ΕΛΛΗΝΙΚΗ ΔΗΜΟΚΡΑΤΙΑ
ΠΕΡΙΦΕΡΕΙΑ ΗΠΕΙΡΟΥ
ΓΕΝΙΚΗ ΔΙΕΥΘΥΝΣΗ ΑΝΑΠΤΥΞΙΑΚΟΥ ΠΡΟΓΡ/ΣΜΟΥ ΠΕΡΙΒΑΛΛΟΝΤΟΣ ΚΑΙ ΥΠΟΔΟΜΩΝ
Δ/ΝΣΗ ΤΕΧΝΙΚΩΝ ΕΡΓΩΝ Π.Ε. ΙΩΑΝΝΙΝΩΝ

ΤΜΗΜΑ ΣΥΓΚΟΙΝΩΝΙΑΚΩΝ ΕΡΓΩΝ

ΘΕΣΗ: ΕΠΑΡΧΙΑΚΗ ΟΔΟΣ ΕΛΕΟΥΣΑΣ - ΚΡΥΑ

ΜΕΛΕΤΗ: **«ΜΕΛΕΤΗ ΑΠΟΚΑΤΑΣΤΑΣΗΣ ΒΛΑΒΩΝ ΚΑΙ ΒΕΛΤΙΩΣΗΣ ΟΔΙΚΗΣ ΛΕΙΤΟΥΡΓΙΑΣ ΥΦΙΣΤΑΜΕΝΩΝ ΓΕΦΥΡΩΝ ΤΗΣ Π.Ε. ΙΩΑΝΝΙΝΩΝ»**

ΥΠΟΕΡΓΟ: «ΒΕΛΤΙΩΣΗΣ ΟΔΙΚΗΣ ΛΕΙΤΟΥΡΓΙΑΣ ΤΜΗΜΑΤΟΣ ΤΟΥ ΟΔΙΚΟΥ ΔΙΚΤΥΟΥ ΕΛΕΟΥΣΑ – ΚΡΥΑ (Ν. ΙΩΑΝΝΙΝΩΝ)»

ΣΤΑΔΙΟ: ΟΡΙΣΤΙΚΗ ΣΤΑΤΙΚΗ ΜΕΛΕΤΗ

Αριθμός Τεύχους: **Σ3.Τ2**

ΘΕΜΑ ΕΓΓΡΑΦΟΥ:

ΥΠΟΛΟΓΙΣΜΟΙ ΦΟΡΕΑ ΤΕΧΝΙΚΟΥ “ΤΑ”

ΚΛΙΜΑΚΕΣ: -

ΕΚΠΟΝΗΣΗ ΜΕΛΕΤΗΣ :

ΑΝΑΔΟΧΟΣ : StaticNet ΜΕΛΕΤΗΤΙΚΗ

	ΗΜΕΡ/ΝΙΑ	ΟΝΟΜΑΤΕΠΩΝΥΜΟ	ΥΠΟΓΡΑΦΗ
ΣΥΝΤΑΞΗ	03-2019	ΓΕΩΡΓΙΟΣ Χ. ΔΡΟΣΟΣ / ΙΩΑΝΝΗΣ Χ. ΔΡΟΣΟΣ	
ΕΛΕΓΧΟΣ	03-2019	ΓΕΩΡΓΙΟΣ Χ. ΔΡΟΣΟΣ / ΙΩΑΝΝΗΣ Χ. ΔΡΟΣΟΣ	
ΕΓΚΡΙΣΗ	03-2019	ΓΕΩΡΓΙΟΣ Χ. ΔΡΟΣΟΣ	

ΔΡΟΣΟΣ ΧΡ. ΓΕΩΡΓΙΟΣ
ΔΙΠΛΩΜ. ΠΟΛΙΤΙΚΟΣ ΜΗΧΑΝΙΚΟΣ, MSc
ΜΕΛΟΣ Τ.Ε.Ε. - ΑΡΙΘ. ΜΗΤΡΩΟΥ 97104
ΑΛΕΞΑΝΔΡΟΥΠΟΛΕΩΣ 7 - ΑΘΗΝΑ ΤΚ 11527
Α.Φ.Μ. 049252515 - ΙΒ' Δ.Ο.Υ. ΑΘΗΝΩΝ
(Τ): 210 74 83 358 - (F): 210 74 83 352

ΑΝΑΘΕΩΡΗΣΕΙΣ

Α / Α	ΗΜΕΡ/ΝΙΑ	ΑΙΤΙΑ Η ΛΟΓΟΣ ΑΝΑΘΕΩΡΗΣΗΣ
Α		
Β		

ΕΓΚΡΙΣΗ ΥΠΗΡΕΣΙΑΣ

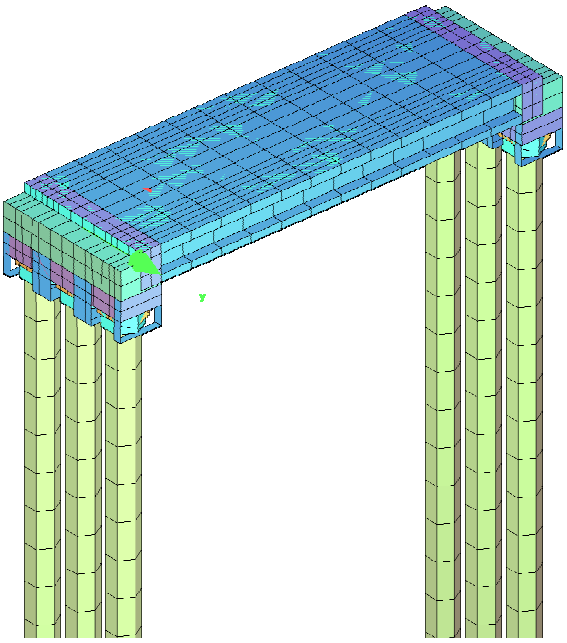
ΕΛΕΓΧΘΗΚΕ ΑΠΟ ΓΕΝΙΚΗΣ ΑΠΟΨΗΣ ΚΑΙ ΕΓΚΡΙΝΕΤΑΙ ΜΕ ΤΙΣ ΑΚΟΛΟΥΘΕΣ ΠΑΡΑΤΗΡΗΣΕΙΣ :

1. _____
2. _____
3. _____

ΙΩΑΝΝΙΝΑ, __/__/2019 Ο ΕΠΙΒΛΕΠΩΝ	ΙΩΑΝΝΙΝΑ, __/__/2019 Ο ΠΡΟΪΣΤΑΜΕΝΟΣ

ΣΤΑΤΙΚΟΙ ΥΠΟΛΟΓΙΣΜΟΙ ΤΕΧΝΙΚΟΥ "ΤΑ"

ΠΕΡΙΕΧΟΜΕΝΑ



ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
18003_C_A_v2-08_CREEP_CSM

00. ΤΕΥΧΟΣ ΣΤΑΤΙΚΗΣ ΜΕΛΕΤΗΣ ΤΕΧΝΙΚΟΥ ΤΑ

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00

1) ΔΕΔΟΜΕΝΑ ΥΛΙΚΩΝ-ΔΙΑΤΟΜΩΝ

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΟΡΙΣΜΟΣ ΥΛΙΚΩΝ

No. 1 C 25/30 (DIN 1045-1)

Youngs-modulus	E	26663	[MPa]	Safetyfactor		1.50	[-]
Poisson-Ratio	mu	0.20	[-]	Strength	fc	21.25	[MPa]
Shear-modulus	G	11109	[MPa]	Nomin. strength	fcn	25.00	[MPa]
Compression modulus		14813	[MPa]	Tens. strength	fctm	2.56	[MPa]
Weight		0.0	[kN/m3]	5 % t.strength	fctk	1.80	[MPa]
Weight buoyancy		0.0	[kN/m3]	95 % t.strength	fctk	3.33	[MPa]
Temp.elongat.coeff.		1.00E-05	[1/°K]	Bond strength	fbd	2.69	[MPa]
				Service strength		33.00	[MPa]
				Fatigue strength		12.75	[MPa]

No. 3 C 25/30 (DIN 1045-1)

Youngs-modulus	E	26663	[MPa]	Safetyfactor		1.50	[-]
Poisson-Ratio	mu	0.20	[-]	Strength	fc	21.25	[MPa]
Shear-modulus	G	11109	[MPa]	Nomin. strength	fcn	25.00	[MPa]
Compression modulus		14813	[MPa]	Tens. strength	fctm	2.56	[MPa]
Weight		0.0	[kN/m3]	5 % t.strength	fctk	1.80	[MPa]
Weight buoyancy		0.0	[kN/m3]	95 % t.strength	fctk	3.33	[MPa]
Temp.elongat.coeff.		1.00E-05	[1/°K]	Bond strength	fbd	2.69	[MPa]
				Service strength		33.00	[MPa]
				Fatigue strength		12.75	[MPa]

No. 4 C 25/30 (DIN 1045-1)

Youngs-modulus	E	26663	[MPa]	Safetyfactor		1.50	[-]
Poisson-Ratio	mu	0.20	[-]	Strength	fc	21.25	[MPa]
Shear-modulus	G	11109	[MPa]	Nomin. strength	fcn	25.00	[MPa]
Compression modulus		14813	[MPa]	Tens. strength	fctm	2.56	[MPa]
Weight		0.0	[kN/m3]	5 % t.strength	fctk	1.80	[MPa]
Weight buoyancy		0.0	[kN/m3]	95 % t.strength	fctk	3.33	[MPa]
Temp.elongat.coeff.		1.00E-05	[1/°K]	Bond strength	fbd	2.69	[MPa]
				Service strength		33.00	[MPa]
				Fatigue strength		12.75	[MPa]

No. 5 C 25/30 (DIN 1045-1)

Youngs-modulus	E	26663	[MPa]	Safetyfactor		1.50	[-]
Poisson-Ratio	mu	0.20	[-]	Strength	fc	21.25	[MPa]
Shear-modulus	G	11109	[MPa]	Nomin. strength	fcn	25.00	[MPa]
Compression modulus		14813	[MPa]	Tens. strength	fctm	2.56	[MPa]
Weight		0.0	[kN/m3]	5 % t.strength	fctk	1.80	[MPa]
Weight buoyancy		0.0	[kN/m3]	95 % t.strength	fctk	3.33	[MPa]
Temp.elongat.coeff.		1.00E-05	[1/°K]	Bond strength	fbd	2.69	[MPa]
				Service strength		33.00	[MPa]
				Fatigue strength		12.75	[MPa]

No. 6 C 25/30 (DIN 1045-1)

Youngs-modulus	E	26663	[MPa]	Safetyfactor		1.50	[-]
Poisson-Ratio	mu	0.20	[-]	Strength	fc	21.25	[MPa]
Shear-modulus	G	11109	[MPa]	Nomin. strength	fcn	25.00	[MPa]
Compression modulus		14813	[MPa]	Tens. strength	fctm	2.56	[MPa]
Weight		32.8	[kN/m3]	5 % t.strength	fctk	1.80	[MPa]
Weight buoyancy		32.8	[kN/m3]	95 % t.strength	fctk	3.33	[MPa]
Temp.elongat.coeff.		1.00E-05	[1/°K]	Bond strength	fbd	2.69	[MPa]
				Service strength		33.00	[MPa]
				Fatigue strength		12.75	[MPa]

No. 7 C 25/30 (DIN 1045-1)

Youngs-modulus	E	26663	[MPa]	Safetyfactor		1.50	[-]
Poisson-Ratio	mu	0.20	[-]	Strength	fc	21.25	[MPa]
Shear-modulus	G	11109	[MPa]	Nomin. strength	fcn	25.00	[MPa]
Compression modulus		14813	[MPa]	Tens. strength	fctm	2.56	[MPa]
Weight		25.0	[kN/m3]	5 % t.strength	fctk	1.80	[MPa]
Weight buoyancy		25.0	[kN/m3]	95 % t.strength	fctk	3.33	[MPa]
Temp.elongat.coeff.		1.00E-05	[1/°K]	Bond strength	fbd	2.69	[MPa]
				Service strength		33.00	[MPa]
				Fatigue strength		12.75	[MPa]

No. 8 C 25/30 (DIN 1045-1)

Youngs-modulus	E	26663	[MPa]	Safetyfactor		1.50	[-]
Poisson-Ratio	mu	0.20	[-]	Strength	fc	21.25	[MPa]
Shear-modulus	G	11109	[MPa]	Nomin. strength	fcn	25.00	[MPa]
Compression modulus		14813	[MPa]	Tens. strength	fctm	2.56	[MPa]
Weight		25.0	[kN/m3]	5 % t.strength	fctk	1.80	[MPa]
Weight buoyancy		25.0	[kN/m3]	95 % t.strength	fctk	3.33	[MPa]
Temp.elongat.coeff.		1.00E-05	[1/°K]	Bond strength	fbd	2.69	[MPa]

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΟΡΙΣΜΟΣ ΥΛΙΚΩΝ

No. 8 C 25/30 (DIN 1045-1)

			Service strength		33.00 [MPa]
			Fatigue strength		12.75 [MPa]

No. 9 C 25/30 (DIN 1045-1)

Youngs-modulus	E	26663 [MPa]	Safetyfactor		1.50 [-]
Poisson-Ratio	mu	0.20 [-]	Strength	fc	21.25 [MPa]
Shear-modulus	G	11109 [MPa]	Nomin. strength	fcn	25.00 [MPa]
Compression modulus		14813 [MPa]	Tens. strength	fctm	2.56 [MPa]
Weight		2.5 [kN/m3]	5 % t.strength	fctk	1.80 [MPa]
Weight buoyancy		2.5 [kN/m3]	95 % t.strength	fctk	3.33 [MPa]
Temp.elongat.coeff.		1.00E-05 [1/°K]	Bond strength	fbd	2.69 [MPa]
			Service strength		33.00 [MPa]
			Fatigue strength		12.75 [MPa]

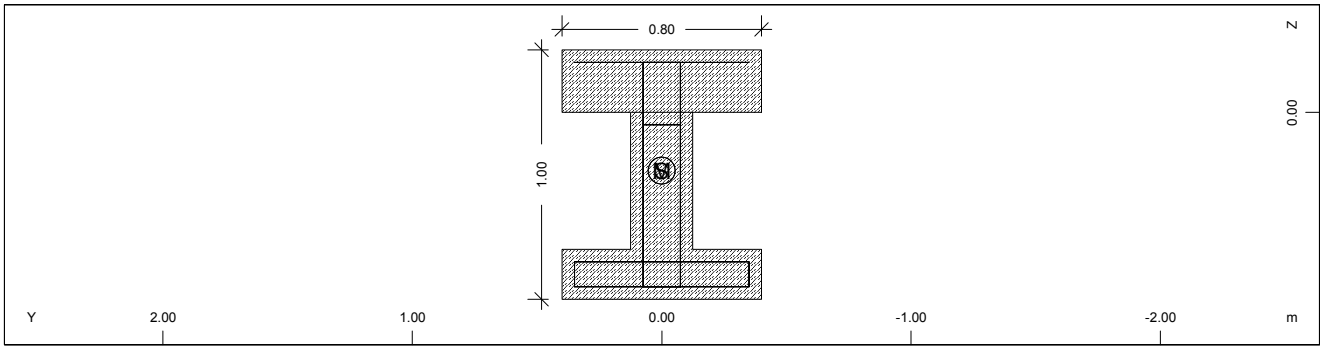
No. 10 C 25/30 (DIN 1045-1)

Youngs-modulus	E	26663 [MPa]	Safetyfactor		1.50 [-]
Poisson-Ratio	mu	0.20 [-]	Strength	fc	21.25 [MPa]
Shear-modulus	G	11109 [MPa]	Nomin. strength	fcn	25.00 [MPa]
Compression modulus		14813 [MPa]	Tens. strength	fctm	2.56 [MPa]
Weight		0.0 [kN/m3]	5 % t.strength	fctk	1.80 [MPa]
Weight buoyancy		0.0 [kN/m3]	95 % t.strength	fctk	3.33 [MPa]
Temp.elongat.coeff.		1.00E-05 [1/°K]	Bond strength	fbd	2.69 [MPa]
			Service strength		33.00 [MPa]
			Fatigue strength		12.75 [MPa]

No. 12 BSt 500 SA (DIN 1045-1)

Youngs-modulus	E	200000 [MPa]	Safetyfactor		1.15 [-]
Poisson-Ratio	mu	0.30 [-]	Yield stress	fy	500.00 [MPa]
Shear-modulus	G	76923 [MPa]	Compr.yield val.	fyc	500.00 [MPa]
Compression modulus		166667 [MPa]	Tens. strength	ft	550.00 [MPa]
Weight		78.5 [kN/m3]	Compr. strength	fc	550.00 [MPa]
Weight buoyancy		78.5 [kN/m3]	Ultim. plast. strain		25.00 [o/oo]
Temp.elongat.coeff.		1.20E-05 [1/°K]	relative bond coeff.		1.00 [-]
max. thickness		32.00 [mm]	EC2 bondcoeff. K1		0.80 [-]
			Hardening modulus		0.00 [MPa]
			Proportional limit		500.00 [MPa]
			Dynamic stress range		169.57 [MPa]

Cross section No. 1

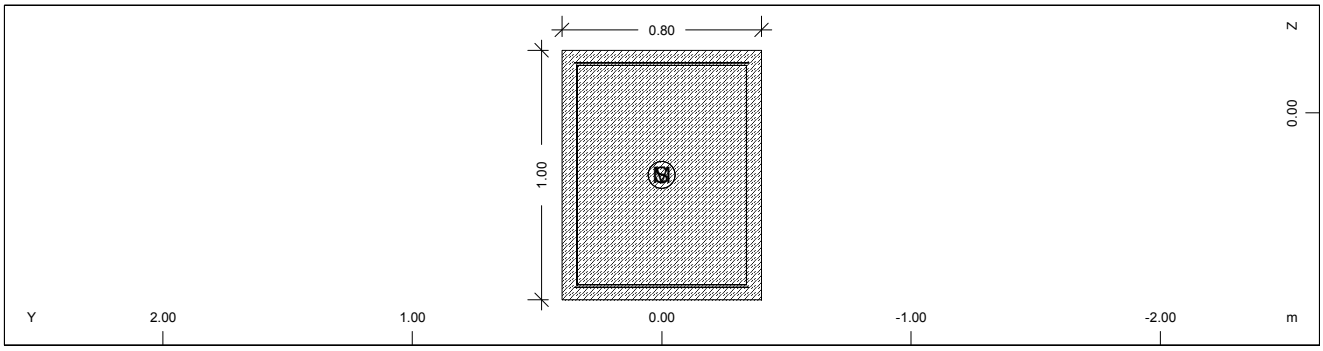


Static properties of cross section

No.	Mat NoR	A[m2] It[m4]	Ay/Az/Ayz [m2]	Iy/Iz/Iyz [m4]	ys/zs [m]	y/z-sc [m]	modules [MPa]	gam [MN/m]
1	=	Composit with materials:		1	4			
	1	4.9904E-01		5.887E-02	0.000	0.000	26663	0.000
	12	9.960E-03		1.992E-02	0.235	0.235	11109	
1.1	=	CS 10						
	1	2.9750E-01		1.440E-02	0.000	0.000	26663	0.000
	12	5.016E-03		9.249E-03	0.477	0.477	11109	
1.2	=	CS 40						
	1	4.9904E-01		5.887E-02	0.000	0.000	26663	0.000
	12	9.960E-03		1.992E-02	0.235	0.235	11109	

Cross section No. 2 - DOKOS-2

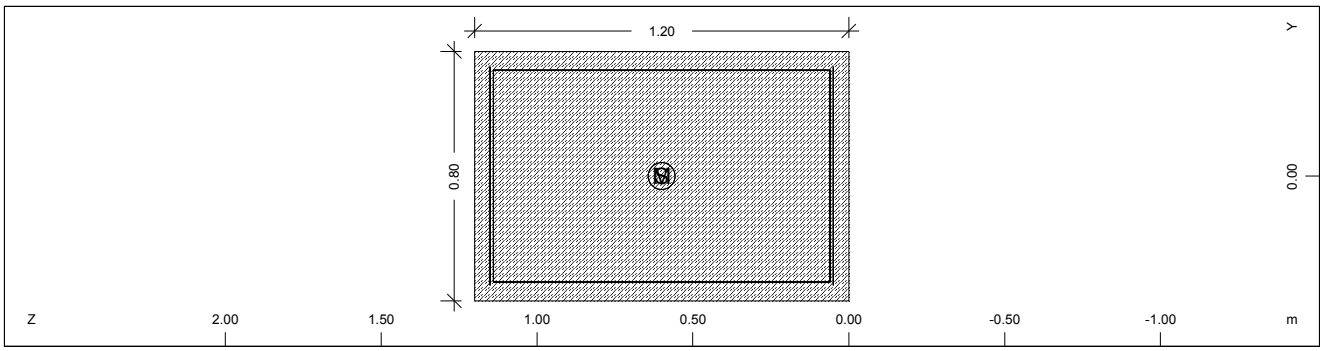
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΟΡΙΣΜΟΣ ΥΛΙΚΩΝ



Static properties of cross section

No.	Mat / NoR	A[m2] / It[m4]	Ay/Az/Ayz [m2]	Iy/Iz/Iyz [m4]	ys/zs [m]	y/z-sc [m]	modules [MPa]	gam [MN/m]
2	=	DOKOS-2						
	3	8.0000E-01		6.667E-02	0.000	0.000	26663	0.000
	12	8.795E-02		4.267E-02	0.250	0.250	11109	

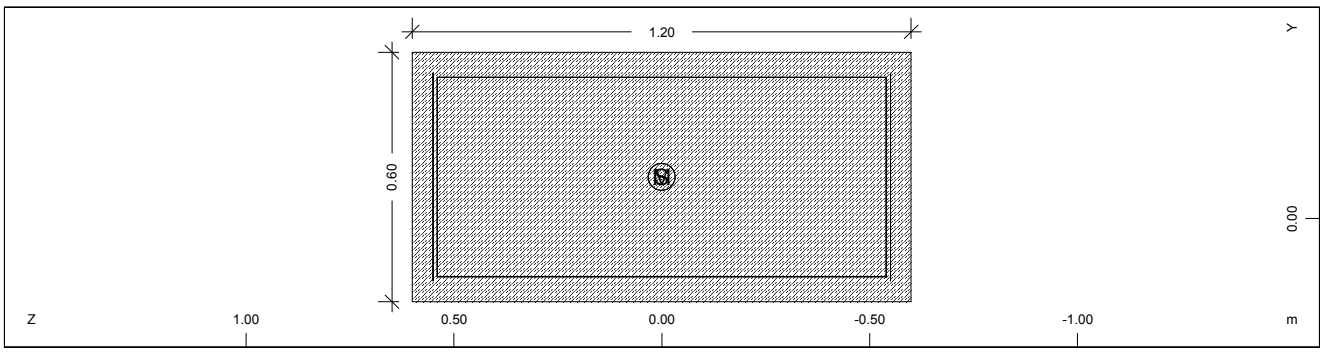
Cross section No. 3 - DOKOS-3



Static properties of cross section

No.	Mat / NoR	A[m2] / It[m4]	Ay/Az/Ayz [m2]	Iy/Iz/Iyz [m4]	ys/zs [m]	y/z-sc [m]	modules [MPa]	gam [MN/m]
3	=	DOKOS-3						
	5	9.6000E-01		1.152E-01	0.000	0.000	26663	0.000
	12	1.203E-01		5.120E-02	0.600	0.600	11109	

Cross section No. 4 - DOKOS-3a

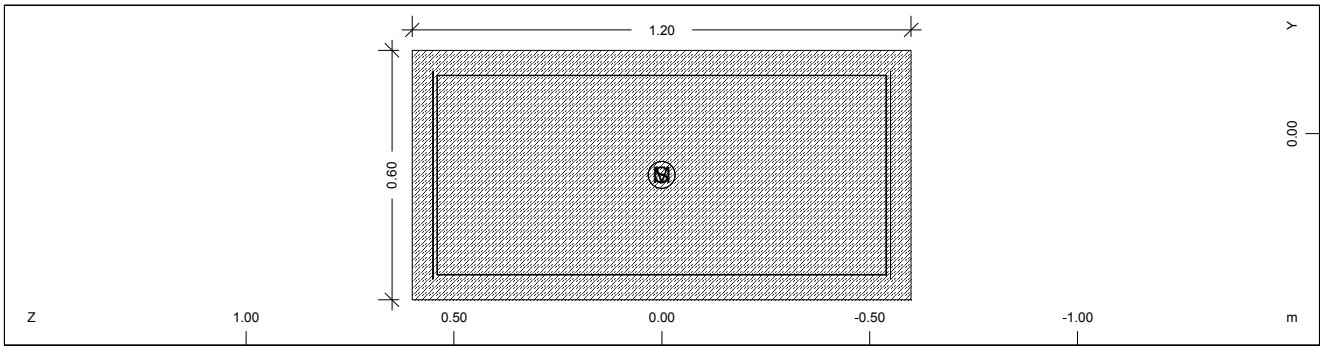


Static properties of cross section

No.	Mat / NoR	A[m2] / It[m4]	Ay/Az/Ayz [m2]	Iy/Iz/Iyz [m4]	ys/zs [m]	y/z-sc [m]	modules [MPa]	gam [MN/m]
4	=	DOKOS-3a						
	5	7.2000E-01		8.640E-02	0.100	0.100	26663	0.000
	12	5.931E-02		2.160E-02	0.000	0.000	11109	

Cross section No. 5 - DOKOS-3b

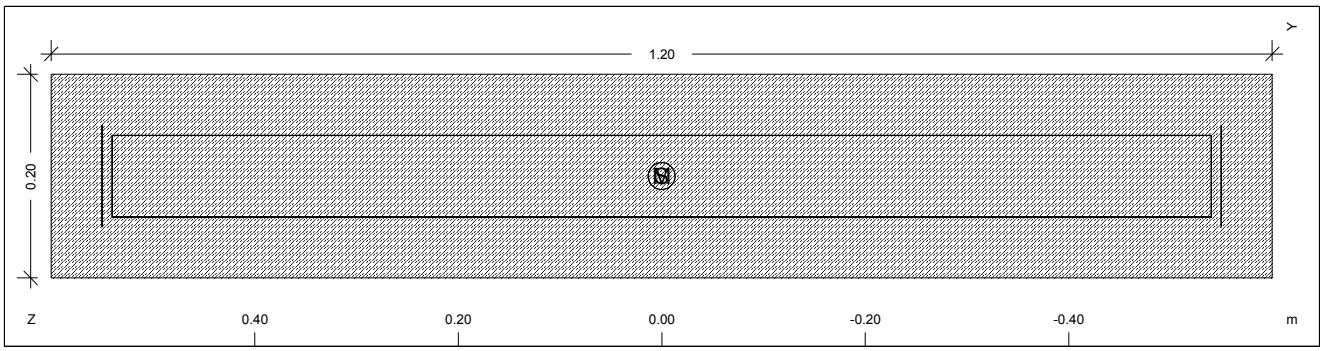
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΟΡΙΣΜΟΣ ΥΛΙΚΩΝ



Static properties of cross section

No.	Mat / NoR	A[m2] / It[m4]	Ay/Az/Ayz [m2]	Iy/Iz/Iyz [m4]	ys/zs [m]	y/z-sc [m]	modules [MPa]	gam [MN/m]
5	=	DOKOS-3b						
	5	7.2000E-01		8.640E-02	-0.100	-0.100	26663	0.000
	12	5.931E-02		2.160E-02	0.000	0.000	11109	

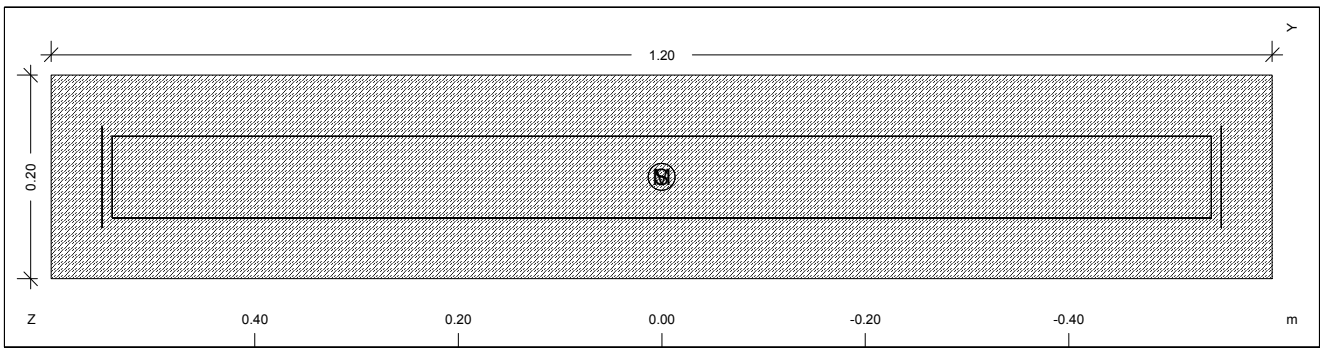
Cross section No. 6 - DOKOS-3c



Static properties of cross section

No.	Mat / NoR	A[m2] / It[m4]	Ay/Az/Ayz [m2]	Iy/Iz/Iyz [m4]	ys/zs [m]	y/z-sc [m]	modules [MPa]	gam [MN/m]
6	=	DOKOS-3c						
	5	2.4000E-01		2.880E-02	0.100	0.100	26663	0.000
	12	2.885E-03		8.000E-04	0.000	0.000	11109	

Cross section No. 7 - DOKOS-3d

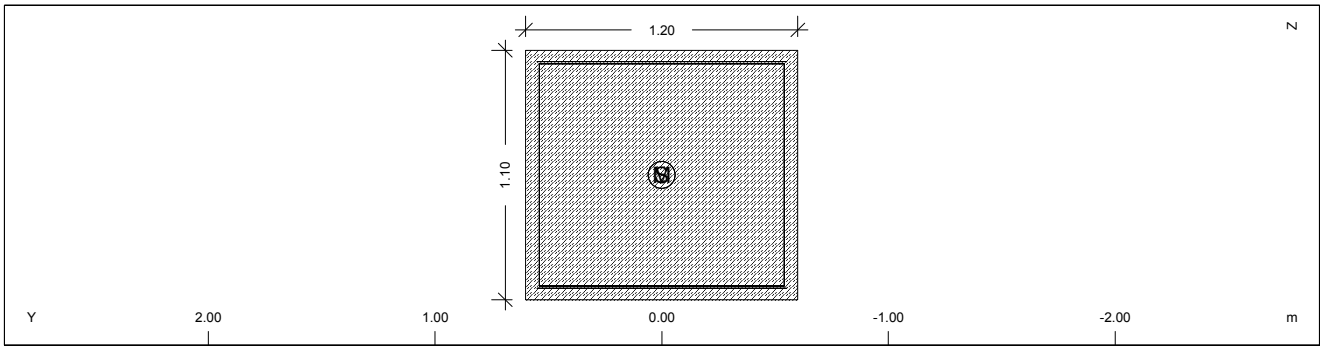


Static properties of cross section

No.	Mat / NoR	A[m2] / It[m4]	Ay/Az/Ayz [m2]	Iy/Iz/Iyz [m4]	ys/zs [m]	y/z-sc [m]	modules [MPa]	gam [MN/m]
7	=	DOKOS-3d						
	5	2.4000E-01		2.880E-02	-0.100	-0.100	26663	0.000
	12	2.885E-03		8.000E-04	0.000	0.000	11109	

Cross section No. 8 - DOKOS-4

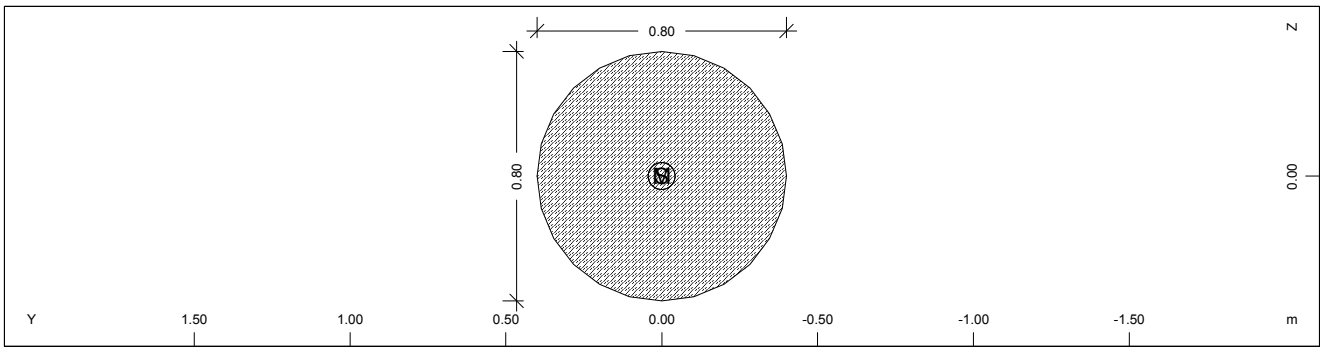
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΟΡΙΣΜΟΣ ΥΛΙΚΩΝ



Static properties of cross section

No.	Mat / NoR	A[m2] / It[m4]	Ay/Az/Ayz [m2]	Iy/Iz/Iyz [m4]	ys/zs [m]	y/z-sc [m]	modules [MPa]	gam [MN/m]
8	=	DOKOS-4						
	8	1.3200E+00		1.331E-01	0.000	0.000	26663	0.033
	12	2.441E-01		1.584E-01	0.550	0.550	11109	

Cross section No. 9 - section pile



Static properties of cross section

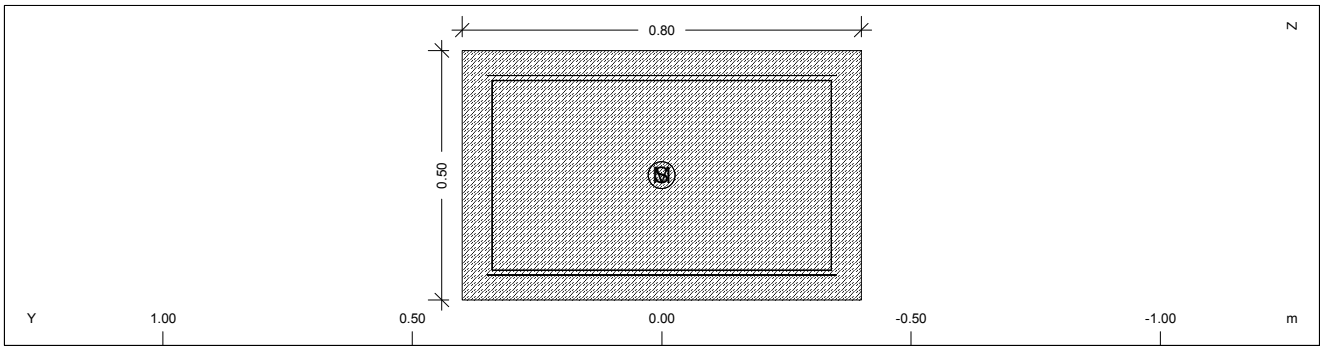
No.	Mat / NoR	A[m2] / It[m4]	Ay/Az/Ayz [m2]	Iy/Iz/Iyz [m4]	ys/zs [m]	y/z-sc [m]	modules [MPa]	gam [MN/m]
9	=	section pile						
	7	5.0265E-01		2.011E-02	0.000	0.000	26663	0.013
	12	4.021E-02		2.011E-02	0.000	0.000	11109	

Cross section No. 10

Static properties of cross section

No.	Mat / NoR	A[m2] / It[m4]	Ay/Az/Ayz [m2]	Iy/Iz/Iyz [m4]	ys/zs [m]	y/z-sc [m]	modules [MPa]	gam [MN/m]
10	9	1.2000E+01		1.000E+03	0.000	0.000	26663	0.030
		2.626E-01		1.000E+03	0.000	0.000	11109	

Cross section No. 11 - CANTIL-1

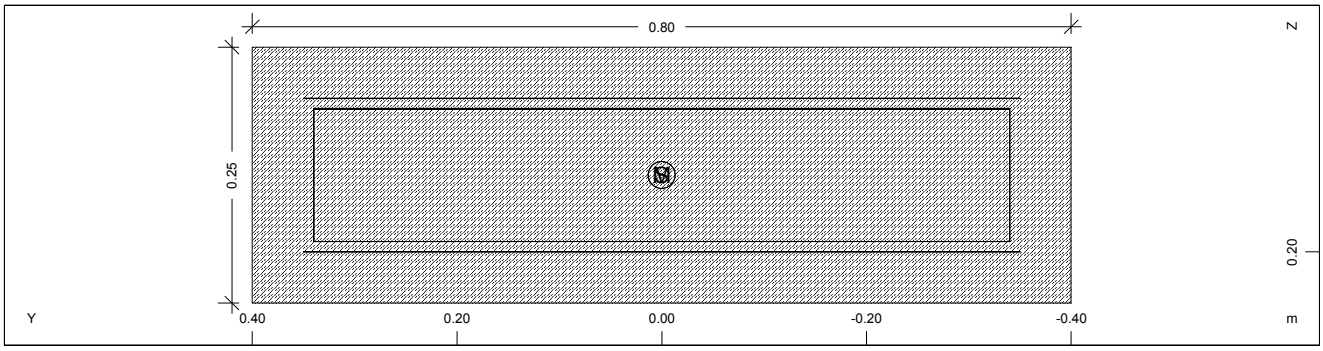


Static properties of cross section

No.	Mat / NoR	A[m2] / It[m4]	Ay/Az/Ayz [m2]	Iy/Iz/Iyz [m4]	ys/zs [m]	y/z-sc [m]	modules [MPa]	gam [MN/m]
11	=	CANTIL-1						
	6	4.0000E-01		8.333E-03	0.000	0.000	26663	0.013
	12	2.038E-02		2.133E-02	0.250	0.250	11109	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΟΡΙΣΜΟΣ ΥΛΙΚΩΝ

Cross section No. 12 - CANTIL-2



Static properties of cross section

No.	Mat NoR	A[m2] It[m4]	Ay/Az/Ayz [m2]	Iy/Iz/Iyz [m4]	ys/zs [m]	y/z-sc [m]	modules [MPa]	gam [MN/m]
12	=	CANTIL-2						
	6	2.0000E-01		1.042E-03	0.000	0.000	26663	0.007
	12	3.351E-03		1.067E-02	0.125	0.125	11109	

Cross section No. 13

Static properties of cross section

No.	Mat NoR	A[m2] It[m4]	Ay/Az/Ayz [m2]	Iy/Iz/Iyz [m4]	ys/zs [m]	y/z-sc [m]	modules [MPa]	gam [MN/m]
13	5	1.0000E+01		1.000E+03	0.000	0.000	26663	0.000
		1.267E-01		1.000E+03	0.000	0.000	11109	

Summary of all beam elements

Cross sections

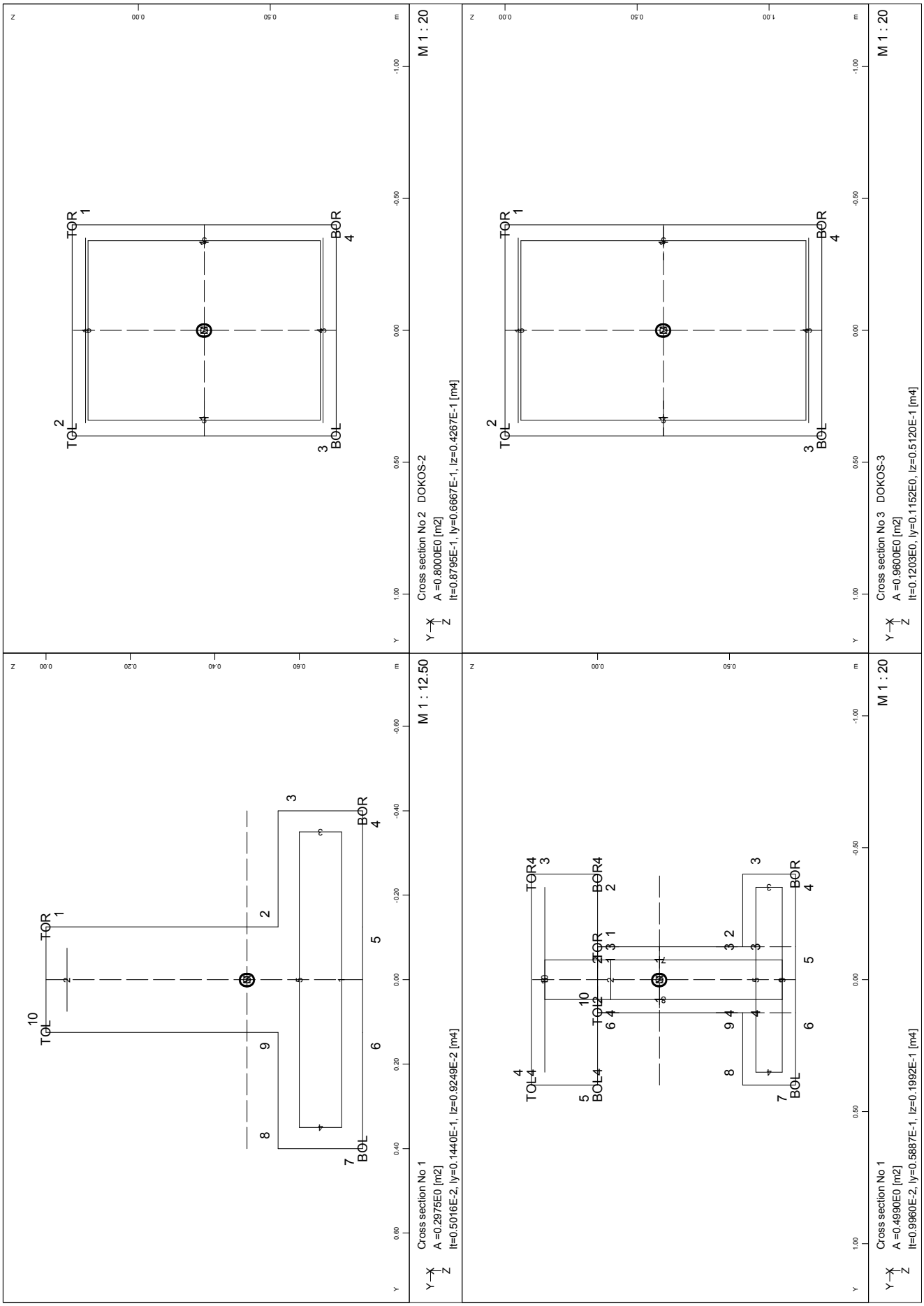
No.	TotLength [m]	Max.Length [m]	TotWeight [t]	Surface [m2]	Title
1	53.000	0.883	0.000	111.300	
2	6.000	0.300	0.000	21.600	DOKOS-2
8	3.200	0.400	10.560	14.720	DOKOS-4
9	150.000	1.000	188.496	376.991	section pile
10	4.800	0.400	14.400		
13	3.300	0.550	0.000		
Sum	220.300		213.456	524.611	

Bore Profile No. 1 ΠΑΣΣΑΛΟΙ ΤΟΙΧΟΥ

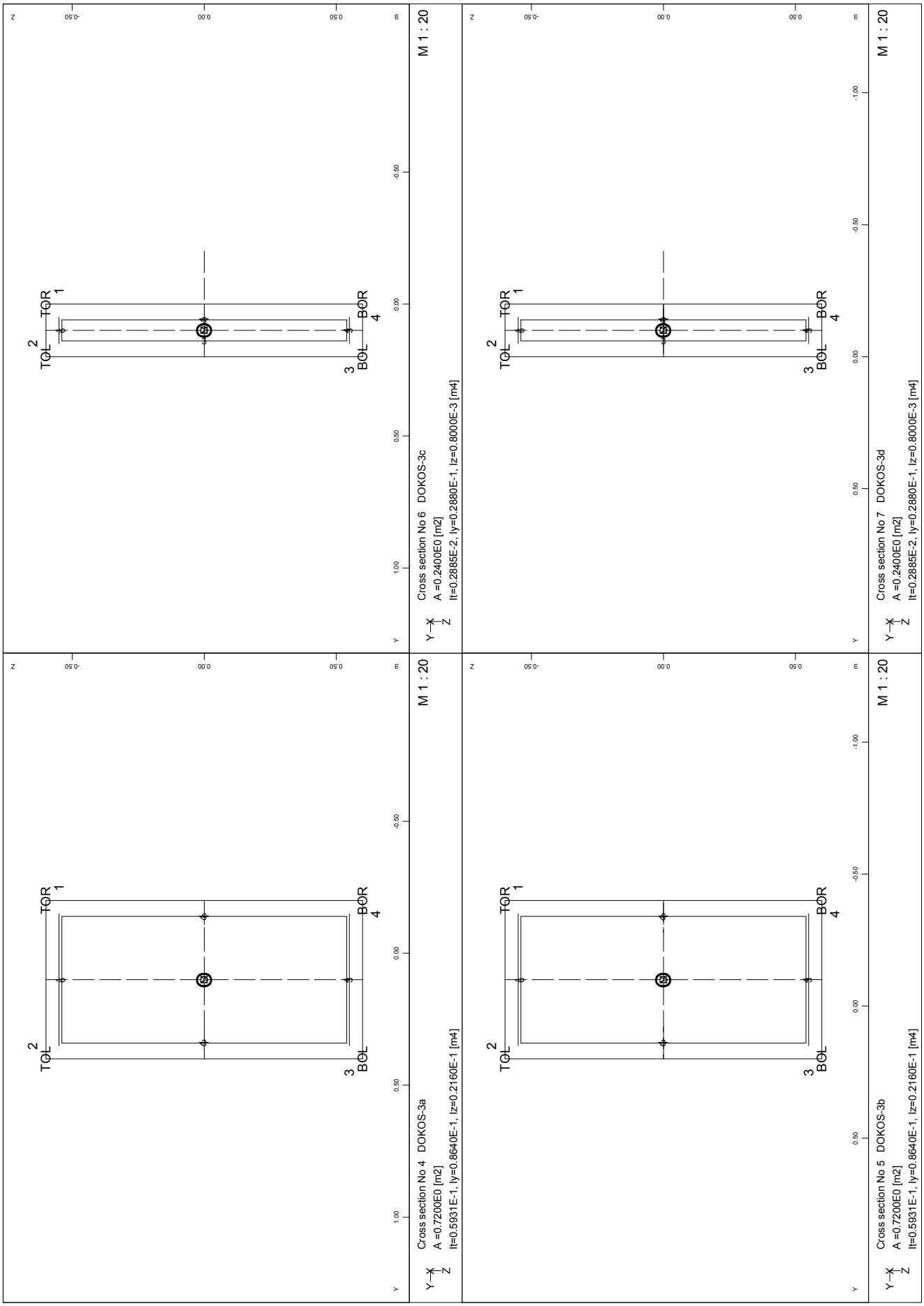
X[m]	Y[m]	Z[m]	dX[-]	dY[-]	dZ[-]	a[°]
0.000	0.000	0.000	0.000	0.000	1.000	0.0

s[m]	K0-t [MN/m2]	K1-t [MN/m2]	K2-t [MN/m2]	K3-t [MN/m2]	P0 [-]	P1 [-]	P2 [-]	P3 [-]	Pmax [MN/m]
4.900	8.480		11.520		1.00	1.00	1.00	1.00	0.468
26.900									1.102

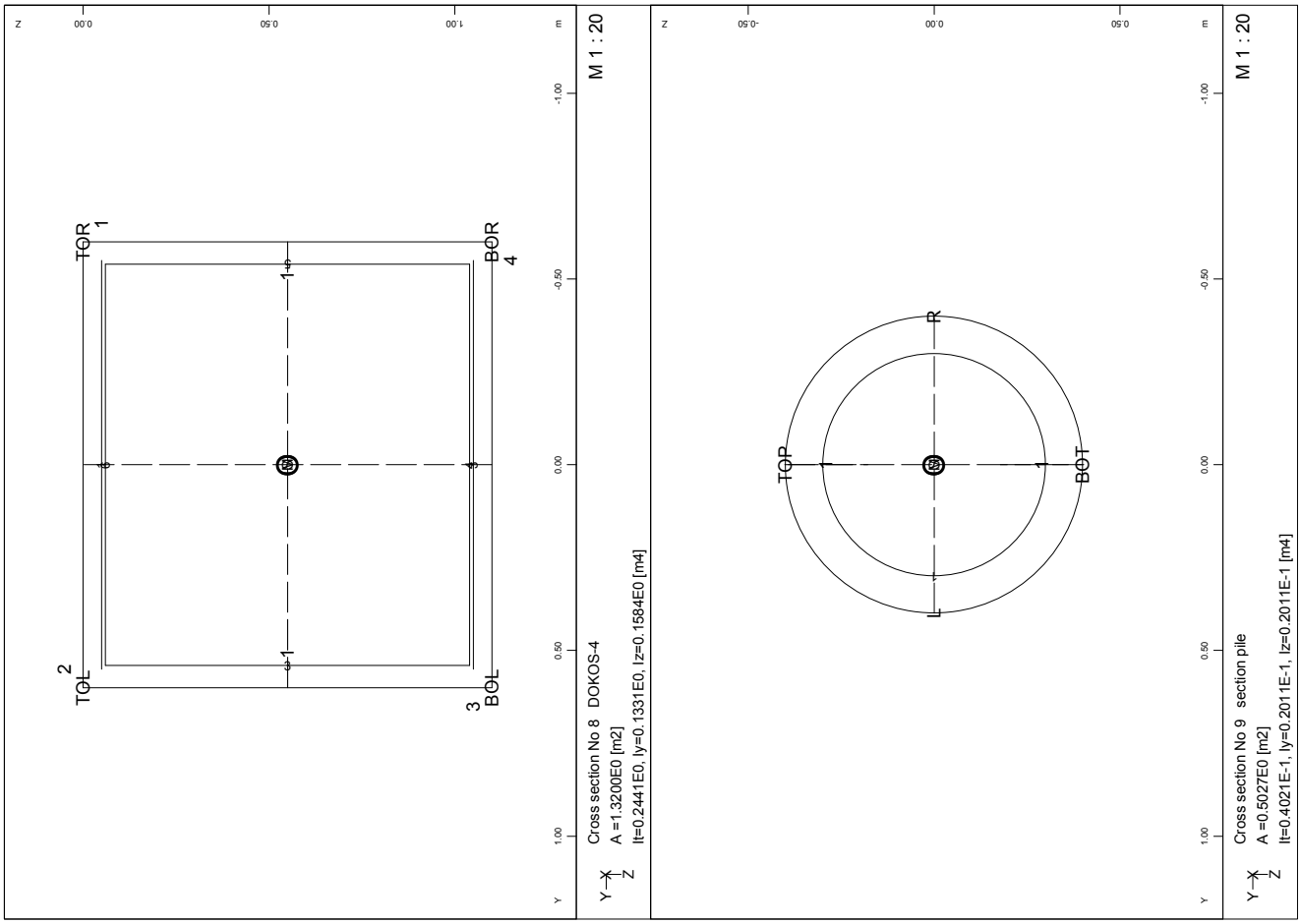
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00



ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00



ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00



2) ΔΕΔΟΜΕΝΑ ΓΕΩΜΕΤΡΙΑΣ ΜΟΝΤΕΛΟΥ

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΟΡΙΣΜΟΣ ΓΕΩΜΕΤΡΙΑΣ ΤΕΧΝΙΚΟΥ

Groups

Grp	number	type	min-no	max-no	Title
1	60	BEAM	1001	1060	
2	20	BEAM	2001	2020	
3	168	QUAD	3001	3168	
4	56	QUAD	4001	4056	
8	56	QUAD	8001	8056	
9	56	QUAD	9001	9056	
10	28	BEAM	10001	10029	
11	6	BEAM	11001	11006	
12	150	BEAM	12001	12150	
13	6	SPRI	13001	13006	

Nodal Coordinates and Supports

Number	X[m]	Y[m]	Z[m]	Support Conditions
101	0.000	2.000	0.000	
102	0.000	1.800	0.000	
103	0.000	1.600	0.000	
104	0.000	1.400	0.000	
105	0.000	1.000	0.000	
106	0.000	0.800	0.000	
107	0.000	0.400	0.000	
108	0.000	0.000	0.000	
109	0.000	-0.400	0.000	
110	0.000	-0.800	0.000	
111	0.000	-1.000	0.000	
112	0.000	-1.400	0.000	
113	0.000	-1.600	0.000	
114	0.000	-1.800	0.000	
115	0.000	-2.000	0.000	
201	0.600	2.000	0.000	
202	0.600	1.800	0.000	
203	0.600	1.600	0.000	
204	0.600	1.400	0.000	
205	0.600	1.000	0.000	
206	0.600	0.800	0.000	
207	0.600	0.400	0.000	
208	0.600	0.000	0.000	
209	0.600	-0.400	0.000	
210	0.600	-0.800	0.000	
211	0.600	-1.000	0.000	
212	0.600	-1.400	0.000	
213	0.600	-1.600	0.000	
214	0.600	-1.800	0.000	
215	0.600	-2.000	0.000	
301	5.900	2.000	0.000	
302	5.900	1.800	0.000	
303	5.900	1.600	0.000	
304	5.900	1.400	0.000	
305	5.900	1.000	0.000	
306	5.900	0.800	0.000	
307	5.900	0.400	0.000	
308	5.900	0.000	0.000	
309	5.900	-0.400	0.000	
310	5.900	-0.800	0.000	
311	5.900	-1.000	0.000	
312	5.900	-1.400	0.000	
313	5.900	-1.600	0.000	
314	5.900	-1.800	0.000	
315	5.900	-2.000	0.000	
401	11.200	2.000	0.000	
402	11.200	1.800	0.000	
403	11.200	1.600	0.000	
404	11.200	1.400	0.000	
405	11.200	1.000	0.000	
406	11.200	0.800	0.000	
407	11.200	0.400	0.000	
408	11.200	0.000	0.000	
409	11.200	-0.400	0.000	
410	11.200	-0.800	0.000	
411	11.200	-1.000	0.000	
412	11.200	-1.400	0.000	
413	11.200	-1.600	0.000	
414	11.200	-1.800	0.000	
415	11.200	-2.000	0.000	
501	11.800	2.000	0.000	
502	11.800	1.800	0.000	
503	11.800	1.600	0.000	
504	11.800	1.400	0.000	
505	11.800	1.000	0.000	
506	11.800	0.800	0.000	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΟΡΙΣΜΟΣ ΓΕΩΜΕΤΡΙΑΣ ΤΕΧΝΙΚΟΥ

Nodal Coordinates and Supports

Number	X[m]	Y[m]	Z[m]	Support Conditions
507	11.800	0.400	0.000	
508	11.800	0.000	0.000	
509	11.800	-0.400	0.000	
510	11.800	-0.800	0.000	
511	11.800	-1.000	0.000	
512	11.800	-1.400	0.000	
513	11.800	-1.600	0.000	
514	11.800	-1.800	0.000	
515	11.800	-2.000	0.000	
1101	0.000	2.000	0.800	
1102	0.000	1.800	0.800	
1103	0.000	1.600	0.800	
1104	0.000	1.400	0.800	
1105	0.000	1.000	0.800	
1106	0.000	0.800	0.800	
1107	0.000	0.400	0.800	
1108	0.000	0.000	0.800	
1109	0.000	-0.400	0.800	
1110	0.000	-0.800	0.800	
1111	0.000	-1.000	0.800	
1112	0.000	-1.400	0.800	
1113	0.000	-1.600	0.800	
1114	0.000	-1.800	0.800	
1115	0.000	-2.000	0.800	
1201	0.600	2.000	0.800	
1202	0.600	1.800	0.800	
1203	0.600	1.600	0.800	
1204	0.600	1.400	0.800	
1205	0.600	1.000	0.800	
1206	0.600	0.800	0.800	
1207	0.600	0.400	0.800	
1208	0.600	0.000	0.800	
1209	0.600	-0.400	0.800	
1210	0.600	-0.800	0.800	
1211	0.600	-1.000	0.800	
1212	0.600	-1.400	0.800	
1213	0.600	-1.600	0.800	
1214	0.600	-1.800	0.800	
1215	0.600	-2.000	0.800	
1223	0.900	1.600	0.800	
1226	0.900	0.800	0.800	
1228	0.900	0.000	0.800	
1230	0.900	-0.800	0.800	
1233	0.900	-1.600	0.800	
1301	5.900	2.000	0.800	
1302	5.900	1.800	0.800	
1303	5.900	1.600	0.800	
1304	5.900	1.400	0.800	
1305	5.900	1.000	0.800	
1306	5.900	0.800	0.800	
1307	5.900	0.400	0.800	
1308	5.900	0.000	0.800	
1309	5.900	-0.400	0.800	
1310	5.900	-0.800	0.800	
1311	5.900	-1.000	0.800	
1312	5.900	-1.400	0.800	
1313	5.900	-1.600	0.800	
1314	5.900	-1.800	0.800	
1315	5.900	-2.000	0.800	
1401	11.200	2.000	0.800	
1402	11.200	1.800	0.800	
1403	11.200	1.600	0.800	
1404	11.200	1.400	0.800	
1405	11.200	1.000	0.800	
1406	11.200	0.800	0.800	
1407	11.200	0.400	0.800	
1408	11.200	0.000	0.800	
1409	11.200	-0.400	0.800	
1410	11.200	-0.800	0.800	
1411	11.200	-1.000	0.800	
1412	11.200	-1.400	0.800	
1413	11.200	-1.600	0.800	
1414	11.200	-1.800	0.800	
1415	11.200	-2.000	0.800	
1423	10.900	1.600	0.800	
1426	10.900	0.800	0.800	
1428	10.900	0.000	0.800	
1430	10.900	-0.800	0.800	
1433	10.900	-1.600	0.800	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΟΡΙΣΜΟΣ ΓΕΩΜΕΤΡΙΑΣ ΤΕΧΝΙΚΟΥ

Nodal Coordinates and Supports

Number	X[m]	Y[m]	Z[m]	Support Conditions
1501	11.800	2.000	0.800	
1502	11.800	1.800	0.800	
1503	11.800	1.600	0.800	
1504	11.800	1.400	0.800	
1505	11.800	1.000	0.800	
1506	11.800	0.800	0.800	
1507	11.800	0.400	0.800	
1508	11.800	0.000	0.800	
1509	11.800	-0.400	0.800	
1510	11.800	-0.800	0.800	
1511	11.800	-1.000	0.800	
1512	11.800	-1.400	0.800	
1513	11.800	-1.600	0.800	
1514	11.800	-1.800	0.800	
1515	11.800	-2.000	0.800	
2101	0.000	2.000	1.350	
2102	0.000	1.800	1.350	
2103	0.000	1.600	1.350	
2104	0.000	1.400	1.350	
2105	0.000	1.000	1.350	
2106	0.000	0.800	1.350	
2107	0.000	0.400	1.350	
2108	0.000	0.000	1.350	
2109	0.000	-0.400	1.350	
2110	0.000	-0.800	1.350	
2111	0.000	-1.000	1.350	
2112	0.000	-1.400	1.350	
2113	0.000	-1.600	1.350	
2114	0.000	-1.800	1.350	
2115	0.000	-2.000	1.350	
2201	0.600	2.000	1.350	
2202	0.600	1.800	1.350	
2203	0.600	1.600	1.350	
2204	0.600	1.400	1.350	
2205	0.600	1.000	1.350	
2206	0.600	0.800	1.350	
2207	0.600	0.400	1.350	
2208	0.600	0.000	1.350	
2209	0.600	-0.400	1.350	
2210	0.600	-0.800	1.350	
2211	0.600	-1.000	1.350	
2212	0.600	-1.400	1.350	
2213	0.600	-1.600	1.350	
2214	0.600	-1.800	1.350	
2215	0.600	-2.000	1.350	
2301	5.900	2.000	1.350	
2302	5.900	1.800	1.350	
2303	5.900	1.600	1.350	
2304	5.900	1.400	1.350	
2305	5.900	1.000	1.350	
2306	5.900	0.800	1.350	
2307	5.900	0.400	1.350	
2308	5.900	0.000	1.350	
2309	5.900	-0.400	1.350	
2310	5.900	-0.800	1.350	
2311	5.900	-1.000	1.350	
2312	5.900	-1.400	1.350	
2313	5.900	-1.600	1.350	
2314	5.900	-1.800	1.350	
2315	5.900	-2.000	1.350	
2401	11.200	2.000	1.350	
2402	11.200	1.800	1.350	
2403	11.200	1.600	1.350	
2404	11.200	1.400	1.350	
2405	11.200	1.000	1.350	
2406	11.200	0.800	1.350	
2407	11.200	0.400	1.350	
2408	11.200	0.000	1.350	
2409	11.200	-0.400	1.350	
2410	11.200	-0.800	1.350	
2411	11.200	-1.000	1.350	
2412	11.200	-1.400	1.350	
2413	11.200	-1.600	1.350	
2414	11.200	-1.800	1.350	
2415	11.200	-2.000	1.350	
2501	11.800	2.000	1.350	
2502	11.800	1.800	1.350	
2503	11.800	1.600	1.350	
2504	11.800	1.400	1.350	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΟΡΙΣΜΟΣ ΓΕΩΜΕΤΡΙΑΣ ΤΕΧΝΙΚΟΥ

Nodal Coordinates and Supports

Number	X[m]	Y[m]	Z[m]	Support Conditions
2505	11.800	1.000	1.350	
2506	11.800	0.800	1.350	
2507	11.800	0.400	1.350	
2508	11.800	0.000	1.350	
2509	11.800	-0.400	1.350	
2510	11.800	-0.800	1.350	
2511	11.800	-1.000	1.350	
2512	11.800	-1.400	1.350	
2513	11.800	-1.600	1.350	
2514	11.800	-1.800	1.350	
2515	11.800	-2.000	1.350	
3101	0.000	2.000	1.900	
3102	0.000	1.800	1.900	
3103	0.000	1.600	1.900	
3104	0.000	1.400	1.900	
3105	0.000	1.000	1.900	
3106	0.000	0.800	1.900	
3107	0.000	0.400	1.900	
3108	0.000	0.000	1.900	
3109	0.000	-0.400	1.900	
3110	0.000	-0.800	1.900	
3111	0.000	-1.000	1.900	
3112	0.000	-1.400	1.900	
3113	0.000	-1.600	1.900	
3114	0.000	-1.800	1.900	
3115	0.000	-2.000	1.900	
3201	0.600	2.000	1.900	
3202	0.600	1.800	1.900	
3203	0.600	1.600	1.900	
3204	0.600	1.400	1.900	
3205	0.600	1.000	1.900	
3206	0.600	0.800	1.900	
3207	0.600	0.400	1.900	
3208	0.600	0.000	1.900	
3209	0.600	-0.400	1.900	
3210	0.600	-0.800	1.900	
3211	0.600	-1.000	1.900	
3212	0.600	-1.400	1.900	
3213	0.600	-1.600	1.900	
3214	0.600	-1.800	1.900	
3215	0.600	-2.000	1.900	
3301	5.900	2.000	1.900	
3302	5.900	1.800	1.900	
3303	5.900	1.600	1.900	
3304	5.900	1.400	1.900	
3305	5.900	1.000	1.900	
3306	5.900	0.800	1.900	
3307	5.900	0.400	1.900	
3308	5.900	0.000	1.900	
3309	5.900	-0.400	1.900	
3310	5.900	-0.800	1.900	
3311	5.900	-1.000	1.900	
3312	5.900	-1.400	1.900	
3313	5.900	-1.600	1.900	
3314	5.900	-1.800	1.900	
3315	5.900	-2.000	1.900	
3401	11.200	2.000	1.900	
3402	11.200	1.800	1.900	
3403	11.200	1.600	1.900	
3404	11.200	1.400	1.900	
3405	11.200	1.000	1.900	
3406	11.200	0.800	1.900	
3407	11.200	0.400	1.900	
3408	11.200	0.000	1.900	
3409	11.200	-0.400	1.900	
3410	11.200	-0.800	1.900	
3411	11.200	-1.000	1.900	
3412	11.200	-1.400	1.900	
3413	11.200	-1.600	1.900	
3414	11.200	-1.800	1.900	
3415	11.200	-2.000	1.900	
3501	11.800	2.000	1.900	
3502	11.800	1.800	1.900	
3503	11.800	1.600	1.900	
3504	11.800	1.400	1.900	
3505	11.800	1.000	1.900	
3506	11.800	0.800	1.900	
3507	11.800	0.400	1.900	
3508	11.800	0.000	1.900	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΟΡΙΣΜΟΣ ΓΕΩΜΕΤΡΙΑΣ ΤΕΧΝΙΚΟΥ

Nodal Coordinates and Supports

Number	X[m]	Y[m]	Z[m]	Support Conditions
3509	11.800	-0.400	1.900	
3510	11.800	-0.800	1.900	
3511	11.800	-1.000	1.900	
3512	11.800	-1.400	1.900	
3513	11.800	-1.600	1.900	
3514	11.800	-1.800	1.900	
3515	11.800	-2.000	1.900	
4101	0.000	2.000	4.900	
4102	0.000	1.800	4.900	
4103	0.000	1.600	4.900	
4104	0.000	1.400	4.900	
4105	0.000	1.000	4.900	
4106	0.000	0.800	4.900	
4107	0.000	0.400	4.900	
4108	0.000	0.000	4.900	
4109	0.000	-0.400	4.900	
4110	0.000	-0.800	4.900	
4111	0.000	-1.000	4.900	
4112	0.000	-1.400	4.900	
4113	0.000	-1.600	4.900	
4114	0.000	-1.800	4.900	
4115	0.000	-2.000	4.900	
4201	0.600	2.000	4.900	
4202	0.600	1.800	4.900	
4203	0.600	1.600	4.900	
4204	0.600	1.400	4.900	
4205	0.600	1.000	4.900	
4206	0.600	0.800	4.900	
4207	0.600	0.400	4.900	
4208	0.600	0.000	4.900	
4209	0.600	-0.400	4.900	
4210	0.600	-0.800	4.900	
4211	0.600	-1.000	4.900	
4212	0.600	-1.400	4.900	
4213	0.600	-1.600	4.900	
4214	0.600	-1.800	4.900	
4215	0.600	-2.000	4.900	
4301	5.900	2.000	4.900	
4302	5.900	1.800	4.900	
4303	5.900	1.600	4.900	
4304	5.900	1.400	4.900	
4305	5.900	1.000	4.900	
4306	5.900	0.800	4.900	
4307	5.900	0.400	4.900	
4308	5.900	0.000	4.900	
4309	5.900	-0.400	4.900	
4310	5.900	-0.800	4.900	
4311	5.900	-1.000	4.900	
4312	5.900	-1.400	4.900	
4313	5.900	-1.600	4.900	
4314	5.900	-1.800	4.900	
4315	5.900	-2.000	4.900	
4401	11.200	2.000	4.900	
4402	11.200	1.800	4.900	
4403	11.200	1.600	4.900	
4404	11.200	1.400	4.900	
4405	11.200	1.000	4.900	
4406	11.200	0.800	4.900	
4407	11.200	0.400	4.900	
4408	11.200	0.000	4.900	
4409	11.200	-0.400	4.900	
4410	11.200	-0.800	4.900	
4411	11.200	-1.000	4.900	
4412	11.200	-1.400	4.900	
4413	11.200	-1.600	4.900	
4414	11.200	-1.800	4.900	
4415	11.200	-2.000	4.900	
4501	11.800	2.000	4.900	
4502	11.800	1.800	4.900	
4503	11.800	1.600	4.900	
4504	11.800	1.400	4.900	
4505	11.800	1.000	4.900	
4506	11.800	0.800	4.900	
4507	11.800	0.400	4.900	
4508	11.800	0.000	4.900	
4509	11.800	-0.400	4.900	
4510	11.800	-0.800	4.900	
4511	11.800	-1.000	4.900	
4512	11.800	-1.400	4.900	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΟΡΙΣΜΟΣ ΓΕΩΜΕΤΡΙΑΣ ΤΕΧΝΙΚΟΥ

Nodal Coordinates and Supports

Number	X[m]	Y[m]	Z[m]	Support Conditions
4513	11.800	-1.600	4.900	
4514	11.800	-1.800	4.900	
4515	11.800	-2.000	4.900	
5101	0.000	2.000	26.900	
5102	0.000	1.800	26.900	
5103	0.000	1.600	26.900	
5104	0.000	1.400	26.900	
5105	0.000	1.000	26.900	
5106	0.000	0.800	26.900	
5107	0.000	0.400	26.900	
5108	0.000	0.000	26.900	
5109	0.000	-0.400	26.900	
5110	0.000	-0.800	26.900	
5111	0.000	-1.000	26.900	
5112	0.000	-1.400	26.900	
5113	0.000	-1.600	26.900	
5114	0.000	-1.800	26.900	
5115	0.000	-2.000	26.900	
5201	0.600	2.000	26.900	
5202	0.600	1.800	26.900	
5203	0.600	1.600	26.900	
5204	0.600	1.400	26.900	
5205	0.600	1.000	26.900	
5206	0.600	0.800	26.900	
5207	0.600	0.400	26.900	
5208	0.600	0.000	26.900	
5209	0.600	-0.400	26.900	
5210	0.600	-0.800	26.900	
5211	0.600	-1.000	26.900	
5212	0.600	-1.400	26.900	
5213	0.600	-1.600	26.900	
5214	0.600	-1.800	26.900	
5215	0.600	-2.000	26.900	
5301	5.900	2.000	26.900	
5302	5.900	1.800	26.900	
5303	5.900	1.600	26.900	
5304	5.900	1.400	26.900	
5305	5.900	1.000	26.900	
5306	5.900	0.800	26.900	
5307	5.900	0.400	26.900	
5308	5.900	0.000	26.900	
5309	5.900	-0.400	26.900	
5310	5.900	-0.800	26.900	
5311	5.900	-1.000	26.900	
5312	5.900	-1.400	26.900	
5313	5.900	-1.600	26.900	
5314	5.900	-1.800	26.900	
5315	5.900	-2.000	26.900	
5401	11.200	2.000	26.900	
5402	11.200	1.800	26.900	
5403	11.200	1.600	26.900	
5404	11.200	1.400	26.900	
5405	11.200	1.000	26.900	
5406	11.200	0.800	26.900	
5407	11.200	0.400	26.900	
5408	11.200	0.000	26.900	
5409	11.200	-0.400	26.900	
5410	11.200	-0.800	26.900	
5411	11.200	-1.000	26.900	
5412	11.200	-1.400	26.900	
5413	11.200	-1.600	26.900	
5414	11.200	-1.800	26.900	
5415	11.200	-2.000	26.900	
5501	11.800	2.000	26.900	
5502	11.800	1.800	26.900	
5503	11.800	1.600	26.900	
5504	11.800	1.400	26.900	
5505	11.800	1.000	26.900	
5506	11.800	0.800	26.900	
5507	11.800	0.400	26.900	
5508	11.800	0.000	26.900	
5509	11.800	-0.400	26.900	
5510	11.800	-0.800	26.900	
5511	11.800	-1.000	26.900	
5512	11.800	-1.400	26.900	
5513	11.800	-1.600	26.900	
5514	11.800	-1.800	26.900	
5515	11.800	-2.000	26.900	
10001	0.300	2.000	0.000	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΟΡΙΣΜΟΣ ΓΕΩΜΕΤΡΙΑΣ ΤΕΧΝΙΚΟΥ

Nodal Coordinates and Supports

Number	X[m]	Y[m]	Z[m]	Support Conditions
10002	1.483	2.000	0.000	
10003	2.367	2.000	0.000	
10004	3.250	2.000	0.000	
10005	4.133	2.000	0.000	
10006	5.017	2.000	0.000	
10007	6.783	2.000	0.000	
10008	7.667	2.000	0.000	
10009	8.550	2.000	0.000	
10010	9.433	2.000	0.000	
10011	10.317	2.000	0.000	
10012	11.500	2.000	0.000	
10013	0.300	1.600	0.000	
10014	1.483	1.600	0.000	
10015	2.367	1.600	0.000	
10016	3.250	1.600	0.000	
10017	4.133	1.600	0.000	
10018	5.017	1.600	0.000	
10019	6.783	1.600	0.000	
10020	7.667	1.600	0.000	
10021	8.550	1.600	0.000	
10022	9.433	1.600	0.000	
10023	10.317	1.600	0.000	
10024	11.500	1.600	0.000	
10025	0.300	0.800	0.000	
10026	1.483	0.800	0.000	
10027	2.367	0.800	0.000	
10028	3.250	0.800	0.000	
10029	4.133	0.800	0.000	
10030	5.017	0.800	0.000	
10031	6.783	0.800	0.000	
10032	7.667	0.800	0.000	
10033	8.550	0.800	0.000	
10034	9.433	0.800	0.000	
10035	10.317	0.800	0.000	
10036	11.500	0.800	0.000	
10037	0.300	0.000	0.000	
10038	1.483	0.000	0.000	
10039	2.367	0.000	0.000	
10040	3.250	0.000	0.000	
10041	4.133	0.000	0.000	
10042	5.017	0.000	0.000	
10043	6.783	0.000	0.000	
10044	7.667	0.000	0.000	
10045	8.550	0.000	0.000	
10046	9.433	0.000	0.000	
10047	10.317	0.000	0.000	
10048	11.500	0.000	0.000	
10049	0.300	-0.800	0.000	
10050	1.483	-0.800	0.000	
10051	2.367	-0.800	0.000	
10052	3.250	-0.800	0.000	
10053	4.133	-0.800	0.000	
10054	5.017	-0.800	0.000	
10055	6.783	-0.800	0.000	
10056	7.667	-0.800	0.000	
10057	8.550	-0.800	0.000	
10058	9.433	-0.800	0.000	
10059	10.317	-0.800	0.000	
10060	11.500	-0.800	0.000	
10061	0.300	-1.600	0.000	
10062	1.483	-1.600	0.000	
10063	2.367	-1.600	0.000	
10064	3.250	-1.600	0.000	
10065	4.133	-1.600	0.000	
10066	5.017	-1.600	0.000	
10067	6.783	-1.600	0.000	
10068	7.667	-1.600	0.000	
10069	8.550	-1.600	0.000	
10070	9.433	-1.600	0.000	
10071	10.317	-1.600	0.000	
10072	11.500	-1.600	0.000	
10073	0.300	-2.000	0.000	
10074	1.483	-2.000	0.000	
10075	2.367	-2.000	0.000	
10076	3.250	-2.000	0.000	
10077	4.133	-2.000	0.000	
10078	5.017	-2.000	0.000	
10079	6.783	-2.000	0.000	
10080	7.667	-2.000	0.000	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΟΡΙΣΜΟΣ ΓΕΩΜΕΤΡΙΑΣ ΤΕΧΝΙΚΟΥ

Nodal Coordinates and Supports

Number	X[m]	Y[m]	Z[m]	Support Conditions
10081	8.550	-2.000	0.000	
10082	9.433	-2.000	0.000	
10083	10.317	-2.000	0.000	
10084	11.500	-2.000	0.000	
10085	1.483	1.800	0.000	
10086	2.367	1.800	0.000	
10087	3.250	1.800	0.000	
10088	4.133	1.800	0.000	
10089	5.017	1.800	0.000	
10090	1.483	1.400	0.000	
10091	2.367	1.400	0.000	
10092	3.250	1.400	0.000	
10093	4.133	1.400	0.000	
10094	5.017	1.400	0.000	
10095	1.483	1.000	0.000	
10096	2.367	1.000	0.000	
10097	3.250	1.000	0.000	
10098	4.133	1.000	0.000	
10099	5.017	1.000	0.000	
10100	1.483	0.400	0.000	
10101	2.367	0.400	0.000	
10102	3.250	0.400	0.000	
10103	4.133	0.400	0.000	
10104	5.017	0.400	0.000	
10105	1.483	-0.400	0.000	
10106	2.367	-0.400	0.000	
10107	3.250	-0.400	0.000	
10108	4.133	-0.400	0.000	
10109	5.017	-0.400	0.000	
10110	1.483	-1.000	0.000	
10111	2.367	-1.000	0.000	
10112	3.250	-1.000	0.000	
10113	4.133	-1.000	0.000	
10114	5.017	-1.000	0.000	
10115	1.483	-1.400	0.000	
10116	2.367	-1.400	0.000	
10117	3.250	-1.400	0.000	
10118	4.133	-1.400	0.000	
10119	5.017	-1.400	0.000	
10120	1.483	-1.800	0.000	
10121	2.367	-1.800	0.000	
10122	3.250	-1.800	0.000	
10123	4.133	-1.800	0.000	
10124	5.017	-1.800	0.000	
10125	6.783	1.800	0.000	
10126	7.667	1.800	0.000	
10127	8.550	1.800	0.000	
10128	9.433	1.800	0.000	
10129	10.317	1.800	0.000	
10130	6.783	1.400	0.000	
10131	7.667	1.400	0.000	
10132	8.550	1.400	0.000	
10133	9.433	1.400	0.000	
10134	10.317	1.400	0.000	
10135	6.783	1.000	0.000	
10136	7.667	1.000	0.000	
10137	8.550	1.000	0.000	
10138	9.433	1.000	0.000	
10139	10.317	1.000	0.000	
10140	6.783	0.400	0.000	
10141	7.667	0.400	0.000	
10142	8.550	0.400	0.000	
10143	9.433	0.400	0.000	
10144	10.317	0.400	0.000	
10145	6.783	-0.400	0.000	
10146	7.667	-0.400	0.000	
10147	8.550	-0.400	0.000	
10148	9.433	-0.400	0.000	
10149	10.317	-0.400	0.000	
10150	6.783	-1.000	0.000	
10151	7.667	-1.000	0.000	
10152	8.550	-1.000	0.000	
10153	9.433	-1.000	0.000	
10154	10.317	-1.000	0.000	
10155	6.783	-1.400	0.000	
10156	7.667	-1.400	0.000	
10157	8.550	-1.400	0.000	
10158	9.433	-1.400	0.000	
10159	10.317	-1.400	0.000	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΟΡΙΣΜΟΣ ΓΕΩΜΕΤΡΙΑΣ ΤΕΧΝΙΚΟΥ

Nodal Coordinates and Supports

Number	X[m]	Y[m]	Z[m]	Support Conditions
10160	6.783	-1.800	0.000	
10161	7.667	-1.800	0.000	
10162	8.550	-1.800	0.000	
10163	9.433	-1.800	0.000	
10164	10.317	-1.800	0.000	
10165	0.300	1.800	0.000	
10166	0.300	1.400	0.000	
10167	0.300	1.000	0.000	
10168	0.300	0.400	0.000	
10169	0.300	-0.400	0.000	
10170	0.300	-1.000	0.000	
10171	0.300	-1.400	0.000	
10172	0.300	-1.800	0.000	
10173	11.500	1.800	0.000	
10174	11.500	1.400	0.000	
10175	11.500	1.000	0.000	
10176	11.500	0.400	0.000	
10177	11.500	-0.400	0.000	
10178	11.500	-1.000	0.000	
10179	11.500	-1.400	0.000	
10180	11.500	-1.800	0.000	
10181	0.000	2.000	0.400	
10182	0.000	1.600	0.400	
10183	0.000	1.800	0.400	
10184	0.000	1.333	0.800	
10185	0.000	1.067	0.800	
10186	0.000	0.800	0.400	
10187	0.000	1.367	0.400	
10188	0.000	1.033	0.400	
10189	0.000	0.000	0.400	
10190	0.000	0.400	0.400	
10191	0.000	-0.800	0.400	
10192	0.000	-0.400	0.400	
10193	0.000	-1.067	0.800	
10194	0.000	-1.333	0.800	
10195	0.000	-1.600	0.400	
10196	0.000	-1.033	0.400	
10197	0.000	-1.367	0.400	
10198	0.000	-2.000	0.400	
10199	0.000	-1.800	0.400	
10200	11.800	2.000	0.400	
10201	11.800	1.600	0.400	
10202	11.800	1.800	0.400	
10203	11.800	0.800	0.400	
10204	11.800	1.333	0.800	
10205	11.800	1.067	0.800	
10206	11.800	1.367	0.400	
10207	11.800	1.033	0.400	
10208	11.800	0.000	0.400	
10209	11.800	0.400	0.400	
10210	11.800	-0.800	0.400	
10211	11.800	-0.400	0.400	
10212	11.800	-1.600	0.400	
10213	11.800	-1.067	0.800	
10214	11.800	-1.333	0.800	
10215	11.800	-1.033	0.400	
10216	11.800	-1.367	0.400	
10217	11.800	-2.000	0.400	
10218	11.800	-1.800	0.400	
10219	0.000	2.000	1.075	
10220	0.000	1.600	1.075	
10221	0.000	1.800	1.075	
10222	0.000	1.333	1.350	
10223	0.000	1.067	1.350	
10224	0.000	0.800	1.075	
10225	0.000	1.333	1.075	
10226	0.000	1.067	1.075	
10227	0.000	0.000	1.075	
10228	0.000	0.400	1.075	
10229	0.000	-0.800	1.075	
10230	0.000	-0.400	1.075	
10231	0.000	-1.067	1.350	
10232	0.000	-1.333	1.350	
10233	0.000	-1.600	1.075	
10234	0.000	-1.067	1.075	
10235	0.000	-1.333	1.075	
10236	0.000	-2.000	1.075	
10237	0.000	-1.800	1.075	
10238	11.800	2.000	1.075	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΟΡΙΣΜΟΣ ΓΕΩΜΕΤΡΙΑΣ ΤΕΧΝΙΚΟΥ

Nodal Coordinates and Supports

Number	X[m]	Y[m]	Z[m]	Support Conditions
10239	11.800	1.600	1.075	
10240	11.800	1.800	1.075	
10241	11.800	0.800	1.075	
10242	11.800	1.333	1.350	
10243	11.800	1.067	1.350	
10244	11.800	1.333	1.075	
10245	11.800	1.067	1.075	
10246	11.800	0.000	1.075	
10247	11.800	0.400	1.075	
10248	11.800	-0.800	1.075	
10249	11.800	-0.400	1.075	
10250	11.800	-1.600	1.075	
10251	11.800	-1.067	1.350	
10252	11.800	-1.333	1.350	
10253	11.800	-1.067	1.075	
10254	11.800	-1.333	1.075	
10255	11.800	-2.000	1.075	
10256	11.800	-1.800	1.075	
10257	0.000	1.400	2.900	
10258	0.000	1.400	3.900	
10259	0.000	0.000	2.900	
10260	0.000	0.000	3.900	
10261	0.000	-1.400	2.900	
10262	0.000	-1.400	3.900	
10263	11.800	1.400	2.900	
10264	11.800	1.400	3.900	
10265	11.800	0.000	2.900	
10266	11.800	0.000	3.900	
10267	11.800	-1.400	2.900	
10268	11.800	-1.400	3.900	
10269	0.000	1.400	5.900	
10270	0.000	1.400	6.900	
10271	0.000	1.400	7.900	
10272	0.000	1.400	8.900	
10273	0.000	1.400	9.900	
10274	0.000	1.400	10.900	
10275	0.000	1.400	11.900	
10276	0.000	1.400	12.900	
10277	0.000	1.400	13.900	
10278	0.000	1.400	14.900	
10279	0.000	1.400	15.900	
10280	0.000	1.400	16.900	
10281	0.000	1.400	17.900	
10282	0.000	1.400	18.900	
10283	0.000	1.400	19.900	
10284	0.000	1.400	20.900	
10285	0.000	1.400	21.900	
10286	0.000	1.400	22.900	
10287	0.000	1.400	23.900	
10288	0.000	1.400	24.900	
10289	0.000	1.400	25.900	
10290	0.000	0.000	5.900	
10291	0.000	0.000	6.900	
10292	0.000	0.000	7.900	
10293	0.000	0.000	8.900	
10294	0.000	0.000	9.900	
10295	0.000	0.000	10.900	
10296	0.000	0.000	11.900	
10297	0.000	0.000	12.900	
10298	0.000	0.000	13.900	
10299	0.000	0.000	14.900	
10300	0.000	0.000	15.900	
10301	0.000	0.000	16.900	
10302	0.000	0.000	17.900	
10303	0.000	0.000	18.900	
10304	0.000	0.000	19.900	
10305	0.000	0.000	20.900	
10306	0.000	0.000	21.900	
10307	0.000	0.000	22.900	
10308	0.000	0.000	23.900	
10309	0.000	0.000	24.900	
10310	0.000	0.000	25.900	
10311	0.000	-1.400	5.900	
10312	0.000	-1.400	6.900	
10313	0.000	-1.400	7.900	
10314	0.000	-1.400	8.900	
10315	0.000	-1.400	9.900	
10316	0.000	-1.400	10.900	
10317	0.000	-1.400	11.900	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΟΡΙΣΜΟΣ ΓΕΩΜΕΤΡΙΑΣ ΤΕΧΝΙΚΟΥ

Nodal Coordinates and Supports

Number	X[m]	Y[m]	Z[m]	Support Conditions
10318	0.000	-1.400	12.900	
10319	0.000	-1.400	13.900	
10320	0.000	-1.400	14.900	
10321	0.000	-1.400	15.900	
10322	0.000	-1.400	16.900	
10323	0.000	-1.400	17.900	
10324	0.000	-1.400	18.900	
10325	0.000	-1.400	19.900	
10326	0.000	-1.400	20.900	
10327	0.000	-1.400	21.900	
10328	0.000	-1.400	22.900	
10329	0.000	-1.400	23.900	
10330	0.000	-1.400	24.900	
10331	0.000	-1.400	25.900	
10332	11.800	1.400	5.900	
10333	11.800	1.400	6.900	
10334	11.800	1.400	7.900	
10335	11.800	1.400	8.900	
10336	11.800	1.400	9.900	
10337	11.800	1.400	10.900	
10338	11.800	1.400	11.900	
10339	11.800	1.400	12.900	
10340	11.800	1.400	13.900	
10341	11.800	1.400	14.900	
10342	11.800	1.400	15.900	
10343	11.800	1.400	16.900	
10344	11.800	1.400	17.900	
10345	11.800	1.400	18.900	
10346	11.800	1.400	19.900	
10347	11.800	1.400	20.900	
10348	11.800	1.400	21.900	
10349	11.800	1.400	22.900	
10350	11.800	1.400	23.900	
10351	11.800	1.400	24.900	
10352	11.800	1.400	25.900	
10353	11.800	0.000	5.900	
10354	11.800	0.000	6.900	
10355	11.800	0.000	7.900	
10356	11.800	0.000	8.900	
10357	11.800	0.000	9.900	
10358	11.800	0.000	10.900	
10359	11.800	0.000	11.900	
10360	11.800	0.000	12.900	
10361	11.800	0.000	13.900	
10362	11.800	0.000	14.900	
10363	11.800	0.000	15.900	
10364	11.800	0.000	16.900	
10365	11.800	0.000	17.900	
10366	11.800	0.000	18.900	
10367	11.800	0.000	19.900	
10368	11.800	0.000	20.900	
10369	11.800	0.000	21.900	
10370	11.800	0.000	22.900	
10371	11.800	0.000	23.900	
10372	11.800	0.000	24.900	
10373	11.800	0.000	25.900	
10374	11.800	-1.400	5.900	
10375	11.800	-1.400	6.900	
10376	11.800	-1.400	7.900	
10377	11.800	-1.400	8.900	
10378	11.800	-1.400	9.900	
10379	11.800	-1.400	10.900	
10380	11.800	-1.400	11.900	
10381	11.800	-1.400	12.900	
10382	11.800	-1.400	13.900	
10383	11.800	-1.400	14.900	
10384	11.800	-1.400	15.900	
10385	11.800	-1.400	16.900	
10386	11.800	-1.400	17.900	
10387	11.800	-1.400	18.900	
10388	11.800	-1.400	19.900	
10389	11.800	-1.400	20.900	
10390	11.800	-1.400	21.900	
10391	11.800	-1.400	22.900	
10392	11.800	-1.400	23.900	
10393	11.800	-1.400	24.900	
10394	11.800	-1.400	25.900	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΟΡΙΣΜΟΣ ΓΕΩΜΕΤΡΙΑΣ ΤΕΧΝΙΚΟΥ

Nodal Coordinates and Supports

Number	X[m]	Y[m]	Z[m]	Support Conditions
MIN	0.000	-2.000	0.000	
MAX	11.800	2.000	26.900	

Flat Elements

Grp	Number	Node	Node	Node	Node	MNo	t[m]	C[kNsec/m2]	direction local x		
3	3001	201	10002	10085	202	5	0.250-		1.000	0.000	0.000
3	3002	10002	10003	10086	10085	5	0.250-		1.000	0.000	0.000
3	3003	10003	10004	10087	10086	5	0.250-		1.000	0.000	0.000
3	3004	10004	10005	10088	10087	5	0.250-		1.000	0.000	0.000
3	3005	10005	10006	10089	10088	5	0.250-		1.000	0.000	0.000
3	3006	10006	301	302	10089	5	0.250-		1.000	0.000	0.000
3	3007	202	10085	10014	203	5	0.250-		1.000	0.000	0.000
3	3008	10085	10086	10015	10014	5	0.250-		1.000	0.000	0.000
3	3009	10086	10087	10016	10015	5	0.250-		1.000	0.000	0.000
3	3010	10087	10088	10017	10016	5	0.250-		1.000	0.000	0.000
3	3011	10088	10089	10018	10017	5	0.250-		1.000	0.000	0.000
3	3012	10089	302	303	10018	5	0.250-		1.000	0.000	0.000
3	3013	203	10014	10090	204	5	0.250-		1.000	0.000	0.000
3	3014	10014	10015	10091	10090	5	0.250-		1.000	0.000	0.000
3	3015	10015	10016	10092	10091	5	0.250-		1.000	0.000	0.000
3	3016	10016	10017	10093	10092	5	0.250-		1.000	0.000	0.000
3	3017	10017	10018	10094	10093	5	0.250-		1.000	0.000	0.000
3	3018	10018	303	304	10094	5	0.250-		1.000	0.000	0.000
3	3019	204	10090	10095	205	5	0.250-		1.000	0.000	0.000
3	3020	10090	10091	10096	10095	5	0.250-		1.000	0.000	0.000
3	3021	10091	10092	10097	10096	5	0.250-		1.000	0.000	0.000
3	3022	10092	10093	10098	10097	5	0.250-		1.000	0.000	0.000
3	3023	10093	10094	10099	10098	5	0.250-		1.000	0.000	0.000
3	3024	10094	304	305	10099	5	0.250-		1.000	0.000	0.000
3	3025	205	10095	10026	206	5	0.250-		1.000	0.000	0.000
3	3026	10095	10096	10027	10026	5	0.250-		1.000	0.000	0.000
3	3027	10096	10097	10028	10027	5	0.250-		1.000	0.000	0.000
3	3028	10097	10098	10029	10028	5	0.250-		1.000	0.000	0.000
3	3029	10098	10099	10030	10029	5	0.250-		1.000	0.000	0.000
3	3030	10099	305	306	10030	5	0.250-		1.000	0.000	0.000
3	3031	206	10026	10100	207	5	0.250-		1.000	0.000	0.000
3	3032	10026	10027	10101	10100	5	0.250-		1.000	0.000	0.000
3	3033	10027	10028	10102	10101	5	0.250-		1.000	0.000	0.000
3	3034	10028	10029	10103	10102	5	0.250-		1.000	0.000	0.000
3	3035	10029	10030	10104	10103	5	0.250-		1.000	0.000	0.000
3	3036	10030	306	307	10104	5	0.250-		1.000	0.000	0.000
3	3037	207	10100	10038	208	5	0.250-		1.000	0.000	0.000
3	3038	10100	10101	10039	10038	5	0.250-		1.000	0.000	0.000
3	3039	10101	10102	10040	10039	5	0.250-		1.000	0.000	0.000
3	3040	10102	10103	10041	10040	5	0.250-		1.000	0.000	0.000
3	3041	10103	10104	10042	10041	5	0.250-		1.000	0.000	0.000
3	3042	10104	307	308	10042	5	0.250-		1.000	0.000	0.000
3	3043	208	10038	10105	209	5	0.250-		1.000	0.000	0.000
3	3044	10038	10039	10106	10105	5	0.250-		1.000	0.000	0.000
3	3045	10039	10040	10107	10106	5	0.250-		1.000	0.000	0.000
3	3046	10040	10041	10108	10107	5	0.250-		1.000	0.000	0.000
3	3047	10041	10042	10109	10108	5	0.250-		1.000	0.000	0.000
3	3048	10042	308	309	10109	5	0.250-		1.000	0.000	0.000
3	3049	209	10105	10050	210	5	0.250-		1.000	0.000	0.000
3	3050	10105	10106	10051	10050	5	0.250-		1.000	0.000	0.000
3	3051	10106	10107	10052	10051	5	0.250-		1.000	0.000	0.000
3	3052	10107	10108	10053	10052	5	0.250-		1.000	0.000	0.000
3	3053	10108	10109	10054	10053	5	0.250-		1.000	0.000	0.000
3	3054	10109	309	310	10054	5	0.250-		1.000	0.000	0.000
3	3055	210	10050	10110	211	5	0.250-		1.000	0.000	0.000
3	3056	10050	10051	10111	10110	5	0.250-		1.000	0.000	0.000
3	3057	10051	10052	10112	10111	5	0.250-		1.000	0.000	0.000
3	3058	10052	10053	10113	10112	5	0.250-		1.000	0.000	0.000
3	3059	10053	10054	10114	10113	5	0.250-		1.000	0.000	0.000
3	3060	10054	310	311	10114	5	0.250-		1.000	0.000	0.000
3	3061	211	10110	10115	212	5	0.250-		1.000	0.000	0.000
3	3062	10110	10111	10116	10115	5	0.250-		1.000	0.000	0.000
3	3063	10111	10112	10117	10116	5	0.250-		1.000	0.000	0.000
3	3064	10112	10113	10118	10117	5	0.250-		1.000	0.000	0.000
3	3065	10113	10114	10119	10118	5	0.250-		1.000	0.000	0.000
3	3066	10114	311	312	10119	5	0.250-		1.000	0.000	0.000
3	3067	212	10115	10062	213	5	0.250-		1.000	0.000	0.000
3	3068	10115	10116	10063	10062	5	0.250-		1.000	0.000	0.000
3	3069	10116	10117	10064	10063	5	0.250-		1.000	0.000	0.000
3	3070	10117	10118	10065	10064	5	0.250-		1.000	0.000	0.000
3	3071	10118	10119	10066	10065	5	0.250-		1.000	0.000	0.000
3	3072	10119	312	313	10066	5	0.250-		1.000	0.000	0.000
3	3073	213	10062	10120	214	5	0.250-		1.000	0.000	0.000

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΟΡΙΣΜΟΣ ΓΕΩΜΕΤΡΙΑΣ ΤΕΧΝΙΚΟΥ

Flat Elements

Grp	Number	Node	Node	Node	Node	MNo	t[m]	C[kNsec/m2]	direction local x		
3	3074	10062	10063	10121	10120	5	0.250-		1.000	0.000	0.000
3	3075	10063	10064	10122	10121	5	0.250-		1.000	0.000	0.000
3	3076	10064	10065	10123	10122	5	0.250-		1.000	0.000	0.000
3	3077	10065	10066	10124	10123	5	0.250-		1.000	0.000	0.000
3	3078	10066	313	314	10124	5	0.250-		1.000	0.000	0.000
3	3079	214	10120	10074	215	5	0.250-		1.000	0.000	0.000
3	3080	10120	10121	10075	10074	5	0.250-		1.000	0.000	0.000
3	3081	10121	10122	10076	10075	5	0.250-		1.000	0.000	0.000
3	3082	10122	10123	10077	10076	5	0.250-		1.000	0.000	0.000
3	3083	10123	10124	10078	10077	5	0.250-		1.000	0.000	0.000
3	3084	10124	314	315	10078	5	0.250-		1.000	0.000	0.000
3	3085	301	10007	10125	302	5	0.250-		1.000	0.000	0.000
3	3086	10007	10008	10126	10125	5	0.250-		1.000	0.000	0.000
3	3087	10008	10009	10127	10126	5	0.250-		1.000	0.000	0.000
3	3088	10009	10010	10128	10127	5	0.250-		1.000	0.000	0.000
3	3089	10010	10011	10129	10128	5	0.250-		1.000	0.000	0.000
3	3090	10011	401	402	10129	5	0.250-		1.000	0.000	0.000
3	3091	302	10125	10019	303	5	0.250-		1.000	0.000	0.000
3	3092	10125	10126	10020	10019	5	0.250-		1.000	0.000	0.000
3	3093	10126	10127	10021	10020	5	0.250-		1.000	0.000	0.000
3	3094	10127	10128	10022	10021	5	0.250-		1.000	0.000	0.000
3	3095	10128	10129	10023	10022	5	0.250-		1.000	0.000	0.000
3	3096	10129	402	403	10023	5	0.250-		1.000	0.000	0.000
3	3097	303	10019	10130	304	5	0.250-		1.000	0.000	0.000
3	3098	10019	10020	10131	10130	5	0.250-		1.000	0.000	0.000
3	3099	10020	10021	10132	10131	5	0.250-		1.000	0.000	0.000
3	3100	10021	10022	10133	10132	5	0.250-		1.000	0.000	0.000
3	3101	10022	10023	10134	10133	5	0.250-		1.000	0.000	0.000
3	3102	10023	403	404	10134	5	0.250-		1.000	0.000	0.000
3	3103	304	10130	10135	305	5	0.250-		1.000	0.000	0.000
3	3104	10130	10131	10136	10135	5	0.250-		1.000	0.000	0.000
3	3105	10131	10132	10137	10136	5	0.250-		1.000	0.000	0.000
3	3106	10132	10133	10138	10137	5	0.250-		1.000	0.000	0.000
3	3107	10133	10134	10139	10138	5	0.250-		1.000	0.000	0.000
3	3108	10134	404	405	10139	5	0.250-		1.000	0.000	0.000
3	3109	305	10135	10031	306	5	0.250-		1.000	0.000	0.000
3	3110	10135	10136	10032	10031	5	0.250-		1.000	0.000	0.000
3	3111	10136	10137	10033	10032	5	0.250-		1.000	0.000	0.000
3	3112	10137	10138	10034	10033	5	0.250-		1.000	0.000	0.000
3	3113	10138	10139	10035	10034	5	0.250-		1.000	0.000	0.000
3	3114	10139	405	406	10035	5	0.250-		1.000	0.000	0.000
3	3115	306	10031	10140	307	5	0.250-		1.000	0.000	0.000
3	3116	10031	10032	10141	10140	5	0.250-		1.000	0.000	0.000
3	3117	10032	10033	10142	10141	5	0.250-		1.000	0.000	0.000
3	3118	10033	10034	10143	10142	5	0.250-		1.000	0.000	0.000
3	3119	10034	10035	10144	10143	5	0.250-		1.000	0.000	0.000
3	3120	10035	406	407	10144	5	0.250-		1.000	0.000	0.000
3	3121	307	10140	10043	308	5	0.250-		1.000	0.000	0.000
3	3122	10140	10141	10044	10043	5	0.250-		1.000	0.000	0.000
3	3123	10141	10142	10045	10044	5	0.250-		1.000	0.000	0.000
3	3124	10142	10143	10046	10045	5	0.250-		1.000	0.000	0.000
3	3125	10143	10144	10047	10046	5	0.250-		1.000	0.000	0.000
3	3126	10144	407	408	10047	5	0.250-		1.000	0.000	0.000
3	3127	308	10043	10145	309	5	0.250-		1.000	0.000	0.000
3	3128	10043	10044	10146	10145	5	0.250-		1.000	0.000	0.000
3	3129	10044	10045	10147	10146	5	0.250-		1.000	0.000	0.000
3	3130	10045	10046	10148	10147	5	0.250-		1.000	0.000	0.000
3	3131	10046	10047	10149	10148	5	0.250-		1.000	0.000	0.000
3	3132	10047	408	409	10149	5	0.250-		1.000	0.000	0.000
3	3133	309	10145	10055	310	5	0.250-		1.000	0.000	0.000
3	3134	10145	10146	10056	10055	5	0.250-		1.000	0.000	0.000
3	3135	10146	10147	10057	10056	5	0.250-		1.000	0.000	0.000
3	3136	10147	10148	10058	10057	5	0.250-		1.000	0.000	0.000
3	3137	10148	10149	10059	10058	5	0.250-		1.000	0.000	0.000
3	3138	10149	409	410	10059	5	0.250-		1.000	0.000	0.000
3	3139	310	10055	10150	311	5	0.250-		1.000	0.000	0.000
3	3140	10055	10056	10151	10150	5	0.250-		1.000	0.000	0.000
3	3141	10056	10057	10152	10151	5	0.250-		1.000	0.000	0.000
3	3142	10057	10058	10153	10152	5	0.250-		1.000	0.000	0.000
3	3143	10058	10059	10154	10153	5	0.250-		1.000	0.000	0.000
3	3144	10059	410	411	10154	5	0.250-		1.000	0.000	0.000
3	3145	311	10150	10155	312	5	0.250-		1.000	0.000	0.000
3	3146	10150	10151	10156	10155	5	0.250-		1.000	0.000	0.000
3	3147	10151	10152	10157	10156	5	0.250-		1.000	0.000	0.000
3	3148	10152	10153	10158	10157	5	0.250-		1.000	0.000	0.000
3	3149	10153	10154	10159	10158	5	0.250-		1.000	0.000	0.000
3	3150	10154	411	412	10159	5	0.250-		1.000	0.000	0.000
3	3151	312	10155	10067	313	5	0.250-		1.000	0.000	0.000
3	3152	10155	10156	10068	10067	5	0.250-		1.000	0.000	0.000

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΟΡΙΣΜΟΣ ΓΕΩΜΕΤΡΙΑΣ ΤΕΧΝΙΚΟΥ

Flat Elements

Grp	Number	Node	Node	Node	Node	MNo	t[m]C[kNsec/m2]	direction local x		
3	3153	10156	10157	10069	10068	5	0.250-	1.000	0.000	0.000
3	3154	10157	10158	10070	10069	5	0.250-	1.000	0.000	0.000
3	3155	10158	10159	10071	10070	5	0.250-	1.000	0.000	0.000
3	3156	10159	412	413	10071	5	0.250-	1.000	0.000	0.000
3	3157	313	10067	10160	314	5	0.250-	1.000	0.000	0.000
3	3158	10067	10068	10161	10160	5	0.250-	1.000	0.000	0.000
3	3159	10068	10069	10162	10161	5	0.250-	1.000	0.000	0.000
3	3160	10069	10070	10163	10162	5	0.250-	1.000	0.000	0.000
3	3161	10070	10071	10164	10163	5	0.250-	1.000	0.000	0.000
3	3162	10071	413	414	10164	5	0.250-	1.000	0.000	0.000
3	3163	314	10160	10079	315	5	0.250-	1.000	0.000	0.000
3	3164	10160	10161	10080	10079	5	0.250-	1.000	0.000	0.000
3	3165	10161	10162	10081	10080	5	0.250-	1.000	0.000	0.000
3	3166	10162	10163	10082	10081	5	0.250-	1.000	0.000	0.000
3	3167	10163	10164	10083	10082	5	0.250-	1.000	0.000	0.000
3	3168	10164	414	415	10083	5	0.250-	1.000	0.000	0.000
4	4001	101	10001	10165	102	5	0.250-	1.000	0.000	0.000
4	4002	10001	201	202	10165	5	0.250-	1.000	0.000	0.000
4	4003	102	10165	10013	103	5	0.250-	1.000	0.000	0.000
4	4004	10165	202	203	10013	5	0.250-	1.000	0.000	0.000
4	4005	103	10013	10166	104	5	0.250-	1.000	0.000	0.000
4	4006	10013	203	204	10166	5	0.250-	1.000	0.000	0.000
4	4007	104	10166	10167	105	5	0.250-	1.000	0.000	0.000
4	4008	10166	204	205	10167	5	0.250-	1.000	0.000	0.000
4	4009	105	10167	10025	106	5	0.250-	1.000	0.000	0.000
4	4010	10167	205	206	10025	5	0.250-	1.000	0.000	0.000
4	4011	106	10025	10168	107	5	0.250-	1.000	0.000	0.000
4	4012	10025	206	207	10168	5	0.250-	1.000	0.000	0.000
4	4013	107	10168	10037	108	5	0.250-	1.000	0.000	0.000
4	4014	10168	207	208	10037	5	0.250-	1.000	0.000	0.000
4	4015	108	10037	10169	109	5	0.250-	1.000	0.000	0.000
4	4016	10037	208	209	10169	5	0.250-	1.000	0.000	0.000
4	4017	109	10169	10049	110	5	0.250-	1.000	0.000	0.000
4	4018	10169	209	210	10049	5	0.250-	1.000	0.000	0.000
4	4019	110	10049	10170	111	5	0.250-	1.000	0.000	0.000
4	4020	10049	210	211	10170	5	0.250-	1.000	0.000	0.000
4	4021	111	10170	10171	112	5	0.250-	1.000	0.000	0.000
4	4022	10170	211	212	10171	5	0.250-	1.000	0.000	0.000
4	4023	112	10171	10061	113	5	0.250-	1.000	0.000	0.000
4	4024	10171	212	213	10061	5	0.250-	1.000	0.000	0.000
4	4025	113	10061	10172	114	5	0.250-	1.000	0.000	0.000
4	4026	10061	213	214	10172	5	0.250-	1.000	0.000	0.000
4	4027	114	10172	10073	115	5	0.250-	1.000	0.000	0.000
4	4028	10172	214	215	10073	5	0.250-	1.000	0.000	0.000
4	4029	401	10012	10173	402	5	0.250-	1.000	0.000	0.000
4	4030	10012	501	502	10173	5	0.250-	1.000	0.000	0.000
4	4031	402	10173	10024	403	5	0.250-	1.000	0.000	0.000
4	4032	10173	502	503	10024	5	0.250-	1.000	0.000	0.000
4	4033	403	10024	10174	404	5	0.250-	1.000	0.000	0.000
4	4034	10024	503	504	10174	5	0.250-	1.000	0.000	0.000
4	4035	404	10174	10175	405	5	0.250-	1.000	0.000	0.000
4	4036	10174	504	505	10175	5	0.250-	1.000	0.000	0.000
4	4037	405	10175	10036	406	5	0.250-	1.000	0.000	0.000
4	4038	10175	505	506	10036	5	0.250-	1.000	0.000	0.000
4	4039	406	10036	10176	407	5	0.250-	1.000	0.000	0.000
4	4040	10036	506	507	10176	5	0.250-	1.000	0.000	0.000
4	4041	407	10176	10048	408	5	0.250-	1.000	0.000	0.000
4	4042	10176	507	508	10048	5	0.250-	1.000	0.000	0.000
4	4043	408	10048	10177	409	5	0.250-	1.000	0.000	0.000
4	4044	10048	508	509	10177	5	0.250-	1.000	0.000	0.000
4	4045	409	10177	10060	410	5	0.250-	1.000	0.000	0.000
4	4046	10177	509	510	10060	5	0.250-	1.000	0.000	0.000
4	4047	410	10060	10178	411	5	0.250-	1.000	0.000	0.000
4	4048	10060	510	511	10178	5	0.250-	1.000	0.000	0.000
4	4049	411	10178	10179	412	5	0.250-	1.000	0.000	0.000
4	4050	10178	511	512	10179	5	0.250-	1.000	0.000	0.000
4	4051	412	10179	10072	413	5	0.250-	1.000	0.000	0.000
4	4052	10179	512	513	10072	5	0.250-	1.000	0.000	0.000
4	4053	413	10072	10180	414	5	0.250-	1.000	0.000	0.000
4	4054	10072	513	514	10180	5	0.250-	1.000	0.000	0.000
4	4055	414	10180	10084	415	5	0.250-	1.000	0.000	0.000
4	4056	10180	514	515	10084	5	0.250-	1.000	0.000	0.000
8	8001	101	10181	10183	102	6	1.200	0.000	1.000	0.000
8	8002	10181	1101	1102	10183	6	1.200	0.000	1.000	0.000
8	8003	102	10183	10182	103	6	1.200	0.000	1.000	0.000
8	8004	10183	1102	1103	10182	6	1.200	0.000	1.000	0.000
8	8005	103	10182	10187	104	6	1.200	0.000	1.000	0.000
8	8006	10182	1103	10184	10187	6	1.200	0.000	1.000	0.000
8	8007	104	10187	10188	105	6	1.200	0.000	1.000	0.000

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΟΡΙΣΜΟΣ ΓΕΩΜΕΤΡΙΑΣ ΤΕΧΝΙΚΟΥ

Flat Elements

Grp	Number	Node	Node	Node	Node	MNo	t[m]	C[kNsec/m2]	direction local x		
8	8008	10187	10184	10185	10188	6	1.200		0.000	1.000	0.000
8	8009	105	10188	10186	106	6	1.200		0.000	1.000	0.000
8	8010	10188	10185	1106	10186	6	1.200		0.000	1.000	0.000
8	8011	106	10186	10190	107	6	1.200		0.000	1.000	0.000
8	8012	10186	1106	1107	10190	6	1.200		0.000	1.000	0.000
8	8013	107	10190	10189	108	6	1.200		0.000	1.000	0.000
8	8014	10190	1107	1108	10189	6	1.200		0.000	1.000	0.000
8	8015	108	10189	10192	109	6	1.200		0.000	1.000	0.000
8	8016	10189	1108	1109	10192	6	1.200		0.000	1.000	0.000
8	8017	109	10192	10191	110	6	1.200		0.000	1.000	0.000
8	8018	10192	1109	1110	10191	6	1.200		0.000	1.000	0.000
8	8019	110	10191	10196	111	6	1.200		0.000	1.000	0.000
8	8020	10191	1110	10193	10196	6	1.200		0.000	1.000	0.000
8	8021	111	10196	10197	112	6	1.200		0.000	1.000	0.000
8	8022	10196	10193	10194	10197	6	1.200		0.000	1.000	0.000
8	8023	112	10197	10195	113	6	1.200		0.000	1.000	0.000
8	8024	10197	10194	1113	10195	6	1.200		0.000	1.000	0.000
8	8025	113	10195	10199	114	6	1.200		0.000	1.000	0.000
8	8026	10195	1113	1114	10199	6	1.200		0.000	1.000	0.000
8	8027	114	10199	10198	115	6	1.200		0.000	1.000	0.000
8	8028	10199	1114	1115	10198	6	1.200		0.000	1.000	0.000
8	8029	501	502	10202	10200	6	1.200		0.000	1.000	0.000
8	8030	502	503	10201	10202	6	1.200		0.000	1.000	0.000
8	8031	10200	10202	1502	1501	6	1.200		0.000	1.000	0.000
8	8032	10202	10201	1503	1502	6	1.200		0.000	1.000	0.000
8	8033	503	504	10206	10201	6	1.200		0.000	1.000	0.000
8	8034	504	505	10207	10206	6	1.200		0.000	1.000	0.000
8	8035	505	506	10203	10207	6	1.200		0.000	1.000	0.000
8	8036	10201	10206	10204	1503	6	1.200		0.000	1.000	0.000
8	8037	10206	10207	10205	10204	6	1.200		0.000	1.000	0.000
8	8038	10207	10203	1506	10205	6	1.200		0.000	1.000	0.000
8	8039	506	507	10209	10203	6	1.200		0.000	1.000	0.000
8	8040	507	508	10208	10209	6	1.200		0.000	1.000	0.000
8	8041	10203	10209	1507	1506	6	1.200		0.000	1.000	0.000
8	8042	10209	10208	1508	1507	6	1.200		0.000	1.000	0.000
8	8043	508	509	10211	10208	6	1.200		0.000	1.000	0.000
8	8044	509	510	10210	10211	6	1.200		0.000	1.000	0.000
8	8045	10208	10211	1509	1508	6	1.200		0.000	1.000	0.000
8	8046	10211	10210	1510	1509	6	1.200		0.000	1.000	0.000
8	8047	510	511	10215	10210	6	1.200		0.000	1.000	0.000
8	8048	511	512	10216	10215	6	1.200		0.000	1.000	0.000
8	8049	512	513	10212	10216	6	1.200		0.000	1.000	0.000
8	8050	10210	10215	10213	1510	6	1.200		0.000	1.000	0.000
8	8051	10215	10216	10214	10213	6	1.200		0.000	1.000	0.000
8	8052	10216	10212	1513	10214	6	1.200		0.000	1.000	0.000
8	8053	513	514	10218	10212	6	1.200		0.000	1.000	0.000
8	8054	514	515	10217	10218	6	1.200		0.000	1.000	0.000
8	8055	10212	10218	1514	1513	6	1.200		0.000	1.000	0.000
8	8056	10218	10217	1515	1514	6	1.200		0.000	1.000	0.000
9	9001	1101	10219	10221	1102	10	1.200		0.000	1.000	0.000
9	9002	10219	2101	2102	10221	10	1.200		0.000	1.000	0.000
9	9003	1102	10221	10220	1103	10	1.200		0.000	1.000	0.000
9	9004	10221	2102	2103	10220	10	1.200		0.000	1.000	0.000
9	9005	1103	10220	10225	10184	10	1.200		0.000	1.000	0.000
9	9006	10220	2103	10222	10225	10	1.200		0.000	1.000	0.000
9	9007	10184	10225	10226	10185	10	1.200		0.000	1.000	0.000
9	9008	10225	10222	10223	10226	10	1.200		0.000	1.000	0.000
9	9009	10185	10226	10224	1106	10	1.200		0.000	1.000	0.000
9	9010	10226	10223	2106	10224	10	1.200		0.000	1.000	0.000
9	9011	1106	10224	10228	1107	10	1.200		0.000	1.000	0.000
9	9012	10224	2106	2107	10228	10	1.200		0.000	1.000	0.000
9	9013	1107	10228	10227	1108	10	1.200		0.000	1.000	0.000
9	9014	10228	2107	2108	10227	10	1.200		0.000	1.000	0.000
9	9015	1108	10227	10230	1109	10	1.200		0.000	1.000	0.000
9	9016	10227	2108	2109	10230	10	1.200		0.000	1.000	0.000
9	9017	1109	10230	10229	1110	10	1.200		0.000	1.000	0.000
9	9018	10230	2109	2110	10229	10	1.200		0.000	1.000	0.000
9	9019	1110	10229	10234	10193	10	1.200		0.000	1.000	0.000
9	9020	10229	2110	10231	10234	10	1.200		0.000	1.000	0.000
9	9021	10193	10234	10235	10194	10	1.200		0.000	1.000	0.000
9	9022	10234	10231	10232	10235	10	1.200		0.000	1.000	0.000
9	9023	10194	10235	10233	1113	10	1.200		0.000	1.000	0.000
9	9024	10235	10232	2113	10233	10	1.200		0.000	1.000	0.000
9	9025	1113	10233	10237	1114	10	1.200		0.000	1.000	0.000
9	9026	10233	2113	2114	10237	10	1.200		0.000	1.000	0.000
9	9027	1114	10237	10236	1115	10	1.200		0.000	1.000	0.000
9	9028	10237	2114	2115	10236	10	1.200		0.000	1.000	0.000
9	9029	1501	1502	10240	10238	10	1.200		0.000	1.000	0.000
9	9030	1502	1503	10239	10240	10	1.200		0.000	1.000	0.000

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΟΡΙΣΜΟΣ ΓΕΩΜΕΤΡΙΑΣ ΤΕΧΝΙΚΟΥ

Flat Elements

Grp	Number	Node	Node	Node	Node	MNo	t[m]C[kNsec/m2]	direction local x		
9	9031	10238	10240	2502	2501	10	1.200	0.000	1.000	0.000
9	9032	10240	10239	2503	2502	10	1.200	0.000	1.000	0.000
9	9033	1503	10204	10244	10239	10	1.200	0.000	1.000	0.000
9	9034	10204	10205	10245	10244	10	1.200	0.000	1.000	0.000
9	9035	10205	1506	10241	10245	10	1.200	0.000	-1.000	0.000
9	9036	10239	10244	10242	2503	10	1.200	0.000	1.000	0.000
9	9037	10244	10245	10243	10242	10	1.200	0.000	1.000	0.000
9	9038	10245	10241	2506	10243	10	1.200	0.000	-1.000	0.000
9	9039	1506	1507	10247	10241	10	1.200	0.000	1.000	0.000
9	9040	1507	1508	10246	10247	10	1.200	0.000	1.000	0.000
9	9041	10241	10247	2507	2506	10	1.200	0.000	1.000	0.000
9	9042	10247	10246	2508	2507	10	1.200	0.000	1.000	0.000
9	9043	1508	1509	10249	10246	10	1.200	0.000	1.000	0.000
9	9044	1509	1510	10248	10249	10	1.200	0.000	1.000	0.000
9	9045	10246	10249	2509	2508	10	1.200	0.000	1.000	0.000
9	9046	10249	10248	2510	2509	10	1.200	0.000	1.000	0.000
9	9047	1510	10213	10253	10248	10	1.200	0.000	1.000	0.000
9	9048	10213	10214	10254	10253	10	1.200	0.000	1.000	0.000
9	9049	10214	1513	10250	10254	10	1.200	0.000	1.000	0.000
9	9050	10248	10253	10251	2510	10	1.200	0.000	1.000	0.000
9	9051	10253	10254	10252	10251	10	1.200	0.000	1.000	0.000
9	9052	10254	10250	2513	10252	10	1.200	0.000	1.000	0.000
9	9053	1513	1514	10256	10250	10	1.200	0.000	1.000	0.000
9	9054	1514	1515	10255	10256	10	1.200	0.000	-1.000	0.000
9	9055	10250	10256	2514	2513	10	1.200	0.000	1.000	0.000
9	9056	10256	10255	2515	2514	10	1.200	0.000	-1.000	0.000

+ after t :Potential with free surface

Beam Elements

Grp	Number	Node	x[m]	NoS	NoP	reference	Hinges	direction local y-axis		
1	1001	203	0.000	1	0-0	0.000	MyMz	0.000	1.000	0.000
		10014	0.883	1		0.883				
1	1002	10014	0.000	1	0-0	0.883		0.000	1.000	0.000
		10015	0.883	1		1.767				
1	1003	10015	0.000	1	0-0	1.767		0.000	1.000	0.000
		10016	0.883	1		2.650				
1	1004	10016	0.000	1	0-0	2.650		0.000	1.000	0.000
		10017	0.883	1		3.533				
1	1005	10017	0.000	1	0-0	3.533		0.000	1.000	0.000
		10018	0.883	1		4.417				
1	1006	10018	0.000	1	0-0	4.417		0.000	1.000	0.000
		303	0.883	1		5.300				
1	1007	303	0.000	1	0-0	5.300		0.000	1.000	0.000
		10019	0.883	1		6.183				
1	1008	10019	0.000	1	0-0	6.183		0.000	1.000	0.000
		10020	0.883	1		7.067				
1	1009	10020	0.000	1	0-0	7.067		0.000	1.000	0.000
		10021	0.883	1		7.950				
1	1010	10021	0.000	1	0-0	7.950		0.000	1.000	0.000
		10022	0.883	1		8.833				
1	1011	10022	0.000	1	0-0	8.833		0.000	1.000	0.000
		10023	0.883	1		9.717				
1	1012	10023	0.000	1	0-0	9.717		0.000	1.000	0.000
		403	0.883	1		10.600	N MtMyMz			
1	1013	206	0.000	1	0-0	0.000	MyMz	0.000	1.000	0.000
		10026	0.883	1		0.883				
1	1014	10026	0.000	1	0-0	0.883		0.000	1.000	0.000
		10027	0.883	1		1.767				
1	1015	10027	0.000	1	0-0	1.767		0.000	1.000	0.000
		10028	0.883	1		2.650				
1	1016	10028	0.000	1	0-0	2.650		0.000	1.000	0.000
		10029	0.883	1		3.533				
1	1017	10029	0.000	1	0-0	3.533		0.000	1.000	0.000
		10030	0.883	1		4.417				
1	1018	10030	0.000	1	0-0	4.417		0.000	1.000	0.000
		306	0.883	1		5.300				
1	1019	306	0.000	1	0-0	5.300		0.000	1.000	0.000
		10031	0.883	1		6.183				
1	1020	10031	0.000	1	0-0	6.183		0.000	1.000	0.000
		10032	0.883	1		7.067				
1	1021	10032	0.000	1	0-0	7.067		0.000	1.000	0.000
		10033	0.883	1		7.950				
1	1022	10033	0.000	1	0-0	7.950		0.000	1.000	0.000
		10034	0.883	1		8.833				
1	1023	10034	0.000	1	0-0	8.833		0.000	1.000	0.000
		10035	0.883	1		9.717				
1	1024	10035	0.000	1	0-0	9.717		0.000	1.000	0.000
		406	0.883	1		10.600	N MtMyMz			
1	1025	208	0.000	1	0-0	0.000	MyMz	0.000	1.000	0.000

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΟΡΙΣΜΟΣ ΓΕΩΜΕΤΡΙΑΣ ΤΕΧΝΙΚΟΥ

Beam Elements

Grp	Number	Node	x[m]	NoS	NoP	reference	Hinges	direction	local	y-axis
1	1025	10038	0.883	1		0.883				
1	1026	10038	0.000	1	0-0	0.883		0.000	1.000	0.000
		10039	0.883	1		1.767				
1	1027	10039	0.000	1	0-0	1.767		0.000	1.000	0.000
		10040	0.883	1		2.650				
1	1028	10040	0.000	1	0-0	2.650		0.000	1.000	0.000
		10041	0.883	1		3.533				
1	1029	10041	0.000	1	0-0	3.533		0.000	1.000	0.000
		10042	0.883	1		4.417				
1	1030	10042	0.000	1	0-0	4.417		0.000	1.000	0.000
		308	0.883	1		5.300				
1	1031	308	0.000	1	0-0	5.300		0.000	1.000	0.000
		10043	0.883	1		6.183				
1	1032	10043	0.000	1	0-0	6.183		0.000	1.000	0.000
		10044	0.883	1		7.067				
1	1033	10044	0.000	1	0-0	7.067		0.000	1.000	0.000
		10045	0.883	1		7.950				
1	1034	10045	0.000	1	0-0	7.950		0.000	1.000	0.000
		10046	0.883	1		8.833				
1	1035	10046	0.000	1	0-0	8.833		0.000	1.000	0.000
		10047	0.883	1		9.717				
1	1036	10047	0.000	1	0-0	9.717		0.000	1.000	0.000
		408	0.883	1		10.600	N MtMyMz			
1	1037	210	0.000	1	0-0	0.000	MyMz	0.000	1.000	0.000
		10050	0.883	1		0.883				
1	1038	10050	0.000	1	0-0	0.883		0.000	1.000	0.000
		10051	0.883	1		1.767				
1	1039	10051	0.000	1	0-0	1.767		0.000	1.000	0.000
		10052	0.883	1		2.650				
1	1040	10052	0.000	1	0-0	2.650		0.000	1.000	0.000
		10053	0.883	1		3.533				
1	1041	10053	0.000	1	0-0	3.533		0.000	1.000	0.000
		10054	0.883	1		4.417				
1	1042	10054	0.000	1	0-0	4.417		0.000	1.000	0.000
		310	0.883	1		5.300				
1	1043	310	0.000	1	0-0	5.300		0.000	1.000	0.000
		10055	0.883	1		6.183				
1	1044	10055	0.000	1	0-0	6.183		0.000	1.000	0.000
		10056	0.883	1		7.067				
1	1045	10056	0.000	1	0-0	7.067		0.000	1.000	0.000
		10057	0.883	1		7.950				
1	1046	10057	0.000	1	0-0	7.950		0.000	1.000	0.000
		10058	0.883	1		8.833				
1	1047	10058	0.000	1	0-0	8.833		0.000	1.000	0.000
		10059	0.883	1		9.717				
1	1048	10059	0.000	1	0-0	9.717		0.000	1.000	0.000
		410	0.883	1		10.600	N MtMyMz			
1	1049	213	0.000	1	0-0	0.000	MyMz	0.000	1.000	0.000
		10062	0.883	1		0.883				
1	1050	10062	0.000	1	0-0	0.883		0.000	1.000	0.000
		10063	0.883	1		1.767				
1	1051	10063	0.000	1	0-0	1.767		0.000	1.000	0.000
		10064	0.883	1		2.650				
1	1052	10064	0.000	1	0-0	2.650		0.000	1.000	0.000
		10065	0.883	1		3.533				
1	1053	10065	0.000	1	0-0	3.533		0.000	1.000	0.000
		10066	0.883	1		4.417				
1	1054	10066	0.000	1	0-0	4.417		0.000	1.000	0.000
		313	0.883	1		5.300				
1	1055	313	0.000	1	0-0	5.300		0.000	1.000	0.000
		10067	0.883	1		6.183				
1	1056	10067	0.000	1	0-0	6.183		0.000	1.000	0.000
		10068	0.883	1		7.067				
1	1057	10068	0.000	1	0-0	7.067		0.000	1.000	0.000
		10069	0.883	1		7.950				
1	1058	10069	0.000	1	0-0	7.950		0.000	1.000	0.000
		10070	0.883	1		8.833				
1	1059	10070	0.000	1	0-0	8.833		0.000	1.000	0.000
		10071	0.883	1		9.717				
1	1060	10071	0.000	1	0-0	9.717		0.000	1.000	0.000
		413	0.883	1		10.600	N MtMyMz			
2	2001	103	0.000	2	0-0	0.000		0.000	1.000	0.000
		10013	0.300	2		0.300				
2	2002	10013	0.000	2	0-0	0.300		0.000	1.000	0.000
		203	0.300	2		0.600				
2	2003	403	0.000	2	0-0	0.000		0.000	1.000	0.000
		10024	0.300	2		0.300				
2	2004	10024	0.000	2	0-0	0.300		0.000	1.000	0.000
		503	0.300	2		0.600				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΟΡΙΣΜΟΣ ΓΕΩΜΕΤΡΙΑΣ ΤΕΧΝΙΚΟΥ

Beam Elements

Grp	Number	Node	x[m]	NoS	NoP	reference	Hinges	direction	local y-axis	
2	2005	106	0.000	2	0-0	0.000		0.000	1.000	0.000
		10025	0.300	2		0.300				
2	2006	10025	0.000	2	0-0	0.300		0.000	1.000	0.000
		206	0.300	2		0.600				
2	2007	406	0.000	2	0-0	0.000		0.000	1.000	0.000
		10036	0.300	2		0.300				
2	2008	10036	0.000	2	0-0	0.300		0.000	1.000	0.000
		506	0.300	2		0.600				
2	2009	108	0.000	2	0-0	0.000		0.000	1.000	0.000
		10037	0.300	2		0.300				
2	2010	10037	0.000	2	0-0	0.300		0.000	1.000	0.000
		208	0.300	2		0.600				
2	2011	408	0.000	2	0-0	0.000		0.000	1.000	0.000
		10048	0.300	2		0.300				
2	2012	10048	0.000	2	0-0	0.300		0.000	1.000	0.000
		508	0.300	2		0.600				
2	2013	110	0.000	2	0-0	0.000		0.000	1.000	0.000
		10049	0.300	2		0.300				
2	2014	10049	0.000	2	0-0	0.300		0.000	1.000	0.000
		210	0.300	2		0.600				
2	2015	410	0.000	2	0-0	0.000		0.000	1.000	0.000
		10060	0.300	2		0.300				
2	2016	10060	0.000	2	0-0	0.300		0.000	1.000	0.000
		510	0.300	2		0.600				
2	2017	113	0.000	2	0-0	0.000		0.000	1.000	0.000
		10061	0.300	2		0.300				
2	2018	10061	0.000	2	0-0	0.300		0.000	1.000	0.000
		213	0.300	2		0.600				
2	2019	413	0.000	2	0-0	0.000		0.000	1.000	0.000
		10072	0.300	2		0.300				
2	2020	10072	0.000	2	0-0	0.300		0.000	1.000	0.000
		513	0.300	2		0.600				
10	10001	2101	0.000	8		0.000		1.000	0.000	0.000
		2102	0.200	8		0.200				
10	10002	2102	0.000	10		0.200		1.000	0.000	0.000
		2103	0.200	10		0.400				
10	10003	2103	0.000	10		0.400		1.000	0.000	0.000
		2104	0.200	10		0.600				
10	10004	2104	0.000	10		0.600		1.000	0.000	0.000
		2105	0.400	10		1.000				
10	10005	2105	0.000	8		1.000		1.000	0.000	0.000
		2106	0.200	8		1.200				
10	10006	2106	0.000	8		1.200		1.000	0.000	0.000
		2107	0.400	8		1.600				
10	10007	2107	0.000	10		1.600		1.000	0.000	0.000
		2108	0.400	10		2.000				
10	10008	2108	0.000	10		2.000		1.000	0.000	0.000
		2109	0.400	10		2.400				
10	10009	2109	0.000	8		2.400		1.000	0.000	0.000
		2110	0.400	8		2.800				
10	10010	2110	0.000	8		2.800		1.000	0.000	0.000
		2111	0.200	8		3.000				
10	10011	2111	0.000	10		3.000		1.000	0.000	0.000
		2112	0.400	10		3.400				
10	10012	2112	0.000	10		3.400		1.000	0.000	0.000
		2113	0.200	10		3.600				
10	10013	2113	0.000	10		3.600		1.000	0.000	0.000
		2114	0.200	10		3.800				
10	10014	2114	0.000	8		3.800		1.000	0.000	0.000
		2115	0.200	8		4.000				
10	10016	2501	0.000	8		0.000		1.000	0.000	0.000
		2502	0.200	8		0.200				
10	10017	2502	0.000	10		0.200		1.000	0.000	0.000
		2503	0.200	10		0.400				
10	10018	2503	0.000	10		0.400		1.000	0.000	0.000
		2504	0.200	10		0.600				
10	10019	2504	0.000	10		0.600		1.000	0.000	0.000
		2505	0.400	10		1.000				
10	10020	2505	0.000	8		1.000		1.000	0.000	0.000
		2506	0.200	8		1.200				
10	10021	2506	0.000	8		1.200		1.000	0.000	0.000
		2507	0.400	8		1.600				
10	10022	2507	0.000	10		1.600		1.000	0.000	0.000
		2508	0.400	10		2.000				
10	10023	2508	0.000	10		2.000		1.000	0.000	0.000
		2509	0.400	10		2.400				
10	10024	2509	0.000	8		2.400		1.000	0.000	0.000
		2510	0.400	8		2.800				
10	10025	2510	0.000	8		2.800		1.000	0.000	0.000

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΟΡΙΣΜΟΣ ΓΕΩΜΕΤΡΙΑΣ ΤΕΧΝΙΚΟΥ

Beam Elements

Grp	Number	Node	x[m]	NoS	NoP	reference	Hinges	direction	local	y-axis
10	10025	2511	0.200	8		3.000				
10	10026	2511	0.000	10		3.000		1.000	0.000	0.000
		2512	0.400	10		3.400				
10	10027	2512	0.000	10		3.400		1.000	0.000	0.000
		2513	0.200	10		3.600				
10	10028	2513	0.000	10		3.600		1.000	0.000	0.000
		2514	0.200	10		3.800				
10	10029	2514	0.000	8		3.800		1.000	0.000	0.000
		2515	0.200	8		4.000				
11	11001	2104	0.000	13		0.000		1.000	0.000	0.000
		3104	0.550	13		0.550				
11	11002	2108	0.000	13		0.000		1.000	0.000	0.000
		3108	0.550	13		0.550				
11	11003	2112	0.000	13		0.000		1.000	0.000	0.000
		3112	0.550	13		0.550				
11	11004	2504	0.000	13		0.000		1.000	0.000	0.000
		3504	0.550	13		0.550				
11	11005	2508	0.000	13		0.000		1.000	0.000	0.000
		3508	0.550	13		0.550				
11	11006	2512	0.000	13		0.000		1.000	0.000	0.000
		3512	0.550	13		0.550				
12	12001	3104	0.000	9	1	0.000		1.000	0.000	0.000
		10257	1.000	9		1.000				
12	12002	10257	0.000	9	1	1.000		1.000	0.000	0.000
		10258	1.000	9		2.000				
12	12003	10258	0.000	9	1	2.000		1.000	0.000	0.000
		4104	1.000	9		3.000				
12	12004	3108	0.000	9	1	0.000		1.000	0.000	0.000
		10259	1.000	9		1.000				
12	12005	10259	0.000	9	1	1.000		1.000	0.000	0.000
		10260	1.000	9		2.000				
12	12006	10260	0.000	9	1	2.000		1.000	0.000	0.000
		4108	1.000	9		3.000				
12	12007	3112	0.000	9	1	0.000		1.000	0.000	0.000
		10261	1.000	9		1.000				
12	12008	10261	0.000	9	1	1.000		1.000	0.000	0.000
		10262	1.000	9		2.000				
12	12009	10262	0.000	9	1	2.000		1.000	0.000	0.000
		4112	1.000	9		3.000				
12	12010	3504	0.000	9	1	0.000		1.000	0.000	0.000
		10263	1.000	9		1.000				
12	12011	10263	0.000	9	1	1.000		1.000	0.000	0.000
		10264	1.000	9		2.000				
12	12012	10264	0.000	9	1	2.000		1.000	0.000	0.000
		4504	1.000	9		3.000				
12	12013	3508	0.000	9	1	0.000		1.000	0.000	0.000
		10265	1.000	9		1.000				
12	12014	10265	0.000	9	1	1.000		1.000	0.000	0.000
		10266	1.000	9		2.000				
12	12015	10266	0.000	9	1	2.000		1.000	0.000	0.000
		4508	1.000	9		3.000				
12	12016	3512	0.000	9	1	0.000		1.000	0.000	0.000
		10267	1.000	9		1.000				
12	12017	10267	0.000	9	1	1.000		1.000	0.000	0.000
		10268	1.000	9		2.000				
12	12018	10268	0.000	9	1	2.000		1.000	0.000	0.000
		4512	1.000	9		3.000				
12	12019	4104	0.000	9	1	0.000		1.000	0.000	0.000
		10269	1.000	9		1.000				
12	12020	10269	0.000	9	1	1.000		1.000	0.000	0.000
		10270	1.000	9		2.000				
12	12021	10270	0.000	9	1	2.000		1.000	0.000	0.000
		10271	1.000	9		3.000				
12	12022	10271	0.000	9	1	3.000		1.000	0.000	0.000
		10272	1.000	9		4.000				
12	12023	10272	0.000	9	1	4.000		1.000	0.000	0.000
		10273	1.000	9		5.000				
12	12024	10273	0.000	9	1	5.000		1.000	0.000	0.000
		10274	1.000	9		6.000				
12	12025	10274	0.000	9	1	6.000		1.000	0.000	0.000
		10275	1.000	9		7.000				
12	12026	10275	0.000	9	1	7.000		1.000	0.000	0.000
		10276	1.000	9		8.000				
12	12027	10276	0.000	9	1	8.000		1.000	0.000	0.000
		10277	1.000	9		9.000				
12	12028	10277	0.000	9	1	9.000		1.000	0.000	0.000
		10278	1.000	9		10.000				
12	12029	10278	0.000	9	1	10.000		1.000	0.000	0.000
		10279	1.000	9		11.000				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΟΡΙΣΜΟΣ ΓΕΩΜΕΤΡΙΑΣ ΤΕΧΝΙΚΟΥ

Beam Elements

Grp	Number	Node	x[m]	NoS	NoP	reference	Hinges	direction	local y-axis	
12	12030	10279	0.000	9	1	11.000		1.000	0.000	0.000
		10280	1.000	9		12.000				
12	12031	10280	0.000	9	1	12.000		1.000	0.000	0.000
		10281	1.000	9		13.000				
12	12032	10281	0.000	9	1	13.000		1.000	0.000	0.000
		10282	1.000	9		14.000				
12	12033	10282	0.000	9	1	14.000		1.000	0.000	0.000
		10283	1.000	9		15.000				
12	12034	10283	0.000	9	1	15.000		1.000	0.000	0.000
		10284	1.000	9		16.000				
12	12035	10284	0.000	9	1	16.000		1.000	0.000	0.000
		10285	1.000	9		17.000				
12	12036	10285	0.000	9	1	17.000		1.000	0.000	0.000
		10286	1.000	9		18.000				
12	12037	10286	0.000	9	1	18.000		1.000	0.000	0.000
		10287	1.000	9		19.000				
12	12038	10287	0.000	9	1	19.000		1.000	0.000	0.000
		10288	1.000	9		20.000				
12	12039	10288	0.000	9	1	20.000		1.000	0.000	0.000
		10289	1.000	9		21.000				
12	12040	10289	0.000	9	1	21.000		1.000	0.000	0.000
		5104	1.000	9		22.000				
12	12041	4108	0.000	9	1	0.000		1.000	0.000	0.000
		10290	1.000	9		1.000				
12	12042	10290	0.000	9	1	1.000		1.000	0.000	0.000
		10291	1.000	9		2.000				
12	12043	10291	0.000	9	1	2.000		1.000	0.000	0.000
		10292	1.000	9		3.000				
12	12044	10292	0.000	9	1	3.000		1.000	0.000	0.000
		10293	1.000	9		4.000				
12	12045	10293	0.000	9	1	4.000		1.000	0.000	0.000
		10294	1.000	9		5.000				
12	12046	10294	0.000	9	1	5.000		1.000	0.000	0.000
		10295	1.000	9		6.000				
12	12047	10295	0.000	9	1	6.000		1.000	0.000	0.000
		10296	1.000	9		7.000				
12	12048	10296	0.000	9	1	7.000		1.000	0.000	0.000
		10297	1.000	9		8.000				
12	12049	10297	0.000	9	1	8.000		1.000	0.000	0.000
		10298	1.000	9		9.000				
12	12050	10298	0.000	9	1	9.000		1.000	0.000	0.000
		10299	1.000	9		10.000				
12	12051	10299	0.000	9	1	10.000		1.000	0.000	0.000
		10300	1.000	9		11.000				
12	12052	10300	0.000	9	1	11.000		1.000	0.000	0.000
		10301	1.000	9		12.000				
12	12053	10301	0.000	9	1	12.000		1.000	0.000	0.000
		10302	1.000	9		13.000				
12	12054	10302	0.000	9	1	13.000		1.000	0.000	0.000
		10303	1.000	9		14.000				
12	12055	10303	0.000	9	1	14.000		1.000	0.000	0.000
		10304	1.000	9		15.000				
12	12056	10304	0.000	9	1	15.000		1.000	0.000	0.000
		10305	1.000	9		16.000				
12	12057	10305	0.000	9	1	16.000		1.000	0.000	0.000
		10306	1.000	9		17.000				
12	12058	10306	0.000	9	1	17.000		1.000	0.000	0.000
		10307	1.000	9		18.000				
12	12059	10307	0.000	9	1	18.000		1.000	0.000	0.000
		10308	1.000	9		19.000				
12	12060	10308	0.000	9	1	19.000		1.000	0.000	0.000
		10309	1.000	9		20.000				
12	12061	10309	0.000	9	1	20.000		1.000	0.000	0.000
		10310	1.000	9		21.000				
12	12062	10310	0.000	9	1	21.000		1.000	0.000	0.000
		5108	1.000	9		22.000				
12	12063	4112	0.000	9	1	0.000		1.000	0.000	0.000
		10311	1.000	9		1.000				
12	12064	10311	0.000	9	1	1.000		1.000	0.000	0.000
		10312	1.000	9		2.000				
12	12065	10312	0.000	9	1	2.000		1.000	0.000	0.000
		10313	1.000	9		3.000				
12	12066	10313	0.000	9	1	3.000		1.000	0.000	0.000
		10314	1.000	9		4.000				
12	12067	10314	0.000	9	1	4.000		1.000	0.000	0.000
		10315	1.000	9		5.000				
12	12068	10315	0.000	9	1	5.000		1.000	0.000	0.000
		10316	1.000	9		6.000				
12	12069	10316	0.000	9	1	6.000		1.000	0.000	0.000

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΟΡΙΣΜΟΣ ΓΕΩΜΕΤΡΙΑΣ ΤΕΧΝΙΚΟΥ

Beam Elements

Grp	Number	Node	x[m]	NoS	NoP	reference	Hinges	direction	local	y-axis
12	12069	10317	1.000	9		7.000				
12	12070	10317	0.000	9	1	7.000		1.000	0.000	0.000
		10318	1.000	9		8.000				
12	12071	10318	0.000	9	1	8.000		1.000	0.000	0.000
		10319	1.000	9		9.000				
12	12072	10319	0.000	9	1	9.000		1.000	0.000	0.000
		10320	1.000	9		10.000				
12	12073	10320	0.000	9	1	10.000		1.000	0.000	0.000
		10321	1.000	9		11.000				
12	12074	10321	0.000	9	1	11.000		1.000	0.000	0.000
		10322	1.000	9		12.000				
12	12075	10322	0.000	9	1	12.000		1.000	0.000	0.000
		10323	1.000	9		13.000				
12	12076	10323	0.000	9	1	13.000		1.000	0.000	0.000
		10324	1.000	9		14.000				
12	12077	10324	0.000	9	1	14.000		1.000	0.000	0.000
		10325	1.000	9		15.000				
12	12078	10325	0.000	9	1	15.000		1.000	0.000	0.000
		10326	1.000	9		16.000				
12	12079	10326	0.000	9	1	16.000		1.000	0.000	0.000
		10327	1.000	9		17.000				
12	12080	10327	0.000	9	1	17.000		1.000	0.000	0.000
		10328	1.000	9		18.000				
12	12081	10328	0.000	9	1	18.000		1.000	0.000	0.000
		10329	1.000	9		19.000				
12	12082	10329	0.000	9	1	19.000		1.000	0.000	0.000
		10330	1.000	9		20.000				
12	12083	10330	0.000	9	1	20.000		1.000	0.000	0.000
		10331	1.000	9		21.000				
12	12084	10331	0.000	9	1	21.000		1.000	0.000	0.000
		5112	1.000	9		22.000				
12	12085	4504	0.000	9	1	0.000		1.000	0.000	0.000
		10332	1.000	9		1.000				
12	12086	10332	0.000	9	1	1.000		1.000	0.000	0.000
		10333	1.000	9		2.000				
12	12087	10333	0.000	9	1	2.000		1.000	0.000	0.000
		10334	1.000	9		3.000				
12	12088	10334	0.000	9	1	3.000		1.000	0.000	0.000
		10335	1.000	9		4.000				
12	12089	10335	0.000	9	1	4.000		1.000	0.000	0.000
		10336	1.000	9		5.000				
12	12090	10336	0.000	9	1	5.000		1.000	0.000	0.000
		10337	1.000	9		6.000				
12	12091	10337	0.000	9	1	6.000		1.000	0.000	0.000
		10338	1.000	9		7.000				
12	12092	10338	0.000	9	1	7.000		1.000	0.000	0.000
		10339	1.000	9		8.000				
12	12093	10339	0.000	9	1	8.000		1.000	0.000	0.000
		10340	1.000	9		9.000				
12	12094	10340	0.000	9	1	9.000		1.000	0.000	0.000
		10341	1.000	9		10.000				
12	12095	10341	0.000	9	1	10.000		1.000	0.000	0.000
		10342	1.000	9		11.000				
12	12096	10342	0.000	9	1	11.000		1.000	0.000	0.000
		10343	1.000	9		12.000				
12	12097	10343	0.000	9	1	12.000		1.000	0.000	0.000
		10344	1.000	9		13.000				
12	12098	10344	0.000	9	1	13.000		1.000	0.000	0.000
		10345	1.000	9		14.000				
12	12099	10345	0.000	9	1	14.000		1.000	0.000	0.000
		10346	1.000	9		15.000				
12	12100	10346	0.000	9	1	15.000		1.000	0.000	0.000
		10347	1.000	9		16.000				
12	12101	10347	0.000	9	1	16.000		1.000	0.000	0.000
		10348	1.000	9		17.000				
12	12102	10348	0.000	9	1	17.000		1.000	0.000	0.000
		10349	1.000	9		18.000				
12	12103	10349	0.000	9	1	18.000		1.000	0.000	0.000
		10350	1.000	9		19.000				
12	12104	10350	0.000	9	1	19.000		1.000	0.000	0.000
		10351	1.000	9		20.000				
12	12105	10351	0.000	9	1	20.000		1.000	0.000	0.000
		10352	1.000	9		21.000				
12	12106	10352	0.000	9	1	21.000		1.000	0.000	0.000
		5504	1.000	9		22.000				
12	12107	4508	0.000	9	1	0.000		1.000	0.000	0.000
		10353	1.000	9		1.000				
12	12108	10353	0.000	9	1	1.000		1.000	0.000	0.000
		10354	1.000	9		2.000				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΟΡΙΣΜΟΣ ΓΕΩΜΕΤΡΙΑΣ ΤΕΧΝΙΚΟΥ

Beam Elements

Grp	Number	Node	x[m]	NoS	NoP	reference	Hinges	direction	local y-axis	
12	12109	10354	0.000	9	1	2.000		1.000	0.000	0.000
		10355	1.000	9		3.000				
12	12110	10355	0.000	9	1	3.000		1.000	0.000	0.000
		10356	1.000	9		4.000				
12	12111	10356	0.000	9	1	4.000		1.000	0.000	0.000
		10357	1.000	9		5.000				
12	12112	10357	0.000	9	1	5.000		1.000	0.000	0.000
		10358	1.000	9		6.000				
12	12113	10358	0.000	9	1	6.000		1.000	0.000	0.000
		10359	1.000	9		7.000				
12	12114	10359	0.000	9	1	7.000		1.000	0.000	0.000
		10360	1.000	9		8.000				
12	12115	10360	0.000	9	1	8.000		1.000	0.000	0.000
		10361	1.000	9		9.000				
12	12116	10361	0.000	9	1	9.000		1.000	0.000	0.000
		10362	1.000	9		10.000				
12	12117	10362	0.000	9	1	10.000		1.000	0.000	0.000
		10363	1.000	9		11.000				
12	12118	10363	0.000	9	1	11.000		1.000	0.000	0.000
		10364	1.000	9		12.000				
12	12119	10364	0.000	9	1	12.000		1.000	0.000	0.000
		10365	1.000	9		13.000				
12	12120	10365	0.000	9	1	13.000		1.000	0.000	0.000
		10366	1.000	9		14.000				
12	12121	10366	0.000	9	1	14.000		1.000	0.000	0.000
		10367	1.000	9		15.000				
12	12122	10367	0.000	9	1	15.000		1.000	0.000	0.000
		10368	1.000	9		16.000				
12	12123	10368	0.000	9	1	16.000		1.000	0.000	0.000
		10369	1.000	9		17.000				
12	12124	10369	0.000	9	1	17.000		1.000	0.000	0.000
		10370	1.000	9		18.000				
12	12125	10370	0.000	9	1	18.000		1.000	0.000	0.000
		10371	1.000	9		19.000				
12	12126	10371	0.000	9	1	19.000		1.000	0.000	0.000
		10372	1.000	9		20.000				
12	12127	10372	0.000	9	1	20.000		1.000	0.000	0.000
		10373	1.000	9		21.000				
12	12128	10373	0.000	9	1	21.000		1.000	0.000	0.000
		5508	1.000	9		22.000				
12	12129	4512	0.000	9	1	0.000		1.000	0.000	0.000
		10374	1.000	9		1.000				
12	12130	10374	0.000	9	1	1.000		1.000	0.000	0.000
		10375	1.000	9		2.000				
12	12131	10375	0.000	9	1	2.000		1.000	0.000	0.000
		10376	1.000	9		3.000				
12	12132	10376	0.000	9	1	3.000		1.000	0.000	0.000
		10377	1.000	9		4.000				
12	12133	10377	0.000	9	1	4.000		1.000	0.000	0.000
		10378	1.000	9		5.000				
12	12134	10378	0.000	9	1	5.000		1.000	0.000	0.000
		10379	1.000	9		6.000				
12	12135	10379	0.000	9	1	6.000		1.000	0.000	0.000
		10380	1.000	9		7.000				
12	12136	10380	0.000	9	1	7.000		1.000	0.000	0.000
		10381	1.000	9		8.000				
12	12137	10381	0.000	9	1	8.000		1.000	0.000	0.000
		10382	1.000	9		9.000				
12	12138	10382	0.000	9	1	9.000		1.000	0.000	0.000
		10383	1.000	9		10.000				
12	12139	10383	0.000	9	1	10.000		1.000	0.000	0.000
		10384	1.000	9		11.000				
12	12140	10384	0.000	9	1	11.000		1.000	0.000	0.000
		10385	1.000	9		12.000				
12	12141	10385	0.000	9	1	12.000		1.000	0.000	0.000
		10386	1.000	9		13.000				
12	12142	10386	0.000	9	1	13.000		1.000	0.000	0.000
		10387	1.000	9		14.000				
12	12143	10387	0.000	9	1	14.000		1.000	0.000	0.000
		10388	1.000	9		15.000				
12	12144	10388	0.000	9	1	15.000		1.000	0.000	0.000
		10389	1.000	9		16.000				
12	12145	10389	0.000	9	1	16.000		1.000	0.000	0.000
		10390	1.000	9		17.000				
12	12146	10390	0.000	9	1	17.000		1.000	0.000	0.000
		10391	1.000	9		18.000				
12	12147	10391	0.000	9	1	18.000		1.000	0.000	0.000
		10392	1.000	9		19.000				
12	12148	10392	0.000	9	1	19.000		1.000	0.000	0.000

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΟΡΙΣΜΟΣ ΓΕΩΜΕΤΡΙΑΣ ΤΕΧΝΙΚΟΥ

Beam Elements

Grp	Number	Node	x[m]	NoS	NoP	reference	Hinges	direction local y-axis		
12	12148	10393	1.000	9		20.000				
12	12149	10393	0.000	9	1	20.000		1.000	0.000	0.000
		10394	1.000	9		21.000				
12	12150	10394	0.000	9	1	21.000		1.000	0.000	0.000
		5512	1.000	9		22.000				

Spring Elements

Grp	Number	Node	Node	dX[-] e-P[-]	dY[-] e-T[-]	dZ[-] e-M[-]	CP[MN/m] DP[kNsec/m]	CT[MN/m] DT[kNsec/m]	CM[MNm] DM[kNmsec]
13	13001	5104		0.000	0.000	1.000	2.513E+02		
13	13002	5108		0.000	0.000	1.000	2.513E+02		
13	13003	5112		0.000	0.000	1.000	2.513E+02		
13	13004	5504		0.000	0.000	1.000	2.513E+02		
13	13005	5508		0.000	0.000	1.000	2.513E+02		
13	13006	5512		0.000	0.000	1.000	2.513E+02		

Summary of all beam elements

Groups

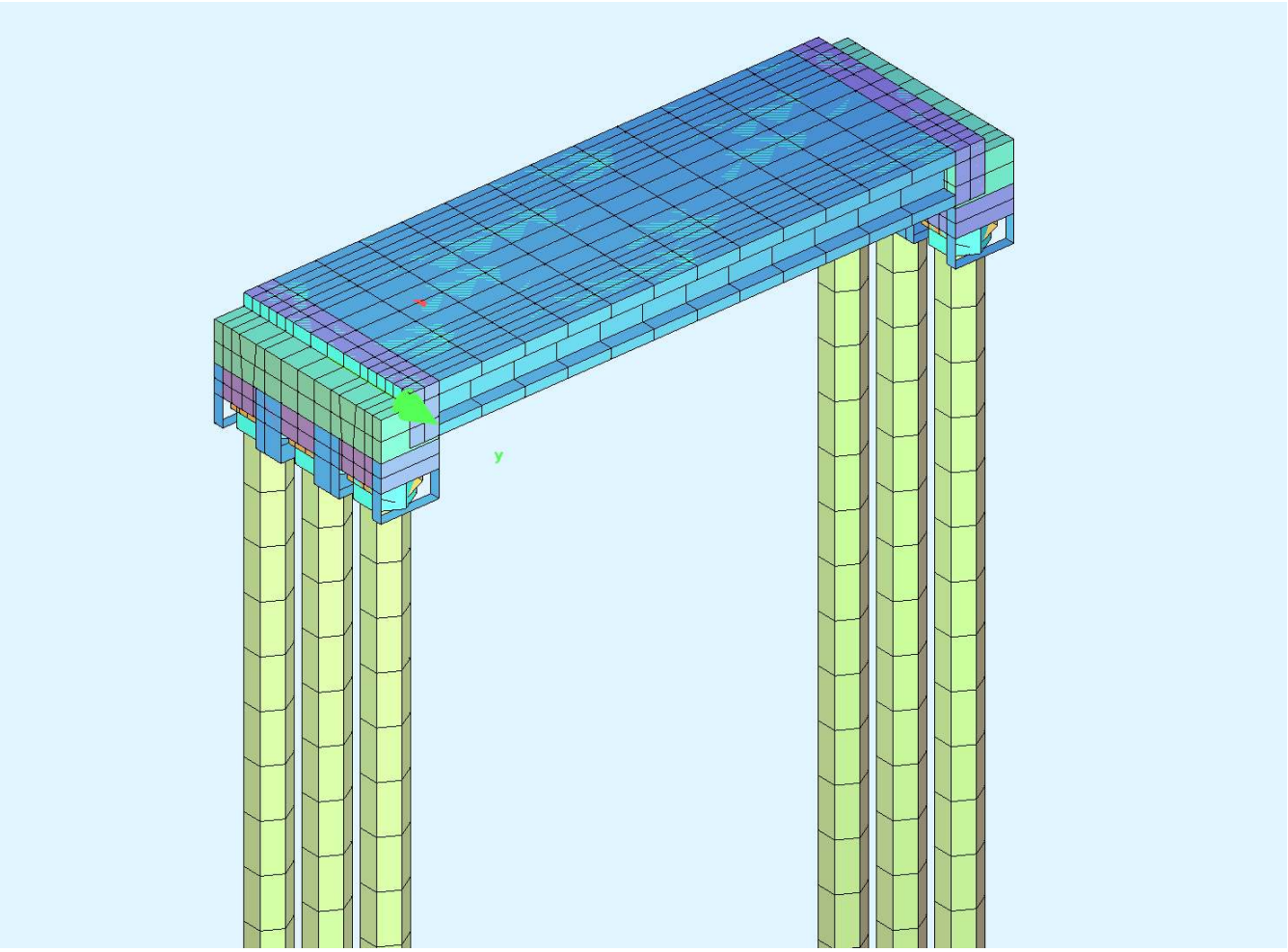
Grp	TotLength [m]	Max.Length [m]	TotWeight [t]	Surface [m2]
1	53.000	0.883	0.000	111.300
2	6.000	0.300	0.000	21.600
10	8.000	0.400	24.960	14.720
11	3.300	0.550	0.000	
12	150.000	1.000	188.496	376.991
Sum	220.300		213.456	524.611

Summary of all planar elements

Groups

Grp	TotArea [m2]	TotVolume [m3]	TotWeight [t]	Material No.
3	42.400	10.600	0.000	5
4	4.800	1.200	0.000	5
8	6.400	7.680	25.200	6
9	4.400	5.280	0.000	10
Sum	58.000	24.760	25.200	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00



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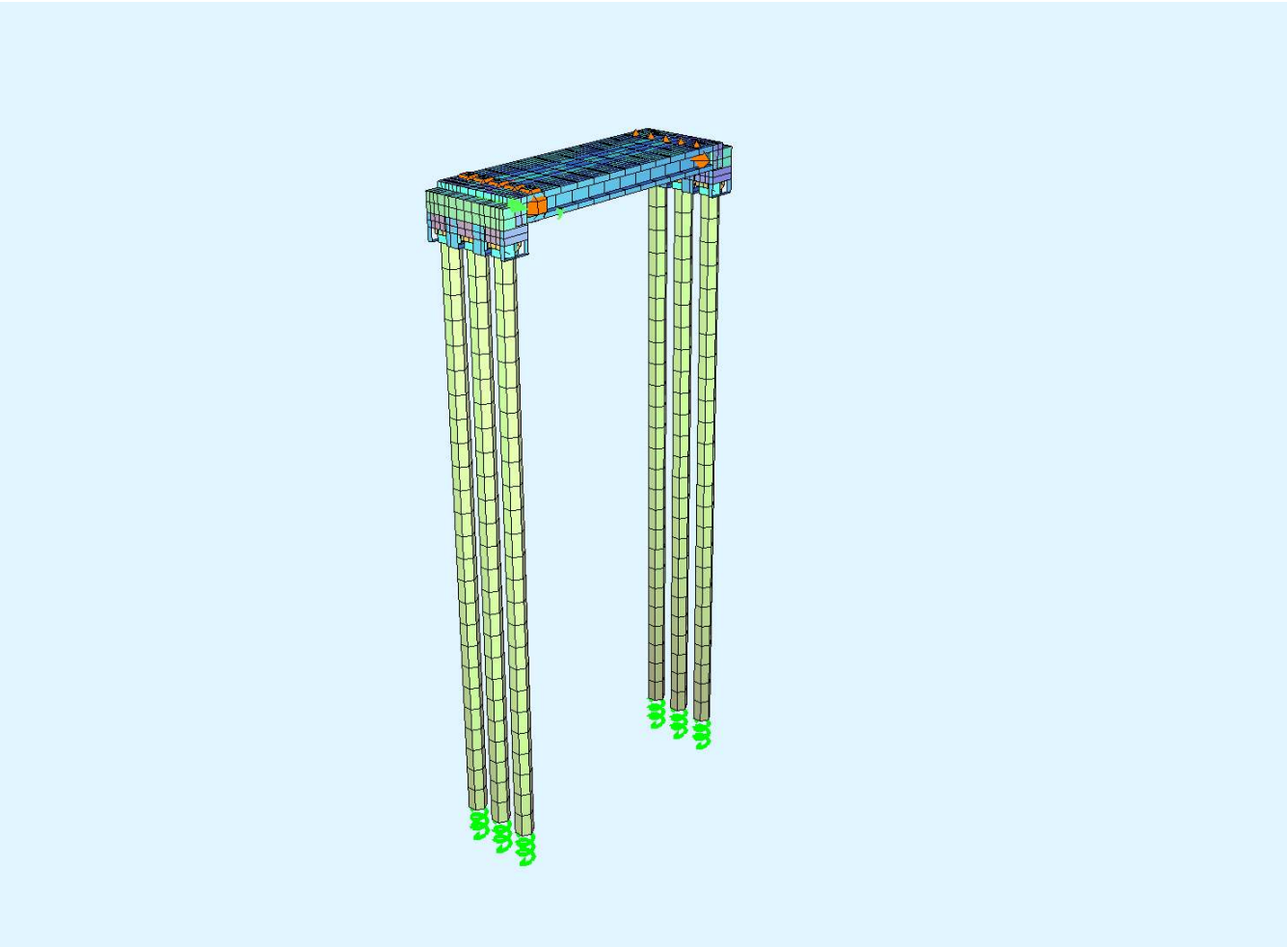

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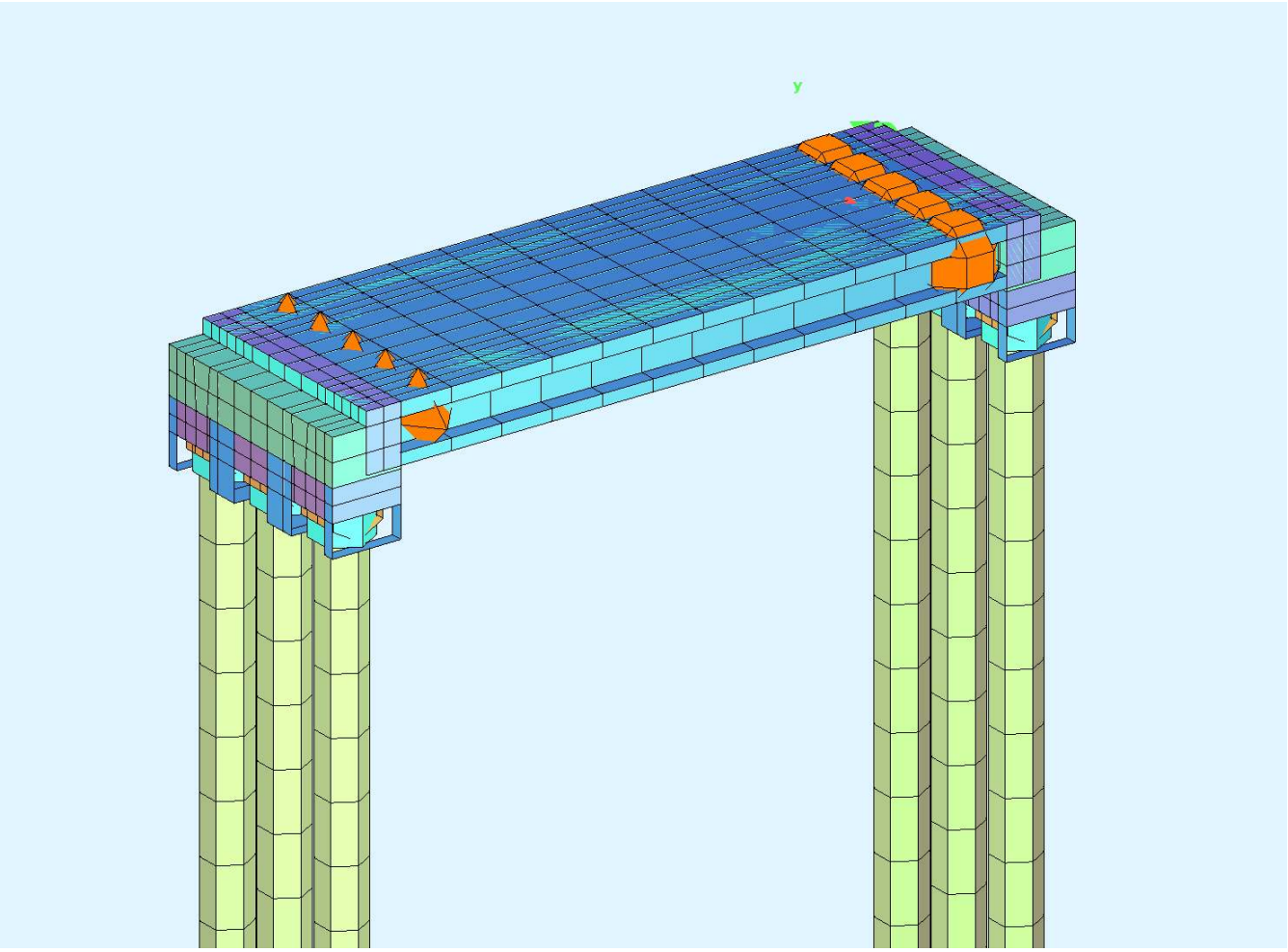

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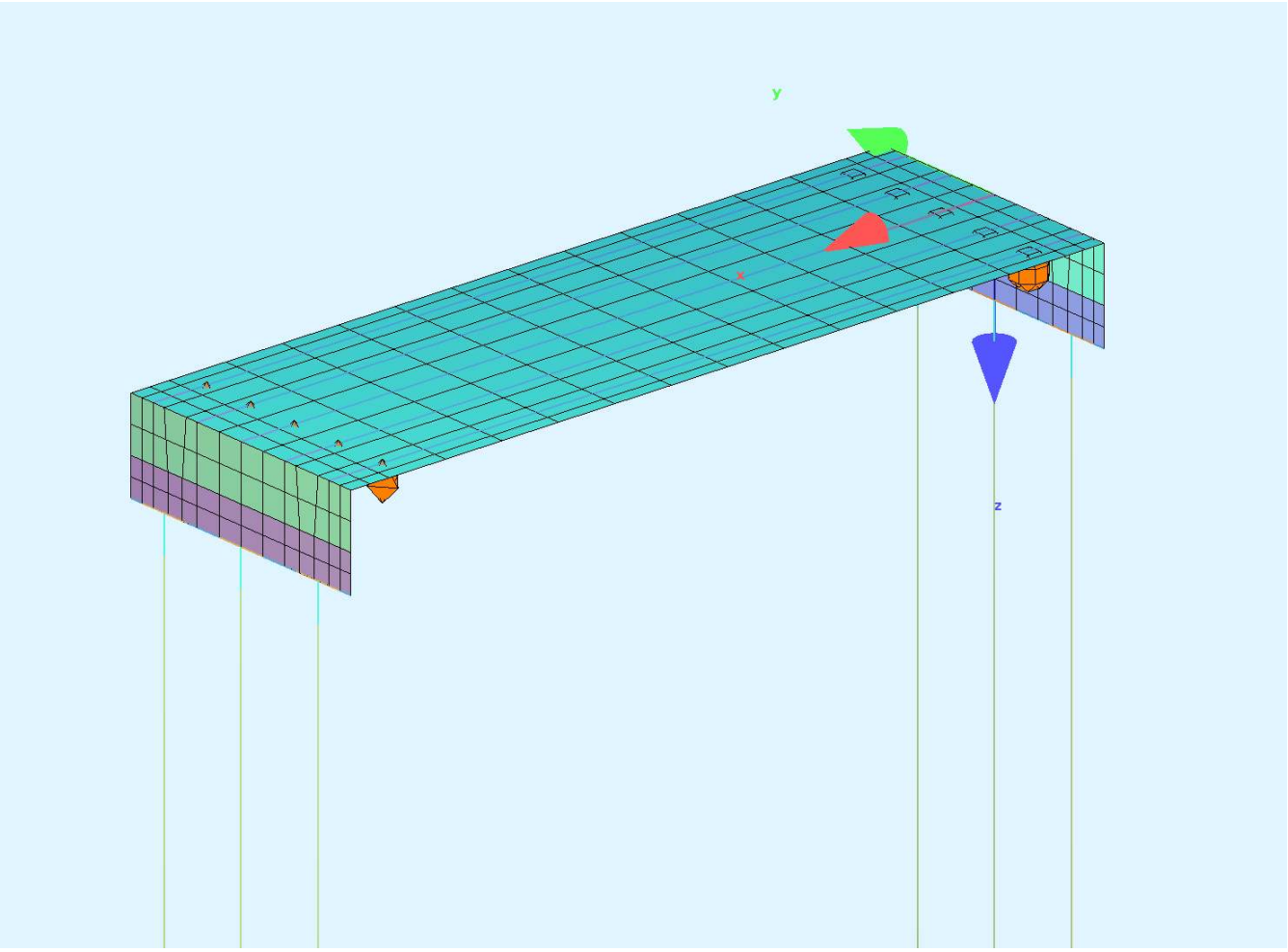

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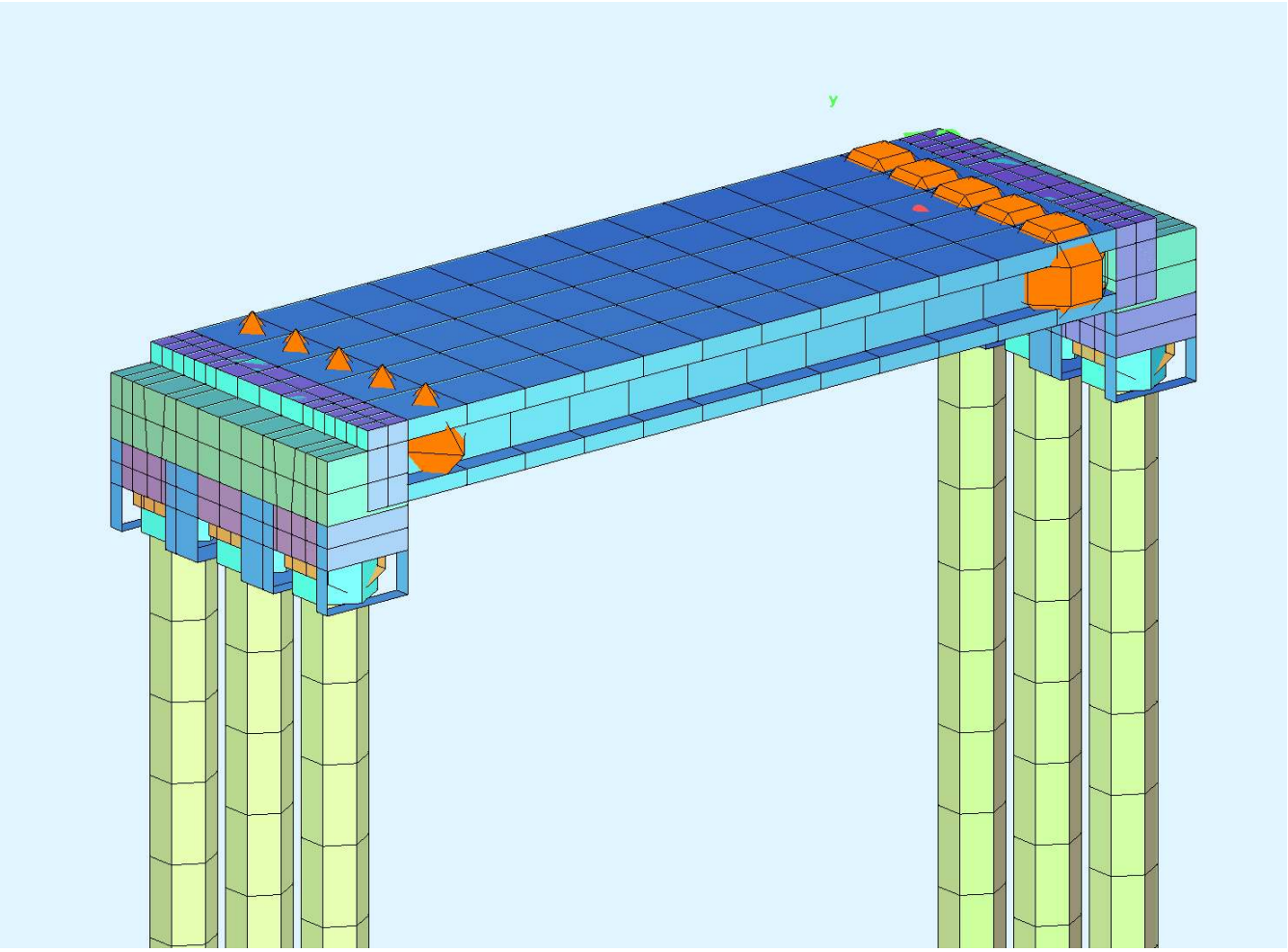

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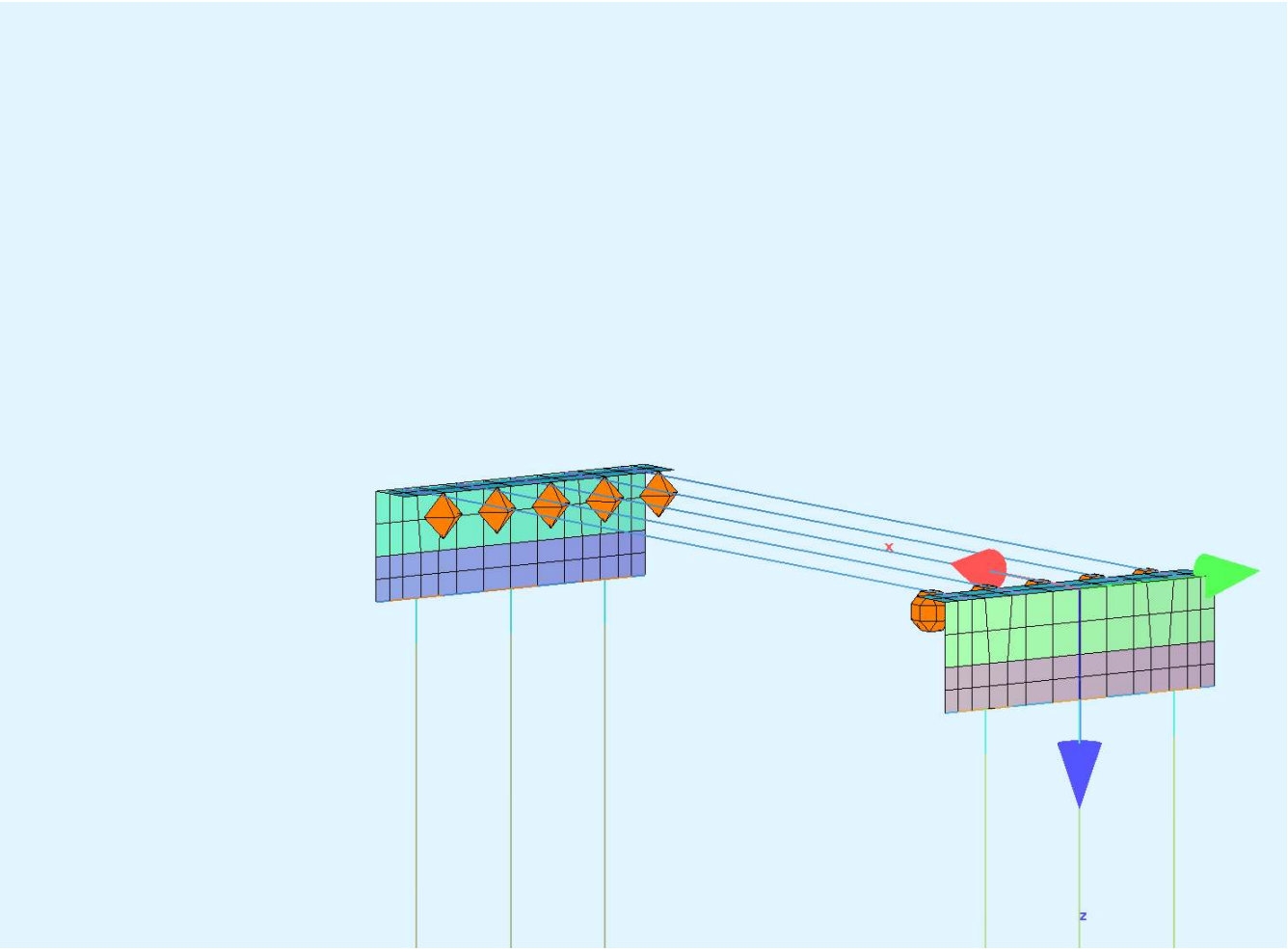

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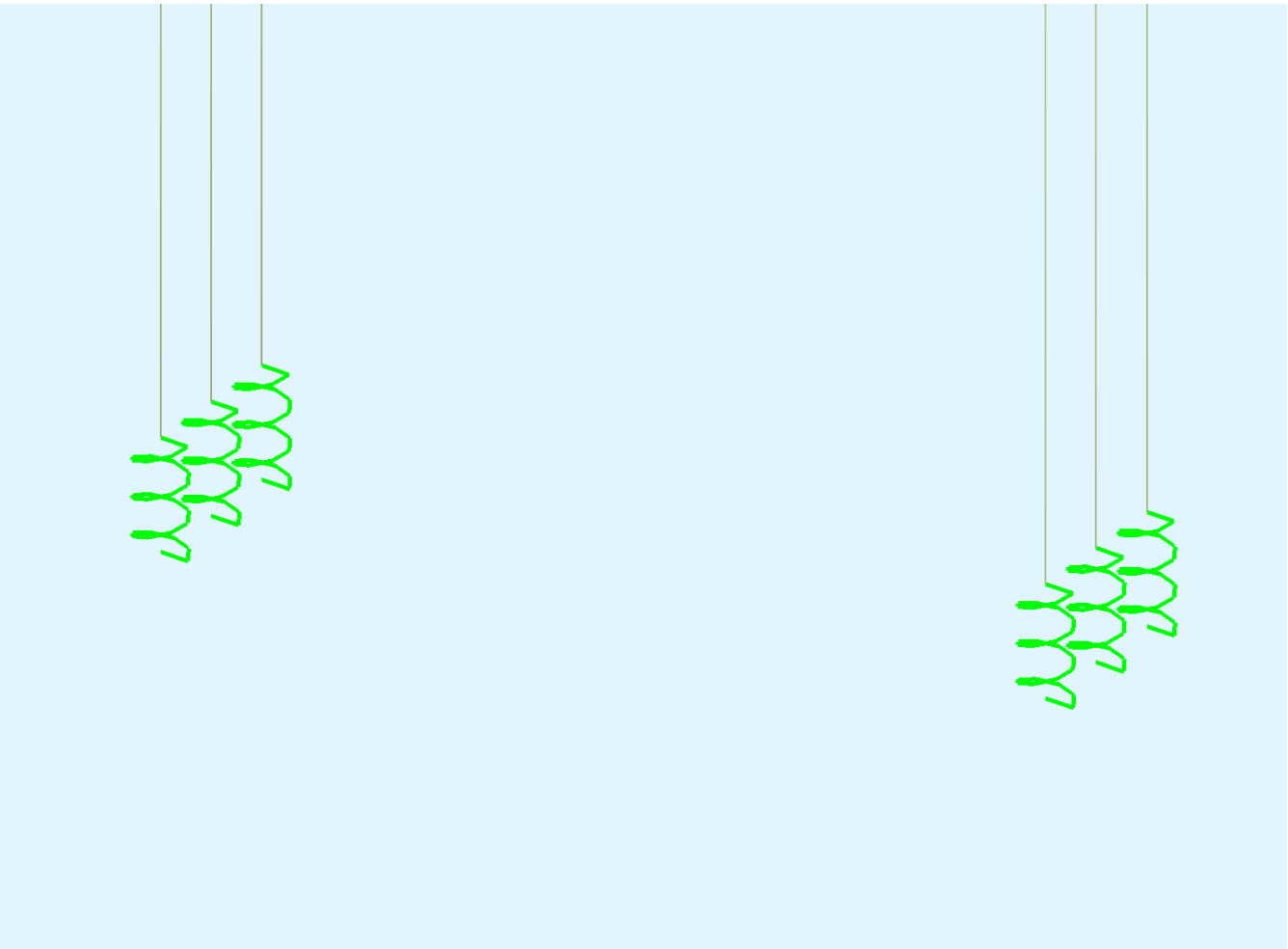
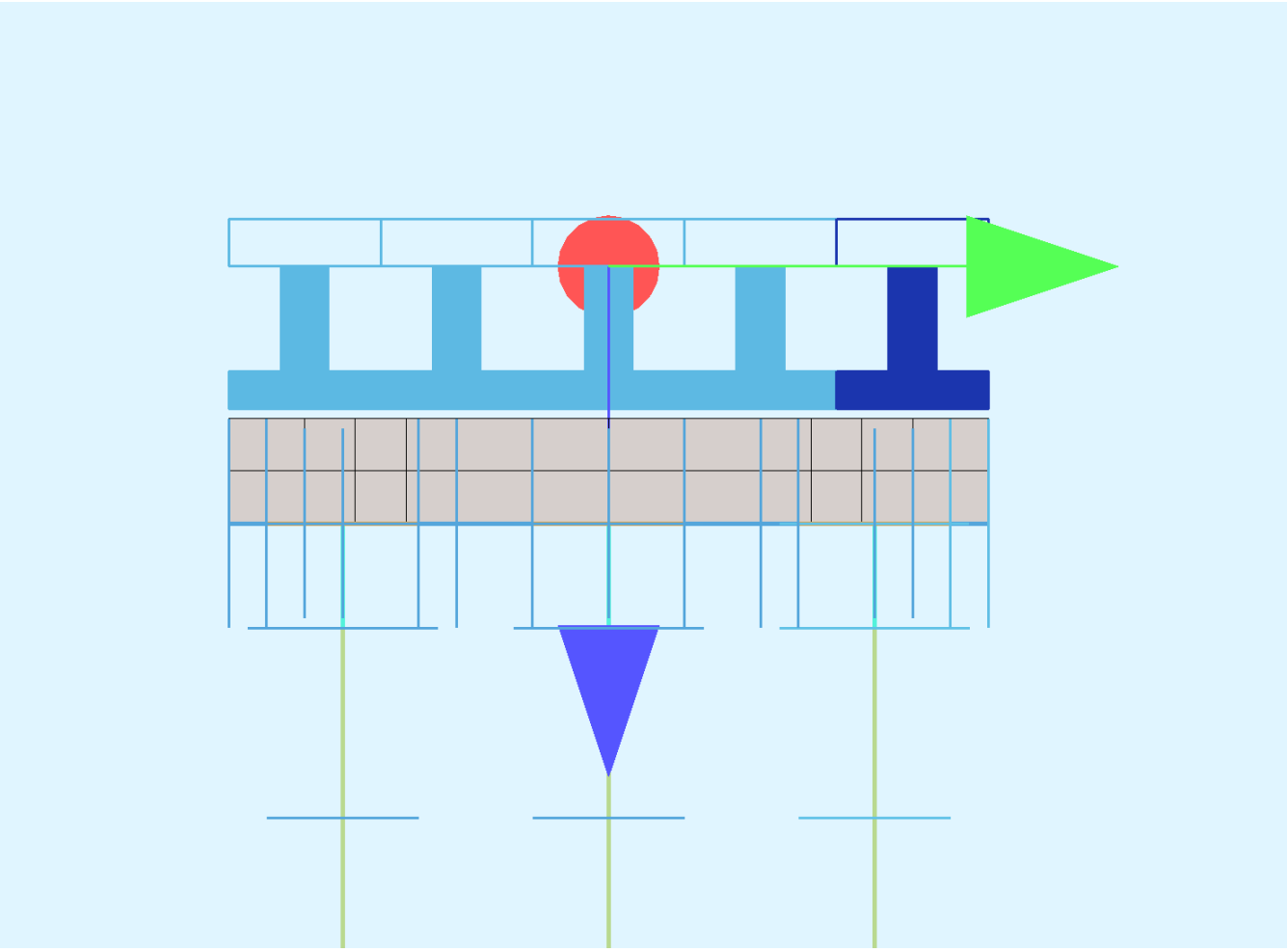
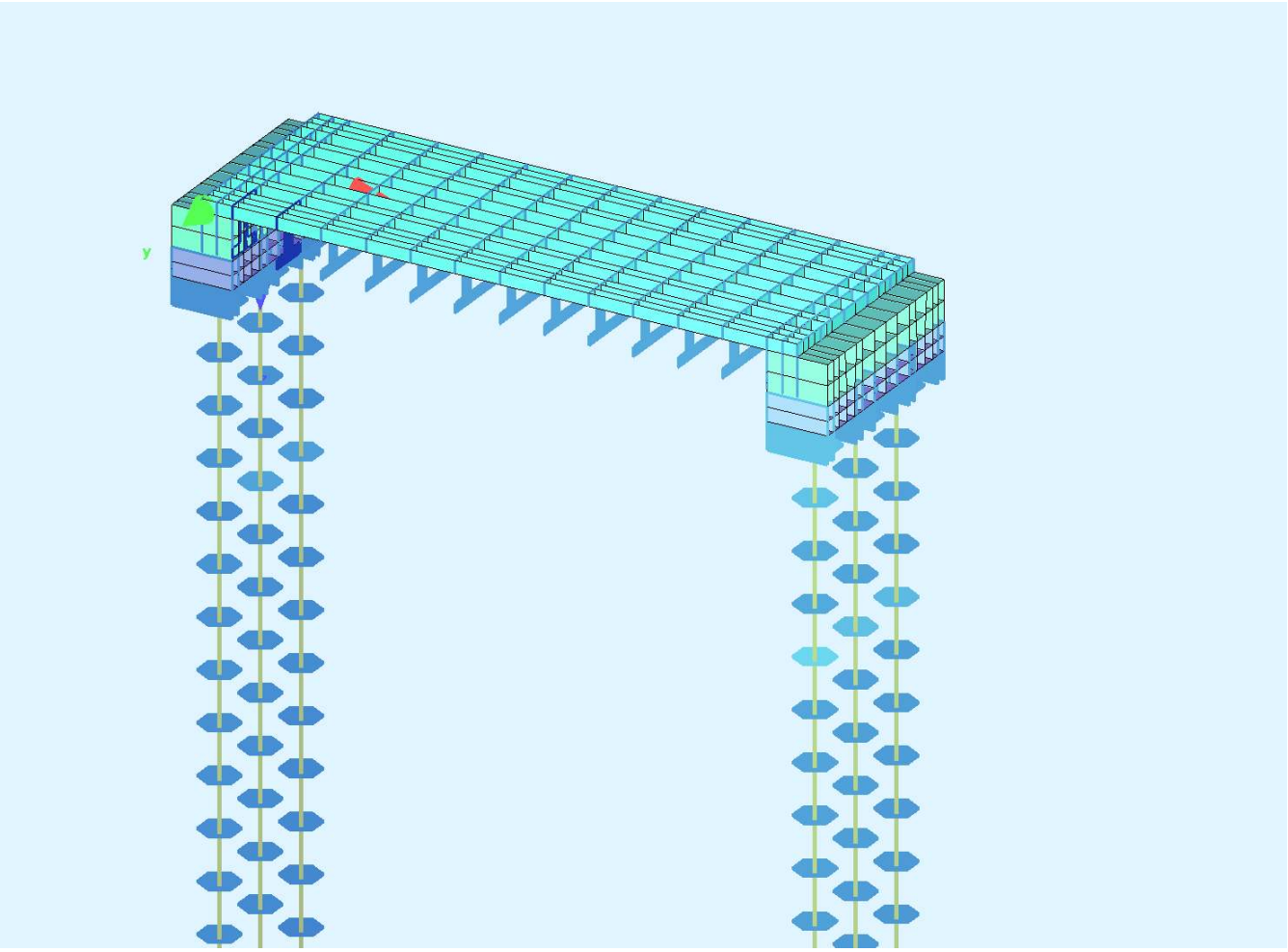


ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00



ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00



ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00

3) ΟΡΙΣΜΟΣ & ΕΠΙΛΥΣΗ ΦΟΡΤΙΣΕΩΝ ΣΕ ΦΑΣΗ-1

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FORTIA ΦΑΣΗ-1

Load Case 1 I.B. ΚΑΤΑΚ.ΣΤΟΙΧΕΙΩΝ

Factor forces and moments		1.000
Factor dead weight	DL-XX	0.000
Factor dead weight	DL-YY	0.000
Factor dead weight	DL-ZZ	1.000
effective construction stage		10 to 10

Load Case 2 I.B. ΔΟΚΩΝ

Factor forces and moments		1.000
Factor dead weight	DL-XX	0.000
Factor dead weight	DL-YY	0.000
Factor dead weight	DL-ZZ	0.000

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
1001	PG	0.000	0.883	0.007	[MN/m]				
1002	PG	0.000	0.883	0.007	[MN/m]				
1003	PG	0.000	0.883	0.007	[MN/m]				
1004	PG	0.000	0.883	0.007	[MN/m]				
1005	PG	0.000	0.883	0.007	[MN/m]				
1006	PG	0.000	0.883	0.007	[MN/m]				
1007	PG	0.000	0.883	0.007	[MN/m]				
1008	PG	0.000	0.883	0.007	[MN/m]				
1009	PG	0.000	0.883	0.007	[MN/m]				
1010	PG	0.000	0.883	0.007	[MN/m]				
1011	PG	0.000	0.883	0.007	[MN/m]				
1012	PG	0.000	0.883	0.007	[MN/m]				
1013	PG	0.000	0.883	0.007	[MN/m]				
1014	PG	0.000	0.883	0.007	[MN/m]				
1015	PG	0.000	0.883	0.007	[MN/m]				
1016	PG	0.000	0.883	0.007	[MN/m]				
1017	PG	0.000	0.883	0.007	[MN/m]				
1018	PG	0.000	0.883	0.007	[MN/m]				
1019	PG	0.000	0.883	0.007	[MN/m]				
1020	PG	0.000	0.883	0.007	[MN/m]				
1021	PG	0.000	0.883	0.007	[MN/m]				
1022	PG	0.000	0.883	0.007	[MN/m]				
1023	PG	0.000	0.883	0.007	[MN/m]				
1024	PG	0.000	0.883	0.007	[MN/m]				
1025	PG	0.000	0.883	0.007	[MN/m]				
1026	PG	0.000	0.883	0.007	[MN/m]				
1027	PG	0.000	0.883	0.007	[MN/m]				
1028	PG	0.000	0.883	0.007	[MN/m]				
1029	PG	0.000	0.883	0.007	[MN/m]				
1030	PG	0.000	0.883	0.007	[MN/m]				
1031	PG	0.000	0.883	0.007	[MN/m]				
1032	PG	0.000	0.883	0.007	[MN/m]				
1033	PG	0.000	0.883	0.007	[MN/m]				
1034	PG	0.000	0.883	0.007	[MN/m]				
1035	PG	0.000	0.883	0.007	[MN/m]				
1036	PG	0.000	0.883	0.007	[MN/m]				
1037	PG	0.000	0.883	0.007	[MN/m]				
1038	PG	0.000	0.883	0.007	[MN/m]				
1039	PG	0.000	0.883	0.007	[MN/m]				
1040	PG	0.000	0.883	0.007	[MN/m]				
1041	PG	0.000	0.883	0.007	[MN/m]				
1042	PG	0.000	0.883	0.007	[MN/m]				
1043	PG	0.000	0.883	0.007	[MN/m]				
1044	PG	0.000	0.883	0.007	[MN/m]				
1045	PG	0.000	0.883	0.007	[MN/m]				
1046	PG	0.000	0.883	0.007	[MN/m]				
1047	PG	0.000	0.883	0.007	[MN/m]				
1048	PG	0.000	0.883	0.007	[MN/m]				
1049	PG	0.000	0.883	0.007	[MN/m]				
1050	PG	0.000	0.883	0.007	[MN/m]				
1051	PG	0.000	0.883	0.007	[MN/m]				
1052	PG	0.000	0.883	0.007	[MN/m]				
1053	PG	0.000	0.883	0.007	[MN/m]				
1054	PG	0.000	0.883	0.007	[MN/m]				
1055	PG	0.000	0.883	0.007	[MN/m]				
1056	PG	0.000	0.883	0.007	[MN/m]				
1057	PG	0.000	0.883	0.007	[MN/m]				
1058	PG	0.000	0.883	0.007	[MN/m]				
1059	PG	0.000	0.883	0.007	[MN/m]				
1060	PG	0.000	0.883	0.007	[MN/m]				

Load Case 3 I.B. ΧΥΤΗΣ ΠΛΑΚΑΣ

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FORTIA ΦΑΣΗ-1

Load Case 3 I.B. ΧΥΤΗΣ ΠΛΑΚΑΣ

Factor forces and moments		1.000
Factor dead weight	DL-XX	0.000
Factor dead weight	DL-YY	0.000
Factor dead weight	DL-ZZ	0.000

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
1001	PG	0.000	0.883	0.005	[MN/m]				
1002	PG	0.000	0.883	0.005	[MN/m]				
1003	PG	0.000	0.883	0.005	[MN/m]				
1004	PG	0.000	0.883	0.005	[MN/m]				
1005	PG	0.000	0.883	0.005	[MN/m]				
1006	PG	0.000	0.883	0.005	[MN/m]				
1007	PG	0.000	0.883	0.005	[MN/m]				
1008	PG	0.000	0.883	0.005	[MN/m]				
1009	PG	0.000	0.883	0.005	[MN/m]				
1010	PG	0.000	0.883	0.005	[MN/m]				
1011	PG	0.000	0.883	0.005	[MN/m]				
1012	PG	0.000	0.883	0.005	[MN/m]				
1013	PG	0.000	0.883	0.005	[MN/m]				
1014	PG	0.000	0.883	0.005	[MN/m]				
1015	PG	0.000	0.883	0.005	[MN/m]				
1016	PG	0.000	0.883	0.005	[MN/m]				
1017	PG	0.000	0.883	0.005	[MN/m]				
1018	PG	0.000	0.883	0.005	[MN/m]				
1019	PG	0.000	0.883	0.005	[MN/m]				
1020	PG	0.000	0.883	0.005	[MN/m]				
1021	PG	0.000	0.883	0.005	[MN/m]				
1022	PG	0.000	0.883	0.005	[MN/m]				
1023	PG	0.000	0.883	0.005	[MN/m]				
1024	PG	0.000	0.883	0.005	[MN/m]				
1025	PG	0.000	0.883	0.005	[MN/m]				
1026	PG	0.000	0.883	0.005	[MN/m]				
1027	PG	0.000	0.883	0.005	[MN/m]				
1028	PG	0.000	0.883	0.005	[MN/m]				
1029	PG	0.000	0.883	0.005	[MN/m]				
1030	PG	0.000	0.883	0.005	[MN/m]				
1031	PG	0.000	0.883	0.005	[MN/m]				
1032	PG	0.000	0.883	0.005	[MN/m]				
1033	PG	0.000	0.883	0.005	[MN/m]				
1034	PG	0.000	0.883	0.005	[MN/m]				
1035	PG	0.000	0.883	0.005	[MN/m]				
1036	PG	0.000	0.883	0.005	[MN/m]				
1037	PG	0.000	0.883	0.005	[MN/m]				
1038	PG	0.000	0.883	0.005	[MN/m]				
1039	PG	0.000	0.883	0.005	[MN/m]				
1040	PG	0.000	0.883	0.005	[MN/m]				
1041	PG	0.000	0.883	0.005	[MN/m]				
1042	PG	0.000	0.883	0.005	[MN/m]				
1043	PG	0.000	0.883	0.005	[MN/m]				
1044	PG	0.000	0.883	0.005	[MN/m]				
1045	PG	0.000	0.883	0.005	[MN/m]				
1046	PG	0.000	0.883	0.005	[MN/m]				
1047	PG	0.000	0.883	0.005	[MN/m]				
1048	PG	0.000	0.883	0.005	[MN/m]				
1049	PG	0.000	0.883	0.005	[MN/m]				
1050	PG	0.000	0.883	0.005	[MN/m]				
1051	PG	0.000	0.883	0.005	[MN/m]				
1052	PG	0.000	0.883	0.005	[MN/m]				
1053	PG	0.000	0.883	0.005	[MN/m]				
1054	PG	0.000	0.883	0.005	[MN/m]				
1055	PG	0.000	0.883	0.005	[MN/m]				
1056	PG	0.000	0.883	0.005	[MN/m]				
1057	PG	0.000	0.883	0.005	[MN/m]				
1058	PG	0.000	0.883	0.005	[MN/m]				
1059	PG	0.000	0.883	0.005	[MN/m]				
1060	PG	0.000	0.883	0.005	[MN/m]				

Load Case 4 ΚΙΝΗΤΟ ΦΑΣΗΣ-1

Factor forces and moments		1.000
Factor dead weight	DL-XX	0.000
Factor dead weight	DL-YY	0.000
Factor dead weight	DL-ZZ	0.000

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
1001	PG	0.000	0.883	0.001	[MN/m]				
1002	PG	0.000	0.883	0.001	[MN/m]				
1003	PG	0.000	0.883	0.001	[MN/m]				
1004	PG	0.000	0.883	0.001	[MN/m]				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FORTIA ΦΑΣΗ-1

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
1005	PG	0.000	0.883	0.001	[MN/m]				
1006	PG	0.000	0.883	0.001	[MN/m]				
1007	PG	0.000	0.883	0.001	[MN/m]				
1008	PG	0.000	0.883	0.001	[MN/m]				
1009	PG	0.000	0.883	0.001	[MN/m]				
1010	PG	0.000	0.883	0.001	[MN/m]				
1011	PG	0.000	0.883	0.001	[MN/m]				
1012	PG	0.000	0.883	0.001	[MN/m]				
1013	PG	0.000	0.883	0.001	[MN/m]				
1014	PG	0.000	0.883	0.001	[MN/m]				
1015	PG	0.000	0.883	0.001	[MN/m]				
1016	PG	0.000	0.883	0.001	[MN/m]				
1017	PG	0.000	0.883	0.001	[MN/m]				
1018	PG	0.000	0.883	0.001	[MN/m]				
1019	PG	0.000	0.883	0.001	[MN/m]				
1020	PG	0.000	0.883	0.001	[MN/m]				
1021	PG	0.000	0.883	0.001	[MN/m]				
1022	PG	0.000	0.883	0.001	[MN/m]				
1023	PG	0.000	0.883	0.001	[MN/m]				
1024	PG	0.000	0.883	0.001	[MN/m]				
1025	PG	0.000	0.883	0.001	[MN/m]				
1026	PG	0.000	0.883	0.001	[MN/m]				
1027	PG	0.000	0.883	0.001	[MN/m]				
1028	PG	0.000	0.883	0.001	[MN/m]				
1029	PG	0.000	0.883	0.001	[MN/m]				
1030	PG	0.000	0.883	0.001	[MN/m]				
1031	PG	0.000	0.883	0.001	[MN/m]				
1032	PG	0.000	0.883	0.001	[MN/m]				
1033	PG	0.000	0.883	0.001	[MN/m]				
1034	PG	0.000	0.883	0.001	[MN/m]				
1035	PG	0.000	0.883	0.001	[MN/m]				
1036	PG	0.000	0.883	0.001	[MN/m]				
1037	PG	0.000	0.883	0.001	[MN/m]				
1038	PG	0.000	0.883	0.001	[MN/m]				
1039	PG	0.000	0.883	0.001	[MN/m]				
1040	PG	0.000	0.883	0.001	[MN/m]				
1041	PG	0.000	0.883	0.001	[MN/m]				
1042	PG	0.000	0.883	0.001	[MN/m]				
1043	PG	0.000	0.883	0.001	[MN/m]				
1044	PG	0.000	0.883	0.001	[MN/m]				
1045	PG	0.000	0.883	0.001	[MN/m]				
1046	PG	0.000	0.883	0.001	[MN/m]				
1047	PG	0.000	0.883	0.001	[MN/m]				
1048	PG	0.000	0.883	0.001	[MN/m]				
1049	PG	0.000	0.883	0.001	[MN/m]				
1050	PG	0.000	0.883	0.001	[MN/m]				
1051	PG	0.000	0.883	0.001	[MN/m]				
1052	PG	0.000	0.883	0.001	[MN/m]				
1053	PG	0.000	0.883	0.001	[MN/m]				
1054	PG	0.000	0.883	0.001	[MN/m]				
1055	PG	0.000	0.883	0.001	[MN/m]				
1056	PG	0.000	0.883	0.001	[MN/m]				
1057	PG	0.000	0.883	0.001	[MN/m]				
1058	PG	0.000	0.883	0.001	[MN/m]				
1059	PG	0.000	0.883	0.001	[MN/m]				
1060	PG	0.000	0.883	0.001	[MN/m]				

Load Case 5 ΩΘΗΣΕΙΣ ΓΑΙΩΝ A1-K0 (Φ-1)

Factor forces and moments		1.000
Factor dead weight	DL-XX	0.000
Factor dead weight	DL-YY	0.000
Factor dead weight	DL-ZZ	0.000

Meshfree Loading

Kind	Referenceto	Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
Area			0.000	2.000	0.800	PXX	0.000	[MN/m2]
			0.000	-2.000	0.800		0.000	[MN/m2]
			0.000	-2.000	1.350		0.005	[MN/m2]
			0.000	2.000	1.350		0.005	[MN/m2]
	QGRP	9	3.000			activated	100.00	percent

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
11001	PXX	0.000	0.550	0.008	0.015 [MN/m]				
11002	PXX	0.000	0.550	0.008	0.015 [MN/m]				
11003	PXX	0.000	0.550	0.008	0.015 [MN/m]				
12001	PXX	0.000	1.000	0.015	0.029 [MN/m]				
12002	PXX	0.000	1.000	0.029	0.043 [MN/m]				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FORTIA ΦΑΣΗ-1

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval	Dimens.	ya[m]	za[m]	ye[m]	ze[m]
12003	PXX	0.000	1.000	0.043	0.057	[MN/m]				
12004	PXX	0.000	1.000	0.015	0.029	[MN/m]				
12005	PXX	0.000	1.000	0.029	0.043	[MN/m]				
12006	PXX	0.000	1.000	0.043	0.057	[MN/m]				
12007	PXX	0.000	1.000	0.015	0.029	[MN/m]				
12008	PXX	0.000	1.000	0.029	0.043	[MN/m]				
12009	PXX	0.000	1.000	0.043	0.057	[MN/m]				

Load Case 6 ΩΘΗΣΕΙΣ ΓΑΙΩΝ Α2-Κ0 (ΦΑΣΗΣ-1)

Factor forces and moments 1.000
Factor dead weight DL-XX 0.000
Factor dead weight DL-YY 0.000
Factor dead weight DL-ZZ 0.000

Meshfree Loading

Kind	Referenceto	Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
Area			11.800	2.000	0.800	PXX	0.000	[MN/m2]
			11.800	-2.000	0.800		0.000	[MN/m2]
			11.800	-2.000	1.350		-0.005	[MN/m2]
			11.800	2.000	1.350		-0.005	[MN/m2]
	QGRP	9	3.000		activated		100.00	percent

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval	Dimens.	ya[m]	za[m]	ye[m]	ze[m]
11004	PXX	0.000	0.550	-0.008	-0.015	[MN/m]				
11005	PXX	0.000	0.550	-0.008	-0.015	[MN/m]				
11006	PXX	0.000	0.550	-0.008	-0.015	[MN/m]				
12010	PXX	0.000	1.000	-0.015	-0.029	[MN/m]				
12011	PXX	0.000	1.000	-0.029	-0.043	[MN/m]				
12012	PXX	0.000	1.000	-0.043	-0.057	[MN/m]				
12013	PXX	0.000	1.000	-0.015	-0.029	[MN/m]				
12014	PXX	0.000	1.000	-0.029	-0.043	[MN/m]				
12015	PXX	0.000	1.000	-0.043	-0.057	[MN/m]				
12016	PXX	0.000	1.000	-0.015	-0.029	[MN/m]				
12017	PXX	0.000	1.000	-0.029	-0.043	[MN/m]				
12018	PXX	0.000	1.000	-0.043	-0.057	[MN/m]				

Load Case 7 ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝ Α1-Κ0 (ΦΑΣΗΣ-1)

Factor forces and moments 1.000
Factor dead weight DL-XX 0.000
Factor dead weight DL-YY 0.000
Factor dead weight DL-ZZ 0.000

Meshfree Loading

Kind	Referenceto	Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
Area			0.000	2.000	0.800	PXX	0.041	[MN/m2]
			0.000	-2.000	0.800		0.041	[MN/m2]
			0.000	-2.000	1.350		0.026	[MN/m2]
			0.000	2.000	1.350		0.026	[MN/m2]
	QGRP	9	3.000		activated		100.00	percent

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval	Dimens.	ya[m]	za[m]	ye[m]	ze[m]
11001	PXX	0.000	0.550	0.037	0.026	[MN/m]				
11002	PXX	0.000	0.550	0.037	0.026	[MN/m]				
11003	PXX	0.000	0.550	0.037	0.026	[MN/m]				
12001	PXX	0.000	1.000	0.026	0.019	[MN/m]				
12002	PXX	0.000	1.000	0.019	0.013	[MN/m]				
12003	PXX	0.000	1.000	0.013	0.007	[MN/m]				
12004	PXX	0.000	1.000	0.026	0.019	[MN/m]				
12005	PXX	0.000	1.000	0.019	0.013	[MN/m]				
12006	PXX	0.000	1.000	0.013	0.007	[MN/m]				
12007	PXX	0.000	1.000	0.026	0.019	[MN/m]				
12008	PXX	0.000	1.000	0.019	0.013	[MN/m]				
12009	PXX	0.000	1.000	0.013	0.007	[MN/m]				

Load Case 8 ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝ Α2-Κ0 (ΦΑΣΗΣ-1)

Factor forces and moments 1.000
Factor dead weight DL-XX 0.000
Factor dead weight DL-YY 0.000
Factor dead weight DL-ZZ 0.000

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FORTIA ΦΑΣΗ-1

Meshfree Loading

Kind	Referenceto	Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
Area			11.800	2.000	0.800	PXX	-0.041	[MN/m2]
			11.800	-2.000	0.800		-0.041	[MN/m2]
			11.800	-2.000	1.350		-0.026	[MN/m2]
			11.800	2.000	1.350		-0.026	[MN/m2]
	QGRP	9	3.000		activated		100.00	percent

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval	Dimens.	ya[m]	za[m]	ye[m]	ze[m]
11004	PXX	0.000	0.550	-0.037	-0.026	[MN/m]				
11005	PXX	0.000	0.550	-0.037	-0.026	[MN/m]				
11006	PXX	0.000	0.550	-0.037	-0.026	[MN/m]				
12010	PXX	0.000	1.000	-0.026	-0.019	[MN/m]				
12011	PXX	0.000	1.000	-0.019	-0.013	[MN/m]				
12012	PXX	0.000	1.000	-0.013	-0.007	[MN/m]				
12013	PXX	0.000	1.000	-0.026	-0.019	[MN/m]				
12014	PXX	0.000	1.000	-0.019	-0.013	[MN/m]				
12015	PXX	0.000	1.000	-0.013	-0.007	[MN/m]				
12016	PXX	0.000	1.000	-0.026	-0.019	[MN/m]				
12017	PXX	0.000	1.000	-0.019	-0.013	[MN/m]				
12018	PXX	0.000	1.000	-0.013	-0.007	[MN/m]				

Load Case 11 ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+0.5*A2(Φ1)

Factor forces and moments 1.000
Factor dead weight DL-XX 0.000
Factor dead weight DL-YY 0.000
Factor dead weight DL-ZZ 0.000
Loads partially copied from load case 5 with factor 0.500
Loads partially copied from load case 6 with factor 0.500

Meshfree Loading

Kind	Referenceto	Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
Area			0.000	2.000	0.800	PXX	0.000	[MN/m2]
			0.000	-2.000	0.800		0.000	[MN/m2]
			0.000	-2.000	1.350		0.003	[MN/m2]
			0.000	2.000	1.350		0.003	[MN/m2]
	QGRP	9	3.000		activated		100.00	percent
Area			11.800	2.000	0.800	PXX	0.000	[MN/m2]
			11.800	-2.000	0.800		0.000	[MN/m2]
			11.800	-2.000	1.350		-0.003	[MN/m2]
			11.800	2.000	1.350		-0.003	[MN/m2]
	QGRP	9	3.000		activated		100.00	percent

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval	Dimens.	ya[m]	za[m]	ye[m]	ze[m]
11001	PXX	0.000	0.550	0.004	0.008	[MN/m]				
11002	PXX	0.000	0.550	0.004	0.008	[MN/m]				
11003	PXX	0.000	0.550	0.004	0.008	[MN/m]				
11004	PXX	0.000	0.550	-0.004	-0.008	[MN/m]				
11005	PXX	0.000	0.550	-0.004	-0.008	[MN/m]				
11006	PXX	0.000	0.550	-0.004	-0.008	[MN/m]				
12001	PXX	0.000	1.000	0.008	0.015	[MN/m]				
12002	PXX	0.000	1.000	0.015	0.022	[MN/m]				
12003	PXX	0.000	1.000	0.022	0.029	[MN/m]				
12004	PXX	0.000	1.000	0.008	0.015	[MN/m]				
12005	PXX	0.000	1.000	0.015	0.022	[MN/m]				
12006	PXX	0.000	1.000	0.022	0.029	[MN/m]				
12007	PXX	0.000	1.000	0.008	0.015	[MN/m]				
12008	PXX	0.000	1.000	0.015	0.022	[MN/m]				
12009	PXX	0.000	1.000	0.022	0.029	[MN/m]				
12010	PXX	0.000	1.000	-0.008	-0.015	[MN/m]				
12011	PXX	0.000	1.000	-0.015	-0.022	[MN/m]				
12012	PXX	0.000	1.000	-0.022	-0.029	[MN/m]				
12013	PXX	0.000	1.000	-0.008	-0.015	[MN/m]				
12014	PXX	0.000	1.000	-0.015	-0.022	[MN/m]				
12015	PXX	0.000	1.000	-0.022	-0.029	[MN/m]				
12016	PXX	0.000	1.000	-0.008	-0.015	[MN/m]				
12017	PXX	0.000	1.000	-0.015	-0.022	[MN/m]				
12018	PXX	0.000	1.000	-0.022	-0.029	[MN/m]				

Load Case 12 ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+0.5*A2(Φ1)

Factor forces and moments 1.000
Factor dead weight DL-XX 0.000
Factor dead weight DL-YY 0.000
Factor dead weight DL-ZZ 0.000
Loads partially copied from load case 5 with factor 1.000
Loads partially copied from load case 6 with factor 0.500

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FORTIA ΦΑΣΗ-1

Meshfree Loading

Kind	Referenceto	Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
Area			0.000	2.000	0.800	PXX	0.000	[MN/m2]
			0.000	-2.000	0.800		0.000	[MN/m2]
			0.000	-2.000	1.350		0.005	[MN/m2]
			0.000	2.000	1.350		0.005	[MN/m2]
	QGRP	9	3.000		activated		100.00	percent
Area			11.800	2.000	0.800	PXX	0.000	[MN/m2]
			11.800	-2.000	0.800		0.000	[MN/m2]
			11.800	-2.000	1.350		-0.003	[MN/m2]
			11.800	2.000	1.350		-0.003	[MN/m2]
	QGRP	9	3.000		activated		100.00	percent

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval	Dimens.	ya[m]	za[m]	ye[m]	ze[m]
11001	PXX	0.000	0.550	0.008	0.015	[MN/m]				
11002	PXX	0.000	0.550	0.008	0.015	[MN/m]				
11003	PXX	0.000	0.550	0.008	0.015	[MN/m]				
11004	PXX	0.000	0.550	-0.004	-0.008	[MN/m]				
11005	PXX	0.000	0.550	-0.004	-0.008	[MN/m]				
11006	PXX	0.000	0.550	-0.004	-0.008	[MN/m]				
12001	PXX	0.000	1.000	0.015	0.029	[MN/m]				
12002	PXX	0.000	1.000	0.029	0.043	[MN/m]				
12003	PXX	0.000	1.000	0.043	0.057	[MN/m]				
12004	PXX	0.000	1.000	0.015	0.029	[MN/m]				
12005	PXX	0.000	1.000	0.029	0.043	[MN/m]				
12006	PXX	0.000	1.000	0.043	0.057	[MN/m]				
12007	PXX	0.000	1.000	0.015	0.029	[MN/m]				
12008	PXX	0.000	1.000	0.029	0.043	[MN/m]				
12009	PXX	0.000	1.000	0.043	0.057	[MN/m]				
12010	PXX	0.000	1.000	-0.008	-0.015	[MN/m]				
12011	PXX	0.000	1.000	-0.015	-0.022	[MN/m]				
12012	PXX	0.000	1.000	-0.022	-0.029	[MN/m]				
12013	PXX	0.000	1.000	-0.008	-0.015	[MN/m]				
12014	PXX	0.000	1.000	-0.015	-0.022	[MN/m]				
12015	PXX	0.000	1.000	-0.022	-0.029	[MN/m]				
12016	PXX	0.000	1.000	-0.008	-0.015	[MN/m]				
12017	PXX	0.000	1.000	-0.015	-0.022	[MN/m]				
12018	PXX	0.000	1.000	-0.022	-0.029	[MN/m]				

Load Case 13 ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+1.0*A2(Φ1)

Factor forces and moments 1.000
Factor dead weight DL-XX 0.000
Factor dead weight DL-YY 0.000
Factor dead weight DL-ZZ 0.000
Loads partially copied from load case 5 with factor 0.500
Loads partially copied from load case 6 with factor 1.000

Meshfree Loading

Kind	Referenceto	Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
Area			0.000	2.000	0.800	PXX	0.000	[MN/m2]
			0.000	-2.000	0.800		0.000	[MN/m2]
			0.000	-2.000	1.350		0.003	[MN/m2]
			0.000	2.000	1.350		0.003	[MN/m2]
	QGRP	9	3.000		activated		100.00	percent
Area			11.800	2.000	0.800	PXX	0.000	[MN/m2]
			11.800	-2.000	0.800		0.000	[MN/m2]
			11.800	-2.000	1.350		-0.005	[MN/m2]
			11.800	2.000	1.350		-0.005	[MN/m2]
	QGRP	9	3.000		activated		100.00	percent

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval	Dimens.	ya[m]	za[m]	ye[m]	ze[m]
11001	PXX	0.000	0.550	0.004	0.008	[MN/m]				
11002	PXX	0.000	0.550	0.004	0.008	[MN/m]				
11003	PXX	0.000	0.550	0.004	0.008	[MN/m]				
11004	PXX	0.000	0.550	-0.008	-0.015	[MN/m]				
11005	PXX	0.000	0.550	-0.008	-0.015	[MN/m]				
11006	PXX	0.000	0.550	-0.008	-0.015	[MN/m]				
12001	PXX	0.000	1.000	0.008	0.015	[MN/m]				
12002	PXX	0.000	1.000	0.015	0.022	[MN/m]				
12003	PXX	0.000	1.000	0.022	0.029	[MN/m]				
12004	PXX	0.000	1.000	0.008	0.015	[MN/m]				
12005	PXX	0.000	1.000	0.015	0.022	[MN/m]				
12006	PXX	0.000	1.000	0.022	0.029	[MN/m]				
12007	PXX	0.000	1.000	0.008	0.015	[MN/m]				
12008	PXX	0.000	1.000	0.015	0.022	[MN/m]				
12009	PXX	0.000	1.000	0.022	0.029	[MN/m]				
12010	PXX	0.000	1.000	-0.015	-0.029	[MN/m]				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FORTIA ΦΑΣΗ-1

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval	Dimens.	ya[m]	za[m]	ye[m]	ze[m]
12011	PXX	0.000	1.000	-0.029	-0.043	[MN/m]				
12012	PXX	0.000	1.000	-0.043	-0.057	[MN/m]				
12013	PXX	0.000	1.000	-0.015	-0.029	[MN/m]				
12014	PXX	0.000	1.000	-0.029	-0.043	[MN/m]				
12015	PXX	0.000	1.000	-0.043	-0.057	[MN/m]				
12016	PXX	0.000	1.000	-0.015	-0.029	[MN/m]				
12017	PXX	0.000	1.000	-0.029	-0.043	[MN/m]				
12018	PXX	0.000	1.000	-0.043	-0.057	[MN/m]				

Load Case 14 ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+1.0*A2(Φ1)

Factor forces and moments 1.000
Factor dead weight DL-XX 0.000
Factor dead weight DL-YY 0.000
Factor dead weight DL-ZZ 0.000
Loads partially copied from load case 5 with factor 1.000
Loads partially copied from load case 6 with factor 1.000

Meshfree Loading

Kind	Referenceto	Projection	Coordinates W[m]	X[m]	Y[m]	Z[m]	Type	Loadvalue	
Area				0.000	2.000	0.800	PXX	0.000	[MN/m2]
				0.000	-2.000	0.800		0.000	[MN/m2]
				0.000	-2.000	1.350		0.005	[MN/m2]
				0.000	2.000	1.350		0.005	[MN/m2]
	QGRP	9	3.000		activated			100.00	percent
Area				11.800	2.000	0.800	PXX	0.000	[MN/m2]
				11.800	-2.000	0.800		0.000	[MN/m2]
				11.800	-2.000	1.350		-0.005	[MN/m2]
				11.800	2.000	1.350		-0.005	[MN/m2]
	QGRP	9	3.000		activated			100.00	percent

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval	Dimens.	ya[m]	za[m]	ye[m]	ze[m]
11001	PXX	0.000	0.550	0.008	0.015	[MN/m]				
11002	PXX	0.000	0.550	0.008	0.015	[MN/m]				
11003	PXX	0.000	0.550	0.008	0.015	[MN/m]				
11004	PXX	0.000	0.550	-0.008	-0.015	[MN/m]				
11005	PXX	0.000	0.550	-0.008	-0.015	[MN/m]				
11006	PXX	0.000	0.550	-0.008	-0.015	[MN/m]				
12001	PXX	0.000	1.000	0.015	0.029	[MN/m]				
12002	PXX	0.000	1.000	0.029	0.043	[MN/m]				
12003	PXX	0.000	1.000	0.043	0.057	[MN/m]				
12004	PXX	0.000	1.000	0.015	0.029	[MN/m]				
12005	PXX	0.000	1.000	0.029	0.043	[MN/m]				
12006	PXX	0.000	1.000	0.043	0.057	[MN/m]				
12007	PXX	0.000	1.000	0.015	0.029	[MN/m]				
12008	PXX	0.000	1.000	0.029	0.043	[MN/m]				
12009	PXX	0.000	1.000	0.043	0.057	[MN/m]				
12010	PXX	0.000	1.000	-0.015	-0.029	[MN/m]				
12011	PXX	0.000	1.000	-0.029	-0.043	[MN/m]				
12012	PXX	0.000	1.000	-0.043	-0.057	[MN/m]				
12013	PXX	0.000	1.000	-0.015	-0.029	[MN/m]				
12014	PXX	0.000	1.000	-0.029	-0.043	[MN/m]				
12015	PXX	0.000	1.000	-0.043	-0.057	[MN/m]				
12016	PXX	0.000	1.000	-0.015	-0.029	[MN/m]				
12017	PXX	0.000	1.000	-0.029	-0.043	[MN/m]				
12018	PXX	0.000	1.000	-0.043	-0.057	[MN/m]				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΕΠΙΛΥΣΗ ΦΑΣΗ-1

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
1	Ι.Β. ΚΑΤΑΚ.ΣΤΟΙΧΕΙΩΝ	0.000	0.000	2.387
2	Ι.Β. ΔΟΚΩΝ	0.000	0.000	0.394
3	Ι.Β. ΧΥΤΗΣ ΠΛΑΚΑΣ	0.000	0.000	0.276
4	ΚΙΝΗΤΟ ΦΑΣΗΣ-1	0.000	0.000	0.042
5	ΩΘΗΣΕΙΣ ΓΑΙΩΝ Α1-Κ0 (Φ-1	0.353	0.000	0.000
6	ΩΘΗΣΕΙΣ ΓΑΙΩΝ Α2-Κ0 (ΦΑΣ	-0.353	0.000	0.000
7	ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝ Α1-Κ0 (Φ	0.272	0.000	0.000
8	ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝ Α2-Κ0 (Φ	-0.272	0.000	0.000
11	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*Α1+0.5	0.000	0.000	0.000
12	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*Α1+0.5	0.176	0.000	0.000
13	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*Α1+1.0	-0.176	0.000	0.000
14	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*Α1+1.0	0.000	0.000	0.000

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00

4) ΟΡΙΣΜΟΣ & ΕΠΙΛΥΣΗ ΦΟΡΤΙΣΕΩΝ ΥΠΟΛΟΓΙΣΜΟΣ ΕΡΠΥΣΜΩΝ ΣΕ ΦΑ

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FORTISEIS ΦΑΣΗ-2

Load Case 31 ΡΥΣΕΙΣ-ΟΔΟΣΤΡΩΣΙΑ
Factor forces and moments 1.000

Meshfree Loading

Kind	Referenceto	Projection	Coordinates W[m]	X[m]	Y[m]	Z[m]	Type	Loadvalue	
Area				0.000	2.000	0.000	PG	0.010	[MN/m2]
				11.800	2.000	0.000		0.010	[MN/m2]
				11.800	-1.000	0.000		0.006	[MN/m2]
				0.000	-1.000	0.000		0.006	[MN/m2]
	QGRP	3	3.000	(--)		activated		89.83	percent
Area				0.000	2.000	0.000	PG	0.010	[MN/m2]
				11.800	2.000	0.000		0.010	[MN/m2]
				11.800	-1.000	0.000		0.006	[MN/m2]
				0.000	-1.000	0.000		0.006	[MN/m2]
	QGRP	4	3.000	(--)		activated		10.17	percent

Load Case 32 PEZODROMIO
Factor forces and moments 1.000

Meshfree Loading

Kind	Referenceto	Projection	Coordinates W[m]	X[m]	Y[m]	Z[m]	Type	Loadvalue	
Area				11.800	-2.000	0.000	PG	0.016	[MN/m2]
				0.000	-2.000	0.000		0.016	[MN/m2]
				0.000	-1.000	0.000		0.016	[MN/m2]
				11.800	-1.000	0.000		0.016	[MN/m2]
	QGRP	3	3.000	(--)		activated		89.83	percent
Area				11.800	-2.000	0.000	PG	0.016	[MN/m2]
				0.000	-2.000	0.000		0.016	[MN/m2]
				0.000	-1.000	0.000		0.016	[MN/m2]
				11.800	-1.000	0.000		0.016	[MN/m2]
	QGRP	4	3.000	(--)		activated		10.17	percent

Load Case 33 ΠΡΟΣΘΕΤΗ ΟΔΟΣΤΡΩΣΙΑ 0.50KN/m2
Factor forces and moments 1.000

Meshfree Loading

Kind	Referenceto	Projection	Coordinates W[m]	X[m]	Y[m]	Z[m]	Type	Loadvalue	
Area				0.000	2.000	0.000	PG	0.001	[MN/m2]
				11.800	2.000	0.000		0.001	[MN/m2]
				11.800	-1.000	0.000		0.001	[MN/m2]
				0.000	-1.000	0.000		0.001	[MN/m2]
	QGRP	3	3.000	(--)		activated		89.83	percent
Area				0.000	2.000	0.000	PG	0.001	[MN/m2]
				11.800	2.000	0.000		0.001	[MN/m2]
				11.800	-1.000	0.000		0.001	[MN/m2]
				0.000	-1.000	0.000		0.001	[MN/m2]
	QGRP	4	3.000	(--)		activated		10.17	percent

Load Case 34 ΩΘΗΣΕΙΣ ΓΑΙΩΝ A1-KO(Φ-2)
Factor forces and moments 1.000

Meshfree Loading

Kind	Referenceto	Projection	Coordinates W[m]	X[m]	Y[m]	Z[m]	Type	Loadvalue	
Area				0.000	2.000	0.000	PXX	0.013	[MN/m2]
				0.000	2.000	0.800		0.013	[MN/m2]
				0.000	-2.000	0.800		0.013	[MN/m2]
				0.000	-2.000	0.000		0.013	[MN/m2]
	QGRP	8	3.000			activated		100.00	percent
Area				0.000	2.000	0.800	PXX	0.013	[MN/m2]
				0.000	-2.000	0.800		0.013	[MN/m2]
				0.000	-2.000	1.350		0.013	[MN/m2]
				0.000	2.000	1.350		0.013	[MN/m2]
	QGRP	9	3.000			activated		100.00	percent

Load Case 35 ΩΘΗΣΕΙΣ ΓΑΙΩΝ A2-KO(Φ-2)
Factor forces and moments 1.000

Meshfree Loading

Kind	Referenceto	Projection	Coordinates W[m]	X[m]	Y[m]	Z[m]	Type	Loadvalue	
Area				11.800	2.000	0.000	PXX	-0.013	[MN/m2]
				11.800	2.000	0.800		-0.013	[MN/m2]
				11.800	-2.000	0.800		-0.013	[MN/m2]
				11.800	-2.000	0.000		-0.013	[MN/m2]
	QGRP	8	3.000			activated		100.00	percent
Area				11.800	2.000	0.800	PXX	-0.013	[MN/m2]
				11.800	-2.000	0.800		-0.013	[MN/m2]

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FORTISEIS ΦΑΣΗ-2

```
Meshfree Loading
Kind     Referenceto  Projection Coordinates                 Type Loadvalue
                           W[m]      X[m]      Y[m]      Z[m]
                                    11.800    -2.000     1.350          -0.013 [MN/m2]
                                    11.800     2.000     1.350          -0.013 [MN/m2]
         QGRP      9       3.000                    activated          100.00 percent

Load Case   36 ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+0.5*A2(Φ1)
Factor forces and moments          1.000
Loads partially  copied from load case    34 with factor    0.500
Loads partially  copied from load case    35 with factor    0.500

Meshfree Loading
Kind     Referenceto  Projection Coordinates                 Type Loadvalue
                           W[m]      X[m]      Y[m]      Z[m]
Area                                 0.000     2.000     0.000 PXX       0.007 [MN/m2]
                                     0.000     2.000     0.800           0.007 [MN/m2]
                                     0.000    -2.000     0.800           0.007 [MN/m2]
                                     0.000    -2.000     0.000           0.007 [MN/m2]
         QGRP      8       3.000                    activated          100.00 percent
Area                                 0.000     2.000     0.800 PXX       0.007 [MN/m2]
                                     0.000    -2.000     0.800           0.007 [MN/m2]
                                     0.000    -2.000     1.350           0.007 [MN/m2]
                                     0.000     2.000     1.350           0.007 [MN/m2]
         QGRP      9       3.000                    activated          100.00 percent
Area                                11.800     2.000     0.000 PXX      -0.007 [MN/m2]
                                    11.800     2.000     0.800          -0.007 [MN/m2]
                                    11.800    -2.000     0.800          -0.007 [MN/m2]
                                    11.800    -2.000     0.000          -0.007 [MN/m2]
         QGRP      8       3.000                    activated          100.00 percent
Area                                11.800     2.000     0.800 PXX      -0.007 [MN/m2]
                                    11.800    -2.000     0.800          -0.007 [MN/m2]
                                    11.800    -2.000     1.350          -0.007 [MN/m2]
                                    11.800     2.000     1.350          -0.007 [MN/m2]
         QGRP      9       3.000                    activated          100.00 percent

Load Case   37 ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+0.5*A2(Φ1)
Factor forces and moments          1.000
Loads partially  copied from load case    34 with factor    1.000
Loads partially  copied from load case    35 with factor    0.500

Meshfree Loading
Kind     Referenceto  Projection Coordinates                 Type Loadvalue
                           W[m]      X[m]      Y[m]      Z[m]
Area                                 0.000     2.000     0.000 PXX       0.013 [MN/m2]
                                     0.000     2.000     0.800           0.013 [MN/m2]
                                     0.000    -2.000     0.800           0.013 [MN/m2]
                                     0.000    -2.000     0.000           0.013 [MN/m2]
         QGRP      8       3.000                    activated          100.00 percent
Area                                 0.000     2.000     0.800 PXX       0.013 [MN/m2]
                                     0.000    -2.000     0.800           0.013 [MN/m2]
                                     0.000    -2.000     1.350           0.013 [MN/m2]
                                     0.000     2.000     1.350           0.013 [MN/m2]
         QGRP      9       3.000                    activated          100.00 percent
Area                                11.800     2.000     0.000 PXX      -0.007 [MN/m2]
                                    11.800     2.000     0.800          -0.007 [MN/m2]
                                    11.800    -2.000     0.800          -0.007 [MN/m2]
                                    11.800    -2.000     0.000          -0.007 [MN/m2]
         QGRP      8       3.000                    activated          100.00 percent
Area                                11.800     2.000     0.800 PXX      -0.007 [MN/m2]
                                    11.800    -2.000     0.800          -0.007 [MN/m2]
                                    11.800    -2.000     1.350          -0.007 [MN/m2]
                                    11.800     2.000     1.350          -0.007 [MN/m2]
         QGRP      9       3.000                    activated          100.00 percent

Load Case   38 ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+1.0*A2(Φ1)
Factor forces and moments          1.000
Loads partially  copied from load case    34 with factor    0.500
Loads partially  copied from load case    35 with factor    1.000

Meshfree Loading
Kind     Referenceto  Projection Coordinates                 Type Loadvalue
                           W[m]      X[m]      Y[m]      Z[m]
Area                                 0.000     2.000     0.000 PXX       0.007 [MN/m2]
                                     0.000     2.000     0.800           0.007 [MN/m2]
                                     0.000    -2.000     0.800           0.007 [MN/m2]
                                     0.000    -2.000     0.000           0.007 [MN/m2]
         QGRP      8       3.000                    activated          100.00 percent
Area                                 0.000     2.000     0.800 PXX       0.007 [MN/m2]
                                     0.000    -2.000     0.800           0.007 [MN/m2]
                                     0.000    -2.000     1.350           0.007 [MN/m2]
```

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FORTISEIS ΦΑΣΗ-2

Meshfree Loading

```
Kind     Referenceto  Projection Coordinates                  Type Loadvalue
                          W[m]      X[m]      Y[m]      Z[m]
                                    0.000     2.000     1.350          0.007 [MN/m2]
         QGRP      9        3.000                   activated         100.00 percent
Area                                11.800     2.000     0.000 PXX     -0.013 [MN/m2]
                                    11.800     2.000     0.800         -0.013 [MN/m2]
                                    11.800    -2.000     0.800         -0.013 [MN/m2]
                                    11.800    -2.000     0.000         -0.013 [MN/m2]
         QGRP      8        3.000                   activated         100.00 percent
Area                                11.800     2.000     0.800 PXX     -0.013 [MN/m2]
                                    11.800    -2.000     0.800         -0.013 [MN/m2]
                                    11.800    -2.000     1.350         -0.013 [MN/m2]
                                    11.800     2.000     1.350         -0.013 [MN/m2]
         QGRP      9        3.000                   activated         100.00 percent
```

Load Case 39 ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+1.0*A2(Φ1)

```
Factor forces and moments          1.000
Loads partially  copied from load case    34 with factor    1.000
Loads partially  copied from load case    35 with factor    1.000
```

Meshfree Loading

```
Kind     Referenceto  Projection Coordinates                  Type Loadvalue
                          W[m]      X[m]      Y[m]      Z[m]
Area                                0.000     2.000     0.000 PXX      0.013 [MN/m2]
                                    0.000     2.000     0.800          0.013 [MN/m2]
                                    0.000    -2.000     0.800          0.013 [MN/m2]
                                    0.000    -2.000     0.000          0.013 [MN/m2]
         QGRP      8        3.000                   activated         100.00 percent
Area                                0.000     2.000     0.800 PXX      0.013 [MN/m2]
                                    0.000    -2.000     0.800          0.013 [MN/m2]
                                    0.000    -2.000     1.350          0.013 [MN/m2]
                                    0.000     2.000     1.350          0.013 [MN/m2]
         QGRP      9        3.000                   activated         100.00 percent
Area                                11.800     2.000     0.000 PXX     -0.013 [MN/m2]
                                    11.800     2.000     0.800         -0.013 [MN/m2]
                                    11.800    -2.000     0.800         -0.013 [MN/m2]
                                    11.800    -2.000     0.000         -0.013 [MN/m2]
         QGRP      8        3.000                   activated         100.00 percent
Area                                11.800     2.000     0.800 PXX     -0.013 [MN/m2]
                                    11.800    -2.000     0.800         -0.013 [MN/m2]
                                    11.800    -2.000     1.350         -0.013 [MN/m2]
                                    11.800     2.000     1.350         -0.013 [MN/m2]
         QGRP      9        3.000                   activated         100.00 percent
```

Load Case 41 L.L.UDL_2.50KN/m2

```
Factor forces and moments          1.000
```

Meshfree Loading

```
Kind     Referenceto  Projection Coordinates                  Type Loadvalue
                          W[m]      X[m]      Y[m]      Z[m]
Area                                0.000     2.000     0.000 PG       0.003 [MN/m2]
                                   11.800     2.000     0.000          0.003 [MN/m2]
                                   11.800    -2.000     0.000          0.003 [MN/m2]
                                    0.000    -2.000     0.000          0.003 [MN/m2]
         QGRP      3        3.000     (--)                activated         89.83 percent
Area                                0.000     2.000     0.000 PG       0.003 [MN/m2]
                                   11.800     2.000     0.000          0.003 [MN/m2]
                                   11.800    -2.000     0.000          0.003 [MN/m2]
                                    0.000    -2.000     0.000          0.003 [MN/m2]
         QGRP      4        3.000     (--)                activated         10.17 percent
```

Load Case 42 L.L.UDL_6.50KN/m2

```
Factor forces and moments          1.000
```

Meshfree Loading

```
Kind     Referenceto  Projection Coordinates                  Type Loadvalue
                          W[m]      X[m]      Y[m]      Z[m]
Area                               11.800    -1.000     0.000 PG       0.006 [MN/m2]
                                    0.000    -1.000     0.000          0.006 [MN/m2]
                                    0.000     2.000     0.000          0.006 [MN/m2]
                                   11.800     2.000     0.000          0.006 [MN/m2]
         QGRP      3        3.000     (--)                activated         89.83 percent
Area                               11.800    -1.000     0.000 PG       0.006 [MN/m2]
                                    0.000    -1.000     0.000          0.006 [MN/m2]
                                    0.000     2.000     0.000          0.006 [MN/m2]
                                   11.800     2.000     0.000          0.006 [MN/m2]
         QGRP      4        3.000     (--)                activated         10.17 percent
```

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FORTISEIS ΦΑΣΗ-2

Load Case 50 TS_RIGHT_Posit.1
Factor forces and moments 1.000

Meshfree Loading

Kind	Referenceto			Projection W[m]	Coordinates X[m]	Y[m]	Z[m]		Type	Loadvalue
Area					0.000	-0.900	0.000	PG		0.211 [MN/m2]
					0.800	-0.900	0.000			0.211 [MN/m2]
					0.800	-0.100	0.000			0.211 [MN/m2]
					0.000	-0.100	0.000			0.211 [MN/m2]
	QGRP	3	ZZ	3.000	(--)		activated			25.00 percent
Area					0.000	-0.900	0.000	PG		0.211 [MN/m2]
					0.800	-0.900	0.000			0.211 [MN/m2]
					0.800	-0.100	0.000			0.211 [MN/m2]
					0.000	-0.100	0.000			0.211 [MN/m2]
	QGRP	4	ZZ	3.000	(--)		activated			75.00 percent
Area					1.200	-0.900	0.000	PG		0.211 [MN/m2]
					2.000	-0.900	0.000			0.211 [MN/m2]
					2.000	-0.100	0.000			0.211 [MN/m2]
					1.200	-0.100	0.000			0.211 [MN/m2]
	QGRP	3	ZZ	3.000			activated			100.00 percent
Area					1.200	-0.900	0.000	PG		0.211 [MN/m2]
					2.000	-0.900	0.000			0.211 [MN/m2]
					2.000	-0.100	0.000			0.211 [MN/m2]
					1.200	-0.100	0.000			0.211 [MN/m2]
	QGRP	4	ZZ	3.000	(--)		activated			0.00 percent
Area					0.000	1.100	0.000	PG		0.211 [MN/m2]
					0.800	1.100	0.000			0.211 [MN/m2]
					0.800	1.900	0.000			0.211 [MN/m2]
					0.000	1.900	0.000			0.211 [MN/m2]
	QGRP	3	ZZ	3.000	(--)		activated			25.00 percent
Area					0.000	1.100	0.000	PG		0.211 [MN/m2]
					0.800	1.100	0.000			0.211 [MN/m2]
					0.800	1.900	0.000			0.211 [MN/m2]
					0.000	1.900	0.000			0.211 [MN/m2]
	QGRP	4	ZZ	3.000	(--)		activated			75.00 percent
Area					1.200	1.100	0.000	PG		0.211 [MN/m2]
					2.000	1.100	0.000			0.211 [MN/m2]
					2.000	1.900	0.000			0.211 [MN/m2]
					1.200	1.900	0.000			0.211 [MN/m2]
	QGRP	3	ZZ	3.000			activated			100.00 percent
Area					1.200	1.100	0.000	PG		0.211 [MN/m2]
					2.000	1.100	0.000			0.211 [MN/m2]
					2.000	1.900	0.000			0.211 [MN/m2]
					1.200	1.900	0.000			0.211 [MN/m2]
	QGRP	4	ZZ	3.000	(--)		activated			0.00 percent

Load Case 51 TS_RIGHT_Posit.2
Factor forces and moments 1.000

Meshfree Loading

Kind	Referenceto			Projection W[m]	Coordinates X[m]	Y[m]	Z[m]		Type	Loadvalue
Area					1.000	-0.900	0.000	PG		0.211 [MN/m2]
					1.800	-0.900	0.000			0.211 [MN/m2]
					1.800	-0.100	0.000			0.211 [MN/m2]
					1.000	-0.100	0.000			0.211 [MN/m2]
	QGRP	3	ZZ	3.000			activated			100.00 percent
Area					1.000	-0.900	0.000	PG		0.211 [MN/m2]
					1.800	-0.900	0.000			0.211 [MN/m2]
					1.800	-0.100	0.000			0.211 [MN/m2]
					1.000	-0.100	0.000			0.211 [MN/m2]
	QGRP	4	ZZ	3.000	(--)		activated			0.00 percent
Area					2.200	-0.900	0.000	PG		0.211 [MN/m2]
					3.000	-0.900	0.000			0.211 [MN/m2]
					3.000	-0.100	0.000			0.211 [MN/m2]
					2.200	-0.100	0.000			0.211 [MN/m2]
	QGRP	3	ZZ	3.000			activated			100.00 percent
Area					2.200	-0.900	0.000	PG		0.211 [MN/m2]
					3.000	-0.900	0.000			0.211 [MN/m2]
					3.000	-0.100	0.000			0.211 [MN/m2]
					2.200	-0.100	0.000			0.211 [MN/m2]
	QGRP	4	ZZ	3.000	(--)		activated			0.00 percent
Area					1.000	1.100	0.000	PG		0.211 [MN/m2]
					1.800	1.100	0.000			0.211 [MN/m2]
					1.800	1.900	0.000			0.211 [MN/m2]
					1.000	1.900	0.000			0.211 [MN/m2]
	QGRP	3	ZZ	3.000			activated			100.00 percent
Area					1.000	1.100	0.000	PG		0.211 [MN/m2]
					1.800	1.100	0.000			0.211 [MN/m2]
					1.800	1.900	0.000			0.211 [MN/m2]

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FORTISEIS ΦΑΣΗ-2

Meshfree Loading

```
Kind     Referenceto  Projection Coordinates                 Type Loadvalue
                          W[m]     X[m]     Y[m]     Z[m]
                                   1.000    1.900    0.000          0.211 [MN/m2]
         QGRP      4  ZZ   3.000    (--)          activated          0.00 percent
Area                               2.200    1.100    0.000 PG       0.211 [MN/m2]
                                   3.000    1.100    0.000          0.211 [MN/m2]
                                   3.000    1.900    0.000          0.211 [MN/m2]
                                   2.200    1.900    0.000          0.211 [MN/m2]
         QGRP      3  ZZ   3.000                   activated        100.00 percent
Area                               2.200    1.100    0.000 PG       0.211 [MN/m2]
                                   3.000    1.100    0.000          0.211 [MN/m2]
                                   3.000    1.900    0.000          0.211 [MN/m2]
                                   2.200    1.900    0.000          0.211 [MN/m2]
         QGRP      4  ZZ   3.000    (--)          activated          0.00 percent
```

Load Case 52 TS_RIGHT_Posit.3
Factor forces and moments 1.000

Meshfree Loading

```
Kind     Referenceto  Projection Coordinates                 Type Loadvalue
                          W[m]     X[m]     Y[m]     Z[m]
Area                               2.000   -0.900    0.000 PG       0.211 [MN/m2]
                                   2.800   -0.900    0.000          0.211 [MN/m2]
                                   2.800   -0.100    0.000          0.211 [MN/m2]
                                   2.000   -0.100    0.000          0.211 [MN/m2]
         QGRP      3  ZZ   3.000                   activated        100.00 percent
Area                               2.000   -0.900    0.000 PG       0.211 [MN/m2]
                                   2.800   -0.900    0.000          0.211 [MN/m2]
                                   2.800   -0.100    0.000          0.211 [MN/m2]
                                   2.000   -0.100    0.000          0.211 [MN/m2]
         QGRP      4  ZZ   3.000    (--)          activated          0.00 percent
Area                               3.200   -0.900    0.000 PG       0.211 [MN/m2]
                                   4.000   -0.900    0.000          0.211 [MN/m2]
                                   4.000   -0.100    0.000          0.211 [MN/m2]
                                   3.200   -0.100    0.000          0.211 [MN/m2]
         QGRP      3  ZZ   3.000                   activated        100.00 percent
Area                               3.200   -0.900    0.000 PG       0.211 [MN/m2]
                                   4.000   -0.900    0.000          0.211 [MN/m2]
                                   4.000   -0.100    0.000          0.211 [MN/m2]
                                   3.200   -0.100    0.000          0.211 [MN/m2]
         QGRP      4  ZZ   3.000    (--)          activated          0.00 percent
Area                               2.000    1.100    0.000 PG       0.211 [MN/m2]
                                   2.800    1.100    0.000          0.211 [MN/m2]
                                   2.800    1.900    0.000          0.211 [MN/m2]
                                   2.000    1.900    0.000          0.211 [MN/m2]
         QGRP      3  ZZ   3.000                   activated        100.00 percent
Area                               2.000    1.100    0.000 PG       0.211 [MN/m2]
                                   2.800    1.100    0.000          0.211 [MN/m2]
                                   2.800    1.900    0.000          0.211 [MN/m2]
                                   2.000    1.900    0.000          0.211 [MN/m2]
         QGRP      4  ZZ   3.000    (--)          activated          0.00 percent
Area                               3.200    1.100    0.000 PG       0.211 [MN/m2]
                                   4.000    1.100    0.000          0.211 [MN/m2]
                                   4.000    1.900    0.000          0.211 [MN/m2]
                                   3.200    1.900    0.000          0.211 [MN/m2]
         QGRP      3  ZZ   3.000                   activated        100.00 percent
Area                               3.200    1.100    0.000 PG       0.211 [MN/m2]
                                   4.000    1.100    0.000          0.211 [MN/m2]
                                   4.000    1.900    0.000          0.211 [MN/m2]
                                   3.200    1.900    0.000          0.211 [MN/m2]
         QGRP      4  ZZ   3.000    (--)          activated          0.00 percent
```

Load Case 53 TS_RIGHT_Posit.4
Factor forces and moments 1.000

Meshfree Loading

```
Kind     Referenceto  Projection Coordinates                 Type Loadvalue
                          W[m]     X[m]     Y[m]     Z[m]
Area                               3.000   -0.900    0.000 PG       0.211 [MN/m2]
                                   3.800   -0.900    0.000          0.211 [MN/m2]
                                   3.800   -0.100    0.000          0.211 [MN/m2]
                                   3.000   -0.100    0.000          0.211 [MN/m2]
         QGRP      3  ZZ   3.000                   activated        100.00 percent
Area                               3.000   -0.900    0.000 PG       0.211 [MN/m2]
                                   3.800   -0.900    0.000          0.211 [MN/m2]
                                   3.800   -0.100    0.000          0.211 [MN/m2]
                                   3.000   -0.100    0.000          0.211 [MN/m2]
         QGRP      4  ZZ   3.000    (--)          activated          0.00 percent
Area                               4.200   -0.900    0.000 PG       0.211 [MN/m2]
                                   5.000   -0.900    0.000          0.211 [MN/m2]
```

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FORTISEIS ΦΑΣΗ-2

Meshfree Loading

```
Kind     Referenceto  Projection Coordinates                  Type Loadvalue
                          W[m]     X[m]     Y[m]     Z[m]
                                   5.000   -0.100    0.000          0.211 [MN/m2]
                                   4.200   -0.100    0.000          0.211 [MN/m2]
         QGRP      3  ZZ   3.000                   activated        100.00 percent
Area                               4.200   -0.900    0.000 PG       0.211 [MN/m2]
                                   5.000   -0.900    0.000          0.211 [MN/m2]
                                   5.000   -0.100    0.000          0.211 [MN/m2]
                                   4.200   -0.100    0.000          0.211 [MN/m2]
         QGRP      4  ZZ   3.000    (--)          activated          0.00 percent
Area                               3.000    1.100    0.000 PG       0.211 [MN/m2]
                                   3.800    1.100    0.000          0.211 [MN/m2]
                                   3.800    1.900    0.000          0.211 [MN/m2]
                                   3.000    1.900    0.000          0.211 [MN/m2]
         QGRP      3  ZZ   3.000                   activated        100.00 percent
Area                               3.000    1.100    0.000 PG       0.211 [MN/m2]
                                   3.800    1.100    0.000          0.211 [MN/m2]
                                   3.800    1.900    0.000          0.211 [MN/m2]
                                   3.000    1.900    0.000          0.211 [MN/m2]
         QGRP      4  ZZ   3.000    (--)          activated          0.00 percent
Area                               4.200    1.100    0.000 PG       0.211 [MN/m2]
                                   5.000    1.100    0.000          0.211 [MN/m2]
                                   5.000    1.900    0.000          0.211 [MN/m2]
                                   4.200    1.900    0.000          0.211 [MN/m2]
         QGRP      3  ZZ   3.000                   activated        100.00 percent
Area                               4.200    1.100    0.000 PG       0.211 [MN/m2]
                                   5.000    1.100    0.000          0.211 [MN/m2]
                                   5.000    1.900    0.000          0.211 [MN/m2]
                                   4.200    1.900    0.000          0.211 [MN/m2]
         QGRP      4  ZZ   3.000    (--)          activated          0.00 percent
```

Load Case 54 TS_RIGHT_Posit.5
Factor forces and moments 1.000

Meshfree Loading

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Kind     Referenceto  Projection Coordinates                  Type Loadvalue
                          W[m]     X[m]     Y[m]     Z[m]
Area                               4.000   -0.900    0.000 PG       0.211 [MN/m2]
                                   4.800   -0.900    0.000          0.211 [MN/m2]
                                   4.800   -0.100    0.000          0.211 [MN/m2]
                                   4.000   -0.100    0.000          0.211 [MN/m2]
         QGRP      3  ZZ   3.000                   activated        100.00 percent
Area                               4.000   -0.900    0.000 PG       0.211 [MN/m2]
                                   4.800   -0.900    0.000          0.211 [MN/m2]
                                   4.800   -0.100    0.000          0.211 [MN/m2]
                                   4.000   -0.100    0.000          0.211 [MN/m2]
         QGRP      4  ZZ   3.000    (--)          activated          0.00 percent
Area                               5.200   -0.900    0.000 PG       0.211 [MN/m2]
                                   6.000   -0.900    0.000          0.211 [MN/m2]
                                   6.000   -0.100    0.000          0.211 [MN/m2]
                                   5.200   -0.100    0.000          0.211 [MN/m2]
         QGRP      3  ZZ   3.000                   activated        100.00 percent
Area                               5.200   -0.900    0.000 PG       0.211 [MN/m2]
                                   6.000   -0.900    0.000          0.211 [MN/m2]
                                   6.000   -0.100    0.000          0.211 [MN/m2]
                                   5.200   -0.100    0.000          0.211 [MN/m2]
         QGRP      4  ZZ   3.000    (--)          activated          0.00 percent
Area                               4.000    1.100    0.000 PG       0.211 [MN/m2]
                                   4.800    1.100    0.000          0.211 [MN/m2]
                                   4.800    1.900    0.000          0.211 [MN/m2]
                                   4.000    1.900    0.000          0.211 [MN/m2]
         QGRP      3  ZZ   3.000                   activated        100.00 percent
Area                               4.000    1.100    0.000 PG       0.211 [MN/m2]
                                   4.800    1.100    0.000          0.211 [MN/m2]
                                   4.800    1.900    0.000          0.211 [MN/m2]
                                   4.000    1.900    0.000          0.211 [MN/m2]
         QGRP      4  ZZ   3.000    (--)          activated          0.00 percent
Area                               5.200    1.100    0.000 PG       0.211 [MN/m2]
                                   6.000    1.100    0.000          0.211 [MN/m2]
                                   6.000    1.900    0.000          0.211 [MN/m2]
                                   5.200    1.900    0.000          0.211 [MN/m2]
         QGRP      3  ZZ   3.000                   activated        100.00 percent
Area                               5.200    1.100    0.000 PG       0.211 [MN/m2]
                                   6.000    1.100    0.000          0.211 [MN/m2]
                                   6.000    1.900    0.000          0.211 [MN/m2]
                                   5.200    1.900    0.000          0.211 [MN/m2]
         QGRP      4  ZZ   3.000    (--)          activated          0.00 percent
```

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FORTISEIS ΦΑΣΗ-2

Load Case 55 TS_RIGHT_Posit.6
Factor forces and moments 1.000

Meshfree Loading

Kind	Referenceto	Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue
Area			5.000	-0.900	0.000	PG	0.211 [MN/m2]
			5.800	-0.900	0.000		0.211 [MN/m2]
			5.800	-0.100	0.000		0.211 [MN/m2]
			5.000	-0.100	0.000		0.211 [MN/m2]
	QGRP 3 ZZ	3.000			activated		100.00 percent
Area			5.000	-0.900	0.000	PG	0.211 [MN/m2]
			5.800	-0.900	0.000		0.211 [MN/m2]
			5.800	-0.100	0.000		0.211 [MN/m2]
			5.000	-0.100	0.000		0.211 [MN/m2]
	QGRP 4 ZZ	3.000	(--)		activated		0.00 percent
Area			6.200	-0.900	0.000	PG	0.211 [MN/m2]
			7.000	-0.900	0.000		0.211 [MN/m2]
			7.000	-0.100	0.000		0.211 [MN/m2]
			6.200	-0.100	0.000		0.211 [MN/m2]
	QGRP 3 ZZ	3.000			activated		100.00 percent
Area			6.200	-0.900	0.000	PG	0.211 [MN/m2]
			7.000	-0.900	0.000		0.211 [MN/m2]
			7.000	-0.100	0.000		0.211 [MN/m2]
			6.200	-0.100	0.000		0.211 [MN/m2]
	QGRP 4 ZZ	3.000	(--)		activated		0.00 percent
Area			5.000	1.100	0.000	PG	0.211 [MN/m2]
			5.800	1.100	0.000		0.211 [MN/m2]
			5.800	1.900	0.000		0.211 [MN/m2]
			5.000	1.900	0.000		0.211 [MN/m2]
	QGRP 3 ZZ	3.000			activated		100.00 percent
Area			5.000	1.100	0.000	PG	0.211 [MN/m2]
			5.800	1.100	0.000		0.211 [MN/m2]
			5.800	1.900	0.000		0.211 [MN/m2]
			5.000	1.900	0.000		0.211 [MN/m2]
	QGRP 4 ZZ	3.000	(--)		activated		0.00 percent
Area			6.200	1.100	0.000	PG	0.211 [MN/m2]
			7.000	1.100	0.000		0.211 [MN/m2]
			7.000	1.900	0.000		0.211 [MN/m2]
			6.200	1.900	0.000		0.211 [MN/m2]
	QGRP 3 ZZ	3.000			activated		100.00 percent
Area			6.200	1.100	0.000	PG	0.211 [MN/m2]
			7.000	1.100	0.000		0.211 [MN/m2]
			7.000	1.900	0.000		0.211 [MN/m2]
			6.200	1.900	0.000		0.211 [MN/m2]
	QGRP 4 ZZ	3.000	(--)		activated		0.00 percent

Load Case 56 TS_RIGHT_Posit.7
Factor forces and moments 1.000

Meshfree Loading

Kind	Referenceto	Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue
Area			6.000	-0.900	0.000	PG	0.211 [MN/m2]
			6.800	-0.900	0.000		0.211 [MN/m2]
			6.800	-0.100	0.000		0.211 [MN/m2]
			6.000	-0.100	0.000		0.211 [MN/m2]
	QGRP 3 ZZ	3.000			activated		100.00 percent
Area			6.000	-0.900	0.000	PG	0.211 [MN/m2]
			6.800	-0.900	0.000		0.211 [MN/m2]
			6.800	-0.100	0.000		0.211 [MN/m2]
			6.000	-0.100	0.000		0.211 [MN/m2]
	QGRP 4 ZZ	3.000	(--)		activated		0.00 percent
Area			7.200	-0.900	0.000	PG	0.211 [MN/m2]
			8.000	-0.900	0.000		0.211 [MN/m2]
			8.000	-0.100	0.000		0.211 [MN/m2]
			7.200	-0.100	0.000		0.211 [MN/m2]
	QGRP 3 ZZ	3.000			activated		100.00 percent
Area			7.200	-0.900	0.000	PG	0.211 [MN/m2]
			8.000	-0.900	0.000		0.211 [MN/m2]
			8.000	-0.100	0.000		0.211 [MN/m2]
			7.200	-0.100	0.000		0.211 [MN/m2]
	QGRP 4 ZZ	3.000	(--)		activated		0.00 percent
Area			6.000	1.100	0.000	PG	0.211 [MN/m2]
			6.800	1.100	0.000		0.211 [MN/m2]
			6.800	1.900	0.000		0.211 [MN/m2]
			6.000	1.900	0.000		0.211 [MN/m2]
	QGRP 3 ZZ	3.000			activated		100.00 percent
Area			6.000	1.100	0.000	PG	0.211 [MN/m2]
			6.800	1.100	0.000		0.211 [MN/m2]
			6.800	1.900	0.000		0.211 [MN/m2]

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FORTISEIS ΦΑΣΗ-2

Meshfree Loading

```
Kind     Referenceto  Projection Coordinates                  Type Loadvalue
                          W[m]     X[m]     Y[m]     Z[m]
                                   6.000    1.900    0.000          0.211 [MN/m2]
         QGRP      4  ZZ   3.000    (--)          activated          0.00 percent
Area                               7.200    1.100    0.000 PG       0.211 [MN/m2]
                                   8.000    1.100    0.000          0.211 [MN/m2]
                                   8.000    1.900    0.000          0.211 [MN/m2]
                                   7.200    1.900    0.000          0.211 [MN/m2]
         QGRP      3  ZZ   3.000                   activated        100.00 percent
Area                               7.200    1.100    0.000 PG       0.211 [MN/m2]
                                   8.000    1.100    0.000          0.211 [MN/m2]
                                   8.000    1.900    0.000          0.211 [MN/m2]
                                   7.200    1.900    0.000          0.211 [MN/m2]
         QGRP      4  ZZ   3.000    (--)          activated          0.00 percent
```

Load Case 57 TS_RIGHT_Posit.8
Factor forces and moments 1.000

Meshfree Loading

```
Kind     Referenceto  Projection Coordinates                  Type Loadvalue
                          W[m]     X[m]     Y[m]     Z[m]
Area                               7.000   -0.900    0.000 PG       0.211 [MN/m2]
                                   7.800   -0.900    0.000          0.211 [MN/m2]
                                   7.800   -0.100    0.000          0.211 [MN/m2]
                                   7.000   -0.100    0.000          0.211 [MN/m2]
         QGRP      3  ZZ   3.000                   activated        100.00 percent
Area                               7.000   -0.900    0.000 PG       0.211 [MN/m2]
                                   7.800   -0.900    0.000          0.211 [MN/m2]
                                   7.800   -0.100    0.000          0.211 [MN/m2]
                                   7.000   -0.100    0.000          0.211 [MN/m2]
         QGRP      4  ZZ   3.000    (--)          activated          0.00 percent
Area                               8.200   -0.900    0.000 PG       0.211 [MN/m2]
                                   9.000   -0.900    0.000          0.211 [MN/m2]
                                   9.000   -0.100    0.000          0.211 [MN/m2]
                                   8.200   -0.100    0.000          0.211 [MN/m2]
         QGRP      3  ZZ   3.000                   activated        100.00 percent
Area                               8.200   -0.900    0.000 PG       0.211 [MN/m2]
                                   9.000   -0.900    0.000          0.211 [MN/m2]
                                   9.000   -0.100    0.000          0.211 [MN/m2]
                                   8.200   -0.100    0.000          0.211 [MN/m2]
         QGRP      4  ZZ   3.000    (--)          activated          0.00 percent
Area                               7.000    1.100    0.000 PG       0.211 [MN/m2]
                                   7.800    1.100    0.000          0.211 [MN/m2]
                                   7.800    1.900    0.000          0.211 [MN/m2]
                                   7.000    1.900    0.000          0.211 [MN/m2]
         QGRP      3  ZZ   3.000                   activated        100.00 percent
Area                               7.000    1.100    0.000 PG       0.211 [MN/m2]
                                   7.800    1.100    0.000          0.211 [MN/m2]
                                   7.800    1.900    0.000          0.211 [MN/m2]
                                   7.000    1.900    0.000          0.211 [MN/m2]
         QGRP      4  ZZ   3.000    (--)          activated          0.00 percent
Area                               8.200    1.100    0.000 PG       0.211 [MN/m2]
                                   9.000    1.100    0.000          0.211 [MN/m2]
                                   9.000    1.900    0.000          0.211 [MN/m2]
                                   8.200    1.900    0.000          0.211 [MN/m2]
         QGRP      3  ZZ   3.000                   activated        100.00 percent
Area                               8.200    1.100    0.000 PG       0.211 [MN/m2]
                                   9.000    1.100    0.000          0.211 [MN/m2]
                                   9.000    1.900    0.000          0.211 [MN/m2]
                                   8.200    1.900    0.000          0.211 [MN/m2]
         QGRP      4  ZZ   3.000    (--)          activated          0.00 percent
```

Load Case 58 TS_RIGHT_Posit.9
Factor forces and moments 1.000

Meshfree Loading

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Kind     Referenceto  Projection Coordinates                  Type Loadvalue
                          W[m]     X[m]     Y[m]     Z[m]
Area                               8.000   -0.900    0.000 PG       0.211 [MN/m2]
                                   8.800   -0.900    0.000          0.211 [MN/m2]
                                   8.800   -0.100    0.000          0.211 [MN/m2]
                                   8.000   -0.100    0.000          0.211 [MN/m2]
         QGRP      3  ZZ   3.000                   activated        100.00 percent
Area                               8.000   -0.900    0.000 PG       0.211 [MN/m2]
                                   8.800   -0.900    0.000          0.211 [MN/m2]
                                   8.800   -0.100    0.000          0.211 [MN/m2]
                                   8.000   -0.100    0.000          0.211 [MN/m2]
         QGRP      4  ZZ   3.000    (--)          activated          0.00 percent
Area                               9.200   -0.900    0.000 PG       0.211 [MN/m2]
                                  10.000   -0.900    0.000          0.211 [MN/m2]
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ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FORTISEIS ΦΑΣΗ-2

Meshfree Loading

Kind	Referenceto		Projection W[m]	Coordinates X[m]	Y[m]	Z[m]		Type	Loadvalue	
				10.000	-0.100	0.000			0.211	[MN/m2]
				9.200	-0.100	0.000			0.211	[MN/m2]
	QGRP	3	ZZ 3.000					activated	100.00	percent
Area				9.200	-0.900	0.000	PG		0.211	[MN/m2]
				10.000	-0.900	0.000			0.211	[MN/m2]
				10.000	-0.100	0.000			0.211	[MN/m2]
				9.200	-0.100	0.000			0.211	[MN/m2]
	QGRP	4	ZZ 3.000	(--)				activated	0.00	percent
Area				8.000	1.100	0.000	PG		0.211	[MN/m2]
				8.800	1.100	0.000			0.211	[MN/m2]
				8.800	1.900	0.000			0.211	[MN/m2]
				8.000	1.900	0.000			0.211	[MN/m2]
	QGRP	3	ZZ 3.000					activated	100.00	percent
Area				8.000	1.100	0.000	PG		0.211	[MN/m2]
				8.800	1.100	0.000			0.211	[MN/m2]
				8.800	1.900	0.000			0.211	[MN/m2]
				8.000	1.900	0.000			0.211	[MN/m2]
	QGRP	4	ZZ 3.000	(--)				activated	0.00	percent
Area				9.200	1.100	0.000	PG		0.211	[MN/m2]
				10.000	1.100	0.000			0.211	[MN/m2]
				10.000	1.900	0.000			0.211	[MN/m2]
				9.200	1.900	0.000			0.211	[MN/m2]
	QGRP	3	ZZ 3.000					activated	100.00	percent
Area				9.200	1.100	0.000	PG		0.211	[MN/m2]
				10.000	1.100	0.000			0.211	[MN/m2]
				10.000	1.900	0.000			0.211	[MN/m2]
				9.200	1.900	0.000			0.211	[MN/m2]
	QGRP	4	ZZ 3.000	(--)				activated	0.00	percent

Load Case 59 TS_RIGHT_Posit.10
Factor forces and moments 1.000

Meshfree Loading

Kind	Referenceto		Projection W[m]	Coordinates X[m]	Y[m]	Z[m]		Type	Loadvalue	
Area				9.000	-0.900	0.000	PG		0.211	[MN/m2]
				9.800	-0.900	0.000			0.211	[MN/m2]
				9.800	-0.100	0.000			0.211	[MN/m2]
				9.000	-0.100	0.000			0.211	[MN/m2]
	QGRP	3	ZZ 3.000					activated	100.00	percent
Area				9.000	-0.900	0.000	PG		0.211	[MN/m2]
				9.800	-0.900	0.000			0.211	[MN/m2]
				9.800	-0.100	0.000			0.211	[MN/m2]
				9.000	-0.100	0.000			0.211	[MN/m2]
	QGRP	4	ZZ 3.000	(--)				activated	0.00	percent
Area				10.200	-0.900	0.000	PG		0.211	[MN/m2]
				11.000	-0.900	0.000			0.211	[MN/m2]
				11.000	-0.100	0.000			0.211	[MN/m2]
				10.200	-0.100	0.000			0.211	[MN/m2]
	QGRP	3	ZZ 3.000					activated	100.00	percent
Area				10.200	-0.900	0.000	PG		0.211	[MN/m2]
				11.000	-0.900	0.000			0.211	[MN/m2]
				11.000	-0.100	0.000			0.211	[MN/m2]
				10.200	-0.100	0.000			0.211	[MN/m2]
	QGRP	4	ZZ 3.000	(--)				activated	0.00	percent
Area				9.000	1.100	0.000	PG		0.211	[MN/m2]
				9.800	1.100	0.000			0.211	[MN/m2]
				9.800	1.900	0.000			0.211	[MN/m2]
				9.000	1.900	0.000			0.211	[MN/m2]
	QGRP	3	ZZ 3.000					activated	100.00	percent
Area				9.000	1.100	0.000	PG		0.211	[MN/m2]
				9.800	1.100	0.000			0.211	[MN/m2]
				9.800	1.900	0.000			0.211	[MN/m2]
				9.000	1.900	0.000			0.211	[MN/m2]
	QGRP	4	ZZ 3.000	(--)				activated	0.00	percent
Area				10.200	1.100	0.000	PG		0.211	[MN/m2]
				11.000	1.100	0.000			0.211	[MN/m2]
				11.000	1.900	0.000			0.211	[MN/m2]
				10.200	1.900	0.000			0.211	[MN/m2]
	QGRP	3	ZZ 3.000					activated	100.00	percent
Area				10.200	1.100	0.000	PG		0.211	[MN/m2]
				11.000	1.100	0.000			0.211	[MN/m2]
				11.000	1.900	0.000			0.211	[MN/m2]
				10.200	1.900	0.000			0.211	[MN/m2]
	QGRP	4	ZZ 3.000	(--)				activated	0.00	percent

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FORTISEIS ΦΑΣΗ-2

Load Case 60 TS_RIGHT_Posit.11
Factor forces and moments 1.000

Meshfree Loading

Kind	Referenceto			Projection W[m]	Coordinates X[m]	Y[m]	Z[m]		Type	Loadvalue	
Area					10.000	-0.900	0.000	PG		0.211	[MN/m2]
					10.800	-0.900	0.000			0.211	[MN/m2]
					10.800	-0.100	0.000			0.211	[MN/m2]
					10.000	-0.100	0.000			0.211	[MN/m2]
	QGRP	3	ZZ	3.000			activated			100.00	percent
Area					10.000	-0.900	0.000	PG		0.211	[MN/m2]
					10.800	-0.900	0.000			0.211	[MN/m2]
					10.800	-0.100	0.000			0.211	[MN/m2]
					10.000	-0.100	0.000			0.211	[MN/m2]
	QGRP	4	ZZ	3.000	(--)		activated			0.00	percent
Area					11.200	-0.900	0.000	PG		0.211	[MN/m2]
					12.000	-0.900	0.000			0.211	[MN/m2]
					12.000	-0.100	0.000			0.211	[MN/m2]
					11.200	-0.100	0.000			0.211	[MN/m2]
	QGRP	3	ZZ	3.000	(--)		activated			0.00	percent
Area					11.200	-0.900	0.000	PG		0.211	[MN/m2]
					12.000	-0.900	0.000			0.211	[MN/m2]
					12.000	-0.100	0.000			0.211	[MN/m2]
					11.200	-0.100	0.000			0.211	[MN/m2]
	QGRP	4	ZZ	3.000	(--)		activated			75.00	percent
Area					10.000	1.100	0.000	PG		0.211	[MN/m2]
					10.800	1.100	0.000			0.211	[MN/m2]
					10.800	1.900	0.000			0.211	[MN/m2]
					10.000	1.900	0.000			0.211	[MN/m2]
	QGRP	3	ZZ	3.000			activated			100.00	percent
Area					10.000	1.100	0.000	PG		0.211	[MN/m2]
					10.800	1.100	0.000			0.211	[MN/m2]
					10.800	1.900	0.000			0.211	[MN/m2]
					10.000	1.900	0.000			0.211	[MN/m2]
	QGRP	4	ZZ	3.000	(--)		activated			0.00	percent
Area					11.200	1.100	0.000	PG		0.211	[MN/m2]
					12.000	1.100	0.000			0.211	[MN/m2]
					12.000	1.900	0.000			0.211	[MN/m2]
					11.200	1.900	0.000			0.211	[MN/m2]
	QGRP	3	ZZ	3.000	(--)		activated			0.00	percent
Area					11.200	1.100	0.000	PG		0.211	[MN/m2]
					12.000	1.100	0.000			0.211	[MN/m2]
					12.000	1.900	0.000			0.211	[MN/m2]
					11.200	1.900	0.000			0.211	[MN/m2]
	QGRP	4	ZZ	3.000	(--)		activated			75.00	percent

Load Case 61 TS_RIGHT_Posit.12
Factor forces and moments 1.000

Meshfree Loading

Kind	Referenceto			Projection W[m]	Coordinates X[m]	Y[m]	Z[m]		Type	Loadvalue	
Area					11.000	-0.900	0.000	PG		0.211	[MN/m2]
					11.800	-0.900	0.000			0.211	[MN/m2]
					11.800	-0.100	0.000			0.211	[MN/m2]
					11.000	-0.100	0.000			0.211	[MN/m2]
	QGRP	3	ZZ	3.000	(--)		activated			25.00	percent
Area					11.000	-0.900	0.000	PG		0.211	[MN/m2]
					11.800	-0.900	0.000			0.211	[MN/m2]
					11.800	-0.100	0.000			0.211	[MN/m2]
					11.000	-0.100	0.000			0.211	[MN/m2]
	QGRP	4	ZZ	3.000	(--)		activated			75.00	percent
Area					12.200	-0.900	0.000	PG		0.211	[MN/m2]
					13.000	-0.900	0.000			0.211	[MN/m2]
					13.000	-0.100	0.000			0.211	[MN/m2]
					12.200	-0.100	0.000			0.211	[MN/m2]
	QGRP	3	ZZ	3.000	(--)		activated			0.00	percent
Area					12.200	-0.900	0.000	PG		0.211	[MN/m2]
					13.000	-0.900	0.000			0.211	[MN/m2]
					13.000	-0.100	0.000			0.211	[MN/m2]
					12.200	-0.100	0.000			0.211	[MN/m2]
	QGRP	4	ZZ	3.000	(--)		activated			0.00	percent
Area					11.000	1.100	0.000	PG		0.211	[MN/m2]
					11.800	1.100	0.000			0.211	[MN/m2]
					11.800	1.900	0.000			0.211	[MN/m2]
					11.000	1.900	0.000			0.211	[MN/m2]
	QGRP	3	ZZ	3.000	(--)		activated			25.00	percent
Area					11.000	1.100	0.000	PG		0.211	[MN/m2]
					11.800	1.100	0.000			0.211	[MN/m2]
					11.800	1.900	0.000			0.211	[MN/m2]

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FORTISEIS ΦΑΣΗ-2

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Meshfree Loading
Kind     Referenceto  Projection Coordinates                 Type Loadvalue
                          W[m]     X[m]     Y[m]     Z[m]
                                  11.000    1.900    0.000          0.211 [MN/m2]
         QGRP      4  ZZ   3.000     (--)          activated         75.00 percent
Area                              12.200    1.100    0.000 PG       0.211 [MN/m2]
                                  13.000    1.100    0.000          0.211 [MN/m2]
                                  13.000    1.900    0.000          0.211 [MN/m2]
                                  12.200    1.900    0.000          0.211 [MN/m2]
         QGRP      3  ZZ   3.000     (--)          activated          0.00 percent
Area                              12.200    1.100    0.000 PG       0.211 [MN/m2]
                                  13.000    1.100    0.000          0.211 [MN/m2]
                                  13.000    1.900    0.000          0.211 [MN/m2]
                                  12.200    1.900    0.000          0.211 [MN/m2]
         QGRP      4  ZZ   3.000     (--)          activated          0.00 percent

Load Case   70 ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A1 2.50-K0 (ΦΑΣΗ-
Factor forces and moments         1.000

Meshfree Loading
Kind     Referenceto  Projection Coordinates                 Type Loadvalue
                          W[m]     X[m]     Y[m]     Z[m]
Area                               0.000    2.000    0.000 PXX      0.001 [MN/m2]
                                   0.000    2.000    0.800          0.001 [MN/m2]
                                   0.000   -2.000    0.800          0.001 [MN/m2]
                                   0.000   -2.000    0.000          0.001 [MN/m2]
         QGRP      8       3.000                   activated        100.00 percent
Area                               0.000    2.000    0.800 PXX      0.001 [MN/m2]
                                   0.000   -2.000    0.800          0.001 [MN/m2]
                                   0.000   -2.000    1.350          0.001 [MN/m2]
                                   0.000    2.000    1.350          0.001 [MN/m2]
         QGRP      9       3.000                   activated        100.00 percent

Load Case   71 ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A2 2.50-K0 (ΦΑΣΗ-
Factor forces and moments         1.000

Meshfree Loading
Kind     Referenceto  Projection Coordinates                 Type Loadvalue
                          W[m]     X[m]     Y[m]     Z[m]
Area                              11.800    2.000    0.000 PXX     -0.001 [MN/m2]
                                  11.800    2.000    0.800         -0.001 [MN/m2]
                                  11.800   -2.000    0.800         -0.001 [MN/m2]
                                  11.800   -2.000    0.000         -0.001 [MN/m2]
         QGRP      8       3.000                   activated        100.00 percent
Area                              11.800    2.000    0.800 PXX     -0.001 [MN/m2]
                                  11.800   -2.000    0.800         -0.001 [MN/m2]
                                  11.800   -2.000    1.350         -0.001 [MN/m2]
                                  11.800    2.000    1.350         -0.001 [MN/m2]
         QGRP      9       3.000                   activated        100.00 percent

Load Case   72 ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A1 6.50-K0 (ΦΑΣΗ-
Factor forces and moments         1.000

Meshfree Loading
Kind     Referenceto  Projection Coordinates                 Type Loadvalue
                          W[m]     X[m]     Y[m]     Z[m]
Area                               0.000    2.000    0.000 PXX      0.003 [MN/m2]
                                   0.000    2.000    0.800          0.003 [MN/m2]
                                   0.000   -2.000    0.800          0.003 [MN/m2]
                                   0.000   -2.000    0.000          0.003 [MN/m2]
         QGRP      8       3.000                   activated        100.00 percent
Area                               0.000    2.000    0.800 PXX      0.003 [MN/m2]
                                   0.000   -2.000    0.800          0.003 [MN/m2]
                                   0.000   -2.000    1.350          0.003 [MN/m2]
                                   0.000    2.000    1.350          0.003 [MN/m2]
         QGRP      9       3.000                   activated        100.00 percent

Load Case   73 ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A2 6.50-K0 (ΦΑΣΗ-
Factor forces and moments         1.000

Meshfree Loading
Kind     Referenceto  Projection Coordinates                 Type Loadvalue
                          W[m]     X[m]     Y[m]     Z[m]
Area                              11.800    2.000    0.000 PXX     -0.003 [MN/m2]
                                  11.800    2.000    0.800         -0.003 [MN/m2]
                                  11.800   -2.000    0.800         -0.003 [MN/m2]
                                  11.800   -2.000    0.000         -0.003 [MN/m2]
         QGRP      8       3.000                   activated        100.00 percent
Area                              11.800    2.000    0.800 PXX     -0.003 [MN/m2]
                                  11.800   -2.000    0.800         -0.003 [MN/m2]
                                  11.800   -2.000    1.350         -0.003 [MN/m2]
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ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FORTISEIS ΦΑΣΗ-2

Meshfree Loading

Kind	Referenceto	Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue
			11.800	2.000	1.350		-0.003 [MN/m2]
	QGRP 9	3.000			activated		100.00 percent

Load Case 74 ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A1 TS1-K0 (ΦΑΣΗ-2
Factor forces and moments 1.000

Meshfree Loading

Kind	Referenceto	Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue
Area			0.000	2.000	0.000	PXX	0.041 [MN/m2]
			0.000	2.000	0.800		0.041 [MN/m2]
			0.000	-2.000	0.800		0.041 [MN/m2]
			0.000	-2.000	0.000		0.041 [MN/m2]
	QGRP 8	3.000			activated		100.00 percent
Area			0.000	2.000	0.800	PXX	0.022 [MN/m2]
			0.000	-2.000	0.800		0.022 [MN/m2]
			0.000	-2.000	1.350		0.016 [MN/m2]
			0.000	2.000	1.350		0.016 [MN/m2]
	QGRP 9	3.000			activated		100.00 percent

Load Case 75 ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A2 TS1-K0 (ΦΑΣΗ-2
Factor forces and moments 1.000

Meshfree Loading

Kind	Referenceto	Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue
Area			11.800	2.000	0.000	PXX	-0.041 [MN/m2]
			11.800	2.000	0.800		-0.041 [MN/m2]
			11.800	-2.000	0.800		-0.041 [MN/m2]
			11.800	-2.000	0.000		-0.041 [MN/m2]
	QGRP 8	3.000			activated		100.00 percent
Area			11.800	2.000	0.800	PXX	-0.022 [MN/m2]
			11.800	-2.000	0.800		-0.022 [MN/m2]
			11.800	-2.000	1.350		-0.016 [MN/m2]
			11.800	2.000	1.350		-0.016 [MN/m2]
	QGRP 9	3.000			activated		100.00 percent

Load Case 80 ΠΙΘΑΝΕΣ ΚΑΘΙΖΗΣΕΙΣ A1
Factor forces and moments 1.000

Load Case 81 ΠΙΘΑΝΕΣ ΚΑΘΙΖΗΣΕΙΣ A2
Factor forces and moments 1.000

Load Case 85 ΔΤΝ = +33
Factor forces and moments 1.000

Load Case 86 ΔΤΝ = -20
Factor forces and moments 1.000

Load Case 87 ΔΤΜ = 13
Factor forces and moments 1.000

Load Case 88 ΔΤΜ = -7
Factor forces and moments 1.000

Load Case 90 (+ΔΤΝ)+0.75*(+ΔΤΜ)
Factor forces and moments 1.000
Loads partially copied from load case 85 with factor 1.000
Loads partially copied from load case 87 with factor 0.750

Load Case 91 (+ΔΤΝ)+0.75*(-ΔΤΜ)
Factor forces and moments 1.000
Loads partially copied from load case 85 with factor 1.000
Loads partially copied from load case 88 with factor 0.750

Load Case 92 (-ΔΤΝ)+0.75*(+ΔΤΜ)
Factor forces and moments 1.000
Loads partially copied from load case 86 with factor 1.000
Loads partially copied from load case 87 with factor 0.750

Load Case 93 (-ΔΤΝ)+0.75*(-ΔΤΜ)
Factor forces and moments 1.000
Loads partially copied from load case 86 with factor 1.000
Loads partially copied from load case 88 with factor 0.750

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
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Load Case 94 0.35*(+ΔΤΝ)+(+ΔΤΜ)
Factor forces and moments 1.000
Loads partially copied from load case 85 with factor 0.350
Loads partially copied from load case 87 with factor 1.000

Load Case 95 0.35*(+ΔΤΝ)+(-ΔΤΜ)
Factor forces and moments 1.000
Loads partially copied from load case 91 with factor 0.350
Loads partially copied from load case 94 with factor 1.000

Load Case 96 0.35*(-ΔΤΝ)+(+ΔΤΜ)
Factor forces and moments 1.000
Loads partially copied from load case 92 with factor 0.350
Loads partially copied from load case 93 with factor 1.000

Load Case 97 0.35*(-ΔΤΝ)+(-ΔΤΜ)
Factor forces and moments 1.000
Loads partially copied from load case 92 with factor 0.350
Loads partially copied from load case 94 with factor 1.000

Load Case 100 CRASH_-Y_Pos.1 _DECK
Factor forces and moments 1.000

Meshfree Loading

Kind	Referenceto		Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
Line				0.000	-1.750	0.000	PYY	-0.480	[MN/m]
				0.500	-1.750	0.000		-0.480	[MN/m]
	QGRP	3	2.000	(--)	activated			0.00	percent
Line				0.000	-1.750	0.000	PYY	-0.480	[MN/m]
				0.500	-1.750	0.000		-0.480	[MN/m]
	QGRP	4	2.000		activated			100.00	percent
Line				0.000	-1.750	0.000	MXX	-0.480	[MNm/m]
				0.500	-1.750	0.000		-0.480	[MNm/m]
	QGRP	3	2.000	(--)	activated			0.00	percent
Line				0.000	-1.750	0.000	MXX	-0.480	[MNm/m]
				0.500	-1.750	0.000		-0.480	[MNm/m]
	QGRP	4	2.000		activated			100.00	percent

Load Case 101 CRASH_-Y_Pos.2 _DECK
Factor forces and moments 1.000

Meshfree Loading

Kind	Referenceto		Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
Line				1.000	-1.750	0.000	PYY	-0.480	[MN/m]
				1.500	-1.750	0.000		-0.480	[MN/m]
	QGRP	3	2.000		activated			100.00	percent
Line				1.000	-1.750	0.000	PYY	-0.480	[MN/m]
				1.500	-1.750	0.000		-0.480	[MN/m]
	QGRP	4	2.000	(--)	activated			0.00	percent
Line				1.000	-1.750	0.000	MXX	-0.480	[MNm/m]
				1.500	-1.750	0.000		-0.480	[MNm/m]
	QGRP	3	2.000		activated			100.00	percent
Line				1.000	-1.750	0.000	MXX	-0.480	[MNm/m]
				1.500	-1.750	0.000		-0.480	[MNm/m]
	QGRP	4	2.000	(--)	activated			0.00	percent

Load Case 102 CRASH_-Y_Pos.3 _DECK
Factor forces and moments 1.000

Meshfree Loading

Kind	Referenceto		Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
Line				2.000	-1.750	0.000	PYY	-0.480	[MN/m]
				2.500	-1.750	0.000		-0.480	[MN/m]
	QGRP	3	2.000		activated			100.00	percent
Line				2.000	-1.750	0.000	PYY	-0.480	[MN/m]
				2.500	-1.750	0.000		-0.480	[MN/m]
	QGRP	4	2.000	(--)	activated			0.00	percent
Line				2.000	-1.750	0.000	MXX	-0.480	[MNm/m]
				2.500	-1.750	0.000		-0.480	[MNm/m]
	QGRP	3	2.000		activated			100.00	percent
Line				2.000	-1.750	0.000	MXX	-0.480	[MNm/m]
				2.500	-1.750	0.000		-0.480	[MNm/m]
	QGRP	4	2.000	(--)	activated			0.00	percent

Load Case 103 CRASH_-Y_Pos.4 _DECK
Factor forces and moments 1.000

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
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Meshfree Loading

Kind	Referenceto		Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
Line				3.000	-1.750	0.000	PYY	-0.480	[MN/m]
				3.500	-1.750	0.000		-0.480	[MN/m]
	QGRP	3	2.000			activated		100.00	percent
Line				3.000	-1.750	0.000	PYY	-0.480	[MN/m]
				3.500	-1.750	0.000		-0.480	[MN/m]
	QGRP	4	2.000	(--)		activated		0.00	percent
Line				3.000	-1.750	0.000	MXX	-0.480	[MNm/m]
				3.500	-1.750	0.000		-0.480	[MNm/m]
	QGRP	3	2.000			activated		100.00	percent
Line				3.000	-1.750	0.000	MXX	-0.480	[MNm/m]
				3.500	-1.750	0.000		-0.480	[MNm/m]
	QGRP	4	2.000	(--)		activated		0.00	percent

Load Case 104 CRASH_-Y_Pos.5 _DECK
Factor forces and moments 1.000

Meshfree Loading

Kind	Referenceto		Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
Line				4.000	-1.750	0.000	PYY	-0.480	[MN/m]
				4.500	-1.750	0.000		-0.480	[MN/m]
	QGRP	3	2.000			activated		100.00	percent
Line				4.000	-1.750	0.000	PYY	-0.480	[MN/m]
				4.500	-1.750	0.000		-0.480	[MN/m]
	QGRP	4	2.000	(--)		activated		0.00	percent
Line				4.000	-1.750	0.000	MXX	-0.480	[MNm/m]
				4.500	-1.750	0.000		-0.480	[MNm/m]
	QGRP	3	2.000			activated		100.00	percent
Line				4.000	-1.750	0.000	MXX	-0.480	[MNm/m]
				4.500	-1.750	0.000		-0.480	[MNm/m]
	QGRP	4	2.000	(--)		activated		0.00	percent

Load Case 105 CRASH_-Y_Pos.6 _DECK
Factor forces and moments 1.000

Meshfree Loading

Kind	Referenceto		Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
Line				5.000	-1.750	0.000	PYY	-0.480	[MN/m]
				5.500	-1.750	0.000		-0.480	[MN/m]
	QGRP	3	2.000			activated		100.00	percent
Line				5.000	-1.750	0.000	PYY	-0.480	[MN/m]
				5.500	-1.750	0.000		-0.480	[MN/m]
	QGRP	4	2.000	(--)		activated		0.00	percent
Line				5.000	-1.750	0.000	MXX	-0.480	[MNm/m]
				5.500	-1.750	0.000		-0.480	[MNm/m]
	QGRP	3	2.000			activated		100.00	percent
Line				5.000	-1.750	0.000	MXX	-0.480	[MNm/m]
				5.500	-1.750	0.000		-0.480	[MNm/m]
	QGRP	4	2.000	(--)		activated		0.00	percent

Load Case 106 CRASH_-Y_Pos.7 _DECK
Factor forces and moments 1.000

Meshfree Loading

Kind	Referenceto		Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
Line				6.000	-1.750	0.000	PYY	-0.480	[MN/m]
				6.500	-1.750	0.000		-0.480	[MN/m]
	QGRP	3	2.000			activated		100.00	percent
Line				6.000	-1.750	0.000	PYY	-0.480	[MN/m]
				6.500	-1.750	0.000		-0.480	[MN/m]
	QGRP	4	2.000	(--)		activated		0.00	percent
Line				6.000	-1.750	0.000	MXX	-0.480	[MNm/m]
				6.500	-1.750	0.000		-0.480	[MNm/m]
	QGRP	3	2.000			activated		100.00	percent
Line				6.000	-1.750	0.000	MXX	-0.480	[MNm/m]
				6.500	-1.750	0.000		-0.480	[MNm/m]
	QGRP	4	2.000	(--)		activated		0.00	percent

Load Case 107 CRASH_-Y_Pos.8 _DECK
Factor forces and moments 1.000

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
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Meshfree Loading

Kind	Referenceto		Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
Line				7.000	-1.750	0.000	PYY	-0.480	[MN/m]
				7.500	-1.750	0.000		-0.480	[MN/m]
	QGRP	3	2.000			activated		100.00	percent
Line				7.000	-1.750	0.000	PYY	-0.480	[MN/m]
				7.500	-1.750	0.000		-0.480	[MN/m]
	QGRP	4	2.000	(--)		activated		0.00	percent
Line				7.000	-1.750	0.000	MXX	-0.480	[MNm/m]
				7.500	-1.750	0.000		-0.480	[MNm/m]
	QGRP	3	2.000			activated		100.00	percent
Line				7.000	-1.750	0.000	MXX	-0.480	[MNm/m]
				7.500	-1.750	0.000		-0.480	[MNm/m]
	QGRP	4	2.000	(--)		activated		0.00	percent

Load Case 108 CRASH_-Y_Pos.9 _DECK
Factor forces and moments 1.000

Meshfree Loading

Kind	Referenceto		Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
Line				8.000	-1.750	0.000	PYY	-0.480	[MN/m]
				8.500	-1.750	0.000		-0.480	[MN/m]
	QGRP	3	2.000			activated		100.00	percent
Line				8.000	-1.750	0.000	PYY	-0.480	[MN/m]
				8.500	-1.750	0.000		-0.480	[MN/m]
	QGRP	4	2.000	(--)		activated		0.00	percent
Line				8.000	-1.750	0.000	MXX	-0.480	[MNm/m]
				8.500	-1.750	0.000		-0.480	[MNm/m]
	QGRP	3	2.000			activated		100.00	percent
Line				8.000	-1.750	0.000	MXX	-0.480	[MNm/m]
				8.500	-1.750	0.000		-0.480	[MNm/m]
	QGRP	4	2.000	(--)		activated		0.00	percent

Load Case 109 CRASH_-Y_Pos.10 _DECK
Factor forces and moments 1.000

Meshfree Loading

Kind	Referenceto		Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
Line				9.000	-1.750	0.000	PYY	-0.480	[MN/m]
				9.500	-1.750	0.000		-0.480	[MN/m]
	QGRP	3	2.000			activated		100.00	percent
Line				9.000	-1.750	0.000	PYY	-0.480	[MN/m]
				9.500	-1.750	0.000		-0.480	[MN/m]
	QGRP	4	2.000	(--)		activated		0.00	percent
Line				9.000	-1.750	0.000	MXX	-0.480	[MNm/m]
				9.500	-1.750	0.000		-0.480	[MNm/m]
	QGRP	3	2.000			activated		100.00	percent
Line				9.000	-1.750	0.000	MXX	-0.480	[MNm/m]
				9.500	-1.750	0.000		-0.480	[MNm/m]
	QGRP	4	2.000	(--)		activated		0.00	percent

Load Case 110 CRASH_-Y_Pos.11 _DECK
Factor forces and moments 1.000

Meshfree Loading

Kind	Referenceto		Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
Line				10.000	-1.750	0.000	PYY	-0.480	[MN/m]
				10.500	-1.750	0.000		-0.480	[MN/m]
	QGRP	3	2.000			activated		100.00	percent
Line				10.000	-1.750	0.000	PYY	-0.480	[MN/m]
				10.500	-1.750	0.000		-0.480	[MN/m]
	QGRP	4	2.000	(--)		activated		0.00	percent
Line				10.000	-1.750	0.000	MXX	-0.480	[MNm/m]
				10.500	-1.750	0.000		-0.480	[MNm/m]
	QGRP	3	2.000			activated		100.00	percent
Line				10.000	-1.750	0.000	MXX	-0.480	[MNm/m]
				10.500	-1.750	0.000		-0.480	[MNm/m]
	QGRP	4	2.000	(--)		activated		0.00	percent

Load Case 111 CRASH_-Y_Pos.12 _DECK
Factor forces and moments 1.000

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FORTISEIS ΦΑΣΗ-2

Meshfree Loading

Kind	Referenceto		Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
Line				11.000	-1.750	0.000	PYY	-0.480	[MN/m]
				11.500	-1.750	0.000		-0.480	[MN/m]
	QGRP	3	2.000	(--)	activated			40.00	percent
Line				11.000	-1.750	0.000	PYY	-0.480	[MN/m]
				11.500	-1.750	0.000		-0.480	[MN/m]
	QGRP	4	2.000	(--)	activated			60.00	percent
Line				11.000	-1.750	0.000	MXX	-0.480	[MNm/m]
				11.500	-1.750	0.000		-0.480	[MNm/m]
	QGRP	3	2.000	(--)	activated			40.00	percent
Line				11.000	-1.750	0.000	MXX	-0.480	[MNm/m]
				11.500	-1.750	0.000		-0.480	[MNm/m]
	QGRP	4	2.000	(--)	activated			60.00	percent

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΕΠΙΛΥΣΗ ΦΑΣΗ-2 (t-beam)

Elementgroups

No	fac-S	fac-L	fac-D	fac-P	fac-B	PLC
1	1.000	1.000	0.000	1.000	1.000	0
2	1.000	1.000	0.000	1.000	1.000	0
3	1.000	1.000	0.000	1.000	1.000	0
4	1.000	1.000	0.000	1.000	1.000	0
8	1.000	1.000	0.000	1.000	1.000	0
9	1.000	1.000	0.000	1.000	1.000	0
10	1.000	1.000	0.000	1.000	1.000	0
11	1.000	1.000	0.000	1.000	1.000	0
12	1.000	1.000	0.000	1.000	1.000	0
13	1.000	1.000	0.000	1.000	1.000	0

Elementgroups construction stage number

no	CS
1	40
2	998
3	998
4	998
8	998
9	998
10	998
11	998
12	998
13	998

Elementgroups activated hinges

no	hinge
1	fixed
2	activ
3	activ
4	activ
8	activ
9	activ
10	activ
11	activ
12	activ
13	activ

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
31	ΡΥΣΕΙΣ-ΟΔΟΣΤΡΩΣΙΑ	0.000	0.000	0.272
32	PEZODROMIO	0.000	0.000	0.192
33	ΠΡΟΣΘΕΤΗ ΟΔΟΣΤΡΩΣΙΑ 0.50	0.000	0.000	0.018
34	ΩΘΗΣΕΙΣ ΓΑΙΩΝ A1-K0(Φ-2)	0.268	0.000	0.000
35	ΩΘΗΣΕΙΣ ΓΑΙΩΝ A2-K0(Φ-2)	-0.268	0.000	0.000
36	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+0.5	0.000	0.000	0.000
37	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+0.5	0.134	0.000	0.000
38	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+1.0	-0.134	0.000	0.000
39	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+1.0	0.000	0.000	0.000
41	L.L.UDL_2.50KN/m2	0.000	0.000	0.118
42	L.L.UDL_6.50KN/m2	0.000	0.000	0.230
50	TS_RIGHT_Posit.1	0.000	0.000	0.540
51	TS_RIGHT_Posit.2	0.000	0.000	0.540
52	TS_RIGHT_Posit.3	0.000	0.000	0.540
53	TS_RIGHT_Posit.4	0.000	0.000	0.540
54	TS_RIGHT_Posit.5	0.000	0.000	0.540
55	TS_RIGHT_Posit.6	0.000	0.000	0.540
56	TS_RIGHT_Posit.7	0.000	0.000	0.540
57	TS_RIGHT_Posit.8	0.000	0.000	0.540
58	TS_RIGHT_Posit.9	0.000	0.000	0.540
59	TS_RIGHT_Posit.10	0.000	0.000	0.540
60	TS_RIGHT_Posit.11	0.000	0.000	0.472
61	TS_RIGHT_Posit.12	0.000	0.000	0.270
70	ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A1 2.50	0.025	0.000	0.000
71	ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A2 2.50	-0.025	0.000	0.000
72	ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A1 6.50	0.066	0.000	0.000
73	ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A2 6.50	-0.066	0.000	0.000
74	ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A1 TS1-	0.310	0.000	0.000
75	ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A2 TS1-	-0.310	0.000	0.000
80	ΠΙΘΑΝΕΣ ΚΑΘΙΖΗΣΕΙΣ A1	0.000	0.000	0.000
81	ΠΙΘΑΝΕΣ ΚΑΘΙΖΗΣΕΙΣ A2	0.000	0.000	0.000
90	(+ΔΤΝ)+0.75*(+ΔΤΜ)	0.000	0.000	0.000
91	(+ΔΤΝ)+0.75*(-ΔΤΜ)	0.000	0.000	0.000
92	(-ΔΤΝ)+0.75*(+ΔΤΜ)	0.000	0.000	0.000
93	(-ΔΤΝ)+0.75*(-ΔΤΜ)	0.000	0.000	0.000
94	0.35*(+ΔΤΝ)+(+ΔΤΜ)	0.000	0.000	0.000
95	0.35*(+ΔΤΝ)+(-ΔΤΜ)	0.000	0.000	0.000
96	0.35*(-ΔΤΝ)+(+ΔΤΜ)	0.000	0.000	0.000
97	0.35*(-ΔΤΝ)+(-ΔΤΜ)	0.000	0.000	0.000

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΕΠΙΛΥΣΗ ΦΑΣΗ-2 (t-beam)

Sum of Reactions and Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
31	ΡΥΣΕΙΣ-ΟΔΟΣΤΡΩΣΙΑ	0.000	0.000	-0.272
		0.000	0.000	0.272
32	PEZODROMIO	0.000	0.000	-0.192
		0.000	0.000	0.192
33	ΠΡΟΣΘΕΤΗ ΟΔΟΣΤΡΩΣΙΑ 0.50	0.000	0.000	-0.018
		0.000	0.000	0.018
34	ΩΘΗΣΕΙΣ ΓΑΙΩΝ A1-K0(Φ-2)	-0.268	0.000	0.000
		0.268	0.000	0.000
35	ΩΘΗΣΕΙΣ ΓΑΙΩΝ A2-K0(Φ-2)	0.268	0.000	0.000
		-0.268	0.000	0.000
36	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+0.5	0.000	0.000	0.000
		0.000	0.000	0.000
37	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+0.5	-0.134	0.000	0.000
		0.134	0.000	0.000
38	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+1.0	0.134	0.000	0.000
		-0.134	0.000	0.000
39	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+1.0	0.000	0.000	0.000
		0.000	0.000	0.000
41	L.L.UDL_2.50KN/m2	0.000	0.000	-0.118
		0.000	0.000	0.118
42	L.L.UDL_6.50KN/m2	0.000	0.000	-0.230
		0.000	0.000	0.230
50	TS_RIGHT_Posit.1	0.000	0.000	-0.540
		0.000	0.000	0.540
51	TS_RIGHT_Posit.2	0.000	0.000	-0.540
		0.000	0.000	0.540
52	TS_RIGHT_Posit.3	0.000	0.000	-0.540
		0.000	0.000	0.540
53	TS_RIGHT_Posit.4	0.000	0.000	-0.540
		0.000	0.000	0.540
54	TS_RIGHT_Posit.5	0.000	0.000	-0.540
		0.000	0.000	0.540
55	TS_RIGHT_Posit.6	0.000	0.000	-0.540
		0.000	0.000	0.540
56	TS_RIGHT_Posit.7	0.000	0.000	-0.540
		0.000	0.000	0.540
57	TS_RIGHT_Posit.8	0.000	0.000	-0.540
		0.000	0.000	0.540
58	TS_RIGHT_Posit.9	0.000	0.000	-0.540
		0.000	0.000	0.540
59	TS_RIGHT_Posit.10	0.000	0.000	-0.540
		0.000	0.000	0.540
60	TS_RIGHT_Posit.11	0.000	0.000	-0.472
		0.000	0.000	0.472
61	TS_RIGHT_Posit.12	0.000	0.000	-0.270
		0.000	0.000	0.270
70	ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A1 2.50	-0.025	0.000	0.000
		0.025	0.000	0.000
71	ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A2 2.50	0.025	0.000	0.000
		-0.025	0.000	0.000
72	ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A1 6.50	-0.066	0.000	0.000
		0.066	0.000	0.000
73	ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A2 6.50	0.066	0.000	0.000
		-0.066	0.000	0.000
74	ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A1 TS1-	-0.310	0.000	0.000
		0.310	0.000	0.000
75	ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A2 TS1-	0.310	0.000	0.000
		-0.310	0.000	0.000
80	ΠΙΘΑΝΕΣ ΚΑΘΙΖΗΣΕΙΣ A1	0.000	0.000	0.000
		0.000	0.000	0.000
81	ΠΙΘΑΝΕΣ ΚΑΘΙΖΗΣΕΙΣ A2	0.000	0.000	0.000
		0.000	0.000	0.000
90	(+ΔΤΝ)+0.75*(+ΔΤΜ)	0.000	0.000	0.000
		0.000	0.000	0.000
91	(+ΔΤΝ)+0.75*(-ΔΤΜ)	0.000	0.000	0.000
		0.000	0.000	0.000
92	(-ΔΤΝ)+0.75*(+ΔΤΜ)	0.000	0.000	0.000
		0.000	0.000	0.000
93	(-ΔΤΝ)+0.75*(-ΔΤΜ)	0.000	0.000	0.000
		0.000	0.000	0.000
94	0.35*(+ΔΤΝ)+(+ΔΤΜ)	0.000	0.000	0.000
		0.000	0.000	0.000
95	0.35*(+ΔΤΝ)+(-ΔΤΜ)	0.000	0.000	0.000
		0.000	0.000	0.000
96	0.35*(-ΔΤΝ)+(+ΔΤΜ)	0.000	0.000	0.000
		0.000	0.000	0.000
97	0.35*(-ΔΤΝ)+(-ΔΤΜ)	0.000	0.000	0.000
		0.000	0.000	0.000

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΕΠΙΛΥΣΗ ΦΑΣΗ-2 (normal)

Elementgroups

No	fac-S	fac-L	fac-D	fac-P	fac-B	PLC
1	1.000	1.000	0.000	1.000	1.000	0
2	1.000	1.000	0.000	1.000	1.000	0
3	1.000	1.000	0.000	1.000	1.000	0
4	1.000	1.000	0.000	1.000	1.000	0
8	1.000	1.000	0.000	1.000	1.000	0
9	1.000	1.000	0.000	1.000	1.000	0
10	1.000	1.000	0.000	1.000	1.000	0
11	1.000	1.000	0.000	1.000	1.000	0
12	1.000	1.000	0.000	1.000	1.000	0
13	1.000	1.000	0.000	1.000	1.000	0

Elementgroups construction stage number

no	CS
1	40
2	998
3	998
4	998
8	998
9	998
10	998
11	998
12	998
13	998

Elementgroups activated hinges

no	hinge
1	fixed
2	activ
3	activ
4	activ
8	activ
9	activ
10	activ
11	activ
12	activ
13	activ

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
100	CRASH_-Y_Pos.1 _DECK	0.000	-0.240	0.000
101	CRASH_-Y_Pos.2 _DECK	0.000	-0.240	0.000
102	CRASH_-Y_Pos.3 _DECK	0.000	-0.240	0.000
103	CRASH_-Y_Pos.4 _DECK	0.000	-0.240	0.000
104	CRASH_-Y_Pos.5 _DECK	0.000	-0.240	0.000
105	CRASH_-Y_Pos.6 _DECK	0.000	-0.240	0.000
106	CRASH_-Y_Pos.7 _DECK	0.000	-0.240	0.000
107	CRASH_-Y_Pos.8 _DECK	0.000	-0.240	0.000
108	CRASH_-Y_Pos.9 _DECK	0.000	-0.240	0.000
109	CRASH_-Y_Pos.10 _DECK	0.000	-0.240	0.000
110	CRASH_-Y_Pos.11 _DECK	0.000	-0.240	0.000
111	CRASH_-Y_Pos.12 _DECK	0.000	-0.240	0.000

Sum of Reactions and Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
100	CRASH_-Y_Pos.1 _DECK	0.000	0.240	0.000
		0.000	-0.240	0.000
101	CRASH_-Y_Pos.2 _DECK	0.000	0.240	0.000
		0.000	-0.240	0.000
102	CRASH_-Y_Pos.3 _DECK	0.000	0.240	0.000
		0.000	-0.240	0.000
103	CRASH_-Y_Pos.4 _DECK	0.000	0.240	0.000
		0.000	-0.240	0.000
104	CRASH_-Y_Pos.5 _DECK	0.000	0.240	0.000
		0.000	-0.240	0.000
105	CRASH_-Y_Pos.6 _DECK	0.000	0.240	0.000
		0.000	-0.240	0.000
106	CRASH_-Y_Pos.7 _DECK	0.000	0.240	0.000
		0.000	-0.240	0.000
107	CRASH_-Y_Pos.8 _DECK	0.000	0.240	0.000
		0.000	-0.240	0.000
108	CRASH_-Y_Pos.9 _DECK	0.000	0.240	0.000
		0.000	-0.240	0.000
109	CRASH_-Y_Pos.10 _DECK	0.000	0.240	0.000
		0.000	-0.240	0.000
110	CRASH_-Y_Pos.11 _DECK	0.000	0.240	0.000
		0.000	-0.240	0.000
111	CRASH_-Y_Pos.12 _DECK	0.000	0.240	0.000
		0.000	-0.240	0.000

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΦΑΣΕΙΣ ΚΑΤΑΣΚΕΥΗΣ-ΥΠΟΛΟΓΙΣΜΟΣ ΕΡΠΥΣΜΟΥ&ΣΥΣΤ.ΞΗΡΑΝΣΗΣ

Global Settings
Dead load of elements is automatically activated.
Creep and shrinkage analysis will be done by AQB.
Creep and shrinkage values are calculated in advance in CSM.

Construction Stages

CS	Type	Time d	RH %	Temp °C	launch m	laun_2 m	Title
10	G_1						G1 activating new group/CS
15	C	7	70	20			K creep step
20	G_2						G2 additional dead load
25	C	30	70	20			K creep step
40	G_3						G2 additional dead load
55	C	60	70	20			K creep step
60	C	219	70	20			K creep step
61	C	681	70	20			K creep step
62	C	2115	70	20			K creep step
63	C	6571	70	20			K creep step
64	C	20414	70	20			K creep step

Group Properties

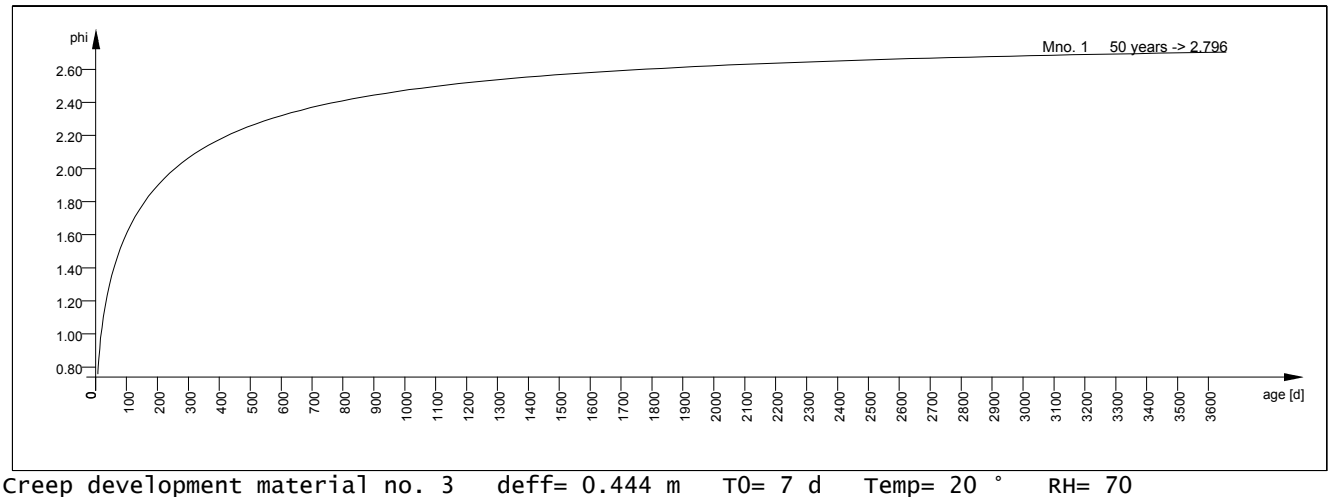
Grp	active from CS	active until CS	deadload from CS	Hfix from	Bedd from	Deck from	T0 d	FAC1	PHIF	QUEA	QEMX
1	10	999	10	40		20	7	1.000			
2	10	999	10			20	7				
3	40	999	40			20	7			0.000	
4	40	999	40			20	7			0.000	
8	10	999	10			20	7				
9	10	999	10			20	7				
10	10	999	10			20	7				
11	10	999	10			20	7				
12	10	999	10			20	7				
13	10	999	10			20	7				

Hfix: beam hinges of this group are fixed starting from this Constrction Stage
Deck: dead weight activation of cross sectional construction stages

Additional Loads

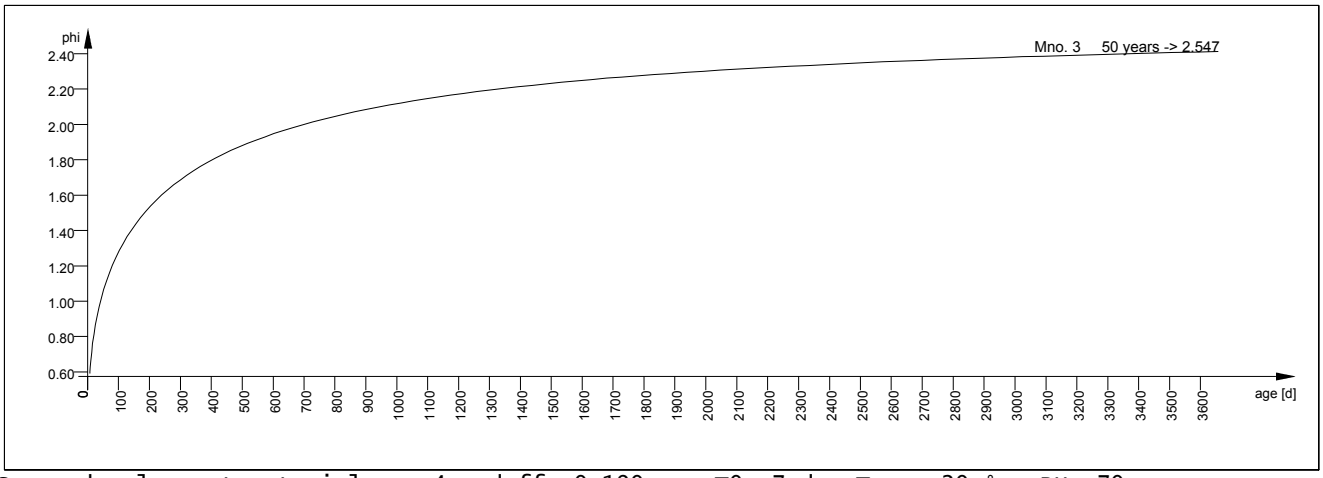
LC	Type	active from CS	active until CS	faktor
2	G_1	10	999	1.000000
3	G_2	20	999	1.000000
31	G_2	40	999	1.000000
32	G_2	40	999	1.000000
33	G_2	40	999	1.000000

Creep development material no. 1 deff= 0.192 m T0= 7 d Temp= 20 ° RH= 70

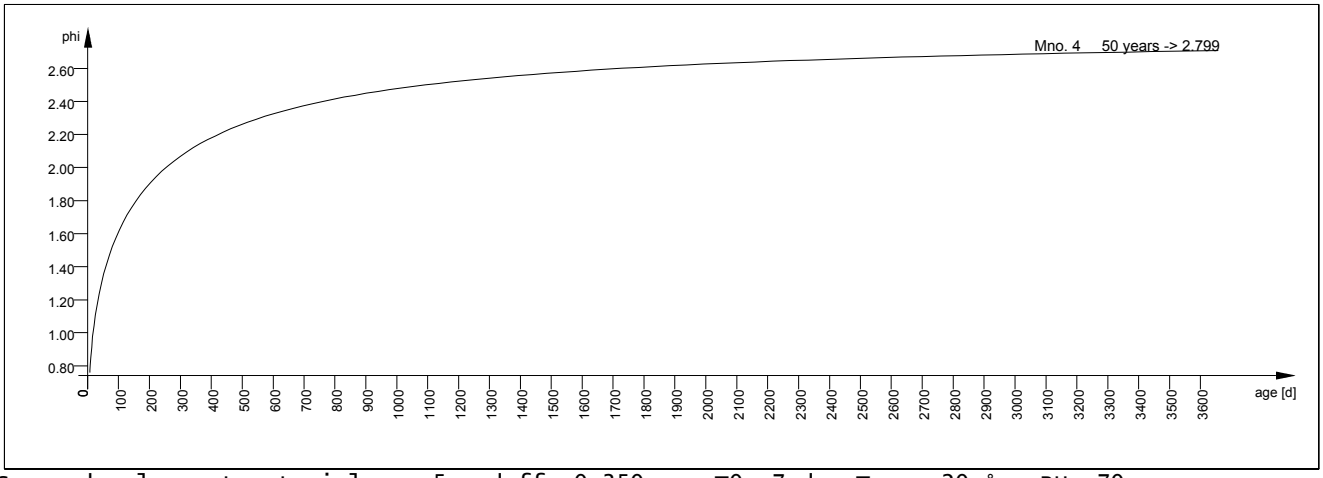


Creep development material no. 3 deff= 0.444 m T0= 7 d Temp= 20 ° RH= 70

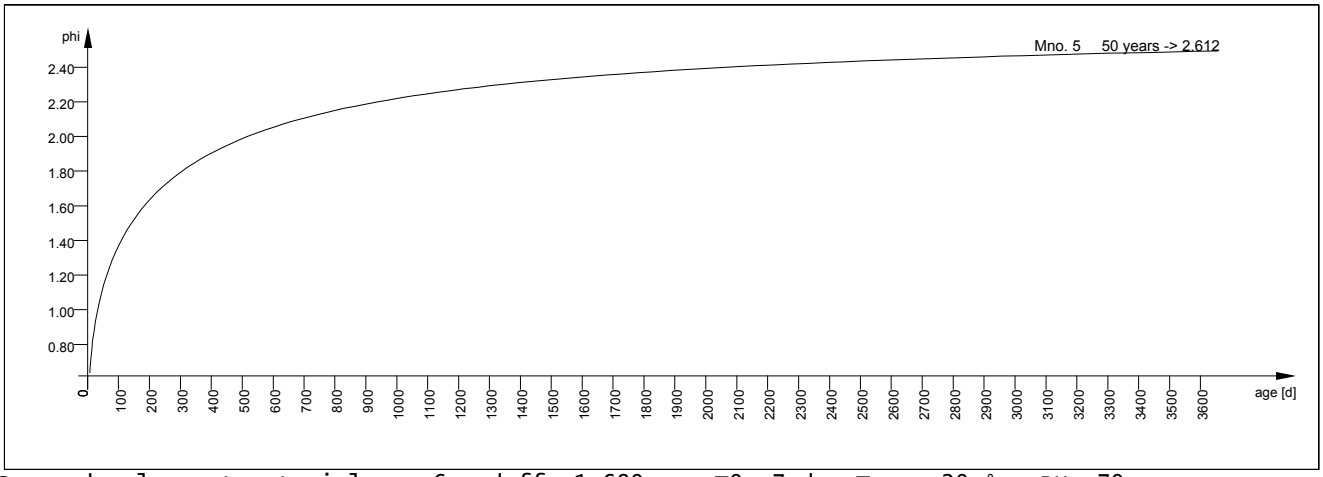
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΦΑΣΕΙΣ ΚΑΤΑΣΚΕΥΗΣ-ΥΠΟΛΟΓΙΣΜΟΣ ΕΡΠΥΣΜΟΥ&ΣΥΣΤ.ΞΗΡΑΝΣΗΣ



Creep development material no. 4 deff= 0.190 m T0= 7 d Temp= 20 ° RH= 70

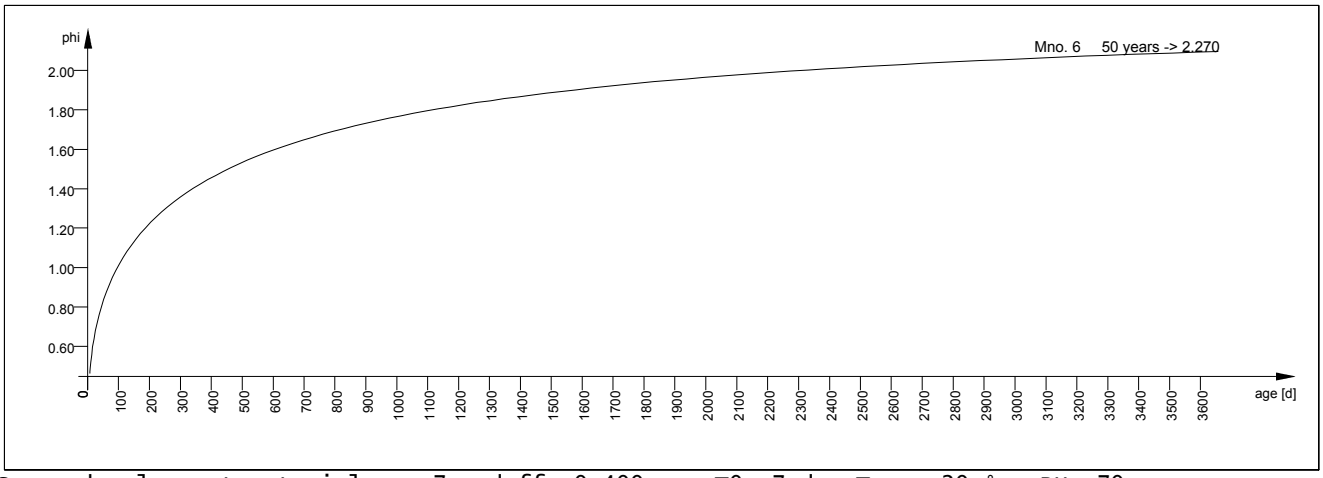


Creep development material no. 5 deff= 0.350 m T0= 7 d Temp= 20 ° RH= 70

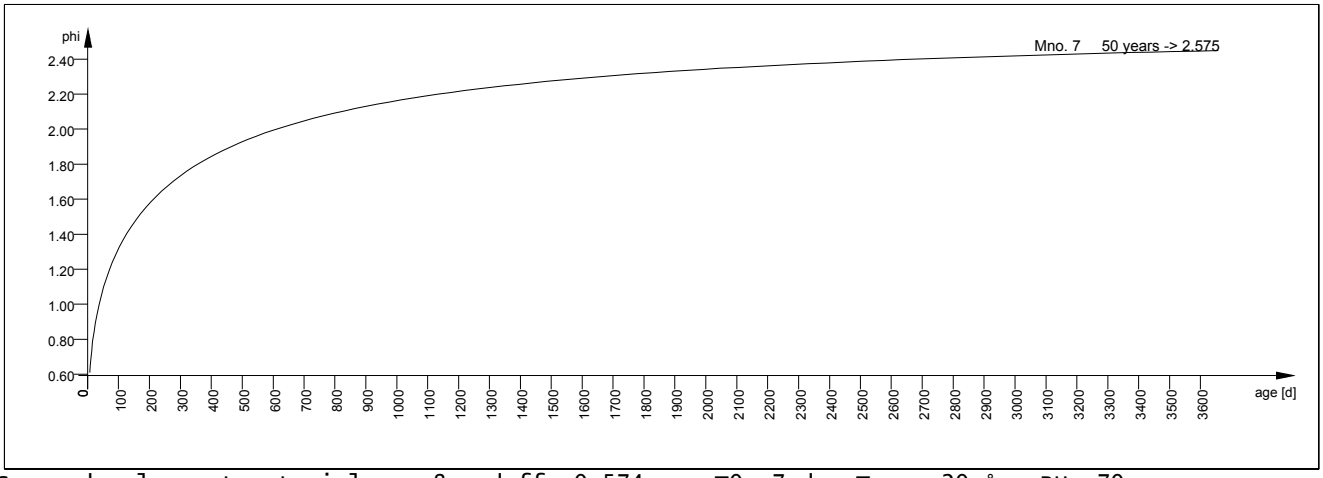


Creep development material no. 6 deff= 1.680 m T0= 7 d Temp= 20 ° RH= 70

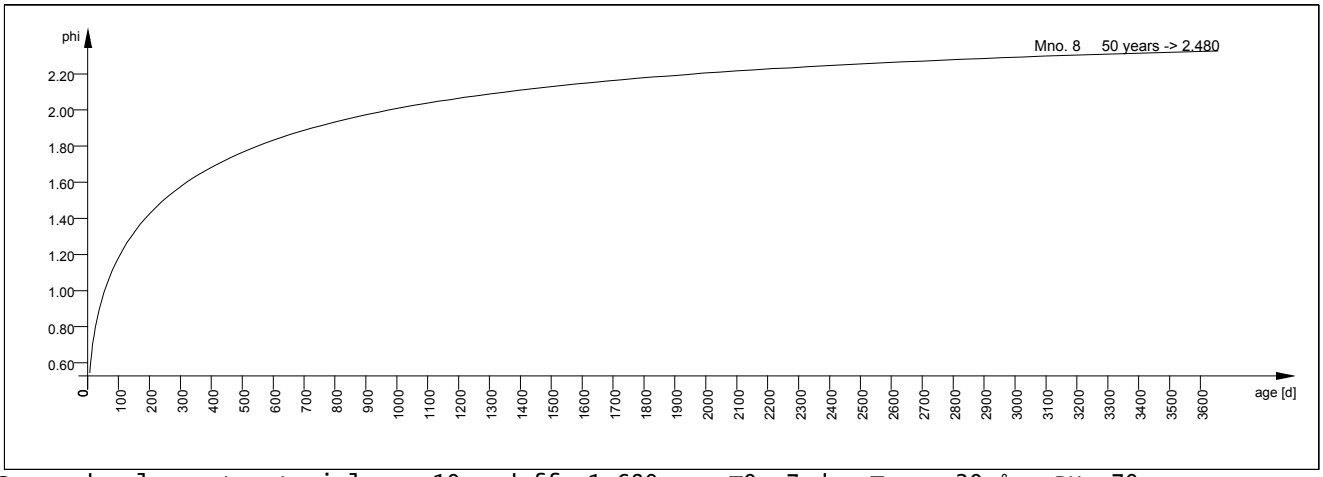
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΦΑΣΕΙΣ ΚΑΤΑΣΚΕΥΗΣ-ΥΠΟΛΟΓΙΣΜΟΣ ΕΡΠΥΣΜΟΥ&ΣΥΣΤ.ΞΗΡΑΝΣΗΣ



Creep development material no. 7 deff= 0.400 m T0= 7 d Temp= 20 ° RH= 70

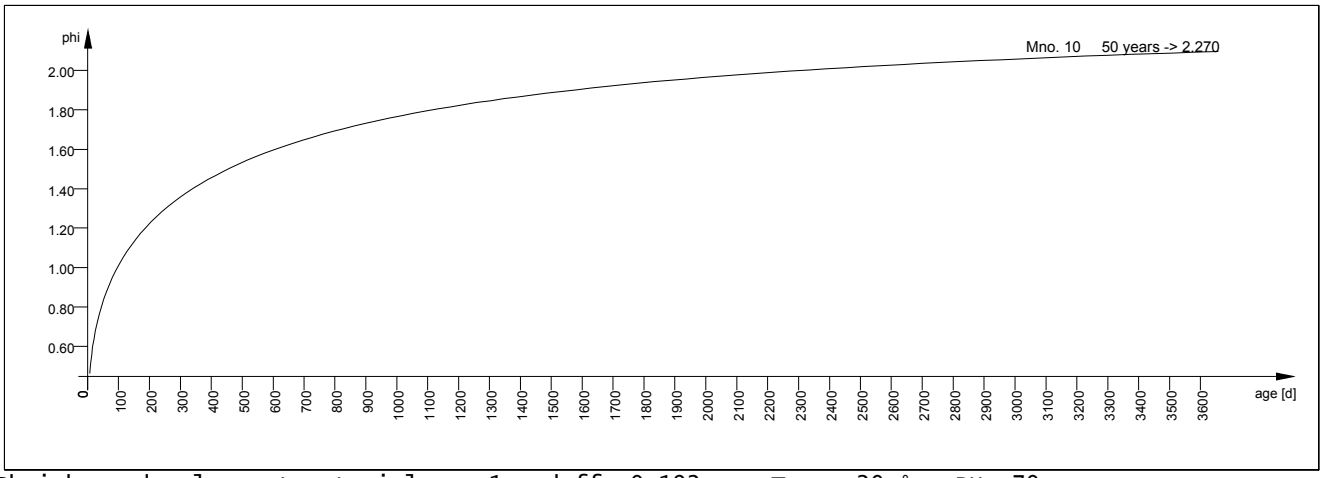


Creep development material no. 8 deff= 0.574 m T0= 7 d Temp= 20 ° RH= 70

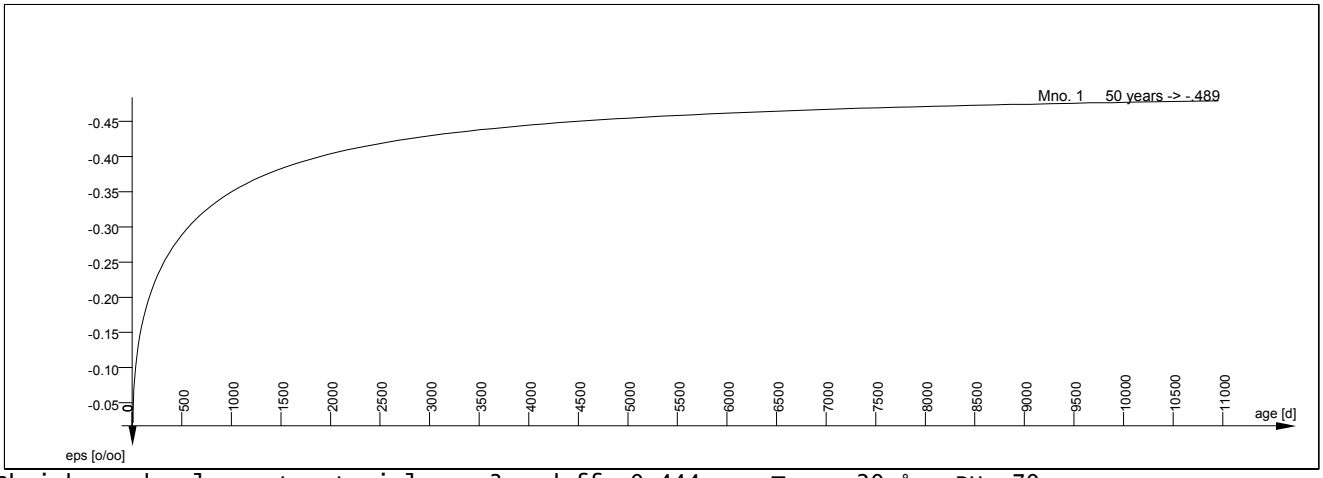


Creep development material no. 10 deff= 1.680 m T0= 7 d Temp= 20 ° RH= 70

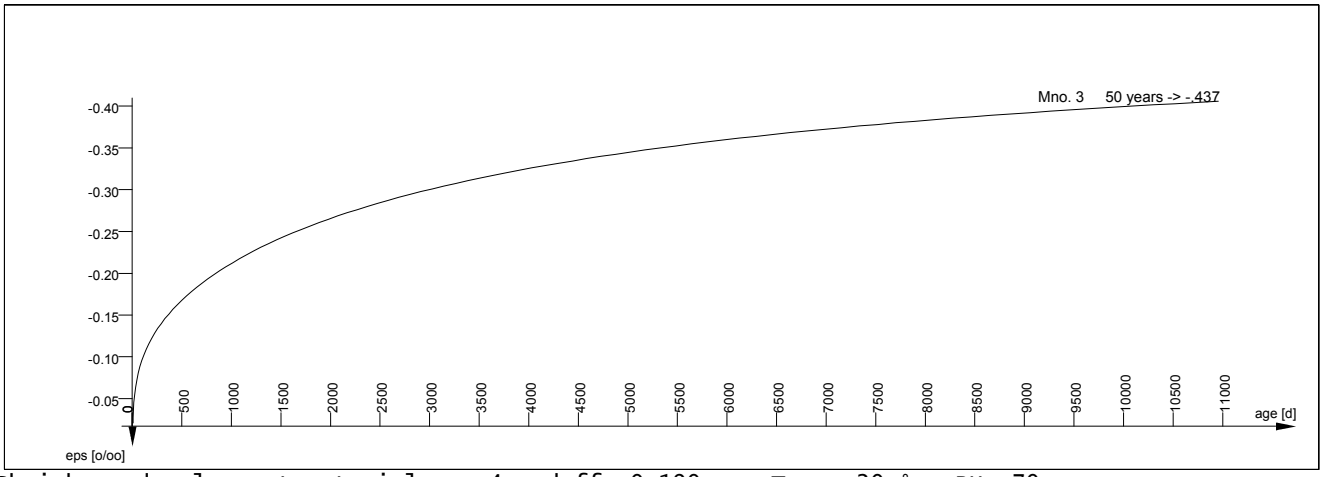
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΦΑΣΕΙΣ ΚΑΤΑΣΚΕΥΗΣ-ΥΠΟΛΟΓΙΣΜΟΣ ΕΡΠΥΣΜΟΥ&ΣΥΣΤ.ΞΗΡΑΝΣΗΣ



Shrinkage development material no. 1 deff= 0.192 m Temp= 20 ° RH= 70

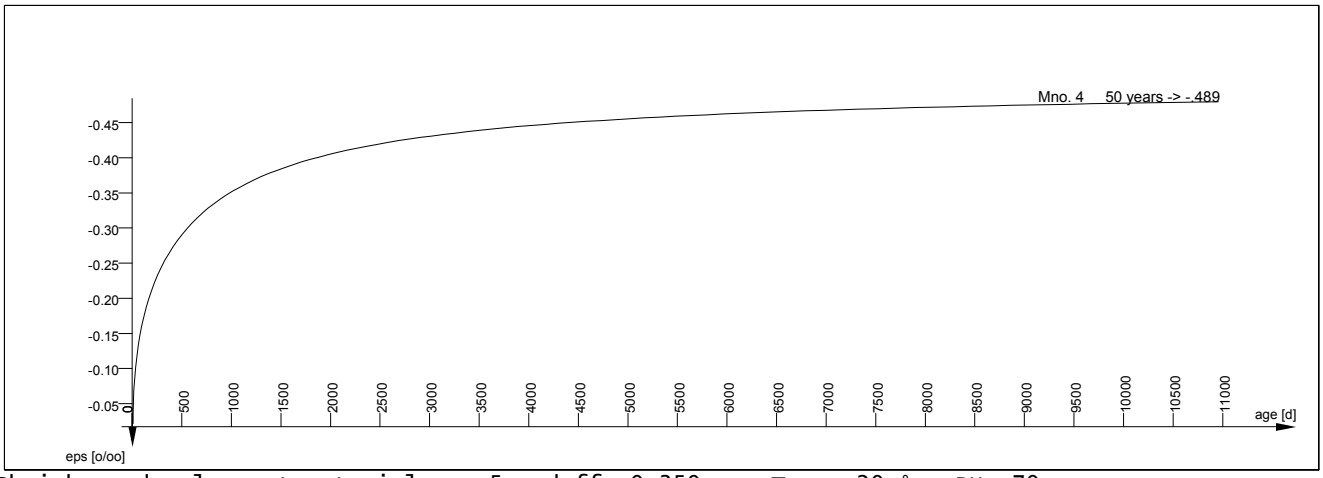


Shrinkage development material no. 3 deff= 0.444 m Temp= 20 ° RH= 70

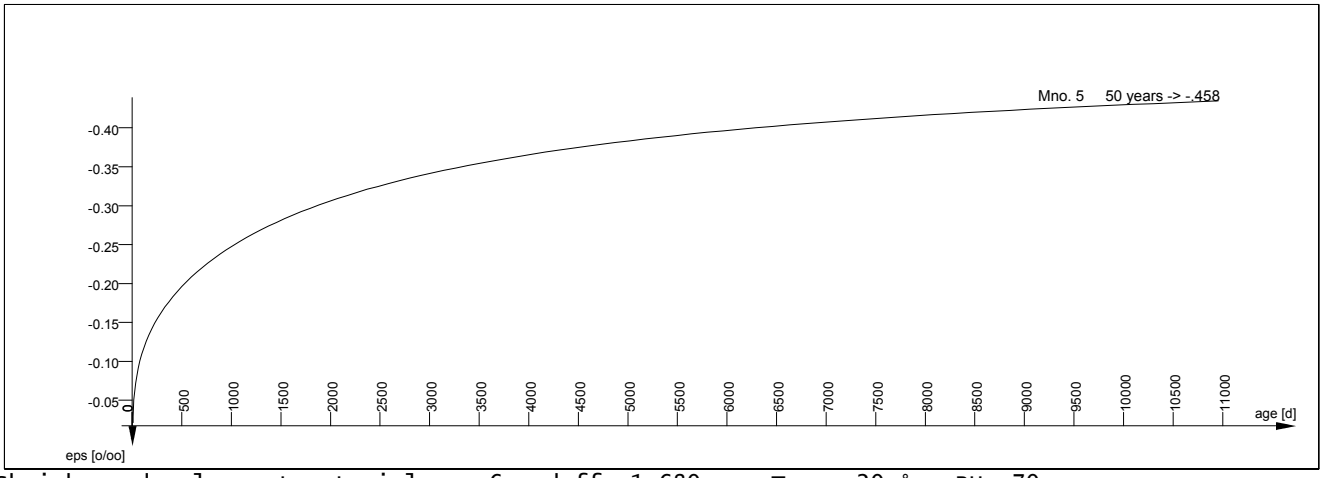


Shrinkage development material no. 4 deff= 0.190 m Temp= 20 ° RH= 70

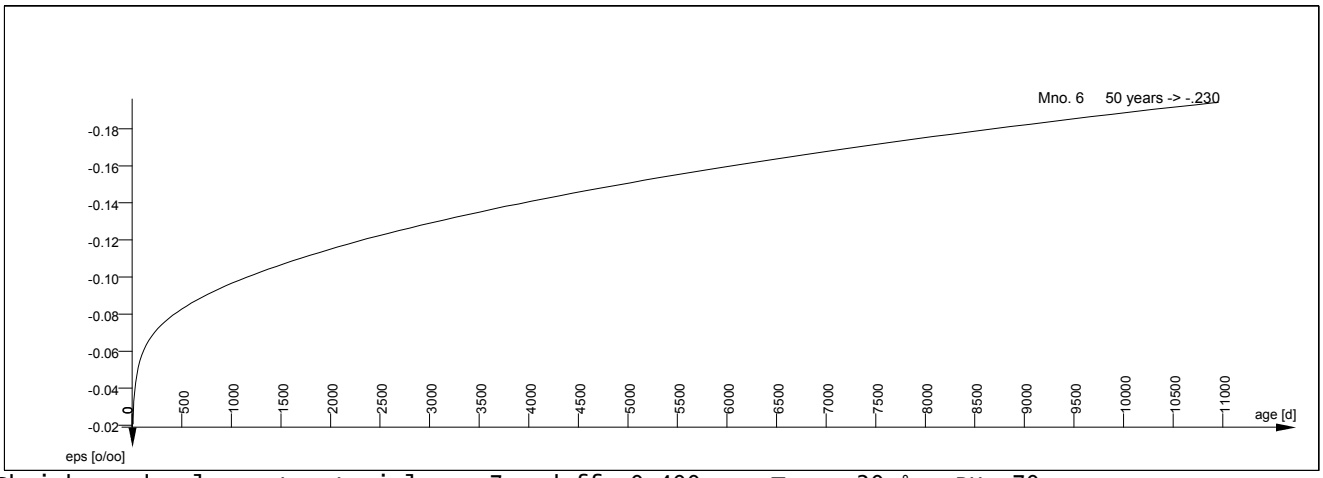
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΦΑΣΕΙΣ ΚΑΤΑΣΚΕΥΗΣ-ΥΠΟΛΟΓΙΣΜΟΣ ΕΡΠΥΣΜΟΥ&ΣΥΣΤ.ΞΗΡΑΝΣΗΣ



Shrinkage development material no. 5 deff= 0.350 m Temp= 20 ° RH= 70

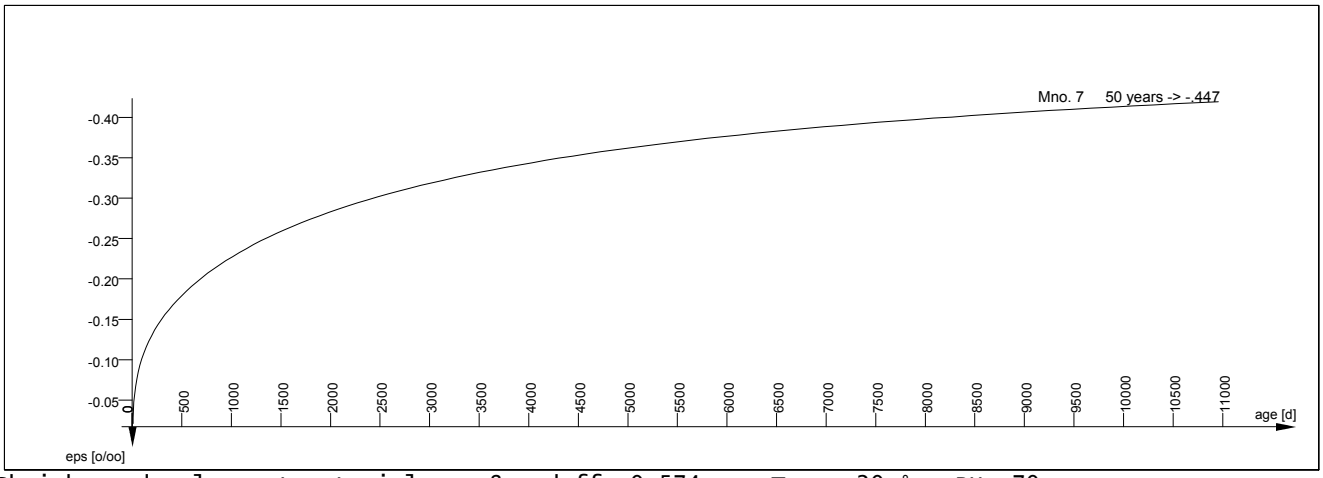


Shrinkage development material no. 6 deff= 1.680 m Temp= 20 ° RH= 70

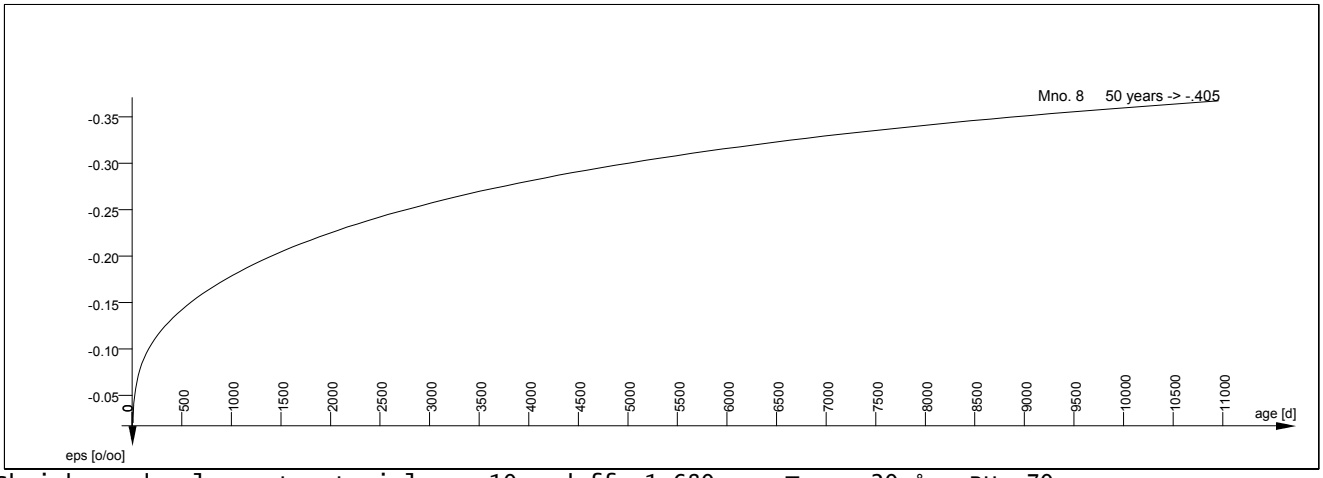


Shrinkage development material no. 7 deff= 0.400 m Temp= 20 ° RH= 70

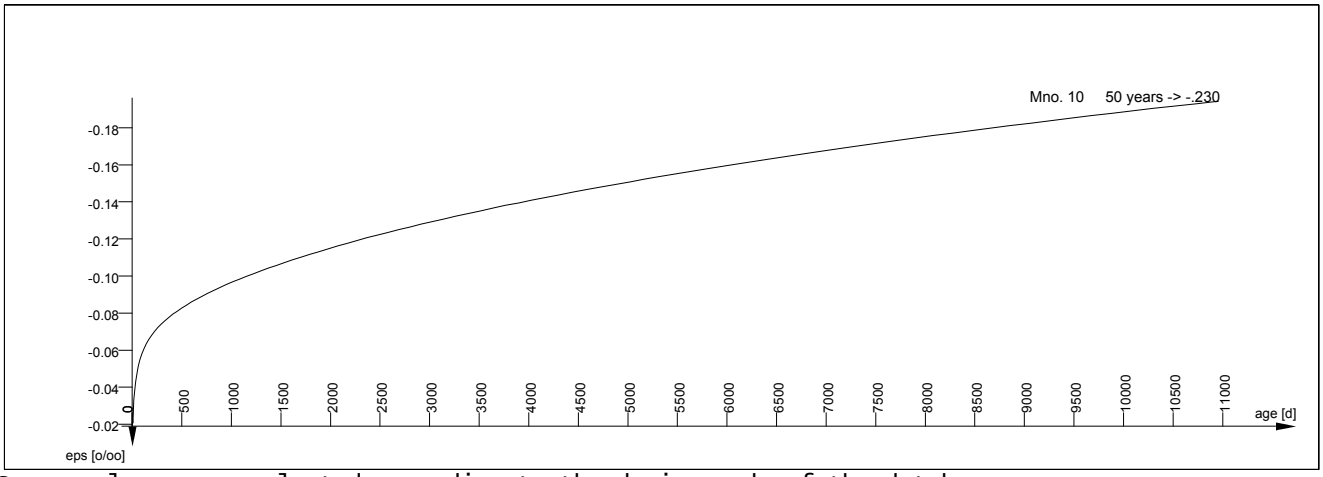
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΦΑΣΕΙΣ ΚΑΤΑΣΚΕΥΗΣ-ΥΠΟΛΟΓΙΣΜΟΣ ΕΡΠΥΣΜΟΥ&ΣΥΣΤ.ΞΗΡΑΝΣΗΣ



Shrinkage development material no. 8 deff= 0.574 m Temp= 20 ° RH= 70



Shrinkage development material no. 10 deff= 1.680 m Temp= 20 ° RH= 70



Creep values are evaluated according to the design code of the database.

Creep Values

Grp	Mno	Type	deff [m]	T0 d	CS 15	CS 25	CS 55	CS 60	CS 61	CS 62	CS 63	CS 64	total
			Time --->		7	30	60	219	681	2115	6571	20414	30097
			RH % --->		70	70	70	70	70	70	70	70	
			Temp --->		20	20	20	20	20	20	20	20	
1	1	beam	0.192	7	0.76	0.47	0.37	0.49	0.39	0.21	0.09	0.03	2.81 / 1.14 **
1	4	beam	0.190	7	-	-	1.41	0.63	0.43	0.22	0.09	0.03	2.81 / 1.14 **
2	3	beam	0.444	7	0.59	0.37	0.30	0.44	0.41	0.27	0.13	0.05	2.56 / 1.14 **
3	5	-	0.350	7	-	-	1.19	0.57	0.44	0.26	0.11	0.04	2.63 / 1.14 **

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΦΑΣΕΙΣ ΚΑΤΑΣΚΕΥΗΣ-ΥΠΟΛΟΓΙΣΜΟΣ ΕΡΠΥΣΜΟΥ&ΣΥΣΤ.ΞΗΡΑΝΣΗΣ

Creep Values

Grp	Mno	Type	deff [m]	T0 d	CS 15	CS 25	CS 55	CS 60	CS 61	CS 62	CS 63	CS 64	total	
			Time --->		7	30	60	219	681	2115	6571	20414	30097	
			RH % --->		70	70	70	70	70	70	70	70		
			Temp --->		20	20	20	20	20	20	20	20		
4	5	-	0.350	7	-	-	1.19	0.57	0.44	0.26	0.11	0.04	2.63 / 1.14	**
8	6	-	1.680	7	0.46	0.30	0.24	0.37	0.39	0.30	0.16	0.06	2.29 / 1.14	**
9	10	-	1.680	7	0.46	0.30	0.24	0.37	0.39	0.30	0.16	0.06	2.29 / 1.14	**
10	8	beam	0.574	7	0.55	0.35	0.28	0.42	0.41	0.29	0.14	0.06	2.50 / 1.14	**
12	7	-	0.400	7	0.61	0.39	0.31	0.45	0.41	0.26	0.12	0.04	2.59 / 1.14	**

** For the ** marked elements, the creep value will be additionally devided by
the printed factor in AQB due to the problem of the reference E-modulus [Ecm/Ec0].
Please refer to Heft 525 Deutscher Ausschuss fuer Stahlbeton S. 65ff.

Shrinkage Values *10^-6

Grp	Mno	Type	deff [m]	T0 d	CS 15	CS 25	CS 55	CS 60	CS 61	CS 62	CS 63	CS 64	total
			Time --->		7	30	60	219	681	2115	6571	20414	30097
			RH % --->		70	70	70	70	70	70	70	70	
			Temp --->		20	20	20	20	20	20	20	20	
1	1	beam	0.192	7	-21.8	-82.2	-55.5	-89.1	-101.	-82.2	-44.7	-18.0	-494.
1	4	beam	0.190	7	-	-	-131.	-109.	-108.	-84.2	-44.6	-17.8	-495.
2	3	beam	0.444	7	-21.1	-44.6	-29.8	-48.1	-68.0	-91.9	-93.6	-62.4	-459.
3	5	-	0.350	7	-	-	-90.3	-69.8	-84.2	-99.3	-84.2	-46.8	-475.
4	5	-	0.350	7	-	-	-90.3	-69.8	-84.2	-99.3	-84.2	-46.8	-475.
8	6	-	1.680	7	-20.6	-23.1	-14.0	-17.4	-21.2	-34.0	-56.1	-83.5	-270.
9	10	-	1.680	7	-20.6	-23.1	-14.0	-17.4	-21.2	-34.0	-56.1	-83.5	-270.
10	8	beam	0.574	7	-20.9	-38.0	-25.0	-39.1	-55.5	-81.2	-97.2	-79.1	-436.
12	7	-	0.400	7	-21.1	-47.8	-32.1	-52.4	-73.3	-94.9	-89.9	-55.3	-467.

Overview loadcases

Total CS displacements and forces starting at loadcase number. 4000
Difference displacements and forces starting at loadcase number. 5000
AQB inner stresses from creep and shrinkage from loadcasenumber. 6000
Stress results of the AQB-LCST-evaluation from loadcasenumber. 7000
[to check prestress normal force after creep and shrinkage :
-> WINGRAF beam normal force LC 7000ff]

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00

5) ΦΑΣΗ-1_ΕΛΕΓΧΟΣ ΦΟΡΕΑ ΣΕ ULS-ΣΤΑΤΙΚΑ

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ULS-BASIKOS SYNDYASMOS 1 (BEAMS)

Combination rule Number 1
FASH_1-ULS
Resulting loadcases type Design Combination
Loadcase selection

Number	factor	type	Title
1	1.35	permanent load grouped in actions	I.B. ΚΑΤΑΚ.ΣΤΟΙΧΕΙΩΝ
2	1.35	permanent load grouped in actions	I.B. ΔΟΚΩΝ
3	1.35	permanent load grouped in actions	I.B. ΧΥΤΗΣ ΠΛΑΚΑΣ
4	1.50	Exclusive LC	A 1 ΚΙΝΗΤΟ ΦΑΣΗΣ-1
11	1.50	Exclusive LC	A 2 ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+0.5
12	1.50	Exclusive LC	A 2 ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+0.5
13	1.50	Exclusive LC	A 2 ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+1.0
14	1.50	Exclusive LC	A 2 ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+1.0
7	1.35	Exclusive LC	A 3 ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝ A1-K0 (Φ
8	1.35	Exclusive LC	A 4 ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝ A2-K0 (Φ

Generated Loadcases

Number	Comb	Title
1001	1	MAX-MY BEAM
1002	1	MIN-MY BEAM
1003	1	MAX-VZ BEAM
1004	1	MIN-VZ BEAM
1005	1	MAX-MZ BEAM
1006	1	MIN-MZ BEAM
1007	1	MAX-VY BEAM
1008	1	MIN-VY BEAM
1009	1	MAX-N BEAM
1010	1	MIN-N BEAM
1011	1	MAX-MT BEAM
1012	1	MIN-MT BEAM

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Selected Beam Elements

FROM	TO	INC	X-VALUE	NC	MEMBER	CS0	CS1	CS2	CS3	CS4	CS5
10001											
10005											
10006											
10009											
10010											
10014											
10016											
10020											
10021											
10024											
10025											
10029											
12000	12150	1									

Default design code is DIN Fachbericht 102 Massivbrȯcken (2003) (Germany)
Klasse(Tab.4.118): D
Wind zone : Binnenland

Materials

No. 1 C 25/30 (DIN 1045-1)
No. 3 C 25/30 (DIN 1045-1)
No. 4 C 25/30 (DIN 1045-1)
No. 5 C 25/30 (DIN 1045-1)
No. 6 C 25/30 (DIN 1045-1)
No. 7 C 25/30 (DIN 1045-1)
No. 8 C 25/30 (DIN 1045-1)
No. 9 C 25/30 (DIN 1045-1)
No. 10 C 25/30 (DIN 1045-1)
No. 12 BSt 500 SA (DIN 1045-1)

Reinforcement will be accounted for sectional values as defined in AQUA
Reinforcements saved as design case LCR 501

Considered Load Cases

No.	refer	act on	Title/type of load case	gam-u	gam-f	psi-0	psi-1	psi-2	psi-1'	
1	part.	CS 0	I.B. ΚΑΤΑΚ.ΣΤΟΙΧΕΙΩΝ	1.35	1.00	1.00	1.00	1.00	1.00	G perm
			G (total dead load)							
2	part.	CS 0	I.B. ΔΟΚΩΝ	1.35	1.00	1.00	1.00	1.00	1.00	G perm
			G (total dead load)							
3	part.	CS 0	I.B. ΧΥΤΗΣ ΠΛΑΚΑΣ	1.35	1.00	1.00	1.00	1.00	1.00	G perm
			G (total dead load)							
4	part.	CS 0	ΚΙΝΗΤΟ ΦΑΣΗΣ-1	1.50	0.00	0.70	0.50	0.30	0.70	Q cond
			Q_A (Pay load residential cat. A)							
7	part.	CS 0	ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝ Α1-ΚΟ (Φ	1.50	0.00	0.70	0.50	0.30	0.70	Q cond
			Q_B (Pay load offices cat. B)							
8	part.	CS 0	ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝ Α2-ΚΟ (Φ	1.50	0.00	0.70	0.50	0.30	0.70	Q cond
			Q_B (Pay load offices cat. B)							
11	part.	CS 0	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*Α1+0.5	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
12	part.	CS 0	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*Α1+0.5	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
13	part.	CS 0	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*Α1+1.0	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
14	part.	CS 0	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*Α1+1.0	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							

Ultimate Load Design

==========

Design for ultimate loads DIN Fachbericht 102 Massivbrȯcken (2003)
Biaxial bending, uniaxial stress calculated in y-z axis

Safety factors	SC-1	SC-2	SC-S	SS-1	SS-2	PIIa
	1.50	1.50	1.50	1.15	1.15	7

Strain limits	C1	C2	S1	S2	Z1	Z2
max	-3.50	-2.00	3.00	25.00	-3.50	25.00

parameters for reinforcements

Minimum reinforcements		compression	min. reinforcem.	maximum-
Bending.	Compress.	e/d N/Npl	requ. section	reinforc.
0.00 [cm2]	0.30 [o/o]	3.50 0.0010	0.00 0.15	9.00

Tensile forces in the longitudinal reinforcements due to shear are NOT accounted for.
Material of sections uses Ultimate Limit strain-stress law with global safety factors
Material of reinforcements uses Ultimate Limit strain-stress law with global safety factors

MNo.	temp lev.	Material-safety [-]	max.compr stress [MPa]	at strain [o/oo]	max.tens stress [MPa]	at strain [o/oo]	tension-stiffening [MPa]
1	0	1.500	-14.17	-2.00	0.00	0.00	
3	0	1.500	-14.17	-2.00	0.00	0.00	
4	0	1.500	-14.17	-2.00	0.00	0.00	
5	0	1.500	-14.17	-2.00	0.00	0.00	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

MNo.	temp lev.	Material-safety [-]	max.compr stress [MPa]	at strain [o/oo]	max.tens stress [MPa]	at strain [o/oo]	tension-stiffening [MPa]
6	0	1.500	-14.17	-2.00	0.00	0.00	
7	0	1.500	-14.17	-2.00	0.00	0.00	
8	0	1.500	-14.17	-2.00	0.00	0.00	
9	0	1.500	-14.17	-2.00	0.00	0.00	
10	0	1.500	-14.17	-2.00	0.00	0.00	
12	0	1.150	-456.52	-25.00	456.52	25.00	

Combinations For Ultimate Design

1013 (gross) max_my-1013
MAX + MY :
1.35 * G + 1.50 * Q_A + 1.50 * L_A + 1.35 * Q_B

1014 (gross) min_my-1014
MIN + MY :
1.35 * G + 1.50 * Q_A + 1.50 * L_A + 1.35 * Q_B

Required Reinforcements

Beam	x[m]	NoS	LC	Ni [MN]	Myi/Mzi [MNm]	e1/yn [o/oo]	e2/zn / mm]	nue C/S	rel tra	As L [cm2]
10001	0.000	8	1013	0.006	0.000	25.00	25.00	1.50	1.00	0.04 0
					0.000			1.15		0.04 1
										0.04 3
										0.35 T
			1014	0.006	0.000	25.00	25.00	1.50	1.00	0.04 0
					0.000			1.15		0.04 1
										0.04 3
										0.41 T
	0.200	8	1013	-0.020	-0.006	0.00	0.00	1.50	not calculated	
										0.38 T
			1014	-0.021	-0.006	0.00	0.00	1.50	not calculated	
										0.37 T
10005	0.000	8	1013	0.029	-0.014	25.00	25.00	1.50	1.00	0.04 0
					0.000			1.15		0.60 3
										1.70 T
			1014	0.031	-0.014	25.00	25.00	1.50	1.00	0.05 0
					0.000			1.15		0.63 3
										1.64 T
	0.200	8	1013	0.031	0.004	25.00	25.00	1.50	1.00	0.23 0
					0.000			1.15		0.31 1
										0.14 3
										1.81 T
			1014	0.029	0.004	25.00	25.00	1.50	1.00	0.21 0
					0.000			1.15		0.30 1
										0.13 3
										1.53 T
10006	0.000	8	1013	0.002	0.004	-0.09	25.00	1.50	1.00	0.10 1
					0.000		4	1.15		0.14 T
			1014	0.002	0.004	-0.09	25.00	1.50	1.00	0.10 1
					0.000		4	1.15		0.11 T
	0.400	8	1013	0.003	0.003	-0.05	25.00	1.50	1.00	0.09 1
					0.000		3	1.15		0.12 T
			1014	0.005	0.002	25.00	25.00	1.50	1.00	0.10 1
					0.000			1.15		0.13 T
10009	0.000	8	1013	0.003	0.003	-0.05	25.00	1.50	1.00	0.09 1
					0.000		3	1.15		0.14 T
			1014	0.005	0.002	25.00	25.00	1.50	1.00	0.10 1
					0.000			1.15		0.11 T
	0.400	8	1013	0.002	0.004	-0.09	25.00	1.50	1.00	0.10 1
					0.000		4	1.15		0.12 T
			1014	0.002	0.004	-0.09	25.00	1.50	1.00	0.10 1
					0.000		4	1.15		0.13 T
10010	0.000	8	1013	0.031	0.004	25.00	25.00	1.50	1.00	0.23 0
					0.000			1.15		0.31 1
										0.14 3
										1.64 T
			1014	0.029	0.004	25.00	25.00	1.50	1.00	0.21 0
					0.000			1.15		0.30 1
										0.13 3
										1.70 T
	0.200	8	1013	0.029	-0.014	25.00	25.00	1.50	1.00	0.04 0
					0.000			1.15		0.60 3
										1.53 T
			1014	0.031	-0.014	25.00	25.00	1.50	1.00	0.05 0
					0.000			1.15		0.63 3
										1.81 T
10014	0.000	8	1013	-0.020	-0.006	0.00	0.00	1.50	not calculated	
										0.38 T
			1014	-0.021	-0.006	0.00	0.00	1.50	not calculated	
										0.37 T

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Required Reinforcements

Beam	x[m]	NoS	LC	Ni [MN]	Myi/Mzi [MNm]	e1/yn [o/oo]	e2/zn / mm]	nue C/S	rel tra	As [cm2]	L
10014	0.200	8	1013	0.006	0.000	25.00	25.00	1.50	1.00	0.04	0
					0.000			1.15		0.04	1
										0.04	3
										0.35	T
			1014	0.006	0.000	25.00	25.00	1.50	1.00	0.04	0
					0.000			1.15		0.04	1
										0.04	3
										0.41	T
10016	0.000	8	1013	0.006	0.000	25.00	25.00	1.50	1.00	0.04	0
					0.000			1.15		0.04	1
										0.04	3
										0.38	T
			1014	0.006	0.000	25.00	25.00	1.50	1.00	0.04	0
					0.000			1.15		0.04	1
										0.04	3
										0.37	T
	0.200	8	1013	-0.020	-0.006	0.00	0.00	1.50	not calculated		
										0.38	T
			1014	-0.021	-0.006	0.00	0.00	1.50	not calculated		
										0.37	T
10020	0.000	8	1013	0.029	-0.014	25.00	25.00	1.50	1.00	0.04	0
					0.000			1.15		0.60	3
										1.53	T
			1014	0.031	-0.014	25.00	25.00	1.50	1.00	0.05	0
					0.000			1.15		0.63	3
										1.81	T
	0.200	8	1013	0.031	0.004	25.00	25.00	1.50	1.00	0.23	0
					0.000			1.15		0.31	1
										0.14	3
										1.64	T
			1014	0.029	0.004	25.00	25.00	1.50	1.00	0.21	0
					0.000			1.15		0.30	1
										0.13	3
										1.70	T
10021	0.000	8	1013	0.006	0.004	-0.05	25.00	1.50	1.00	0.15	1
					0.000		3	1.15		0.12	T
			1014	0.006	0.004	-0.05	25.00	1.50	1.00	0.14	1
					0.000		3	1.15		0.13	T
	0.400	8	1013	0.003	0.003	-0.05	25.00	1.50	1.00	0.09	1
					0.000		3	1.15		0.14	T
			1014	0.005	0.002	25.00	25.00	1.50	1.00	0.10	1
					0.000			1.15		0.11	T
10024	0.000	8	1013	0.003	0.003	-0.05	25.00	1.50	1.00	0.09	1
					0.000		3	1.15		0.14	T
			1014	0.005	0.002	25.00	25.00	1.50	1.00	0.10	1
					0.000			1.15		0.11	T
	0.400	8	1013	0.002	0.004	-0.09	25.00	1.50	1.00	0.10	1
					0.000		4	1.15		0.14	T
			1014	0.002	0.004	-0.09	25.00	1.50	1.00	0.10	1
					0.000		4	1.15		0.11	T
10025	0.000	8	1013	0.031	0.004	25.00	25.00	1.50	1.00	0.23	0
					0.000			1.15		0.31	1
										0.14	3
										1.81	T
			1014	0.029	0.004	25.00	25.00	1.50	1.00	0.21	0
					0.000			1.15		0.30	1
										0.13	3
										1.53	T
	0.200	8	1013	0.029	-0.014	25.00	25.00	1.50	1.00	0.04	0
					0.000			1.15		0.60	3
										1.70	T
			1014	0.031	-0.014	25.00	25.00	1.50	1.00	0.05	0
					0.000			1.15		0.63	3
										1.64	T
10029	0.000	8	1013	-0.018	-0.006	0.00	0.00	1.50	not calculated		
										0.38	T
			1014	-0.019	-0.006	0.00	0.00	1.50	not calculated		
										0.37	T
	0.200	8	1013	0.006	0.000	25.00	25.00	1.50	1.00	0.04	0
					0.000			1.15		0.04	1
										0.04	3
										0.34	T
			1014	0.006	0.000	25.00	25.00	1.50	1.00	0.04	0
					0.000			1.15		0.04	1
										0.04	3
										0.41	T
12001	0.000	9	1013	-1.585	0.002	-3.50	4.93	1.50	5.78	15.08	1
					0.557	110	-9999	1.15			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Required Reinforcements

Beam	x[m]	NoS	LC	Ni [MN]	Myi/Mzi [MNm]	e1/yn [o/oo]	e2/zn / mm]	nue C/S	rel tra	As [cm2]	L
12001	0.000	9	1014	-0.883	0.001	-3.50	7.68	1.50	3.35	15.08	1
					0.433	181	-9999	1.15			
	1.000	9	1013	-1.751	0.001	-3.50	4.48	1.50	6.02	15.08	1
					0.580	93	-9999	1.15			
			1014	-0.394	0.000	-3.50	11.17	1.50	1.41	15.08	1
					0.322	233	-9999	1.15			
12002	0.000	9	1013	-1.751	0.001	-3.50	4.48	1.50	6.02	15.08	1
					0.580	93	-9999	1.15			
			1014	-0.394	0.000	-3.50	11.17	1.50	1.41	15.08	1
					0.322	233	-9999	1.15			
	1.000	9	1013	-1.917	0.001	-3.50	4.07	1.50	6.22	15.08	1
					0.601	77	-9999	1.15			
			1014	-0.297	0.000	-3.50	10.00	1.50	1.00	24.24	1
					0.399	219	-9999	1.15			
12003	0.000	9	1013	-1.917	0.001	-3.50	4.07	1.50	6.22	15.08	1
					0.601	77	-9999	1.15			
			1014	-0.297	0.000	-3.50	10.00	1.50	1.00	24.24	1
					0.399	219	-9999	1.15			
	1.000	9	1013	-0.325	0.000	-3.50	7.24	1.50	1.00	49.35	1
					0.659	172	-9999	1.15			
			1014	-2.193	0.001	-3.50	3.49	1.50	6.97	15.08	1
					0.630	50	-9999	1.15			
12004	0.000	9	1013	-1.628	0.000	-3.50	4.81	1.50	6.18	15.08	1
					0.563	106	9999	1.15			
			1014	-0.855	0.000	-3.50	7.83	1.50	3.12	15.08	1
					0.427	184	9999	1.15			
	1.000	9	1013	-1.812	0.000	-3.50	4.32	1.50	6.46	15.08	1
					0.588	87	9999	1.15			
			1014	-0.397	0.000	-3.50	11.14	1.50	1.36	15.08	1
					0.323	233	9999	1.15			
12005	0.000	9	1013	-1.812	0.000	-3.50	4.32	1.50	6.46	15.08	1
					0.588	87	9999	1.15			
			1014	-0.397	0.000	-3.50	11.14	1.50	1.36	15.08	1
					0.323	233	9999	1.15			
	1.000	9	1013	-1.996	0.000	-3.50	3.90	1.50	6.71	15.08	1
					0.610	69	9999	1.15			
			1014	-0.308	0.000	-3.50	9.85	1.50	1.00	24.68	1
					0.406	217	9999	1.15			
12006	0.000	9	1013	-1.996	0.000	-3.50	3.90	1.50	6.71	15.08	1
					0.610	69	9999	1.15			
			1014	-0.308	0.000	-3.50	9.85	1.50	1.00	24.68	1
					0.406	217	9999	1.15			
	1.000	9	1013	-0.315	0.000	-3.50	7.30	1.50	1.00	48.95	1
					0.653	174	9999	1.15			
			1014	-2.066	0.000	-3.50	3.75	1.50	6.35	15.08	1
					0.617	62	9999	1.15			
12007	0.000	9	1013	-1.664	-0.002	-3.50	4.71	1.50	6.31	15.08	1
					0.568	102	9999	1.15			
			1014	-0.870	-0.001	-3.50	7.75	1.50	3.17	15.08	1
					0.431	183	9999	1.15			
	1.000	9	1013	-1.842	-0.001	-3.50	4.25	1.50	6.57	15.08	1
					0.592	84	9999	1.15			
			1014	-0.400	0.000	-3.50	11.11	1.50	1.37	15.08	1
					0.323	233	9999	1.15			
12008	0.000	9	1013	-1.842	-0.001	-3.50	4.25	1.50	6.57	15.08	1
					0.592	84	9999	1.15			
			1014	-0.400	0.000	-3.50	11.11	1.50	1.37	15.08	1
					0.323	233	9999	1.15			
	1.000	9	1013	-2.019	-0.001	-3.50	3.85	1.50	6.79	15.08	1
					0.612	67	9999	1.15			
			1014	-0.308	0.000	-3.50	9.87	1.50	1.00	24.60	1
					0.406	217	9999	1.15			
12009	0.000	9	1013	-2.019	-0.001	-3.50	3.85	1.50	6.79	15.08	1
					0.612	67	9999	1.15			
			1014	-0.308	0.000	-3.50	9.87	1.50	1.00	24.60	1
					0.406	217	9999	1.15			
	1.000	9	1013	-0.314	0.000	-3.50	7.30	1.50	1.00	48.90	1
					0.652	174	9999	1.15			
			1014	-2.081	-0.001	-3.50	3.71	1.50	6.40	15.08	1
					0.619	61	9999	1.15			
12010	0.000	9	1013	-1.585	0.002	-3.50	4.93	1.50	5.78	15.08	1
					-0.557	-109	-9999	1.15			
			1014	-0.883	0.001	-3.50	7.68	1.50	3.35	15.08	1
					-0.433	-180	-9999	1.15			
	1.000	9	1013	-1.751	0.001	-3.50	4.48	1.50	6.02	15.08	1
					-0.580	-92	-9999	1.15			
			1014	-0.394	0.000	-3.50	11.17	1.50	1.41	15.08	1
					-0.322	-232	-9999	1.15			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Required Reinforcements

Beam	x[m]	NoS	LC	Ni [MN]	Myi/Mzi [MNm]	e1/yn [o/oo]	e2/zn / mm]	nue C/S	rel tra	As [cm2]	L
12011	0.000	9	1013	-1.751	0.001	-3.50	4.48	1.50	6.02	15.08	1
					-0.580	-92	-9999	1.15			
			1014	-0.394	0.000	-3.50	11.17	1.50	1.41	15.08	1
					-0.322	-232	-9999	1.15			
	1.000	9	1013	-1.917	0.001	-3.50	4.07	1.50	6.22	15.08	1
					-0.601	-76	-9999	1.15			
			1014	-0.297	0.000	-3.50	10.00	1.50	1.00	24.24	1
					-0.399	-218	-9999	1.15			
12012	0.000	9	1013	-1.917	0.001	-3.50	4.07	1.50	6.22	15.08	1
					-0.601	-76	-9999	1.15			
			1014	-0.297	0.000	-3.50	10.00	1.50	1.00	24.24	1
					-0.399	-218	-9999	1.15			
	1.000	9	1013	-2.081	0.001	-3.50	3.71	1.50	6.40	15.08	1
					-0.619	-60	-9999	1.15			
			1014	-0.314	0.000	-3.50	7.30	1.50	1.00	48.90	1
					-0.652	-173	-9999	1.15			
12013	0.000	9	1013	-0.868	0.000	-3.50	7.76	1.50	3.29	15.08	1
					-0.430	-182	-9999	1.15			
			1014	-1.551	0.000	-3.50	5.03	1.50	5.66	15.08	1
					-0.552	-112	9999	1.15			
	1.000	9	1013	-0.392	0.000	-3.50	11.19	1.50	1.40	15.08	1
					-0.321	-233	-9999	1.15			
			1014	-1.722	0.000	-3.50	4.55	1.50	5.91	15.08	1
					-0.576	-95	9999	1.15			
12014	0.000	9	1013	-0.392	0.000	-3.50	11.19	1.50	1.40	15.08	1
					-0.321	-233	-9999	1.15			
			1014	-1.722	0.000	-3.50	4.55	1.50	5.91	15.08	1
					-0.576	-95	9999	1.15			
	1.000	9	1013	-0.298	0.000	-3.50	9.99	1.50	1.00	24.31	1
					-0.400	-218	-9999	1.15			
			1014	-1.894	0.000	-3.50	4.13	1.50	6.15	15.08	1
					-0.598	-78	9999	1.15			
12015	0.000	9	1013	-0.298	0.000	-3.50	9.99	1.50	1.00	24.31	1
					-0.400	-218	-9999	1.15			
			1014	-1.894	0.000	-3.50	4.13	1.50	6.15	15.08	1
					-0.598	-78	9999	1.15			
	1.000	9	1013	-0.315	0.000	-3.50	7.30	1.50	1.00	48.95	1
					-0.653	-173	9999	1.15			
			1014	-2.066	0.000	-3.50	3.75	1.50	6.35	15.08	1
					-0.617	-61	9999	1.15			
12016	0.000	9	1013	-1.664	-0.002	-3.50	4.71	1.50	6.31	15.08	1
					-0.568	-101	9999	1.15			
			1014	-0.870	-0.001	-3.50	7.75	1.50	3.17	15.08	1
					-0.431	-182	9999	1.15			
	1.000	9	1013	-1.842	-0.001	-3.50	4.25	1.50	6.57	15.08	1
					-0.592	-83	9999	1.15			
			1014	-0.400	0.000	-3.50	11.11	1.50	1.37	15.08	1
					-0.323	-232	9999	1.15			
12017	0.000	9	1013	-1.842	-0.001	-3.50	4.25	1.50	6.57	15.08	1
					-0.592	-83	9999	1.15			
			1014	-0.400	0.000	-3.50	11.11	1.50	1.37	15.08	1
					-0.323	-232	9999	1.15			
	1.000	9	1013	-2.019	-0.001	-3.50	3.85	1.50	6.79	15.08	1
					-0.612	-66	9999	1.15			
			1014	-0.308	0.000	-3.50	9.87	1.50	1.00	24.60	1
					-0.406	-216	9999	1.15			
12018	0.000	9	1013	-2.019	-0.001	-3.50	3.85	1.50	6.79	15.08	1
					-0.612	-66	9999	1.15			
			1014	-0.308	0.000	-3.50	9.87	1.50	1.00	24.60	1
					-0.406	-216	9999	1.15			
	1.000	9	1013	-2.193	-0.001	-3.50	3.49	1.50	6.97	15.08	1
					-0.630	-49	9999	1.15			
			1014	-0.325	0.000	-3.50	7.24	1.50	1.00	49.35	1
					-0.659	-171	9999	1.15			
12019	0.000	9	1013	-0.325	0.000	-3.50	7.24	1.50	1.00	49.35	1
					0.659	172	-9999	1.15			
			1014	-2.193	0.001	-3.50	3.49	1.50	6.97	15.08	1
					0.630	50	-9999	1.15			
	1.000	9	1013	-0.342	0.000	-3.50	6.18	1.50	1.00	70.29	1
					0.857	147	-9999	1.15			
			1014	-2.542	0.000	-3.50	2.88	1.50	7.67	15.08	1
					0.656	17	-9999	1.15			
12020	0.000	9	1013	-0.342	0.000	-3.50	6.19	1.50	1.00	70.23	1
					0.856	148	-9999	1.15			
			1014	-2.543	0.000	-3.50	2.88	1.50	7.67	15.08	1
					0.657	16	-9999	1.15			
	1.000	9	1013	-0.359	0.000	-3.50	6.01	1.50	1.00	74.07	1
					0.894	143	-9999	1.15			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Required Reinforcements

Beam	x[m]	NoS	LC	Ni [MN]	Myi/Mzi [MNm]	e1/yn [o/oo]	e2/zn / mm]	nue C/S	rel tra	As [cm2]	L
12020	1.000	9	1014	-3.173	0.000	-3.50	2.04	1.50	9.11	15.08	1
					0.676	-42	9999	1.15			
12021	0.000	9	1013	-0.359	0.000	-3.50	6.01	1.50	1.00	74.03	1
					0.893	143	-9999	1.15			
			1014	-3.174	0.000	-3.50	2.04	1.50	9.11	15.08	1
					0.676	-42	9999	1.15			
	1.000	9	1013	-0.365	0.000	-3.50	6.29	1.50	1.00	65.97	1
					0.821	150	9999	1.15			
			1014	-3.892	0.000	-3.50	1.31	1.50	9.99	15.08	1
					0.662	-109	-9999	1.15			
12022	0.000	9	1013	-0.365	0.000	-3.50	6.29	1.50	1.00	65.94	1
					0.821	150	9999	1.15			
			1014	-3.893	0.000	-3.50	1.31	1.50	9.99	15.08	1
					0.662	-109	-9999	1.15			
	1.000	9	1013	-0.382	0.000	-3.50	6.89	1.50	1.00	52.06	1
					0.695	165	9999	1.15			
			1014	-4.859	0.000	-3.50	0.55	1.50	9.99	15.08	1
					0.591	-204	-9999	1.15			
12023	0.000	9	1013	-0.382	0.000	-3.50	6.89	1.50	1.00	52.04	1
					0.695	165	9999	1.15			
			1014	-4.859	0.000	-3.50	0.55	1.50	9.99	15.08	1
					0.591	-204	-9999	1.15			
	1.000	9	1013	-0.399	0.000	-3.50	7.94	1.50	1.00	36.21	1
					0.545	186	9999	1.15			
			1014	-5.765	0.000	-3.50	-0.02	1.50	9.99	15.08	1
					0.465	-303	-9999	1.15			
12024	0.000	9	1013	-0.399	0.000	-3.50	7.94	1.50	1.00	36.20	1
					0.545	186	9999	1.15			
			1014	-5.765	0.000	-3.50	-0.02	1.50	9.99	15.08	1
					0.465	-303	-9999	1.15			
	1.000	9	1013	-0.416	0.000	-3.50	9.67	1.50	1.00	21.35	1
					0.396	214	9999	1.15			
			1014	-6.509	0.000	-3.50	-0.44	1.50	9.99	15.08	1
					0.314	-400	-9999	1.15			
12025	0.000	9	1013	-0.416	0.000	-3.50	9.67	1.50	1.00	21.35	1
					0.396	214	9999	1.15			
			1014	-6.509	0.000	-3.50	-0.44	1.50	9.99	15.08	1
					0.314	-400	-9999	1.15			
	1.000	9	1013	-0.617	0.000	-3.50	9.32	1.50	1.42	15.08	1
					0.375	209	9999	1.15			
			1014	-7.064	0.000	-3.09	-0.55	1.50	9.99	15.08	1
					0.174	-574	-9999	1.15			
12026	0.000	9	1013	-0.617	0.000	-3.50	9.32	1.50	1.42	15.08	1
					0.375	209	9999	1.15			
			1014	-7.063	0.000	-3.09	-0.55	1.50	9.99	15.08	1
					0.174	-573	-9999	1.15			
	1.000	9	1013	-1.633	0.000	-3.50	4.79	1.50	3.63	15.08	1
					0.564	105	9999	1.15			
			1014	-7.497	0.000	-2.61	-1.18	1.50	9.99	15.08	1
					0.065	-1059	-9999	1.15			
12027	0.000	9	1013	-1.632	0.000	-3.50	4.80	1.50	3.63	15.08	1
					0.564	105	9999	1.15			
			1014	-7.496	0.000	-2.61	-1.18	1.50	9.99	15.08	1
					0.066	-1058	-9999	1.15			
	1.000	9	1013	-4.094	0.000	-3.50	1.14	1.50	8.77	15.08	1
					0.652	-128	-9999	1.15			
			1014	-7.707	0.000	-2.14	-1.81	1.50	9.99	15.08	1
					-0.008	4769	-9999	1.15			
12028	0.000	9	1013	-4.092	0.000	-3.50	1.14	1.50	8.76	15.08	1
					0.652	-128	-9999	1.15			
			1014	-7.708	0.000	-2.14	-1.81	1.50	9.99	15.08	1
					-0.008	4798	-9999	1.15			
	1.000	9	1013	-6.736	0.000	-3.35	-0.21	1.50	9.99	15.08	1
					0.257	-452	-9999	1.15			
			1014	-7.570	0.000	-2.50	-1.33	1.50	9.99	15.08	1
					-0.047	1317	-9999	1.15			
12029	0.000	9	1013	-6.734	0.000	-3.35	-0.20	1.50	9.99	15.08	1
					0.257	-452	-9999	1.15			
			1014	-7.570	0.000	-2.50	-1.33	1.50	9.99	15.08	1
					-0.047	1318	-9999	1.15			
	1.000	9	1013	-6.881	0.000	-3.24	-0.35	1.50	9.99	15.08	1
					-0.220	498	-9999	1.15			
			1014	-7.512	0.000	-2.59	-1.21	1.50	9.99	15.08	1
					-0.062	1103	-9999	1.15			
12030	0.000	9	1013	-6.883	0.000	-3.24	-0.35	1.50	9.99	15.08	1
					-0.220	498	-9999	1.15			
			1014	-7.513	0.000	-2.59	-1.21	1.50	9.99	15.08	1
					-0.061	1103	-9999	1.15			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Required Reinforcements

Beam	x[m]	NoS	LC	Ni [MN]	Myi/Mzi [MNm]	e1/yn [o/oo]	e2/zn / mm]	nue C/S	rel tra	As [cm2]	L
12030	1.000	9	1013	-7.524	0.000	-2.57	-1.23	1.50	9.99	15.08	1
					-0.059	1138	-9999	1.15			
			1014	-6.128	0.000	-3.50	-0.22	1.50	9.99	15.08	1
					-0.396	348	-9999	1.15			
12031	0.000	9	1013	-7.524	0.000	-2.57	-1.23	1.50	9.99	15.08	1
					-0.059	1138	-9999	1.15			
			1014	-6.129	0.000	-3.50	-0.22	1.50	9.99	15.08	1
					-0.396	348	-9999	1.15			
	1.000	9	1013	-7.554	0.000	-2.53	-1.30	1.50	9.99	15.08	1
					-0.051	1248	-9999	1.15			
			1014	-5.925	0.000	-3.50	-0.11	1.50	9.99	15.08	1
					-0.436	323	-9999	1.15			
12032	0.000	9	1013	-7.554	0.000	-2.53	-1.30	1.50	9.99	15.08	1
					-0.051	1248	-9999	1.15			
			1014	-5.925	0.000	-3.50	-0.11	1.50	9.99	15.08	1
					-0.436	323	-9999	1.15			
	1.000	9	1013	-7.589	0.000	-2.46	-1.38	1.50	9.99	15.08	1
					-0.042	1419	9999	1.15			
			1014	-6.023	0.000	-3.50	-0.16	1.50	9.99	15.08	1
					-0.417	335	9999	1.15			
12033	0.000	9	1013	-7.589	0.000	-2.46	-1.38	1.50	9.99	15.08	1
					-0.042	1419	9999	1.15			
			1014	-6.023	0.000	-3.50	-0.16	1.50	9.99	15.08	1
					-0.417	335	9999	1.15			
	1.000	9	1013	-7.635	0.000	-2.37	-1.50	1.50	9.99	15.08	1
					-0.030	1779	9999	1.15			
			1014	-6.307	0.000	-3.50	-0.32	1.50	9.99	15.08	1
					-0.359	372	9999	1.15			
12034	0.000	9	1013	-7.635	0.000	-2.37	-1.50	1.50	9.99	15.08	1
					-0.030	1779	9999	1.15			
			1014	-6.307	0.000	-3.50	-0.32	1.50	9.99	15.08	1
					-0.359	372	9999	1.15			
	1.000	9	1013	-7.673	0.000	-2.28	-1.63	1.50	9.99	15.08	1
					-0.020	2416	9999	1.15			
			1014	-6.638	0.000	-3.41	-0.11	1.50	9.99	15.08	1
					-0.282	428	9999	1.15			
12035	0.000	9	1013	-7.673	0.000	-2.28	-1.63	1.50	9.99	15.08	1
					-0.020	2416	9999	1.15			
			1014	-6.638	0.000	-3.41	-0.11	1.50	9.99	15.08	1
					-0.282	428	9999	1.15			
	1.000	9	1013	-7.697	0.000	-2.19	-1.74	1.50	9.99	15.08	1
					-0.012	3522	9999	1.15			
			1014	-6.961	0.000	-3.17	-0.44	1.50	9.99	15.08	1
					-0.200	528	9999	1.15			
12036	0.000	9	1013	-7.697	0.000	-2.19	-1.74	1.50	9.99	15.08	1
					-0.012	3522	9999	1.15			
			1014	-6.961	0.000	-3.17	-0.44	1.50	9.99	15.08	1
					-0.200	528	9999	1.15			
	1.000	9	1013	-7.712	0.000	-2.12	-1.84	1.50	9.99	15.08	1
					-0.006	5787	9999	1.15			
			1014	-7.252	0.000	-2.91	-0.79	1.50	9.99	15.08	1
					-0.127	698	9999	1.15			
12037	0.000	9	1013	-7.712	0.000	-2.12	-1.84	1.50	9.99	15.08	1
					-0.006	5786	9999	1.15			
			1014	-7.252	0.000	-2.91	-0.79	1.50	9.99	15.08	1
					-0.127	698	9999	1.15			
	1.000	9	1013	-7.720	0.000	-2.06	-1.92	1.50	9.99	15.08	1
					-0.003	9999	9999	1.15			
			1014	-7.483	0.000	-2.63	-1.16	1.50	9.99	15.08	1
					-0.069	1026	9999	1.15			
12038	0.000	9	1013	-7.720	0.000	-2.06	-1.92	1.50	9.99	15.08	1
					-0.003	9999	9999	1.15			
			1014	-7.483	0.000	-2.63	-1.16	1.50	9.99	15.08	1
					-0.069	1025	9999	1.15			
	1.000	9	1013	-7.723	0.000	-2.02	-1.97	1.50	9.99	15.08	1
					-0.001	9999	9999	1.15			
			1014	-7.641	0.000	-2.36	-1.52	1.50	9.99	15.08	1
					-0.029	1853	9999	1.15			
12039	0.000	9	1013	-7.723	0.000	-2.02	-1.97	1.50	9.99	15.08	1
					-0.001	9999	9999	1.15			
			1014	-7.641	0.000	-2.36	-1.52	1.50	9.99	15.08	1
					-0.029	1852	9999	1.15			
	1.000	9	1013	-7.724	0.000	-2.00	-2.00	1.50	9.99	15.08	1
					0.000	9999	9999	1.15			
			1014	-7.711	0.000	-2.12	-1.84	1.50	9.99	15.08	1
					-0.007	5586	9999	1.15			
12040	0.000	9	1013	-7.724	0.000	-2.00	-2.00	1.50	9.99	15.08	1
					0.000	9999	9999	1.15			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Required Reinforcements

Beam	x[m]	NoS	LC	Ni [MN]	Myi/Mzi [MNm]	e1/yn [o/oo]	e2/zn / mm]	nue C/S	rel tra	As [cm2]	L
12040	0.000	9	1014	-7.711	0.000	-2.12	-1.84	1.50	9.99	15.08	1
					-0.007	5574	9999	1.15			
	1.000	9	1013	-7.724	0.000	-2.00	-2.00	1.50	9.99	15.08	1
					0.000	-9999	-9999	1.15			
			1014	-7.724	0.000	-2.00	-2.00	1.50	9.99	15.08	1
					0.000	-9999	-9999	1.15			
12041	0.000	9	1013	-0.315	0.000	-3.50	7.30	1.50	1.00	48.95	1
					0.653	174	9999	1.15			
			1014	-2.066	0.000	-3.50	3.75	1.50	6.35	15.08	1
					0.617	62	9999	1.15			
	1.000	9	1013	-0.332	0.000	-3.50	6.22	1.50	1.00	69.85	1
					0.851	149	9999	1.15			
			1014	-2.411	0.000	-3.50	3.10	1.50	7.05	15.08	1
					0.648	29	9999	1.15			
12042	0.000	9	1013	-0.332	0.000	-3.50	6.23	1.50	1.00	69.79	1
					0.850	149	9999	1.15			
			1014	-2.412	0.000	-3.50	3.09	1.50	7.05	15.08	1
					0.648	29	9999	1.15			
	1.000	9	1013	-0.348	0.000	-3.50	6.05	1.50	1.00	73.69	1
					0.889	144	9999	1.15			
			1014	-3.050	0.000	-3.50	2.18	1.50	8.50	15.08	1
					0.676	-31	-9999	1.15			
12043	0.000	9	1013	-0.348	0.000	-3.50	6.05	1.50	1.00	73.64	1
					0.888	144	9999	1.15			
			1014	-3.051	0.000	-3.50	2.18	1.50	8.50	15.08	1
					0.676	-31	-9999	1.15			
	1.000	9	1013	-0.365	0.000	-3.50	6.29	1.50	1.00	65.97	1
					0.821	150	9999	1.15			
			1014	-3.894	0.000	-3.50	1.31	1.50	9.99	15.08	1
					0.661	-109	-9999	1.15			
12044	0.000	9	1013	-0.365	0.000	-3.50	6.29	1.50	1.00	65.94	1
					0.821	150	9999	1.15			
			1014	-3.894	0.000	-3.50	1.31	1.50	9.99	15.08	1
					0.661	-109	-9999	1.15			
	1.000	9	1013	-0.382	0.000	-3.50	6.89	1.50	1.00	52.04	1
					0.695	165	9999	1.15			
			1014	-4.864	0.000	-3.50	0.55	1.50	9.99	15.08	1
					0.591	-205	-9999	1.15			
12045	0.000	9	1013	-0.382	0.000	-3.50	6.89	1.50	1.00	52.03	1
					0.695	165	9999	1.15			
			1014	-4.864	0.000	-3.50	0.55	1.50	9.99	15.08	1
					0.591	-205	-9999	1.15			
	1.000	9	1013	-0.399	0.000	-3.50	7.94	1.50	1.00	36.19	1
					0.545	186	9999	1.15			
			1014	-5.772	0.000	-3.50	-0.02	1.50	9.99	15.08	1
					0.464	-304	-9999	1.15			
12046	0.000	9	1013	-0.399	0.000	-3.50	7.94	1.50	1.00	36.19	1
					0.545	186	9999	1.15			
			1014	-5.772	0.000	-3.50	-0.02	1.50	9.99	15.08	1
					0.464	-304	-9999	1.15			
	1.000	9	1013	-0.416	0.000	-3.50	9.67	1.50	1.00	21.33	1
					0.396	215	9999	1.15			
			1014	-6.514	0.000	-3.50	0.00	1.50	9.99	15.08	1
					0.313	-401	-9999	1.15			
12047	0.000	9	1013	-0.416	0.000	-3.50	9.67	1.50	1.00	21.33	1
					0.396	215	9999	1.15			
			1014	-6.514	0.000	-3.50	0.00	1.50	9.99	15.08	1
					0.313	-400	-9999	1.15			
	1.000	9	1013	-0.618	0.000	-3.50	9.31	1.50	1.43	15.08	1
					0.375	209	9999	1.15			
			1014	-7.070	0.000	-3.08	-0.56	1.50	9.99	15.08	1
					0.173	-577	-9999	1.15			
12048	0.000	9	1013	-0.618	0.000	-3.50	9.31	1.50	1.43	15.08	1
					0.375	209	9999	1.15			
			1014	-7.070	0.000	-3.08	-0.56	1.50	9.99	15.08	1
					0.173	-577	-9999	1.15			
	1.000	9	1013	-1.636	0.000	-3.50	4.79	1.50	3.63	15.08	1
					0.564	105	9999	1.15			
			1014	-7.503	0.000	-2.61	-1.19	1.50	9.99	15.08	1
					0.064	-1074	-9999	1.15			
12049	0.000	9	1013	-1.635	0.000	-3.50	4.79	1.50	3.63	15.08	1
					0.564	105	9999	1.15			
			1014	-7.502	0.000	-2.61	-1.19	1.50	9.99	15.08	1
					0.064	-1073	-9999	1.15			
	1.000	9	1013	-4.168	0.000	-3.50	1.07	1.50	8.72	15.08	1
					0.648	-135	9999	1.15			
			1014	-7.706	0.000	-2.15	-1.80	1.50	9.99	15.08	1
					-0.009	4459	9999	1.15			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Required Reinforcements

Beam	x[m]	NoS	LC	Ni [MN]	Myi/Mzi [MNm]	e1/yn [o/oo]	e2/zn / mm]	nue C/S	rel tra	As [cm2]	L
12050	0.000	9	1013	-4.166	0.000	-3.50	1.08	1.50	8.72	15.08	1
					0.648	-135	9999	1.15			
			1014	-7.706	0.000	-2.15	-1.80	1.50	9.99	15.08	1
					-0.009	4483	9999	1.15			
	1.000	9	1013	-6.767	0.000	-3.32	-0.24	1.50	9.99	15.08	1
					0.249	-461	9999	1.15			
			1014	-7.575	0.000	-2.49	-1.35	1.50	9.99	15.08	1
					-0.046	1343	9999	1.15			
12051	0.000	9	1013	-6.766	0.000	-3.32	-0.24	1.50	9.99	15.08	1
					0.250	-460	9999	1.15			
			1014	-7.576	0.000	-2.49	-1.35	1.50	9.99	15.08	1
					-0.046	1346	9999	1.15			
	1.000	9	1013	-6.883	0.000	-3.24	-0.35	1.50	9.99	15.08	1
					-0.220	498	9999	1.15			
			1014	-7.522	0.000	-2.58	-1.23	1.50	9.99	15.08	1
					-0.059	1131	9999	1.15			
12052	0.000	9	1013	-6.884	0.000	-3.23	-0.35	1.50	9.99	15.08	1
					-0.220	499	9999	1.15			
			1014	-7.522	0.000	-2.58	-1.23	1.50	9.99	15.08	1
					-0.059	1131	9999	1.15			
	1.000	9	1013	-7.512	0.000	-2.59	-1.21	1.50	9.99	15.08	1
					-0.062	1101	9999	1.15			
			1014	-6.110	0.000	-3.50	-0.21	1.50	9.99	15.08	1
					-0.400	346	9999	1.15			
12053	0.000	9	1013	-7.512	0.000	-2.59	-1.21	1.50	9.99	15.08	1
					-0.062	1102	9999	1.15			
			1014	-6.111	0.000	-3.50	-0.21	1.50	9.99	15.08	1
					-0.400	346	9999	1.15			
	1.000	9	1013	-7.544	0.000	-2.54	-1.28	1.50	9.99	15.08	1
					-0.054	1208	9999	1.15			
			1014	-5.902	0.000	-3.50	-0.10	1.50	9.99	15.08	1
					-0.440	320	9999	1.15			
12054	0.000	9	1013	-7.544	0.000	-2.54	-1.28	1.50	9.99	15.08	1
					-0.054	1208	9999	1.15			
			1014	-5.903	0.000	-3.50	-0.10	1.50	9.99	15.08	1
					-0.440	320	9999	1.15			
	1.000	9	1013	-7.589	0.000	-2.46	-1.38	1.50	9.99	15.08	1
					-0.042	1418	9999	1.15			
			1014	-6.023	0.000	-3.50	-0.16	1.50	9.99	15.08	1
					-0.417	335	9999	1.15			
12055	0.000	9	1013	-7.589	0.000	-2.46	-1.38	1.50	9.99	15.08	1
					-0.042	1419	9999	1.15			
			1014	-6.024	0.000	-3.50	-0.16	1.50	9.99	15.08	1
					-0.417	335	9999	1.15			
	1.000	9	1013	-7.635	0.000	-2.37	-1.50	1.50	9.99	15.08	1
					-0.030	1781	9999	1.15			
			1014	-6.307	0.000	-3.50	-0.32	1.50	9.99	15.08	1
					-0.359	372	9999	1.15			
12056	0.000	9	1013	-7.635	0.000	-2.37	-1.50	1.50	9.99	15.08	1
					-0.030	1781	9999	1.15			
			1014	-6.308	0.000	-3.50	-0.32	1.50	9.99	15.08	1
					-0.359	372	9999	1.15			
	1.000	9	1013	-7.673	0.000	-2.28	-1.63	1.50	9.99	15.08	1
					-0.020	2422	9999	1.15			
			1014	-6.638	0.000	-3.41	-0.11	1.50	9.99	15.08	1
					-0.282	428	9999	1.15			
12057	0.000	9	1013	-7.673	0.000	-2.28	-1.63	1.50	9.99	15.08	1
					-0.020	2422	9999	1.15			
			1014	-6.638	0.000	-3.41	-0.12	1.50	9.99	15.08	1
					-0.282	428	9999	1.15			
	1.000	9	1013	-7.697	0.000	-2.19	-1.75	1.50	9.99	15.08	1
					-0.012	3535	9999	1.15			
			1014	-6.961	0.000	-3.17	-0.44	1.50	9.99	15.08	1
					-0.200	528	9999	1.15			
12058	0.000	9	1013	-7.697	0.000	-2.19	-1.75	1.50	9.99	15.08	1
					-0.012	3535	9999	1.15			
			1014	-6.961	0.000	-3.17	-0.44	1.50	9.99	15.08	1
					-0.200	528	9999	1.15			
	1.000	9	1013	-7.712	0.000	-2.12	-1.84	1.50	9.99	15.08	1
					-0.006	5822	9999	1.15			
			1014	-7.252	0.000	-2.91	-0.79	1.50	9.99	15.08	1
					-0.127	698	9999	1.15			
12059	0.000	9	1013	-7.712	0.000	-2.12	-1.84	1.50	9.99	15.08	1
					-0.006	5821	9999	1.15			
			1014	-7.252	0.000	-2.91	-0.79	1.50	9.99	15.08	1
					-0.127	698	9999	1.15			
	1.000	9	1013	-7.720	0.000	-2.06	-1.92	1.50	9.99	15.08	1
					-0.003	9999	9999	1.15			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Required Reinforcements

Beam	x[m]	NoS	LC	Ni [MN]	Myi/Mzi [MNm]	e1/yn [o/oo]	e2/zn / mm]	nue C/S	rel tra	As [cm2]	L
12059	1.000	9	1014	-7.483	0.000	-2.63	-1.16	1.50	9.99	15.08	1
					-0.069	1026	9999	1.15			
12060	0.000	9	1013	-7.720	0.000	-2.06	-1.92	1.50	9.99	15.08	1
					-0.003	9999	9999	1.15			
			1014	-7.483	0.000	-2.63	-1.16	1.50	9.99	15.08	1
					-0.069	1026	9999	1.15			
	1.000	9	1013	-7.723	0.000	-2.02	-1.97	1.50	9.99	15.08	1
					-0.001	9999	9999	1.15			
			1014	-7.641	0.000	-2.36	-1.52	1.50	9.99	15.08	1
					-0.029	1854	9999	1.15			
12061	0.000	9	1013	-7.723	0.000	-2.02	-1.97	1.50	9.99	15.08	1
					-0.001	9999	9999	1.15			
			1014	-7.641	0.000	-2.36	-1.52	1.50	9.99	15.08	1
					-0.029	1853	9999	1.15			
	1.000	9	1013	-7.724	0.000	-2.00	-2.00	1.50	9.99	15.08	1
					0.000	9999	9999	1.15			
			1014	-7.711	0.000	-2.12	-1.84	1.50	9.99	15.08	1
					-0.007	5590	9999	1.15			
12062	0.000	9	1013	-7.724	0.000	-2.00	-2.00	1.50	9.99	15.08	1
					0.000	9999	9999	1.15			
			1014	-7.711	0.000	-2.12	-1.84	1.50	9.99	15.08	1
					-0.007	5578	9999	1.15			
	1.000	9	1013	-7.724	0.000	-2.00	-2.00	1.50	9.99	15.08	1
					0.000	-9999	-9999	1.15			
			1014	-7.724	0.000	-2.00	-2.00	1.50	9.99	15.08	1
					0.000	-9999	-9999	1.15			
12063	0.000	9	1013	-0.314	0.000	-3.50	7.30	1.50	1.00	48.90	1
					0.652	174	9999	1.15			
			1014	-2.081	-0.001	-3.50	3.71	1.50	6.40	15.08	1
					0.619	61	9999	1.15			
	1.000	9	1013	-0.331	0.000	-3.50	6.22	1.50	1.00	69.82	1
					0.851	149	9999	1.15			
			1014	-2.420	0.000	-3.50	3.08	1.50	7.08	15.08	1
					0.648	28	9999	1.15			
12064	0.000	9	1013	-0.331	0.000	-3.50	6.23	1.50	1.00	69.76	1
					0.850	149	9999	1.15			
			1014	-2.421	0.000	-3.50	3.08	1.50	7.08	15.08	1
					0.648	28	9999	1.15			
	1.000	9	1013	-0.348	0.000	-3.50	6.05	1.50	1.00	73.68	1
					0.888	144	9999	1.15			
			1014	-3.053	0.000	-3.50	2.17	1.50	8.51	15.08	1
					0.676	-31	-9999	1.15			
12065	0.000	9	1013	-0.348	0.000	-3.50	6.05	1.50	1.00	73.63	1
					0.888	144	9999	1.15			
			1014	-3.054	0.000	-3.50	2.17	1.50	8.51	15.08	1
					0.676	-31	-9999	1.15			
	1.000	9	1013	-0.376	0.000	-3.50	6.25	1.50	1.00	66.24	1
					0.825	149	-9999	1.15			
			1014	-4.015	0.000	-3.50	1.20	1.50	9.99	15.08	1
					0.656	-120	9999	1.15			
12066	0.000	9	1013	-0.376	0.000	-3.50	6.26	1.50	1.00	66.21	1
					0.825	149	-9999	1.15			
			1014	-4.015	0.000	-3.50	1.20	1.50	9.99	15.08	1
					0.656	-120	9999	1.15			
	1.000	9	1013	-0.393	0.000	-3.50	6.85	1.50	1.00	52.19	1
					0.698	164	-9999	1.15			
			1014	-4.963	0.000	-3.50	0.48	1.50	9.99	15.08	1
					0.580	-215	9999	1.15			
12067	0.000	9	1013	-0.393	0.000	-3.50	6.85	1.50	1.00	52.17	1
					0.698	164	-9999	1.15			
			1014	-4.963	0.000	-3.50	0.48	1.50	9.99	15.08	1
					0.580	-215	9999	1.15			
	1.000	9	1013	-0.410	0.000	-3.50	7.89	1.50	1.00	36.21	1
					0.547	185	-9999	1.15			
			1014	-5.840	0.000	-3.50	-0.06	1.50	9.99	15.08	1
					0.452	-312	9999	1.15			
12068	0.000	9	1013	-0.410	0.000	-3.50	7.89	1.50	1.00	36.20	1
					0.547	185	-9999	1.15			
			1014	-5.840	0.000	-3.50	-0.06	1.50	9.99	15.08	1
					0.452	-312	9999	1.15			
	1.000	9	1013	-0.427	0.000	-3.50	9.62	1.50	1.00	21.25	1
					0.397	214	-9999	1.15			
			1014	-6.552	0.000	-3.47	-0.04	1.50	9.99	15.08	1
					0.303	-408	9999	1.15			
12069	0.000	9	1013	-0.427	0.000	-3.50	9.62	1.50	1.00	21.24	1
					0.397	214	-9999	1.15			
			1014	-6.552	0.000	-3.47	-0.04	1.50	9.99	15.08	1
					0.303	-408	9999	1.15			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Required Reinforcements

Beam	x[m]	NoS	LC	Ni [MN]	Myi/Mzi [MNm]	e1/yn [o/oo]	e2/zn / mm]	nue C/S	rel tra	As [cm2]	L
12069	1.000	9	1013	-0.639	0.000	-3.50	9.16	1.50	1.44	15.08	1
					0.380	207	-9999	1.15			
			1014	-7.091	0.000	-3.06	-0.58	1.50	9.99	15.08	1
					0.168	-588	9999	1.15			
12070	0.000	9	1013	-0.639	0.000	-3.50	9.16	1.50	1.44	15.08	1
					0.380	207	-9999	1.15			
			1014	-7.091	0.000	-3.06	-0.58	1.50	9.99	15.08	1
					0.168	-588	9999	1.15			
	1.000	9	1013	-1.693	0.000	-3.50	4.63	1.50	3.68	15.08	1
					0.572	99	-9999	1.15			
			1014	-7.508	0.000	-2.60	-1.20	1.50	9.99	15.08	1
					0.063	-1089	9999	1.15			
12071	0.000	9	1013	-1.693	0.000	-3.50	4.63	1.50	3.67	15.08	1
					0.572	99	-9999	1.15			
			1014	-7.507	0.000	-2.60	-1.20	1.50	9.99	15.08	1
					0.063	-1088	9999	1.15			
	1.000	9	1013	-4.164	0.000	-3.50	1.08	1.50	8.72	15.08	1
					0.648	-135	9999	1.15			
			1014	-7.708	0.000	-2.14	-1.82	1.50	9.99	15.08	1
					-0.008	4941	9999	1.15			
12072	0.000	9	1013	-4.162	0.000	-3.50	1.08	1.50	8.71	15.08	1
					0.648	-135	9999	1.15			
			1014	-7.708	0.000	-2.14	-1.82	1.50	9.99	15.08	1
					-0.008	4971	9999	1.15			
	1.000	9	1013	-6.764	0.000	-3.32	-0.23	1.50	9.99	15.08	1
					0.250	-460	9999	1.15			
			1014	-7.578	0.000	-2.48	-1.35	1.50	9.99	15.08	1
					-0.045	1359	9999	1.15			
12073	0.000	9	1013	-6.763	0.000	-3.33	-0.23	1.50	9.99	15.08	1
					0.250	-460	9999	1.15			
			1014	-7.578	0.000	-2.48	-1.35	1.50	9.99	15.08	1
					-0.045	1360	9999	1.15			
	1.000	9	1013	-6.884	0.000	-3.23	-0.35	1.50	9.99	15.08	1
					-0.220	499	9999	1.15			
			1014	-7.523	0.000	-2.57	-1.23	1.50	9.99	15.08	1
					-0.059	1136	9999	1.15			
12074	0.000	9	1013	-6.886	0.000	-3.23	-0.36	1.50	9.99	15.08	1
					-0.219	499	9999	1.15			
			1014	-7.523	0.000	-2.57	-1.23	1.50	9.99	15.08	1
					-0.059	1137	9999	1.15			
	1.000	9	1013	-7.513	0.000	-2.59	-1.21	1.50	9.99	15.08	1
					-0.061	1104	9999	1.15			
			1014	-6.110	0.000	-3.50	-0.21	1.50	9.99	15.08	1
					-0.400	346	9999	1.15			
12075	0.000	9	1013	-7.513	0.000	-2.59	-1.21	1.50	9.99	15.08	1
					-0.061	1104	9999	1.15			
			1014	-6.111	0.000	-3.50	-0.21	1.50	9.99	15.08	1
					-0.400	346	9999	1.15			
	1.000	9	1013	-7.544	0.000	-2.54	-1.28	1.50	9.99	15.08	1
					-0.053	1210	9999	1.15			
			1014	-5.902	0.000	-3.50	-0.10	1.50	9.99	15.08	1
					-0.440	320	9999	1.15			
12076	0.000	9	1013	-7.544	0.000	-2.54	-1.28	1.50	9.99	15.08	1
					-0.053	1210	9999	1.15			
			1014	-5.903	0.000	-3.50	-0.10	1.50	9.99	15.08	1
					-0.440	320	9999	1.15			
	1.000	9	1013	-7.597	0.000	-2.45	-1.40	1.50	9.99	15.08	1
					-0.040	1467	-9999	1.15			
			1014	-6.044	0.000	-3.50	-0.18	1.50	9.99	15.08	1
					-0.413	338	-9999	1.15			
12077	0.000	9	1013	-7.597	0.000	-2.45	-1.40	1.50	9.99	15.08	1
					-0.040	1468	-9999	1.15			
			1014	-6.045	0.000	-3.50	-0.18	1.50	9.99	15.08	1
					-0.413	338	-9999	1.15			
	1.000	9	1013	-7.640	0.000	-2.36	-1.52	1.50	9.99	15.08	1
					-0.029	1843	-9999	1.15			
			1014	-6.325	0.000	-3.50	-0.33	1.50	9.99	15.08	1
					-0.355	374	-9999	1.15			
12078	0.000	9	1013	-7.640	0.000	-2.36	-1.52	1.50	9.99	15.08	1
					-0.029	1843	-9999	1.15			
			1014	-6.325	0.000	-3.50	-0.33	1.50	9.99	15.08	1
					-0.355	374	-9999	1.15			
	1.000	9	1013	-7.676	0.000	-2.27	-1.64	1.50	9.99	15.08	1
					-0.019	2511	-9999	1.15			
			1014	-6.651	0.000	-3.41	-0.13	1.50	9.99	15.08	1
					-0.278	431	-9999	1.15			
12079	0.000	9	1013	-7.676	0.000	-2.27	-1.64	1.50	9.99	15.08	1
					-0.019	2511	-9999	1.15			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Required Reinforcements

Beam	x[m]	NoS	LC	Ni [MN]	Myi/Mzi [MNm]	e1/yn [o/oo]	e2/zn / mm]	nue C/S	rel tra	As [cm2]	L
12079	0.000	9	1014	-6.651	0.000	-3.40	-0.13	1.50	9.99	15.08	1
					-0.278	431	-9999	1.15			
	1.000	9	1013	-7.699	0.000	-2.18	-1.75	1.50	9.99	15.08	1
					-0.011	3658	-9999	1.15			
			1014	-6.970	0.000	-3.17	-0.45	1.50	9.99	15.08	1
					-0.198	532	-9999	1.15			
12080	0.000	9	1013	-7.699	0.000	-2.18	-1.75	1.50	9.99	15.08	1
					-0.011	3657	-9999	1.15			
			1014	-6.970	0.000	-3.17	-0.45	1.50	9.99	15.08	1
					-0.198	532	-9999	1.15			
	1.000	9	1013	-7.712	0.000	-2.11	-1.85	1.50	9.99	15.08	1
					-0.006	6028	-9999	1.15			
			1014	-7.258	0.000	-2.90	-0.80	1.50	9.99	15.08	1
					-0.125	704	-9999	1.15			
12081	0.000	9	1013	-7.712	0.000	-2.11	-1.85	1.50	9.99	15.08	1
					-0.006	6026	-9999	1.15			
			1014	-7.258	0.000	-2.90	-0.80	1.50	9.99	15.08	1
					-0.125	703	-9999	1.15			
	1.000	9	1013	-7.720	0.000	-2.06	-1.93	1.50	9.99	15.08	1
					-0.003	9999	-9999	1.15			
			1014	-7.487	0.000	-2.63	-1.16	1.50	9.99	15.08	1
					-0.068	1035	-9999	1.15			
12082	0.000	9	1013	-7.720	0.000	-2.06	-1.93	1.50	9.99	15.08	1
					-0.003	9999	-9999	1.15			
			1014	-7.487	0.000	-2.63	-1.16	1.50	9.99	15.08	1
					-0.068	1034	-9999	1.15			
	1.000	9	1013	-7.723	0.000	-2.02	-1.97	1.50	9.99	15.08	1
					-0.001	9999	-9999	1.15			
			1014	-7.643	0.000	-2.36	-1.53	1.50	9.99	15.08	1
					-0.028	1872	-9999	1.15			
12083	0.000	9	1013	-7.723	0.000	-2.02	-1.97	1.50	9.99	15.08	1
					-0.001	9999	-9999	1.15			
			1014	-7.642	0.000	-2.36	-1.53	1.50	9.99	15.08	1
					-0.028	1871	-9999	1.15			
	1.000	9	1013	-7.724	0.000	-2.00	-2.00	1.50	9.99	15.08	1
					0.000	9999	-9999	1.15			
			1014	-7.711	0.000	-2.12	-1.84	1.50	9.99	15.08	1
					-0.007	5651	-9999	1.15			
12084	0.000	9	1013	-7.724	0.000	-2.00	-2.00	1.50	9.99	15.08	1
					0.000	9999	-9999	1.15			
			1014	-7.711	0.000	-2.12	-1.84	1.50	9.99	15.08	1
					-0.007	5638	-9999	1.15			
	1.000	9	1013	-7.724	0.000	-2.00	-2.00	1.50	9.99	15.08	1
					0.000	-9999	9999	1.15			
			1014	-7.724	0.000	-2.00	-2.00	1.50	9.99	15.08	1
					0.000	-9999	9999	1.15			
12085	0.000	9	1013	-2.081	0.001	-3.50	3.71	1.50	6.40	15.08	1
					-0.619	-60	-9999	1.15			
			1014	-0.314	0.000	-3.50	7.30	1.50	1.00	48.90	1
					-0.652	-173	-9999	1.15			
	1.000	9	1013	-2.420	0.000	-3.50	3.08	1.50	7.08	15.08	1
					-0.648	-27	-9999	1.15			
			1014	-0.331	0.000	-3.50	6.22	1.50	1.00	69.82	1
					-0.851	-148	-9999	1.15			
12086	0.000	9	1013	-2.421	0.000	-3.50	3.08	1.50	7.08	15.08	1
					-0.648	-27	-9999	1.15			
			1014	-0.331	0.000	-3.50	6.23	1.50	1.00	69.76	1
					-0.850	-148	-9999	1.15			
	1.000	9	1013	-3.053	0.000	-3.50	2.17	1.50	8.51	15.08	1
					-0.676	32	9999	1.15			
			1014	-0.348	0.000	-3.50	6.05	1.50	1.00	73.68	1
					-0.888	-143	-9999	1.15			
12087	0.000	9	1013	-3.054	0.000	-3.50	2.17	1.50	8.51	15.08	1
					-0.676	32	9999	1.15			
			1014	-0.348	0.000	-3.50	6.05	1.50	1.00	73.63	1
					-0.888	-143	-9999	1.15			
	1.000	9	1013	-4.015	0.000	-3.50	1.20	1.50	9.99	15.08	1
					-0.656	121	-9999	1.15			
			1014	-0.376	0.000	-3.50	6.25	1.50	1.00	66.24	1
					-0.825	-148	9999	1.15			
12088	0.000	9	1013	-4.015	0.000	-3.50	1.20	1.50	9.99	15.08	1
					-0.656	121	-9999	1.15			
			1014	-0.376	0.000	-3.50	6.26	1.50	1.00	66.21	1
					-0.825	-148	9999	1.15			
	1.000	9	1013	-0.382	0.000	-3.50	6.89	1.50	1.00	52.06	1
					-0.695	-164	9999	1.15			
			1014	-4.859	0.000	-3.50	0.55	1.50	9.99	15.08	1
					-0.591	205	-9999	1.15			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Required Reinforcements

Beam	x[m]	NoS	LC	Ni [MN]	Myi/Mzi [MNm]	e1/yn [o/oo]	e2/zn / mm]	nue C/S	rel tra	As [cm2]	L
12089	0.000	9	1013	-0.382	0.000	-3.50	6.89	1.50	1.00	52.04	1
					-0.695	-164	9999	1.15			
			1014	-4.859	0.000	-3.50	0.55	1.50	9.99	15.08	1
					-0.591	205	-9999	1.15			
	1.000	9	1013	-0.399	0.000	-3.50	7.94	1.50	1.00	36.21	1
					-0.545	-185	9999	1.15			
			1014	-5.765	0.000	-3.50	-0.02	1.50	9.99	15.08	1
					-0.465	304	-9999	1.15			
12090	0.000	9	1013	-0.399	0.000	-3.50	7.94	1.50	1.00	36.20	1
					-0.545	-185	9999	1.15			
			1014	-5.765	0.000	-3.50	-0.02	1.50	9.99	15.08	1
					-0.465	304	-9999	1.15			
	1.000	9	1013	-0.416	0.000	-3.50	9.67	1.50	1.00	21.35	1
					-0.396	-213	9999	1.15			
			1014	-6.509	0.000	-3.50	-0.44	1.50	9.99	15.08	1
					-0.314	401	-9999	1.15			
12091	0.000	9	1013	-0.416	0.000	-3.50	9.67	1.50	1.00	21.35	1
					-0.396	-213	9999	1.15			
			1014	-6.509	0.000	-3.50	-0.44	1.50	9.99	15.08	1
					-0.314	401	-9999	1.15			
	1.000	9	1013	-0.617	0.000	-3.50	9.32	1.50	1.42	15.08	1
					-0.375	-208	9999	1.15			
			1014	-7.064	0.000	-3.09	-0.55	1.50	9.99	15.08	1
					-0.174	575	-9999	1.15			
12092	0.000	9	1013	-0.617	0.000	-3.50	9.32	1.50	1.42	15.08	1
					-0.375	-208	9999	1.15			
			1014	-7.063	0.000	-3.09	-0.55	1.50	9.99	15.08	1
					-0.174	574	-9999	1.15			
	1.000	9	1013	-1.633	0.000	-3.50	4.79	1.50	3.63	15.08	1
					-0.564	-104	9999	1.15			
			1014	-7.497	0.000	-2.61	-1.18	1.50	9.99	15.08	1
					-0.065	1060	-9999	1.15			
12093	0.000	9	1013	-1.632	0.000	-3.50	4.80	1.50	3.63	15.08	1
					-0.564	-104	9999	1.15			
			1014	-7.496	0.000	-2.61	-1.18	1.50	9.99	15.08	1
					-0.066	1059	-9999	1.15			
	1.000	9	1013	-4.094	0.000	-3.50	1.14	1.50	8.77	15.08	1
					-0.652	129	-9999	1.15			
			1014	-7.707	0.000	-2.14	-1.81	1.50	9.99	15.08	1
					0.008	-4768	-9999	1.15			
12094	0.000	9	1013	-4.092	0.000	-3.50	1.14	1.50	8.76	15.08	1
					-0.652	129	-9999	1.15			
			1014	-7.708	0.000	-2.14	-1.81	1.50	9.99	15.08	1
					0.008	-4797	-9999	1.15			
	1.000	9	1013	-6.736	0.000	-3.35	-0.21	1.50	9.99	15.08	1
					-0.257	453	-9999	1.15			
			1014	-7.570	0.000	-2.50	-1.33	1.50	9.99	15.08	1
					0.047	-1316	-9999	1.15			
12095	0.000	9	1013	-6.734	0.000	-3.35	-0.20	1.50	9.99	15.08	1
					-0.257	453	-9999	1.15			
			1014	-7.570	0.000	-2.50	-1.33	1.50	9.99	15.08	1
					0.047	-1317	-9999	1.15			
	1.000	9	1013	-6.881	0.000	-3.24	-0.35	1.50	9.99	15.08	1
					0.220	-497	-9999	1.15			
			1014	-7.512	0.000	-2.59	-1.21	1.50	9.99	15.08	1
					0.062	-1102	-9999	1.15			
12096	0.000	9	1013	-6.883	0.000	-3.24	-0.35	1.50	9.99	15.08	1
					0.220	-497	-9999	1.15			
			1014	-7.513	0.000	-2.59	-1.21	1.50	9.99	15.08	1
					0.061	-1102	-9999	1.15			
	1.000	9	1013	-6.110	0.000	-3.50	-0.21	1.50	9.99	15.08	1
					0.400	-345	-9999	1.15			
			1014	-7.513	0.000	-2.59	-1.21	1.50	9.99	15.08	1
					0.061	-1103	-9999	1.15			
12097	0.000	9	1013	-6.111	0.000	-3.50	-0.21	1.50	9.99	15.08	1
					0.400	-345	-9999	1.15			
			1014	-7.513	0.000	-2.59	-1.21	1.50	9.99	15.08	1
					0.061	-1103	-9999	1.15			
	1.000	9	1013	-5.902	0.000	-3.50	-0.10	1.50	9.99	15.08	1
					0.440	-319	-9999	1.15			
			1014	-7.544	0.000	-2.54	-1.28	1.50	9.99	15.08	1
					0.053	-1209	-9999	1.15			
12098	0.000	9	1013	-5.903	0.000	-3.50	-0.10	1.50	9.99	15.08	1
					0.440	-319	-9999	1.15			
			1014	-7.544	0.000	-2.54	-1.28	1.50	9.99	15.08	1
					0.053	-1209	-9999	1.15			
	1.000	9	1013	-6.044	0.000	-3.50	-0.18	1.50	9.99	15.08	1
					0.413	-337	9999	1.15			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Required Reinforcements

Beam	x[m]	NoS	LC	Ni [MN]	Myi/Mzi [MNm]	e1/yn [o/oo]	e2/zn / mm]	nue C/S	rel tra	As [cm2]	L
12098	1.000	9	1014	-7.597	0.000	-2.45	-1.40	1.50	9.99	15.08	1
					0.040	-1466	9999	1.15			
12099	0.000	9	1013	-6.045	0.000	-3.50	-0.18	1.50	9.99	15.08	1
					0.413	-337	9999	1.15			
			1014	-7.597	0.000	-2.45	-1.40	1.50	9.99	15.08	1
					0.040	-1467	9999	1.15			
	1.000	9	1013	-7.635	0.000	-2.37	-1.50	1.50	9.99	15.08	1
					0.030	-1778	9999	1.15			
			1014	-6.307	0.000	-3.50	-0.32	1.50	9.99	15.08	1
					0.359	-371	9999	1.15			
12100	0.000	9	1013	-7.635	0.000	-2.37	-1.50	1.50	9.99	15.08	1
					0.030	-1778	9999	1.15			
			1014	-6.307	0.000	-3.50	-0.32	1.50	9.99	15.08	1
					0.359	-371	9999	1.15			
	1.000	9	1013	-7.673	0.000	-2.28	-1.63	1.50	9.99	15.08	1
					0.020	-2415	9999	1.15			
			1014	-6.638	0.000	-3.41	-0.11	1.50	9.99	15.08	1
					0.282	-427	9999	1.15			
12101	0.000	9	1013	-7.673	0.000	-2.28	-1.63	1.50	9.99	15.08	1
					0.020	-2415	9999	1.15			
			1014	-6.638	0.000	-3.41	-0.11	1.50	9.99	15.08	1
					0.282	-427	9999	1.15			
	1.000	9	1013	-7.697	0.000	-2.19	-1.74	1.50	9.99	15.08	1
					0.012	-3521	9999	1.15			
			1014	-6.961	0.000	-3.17	-0.44	1.50	9.99	15.08	1
					0.200	-527	9999	1.15			
12102	0.000	9	1013	-7.697	0.000	-2.19	-1.74	1.50	9.99	15.08	1
					0.012	-3521	9999	1.15			
			1014	-6.961	0.000	-3.17	-0.44	1.50	9.99	15.08	1
					0.200	-527	9999	1.15			
	1.000	9	1013	-7.712	0.000	-2.12	-1.84	1.50	9.99	15.08	1
					0.006	-5786	9999	1.15			
			1014	-7.252	0.000	-2.91	-0.79	1.50	9.99	15.08	1
					0.127	-697	9999	1.15			
12103	0.000	9	1013	-7.712	0.000	-2.12	-1.84	1.50	9.99	15.08	1
					0.006	-5785	9999	1.15			
			1014	-7.252	0.000	-2.91	-0.79	1.50	9.99	15.08	1
					0.127	-697	9999	1.15			
	1.000	9	1013	-7.720	0.000	-2.06	-1.92	1.50	9.99	15.08	1
					0.003	-9999	9999	1.15			
			1014	-7.483	0.000	-2.63	-1.16	1.50	9.99	15.08	1
					0.069	-1025	9999	1.15			
12104	0.000	9	1013	-7.720	0.000	-2.06	-1.92	1.50	9.99	15.08	1
					0.003	-9999	9999	1.15			
			1014	-7.483	0.000	-2.63	-1.16	1.50	9.99	15.08	1
					0.069	-1024	9999	1.15			
	1.000	9	1013	-7.723	0.000	-2.02	-1.97	1.50	9.99	15.08	1
					0.001	-9999	9999	1.15			
			1014	-7.641	0.000	-2.36	-1.52	1.50	9.99	15.08	1
					0.029	-1852	9999	1.15			
12105	0.000	9	1013	-7.723	0.000	-2.02	-1.97	1.50	9.99	15.08	1
					0.001	-9999	9999	1.15			
			1014	-7.641	0.000	-2.36	-1.52	1.50	9.99	15.08	1
					0.029	-1851	9999	1.15			
	1.000	9	1013	-7.724	0.000	-2.00	-2.00	1.50	9.99	15.08	1
					0.000	-9999	9999	1.15			
			1014	-7.711	0.000	-2.12	-1.84	1.50	9.99	15.08	1
					0.007	-5585	9999	1.15			
12106	0.000	9	1013	-7.724	0.000	-2.00	-2.00	1.50	9.99	15.08	1
					0.000	-9999	9999	1.15			
			1014	-7.711	0.000	-2.12	-1.84	1.50	9.99	15.08	1
					0.007	-5573	9999	1.15			
	1.000	9	1013	-7.724	0.000	-2.00	-2.00	1.50	9.99	15.08	1
					0.000	9999	-9999	1.15			
			1014	-7.724	0.000	-2.00	-2.00	1.50	9.99	15.08	1
					0.000	9999	-9999	1.15			
12107	0.000	9	1013	-0.315	0.000	-3.50	7.30	1.50	1.00	48.95	1
					-0.653	-173	9999	1.15			
			1014	-2.066	0.000	-3.50	3.75	1.50	6.35	15.08	1
					-0.617	-61	9999	1.15			
	1.000	9	1013	-0.332	0.000	-3.50	6.22	1.50	1.00	69.85	1
					-0.851	-148	9999	1.15			
			1014	-2.411	0.000	-3.50	3.10	1.50	7.05	15.08	1
					-0.648	-28	9999	1.15			
12108	0.000	9	1013	-0.332	0.000	-3.50	6.23	1.50	1.00	69.79	1
					-0.850	-148	9999	1.15			
			1014	-2.412	0.000	-3.50	3.09	1.50	7.05	15.08	1
					-0.648	-28	9999	1.15			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Required Reinforcements

Beam	x[m]	NoS	LC	Ni [MN]	Myi/Mzi [MNm]	e1/yn [o/oo]	e2/zn / mm]	nue C/S	rel tra	As [cm2]	L
12108	1.000	9	1013	-0.348	0.000	-3.50	6.05	1.50	1.00	73.69	1
					-0.889	-143	9999	1.15			
			1014	-3.050	0.000	-3.50	2.18	1.50	8.50	15.08	1
					-0.676	32	-9999	1.15			
12109	0.000	9	1013	-0.348	0.000	-3.50	6.05	1.50	1.00	73.64	1
					-0.888	-143	9999	1.15			
			1014	-3.051	0.000	-3.50	2.18	1.50	8.50	15.08	1
					-0.676	32	-9999	1.15			
	1.000	9	1013	-4.016	0.000	-3.50	1.20	1.50	9.99	15.08	1
					-0.656	121	-9999	1.15			
			1014	-0.376	0.000	-3.50	6.25	1.50	1.00	66.23	1
					-0.825	-148	9999	1.15			
12110	0.000	9	1013	-4.017	0.000	-3.50	1.20	1.50	9.99	15.08	1
					-0.656	122	-9999	1.15			
			1014	-0.376	0.000	-3.50	6.26	1.50	1.00	66.21	1
					-0.825	-148	9999	1.15			
	1.000	9	1013	-4.967	0.000	-3.50	0.48	1.50	9.99	15.08	1
					-0.579	216	-9999	1.15			
			1014	-0.393	0.000	-3.50	6.85	1.50	1.00	52.17	1
					-0.698	-163	9999	1.15			
12111	0.000	9	1013	-4.968	0.000	-3.50	0.48	1.50	9.99	15.08	1
					-0.579	216	-9999	1.15			
			1014	-0.393	0.000	-3.50	6.85	1.50	1.00	52.16	1
					-0.698	-163	9999	1.15			
	1.000	9	1013	-5.846	0.000	-3.50	-0.06	1.50	9.99	15.08	1
					-0.450	313	-9999	1.15			
			1014	-0.410	0.000	-3.50	7.90	1.50	1.00	36.19	1
					-0.547	-184	9999	1.15			
12112	0.000	9	1013	-5.846	0.000	-3.50	-0.06	1.50	9.99	15.08	1
					-0.450	313	-9999	1.15			
			1014	-0.410	0.000	-3.50	7.90	1.50	1.00	36.19	1
					-0.547	-185	9999	1.15			
	1.000	9	1013	-6.558	0.000	-3.47	-0.04	1.50	9.99	15.08	1
					-0.302	411	-9999	1.15			
			1014	-0.427	0.000	-3.50	9.62	1.50	1.00	21.23	1
					-0.397	-213	9999	1.15			
12113	0.000	9	1013	-6.558	0.000	-3.47	-0.04	1.50	9.99	15.08	1
					-0.302	411	-9999	1.15			
			1014	-0.427	0.000	-3.50	9.62	1.50	1.00	21.23	1
					-0.397	-213	9999	1.15			
	1.000	9	1013	-7.097	0.000	-3.06	-0.59	1.50	9.99	15.08	1
					-0.166	592	-9999	1.15			
			1014	-0.640	0.000	-3.50	9.16	1.50	1.44	15.08	1
					-0.380	-206	9999	1.15			
12114	0.000	9	1013	-7.097	0.000	-3.06	-0.59	1.50	9.99	15.08	1
					-0.166	592	-9999	1.15			
			1014	-0.640	0.000	-3.50	9.16	1.50	1.44	15.08	1
					-0.380	-206	9999	1.15			
	1.000	9	1013	-7.513	0.000	-2.59	-1.21	1.50	9.99	15.08	1
					-0.061	1106	-9999	1.15			
			1014	-1.696	0.000	-3.50	4.62	1.50	3.68	15.08	1
					-0.573	-98	9999	1.15			
12115	0.000	9	1013	-7.513	0.000	-2.59	-1.21	1.50	9.99	15.08	1
					-0.061	1105	-9999	1.15			
			1014	-1.695	0.000	-3.50	4.62	1.50	3.68	15.08	1
					-0.573	-98	9999	1.15			
	1.000	9	1013	-7.706	0.000	-2.15	-1.80	1.50	9.99	15.08	1
					0.009	-4458	-9999	1.15			
			1014	-4.168	0.000	-3.50	1.07	1.50	8.72	15.08	1
					-0.648	136	-9999	1.15			
12116	0.000	9	1013	-7.706	0.000	-2.15	-1.80	1.50	9.99	15.08	1
					0.009	-4482	-9999	1.15			
			1014	-4.166	0.000	-3.50	1.08	1.50	8.72	15.08	1
					-0.648	136	-9999	1.15			
	1.000	9	1013	-7.567	0.000	-2.50	-1.33	1.50	9.99	15.08	1
					0.048	-1302	9999	1.15			
			1014	-6.739	0.000	-3.34	-0.21	1.50	9.99	15.08	1
					-0.256	454	-9999	1.15			
12117	0.000	9	1013	-7.567	0.000	-2.50	-1.33	1.50	9.99	15.08	1
					0.048	-1303	9999	1.15			
			1014	-6.737	0.000	-3.34	-0.21	1.50	9.99	15.08	1
					-0.257	453	-9999	1.15			
	1.000	9	1013	-7.511	0.000	-2.59	-1.21	1.50	9.99	15.08	1
					0.062	-1097	9999	1.15			
			1014	-6.880	0.000	-3.24	-0.35	1.50	9.99	15.08	1
					0.221	-496	-9999	1.15			
12118	0.000	9	1013	-7.511	0.000	-2.59	-1.21	1.50	9.99	15.08	1
					0.062	-1097	9999	1.15			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Required Reinforcements

Beam	x[m]	NoS	LC	Ni [MN]	Myi/Mzi [MNm]	e1/yn [o/oo]	e2/zn / mm]	nue C/S	rel tra	As [cm2]	L
12118	0.000	9	1014	-6.881	0.000	-3.24	-0.35	1.50	9.99	15.08	1
					0.220	-497	-9999	1.15			
	1.000	9	1013	-7.512	0.000	-2.59	-1.21	1.50	9.99	15.08	1
					0.062	-1100	9999	1.15			
			1014	-6.110	0.000	-3.50	-0.21	1.50	9.99	15.08	1
					0.400	-345	9999	1.15			
12119	0.000	9	1013	-7.512	0.000	-2.59	-1.21	1.50	9.99	15.08	1
					0.062	-1101	9999	1.15			
			1014	-6.111	0.000	-3.50	-0.21	1.50	9.99	15.08	1
					0.400	-345	9999	1.15			
	1.000	9	1013	-7.544	0.000	-2.54	-1.28	1.50	9.99	15.08	1
					0.054	-1207	9999	1.15			
			1014	-5.902	0.000	-3.50	-0.10	1.50	9.99	15.08	1
					0.440	-319	9999	1.15			
12120	0.000	9	1013	-7.544	0.000	-2.54	-1.28	1.50	9.99	15.08	1
					0.054	-1207	9999	1.15			
			1014	-5.903	0.000	-3.50	-0.10	1.50	9.99	15.08	1
					0.440	-319	9999	1.15			
	1.000	9	1013	-6.045	0.000	-3.50	-0.18	1.50	9.99	15.08	1
					0.413	-337	9999	1.15			
			1014	-7.597	0.000	-2.45	-1.40	1.50	9.99	15.08	1
					0.040	-1466	9999	1.15			
12121	0.000	9	1013	-6.045	0.000	-3.50	-0.18	1.50	9.99	15.08	1
					0.413	-337	9999	1.15			
			1014	-7.597	0.000	-2.45	-1.40	1.50	9.99	15.08	1
					0.040	-1466	9999	1.15			
	1.000	9	1013	-6.325	0.000	-3.50	-0.33	1.50	9.99	15.08	1
					0.355	-373	9999	1.15			
			1014	-7.640	0.000	-2.36	-1.52	1.50	9.99	15.08	1
					0.029	-1844	9999	1.15			
12122	0.000	9	1013	-6.326	0.000	-3.50	-0.33	1.50	9.99	15.08	1
					0.355	-373	9999	1.15			
			1014	-7.640	0.000	-2.36	-1.52	1.50	9.99	15.08	1
					0.029	-1844	9999	1.15			
	1.000	9	1013	-6.651	0.000	-3.40	-0.13	1.50	9.99	15.08	1
					0.278	-430	9999	1.15			
			1014	-7.677	0.000	-2.27	-1.64	1.50	9.99	15.08	1
					0.019	-2515	9999	1.15			
12123	0.000	9	1013	-6.652	0.000	-3.40	-0.13	1.50	9.99	15.08	1
					0.278	-431	9999	1.15			
			1014	-7.677	0.000	-2.27	-1.64	1.50	9.99	15.08	1
					0.019	-2515	9999	1.15			
	1.000	9	1013	-6.971	0.000	-3.17	-0.45	1.50	9.99	15.08	1
					0.198	-531	9999	1.15			
			1014	-7.699	0.000	-2.18	-1.75	1.50	9.99	15.08	1
					0.011	-3670	9999	1.15			
12124	0.000	9	1013	-6.971	0.000	-3.17	-0.45	1.50	9.99	15.08	1
					0.198	-531	9999	1.15			
			1014	-7.699	0.000	-2.18	-1.75	1.50	9.99	15.08	1
					0.011	-3670	9999	1.15			
	1.000	9	1013	-7.259	0.000	-2.90	-0.80	1.50	9.99	15.08	1
					0.125	-703	9999	1.15			
			1014	-7.713	0.000	-2.11	-1.85	1.50	9.99	15.08	1
					0.006	-6064	9999	1.15			
12125	0.000	9	1013	-7.258	0.000	-2.90	-0.80	1.50	9.99	15.08	1
					0.125	-703	9999	1.15			
			1014	-7.713	0.000	-2.11	-1.85	1.50	9.99	15.08	1
					0.006	-6062	9999	1.15			
	1.000	9	1013	-7.487	0.000	-2.63	-1.16	1.50	9.99	15.08	1
					0.068	-1034	9999	1.15			
			1014	-7.720	0.000	-2.06	-1.93	1.50	9.99	15.08	1
					0.003	-9999	9999	1.15			
12126	0.000	9	1013	-7.487	0.000	-2.63	-1.16	1.50	9.99	15.08	1
					0.068	-1034	9999	1.15			
			1014	-7.720	0.000	-2.06	-1.93	1.50	9.99	15.08	1
					0.003	-9999	9999	1.15			
	1.000	9	1013	-7.643	0.000	-2.36	-1.53	1.50	9.99	15.08	1
					0.028	-1872	9999	1.15			
			1014	-7.723	0.000	-2.02	-1.97	1.50	9.99	15.08	1
					0.001	-9999	9999	1.15			
12127	0.000	9	1013	-7.643	0.000	-2.36	-1.53	1.50	9.99	15.08	1
					0.028	-1871	9999	1.15			
			1014	-7.723	0.000	-2.02	-1.97	1.50	9.99	15.08	1
					0.001	-9999	9999	1.15			
	1.000	9	1013	-7.711	0.000	-2.12	-1.84	1.50	9.99	15.08	1
					0.006	-5654	9999	1.15			
			1014	-7.724	0.000	-2.00	-2.00	1.50	9.99	15.08	1
					0.000	-9999	9999	1.15			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Required Reinforcements

Beam	x[m]	NoS	LC	Ni [MN]	Myi/Mzi [MNm]	e1/yn [o/oo]	e2/zn / mm]	nue C/S	rel tra	As [cm2]	L
12128	0.000	9	1013	-7.711	0.000	-2.12	-1.84	1.50	9.99	15.08	1
					0.007	-5641	9999	1.15			
			1014	-7.724	0.000	-2.00	-2.00	1.50	9.99	15.08	1
					0.000	-9999	9999	1.15			
	1.000	9	1013	-7.724	0.000	-2.00	-2.00	1.50	9.99	15.08	1
					0.000	9999	-9999	1.15			
			1014	-7.724	0.000	-2.00	-2.00	1.50	9.99	15.08	1
					0.000	9999	-9999	1.15			
12129	0.000	9	1013	-2.193	-0.001	-3.50	3.49	1.50	6.97	15.08	1
					-0.630	-49	9999	1.15			
			1014	-0.325	0.000	-3.50	7.24	1.50	1.00	49.35	1
					-0.659	-171	9999	1.15			
	1.000	9	1013	-2.542	0.000	-3.50	2.88	1.50	7.67	15.08	1
					-0.656	-16	9999	1.15			
			1014	-0.342	0.000	-3.50	6.18	1.50	1.00	70.29	1
					-0.857	-146	9999	1.15			
12130	0.000	9	1013	-2.543	0.000	-3.50	2.88	1.50	7.67	15.08	1
					-0.657	-15	9999	1.15			
			1014	-0.342	0.000	-3.50	6.19	1.50	1.00	70.23	1
					-0.856	-147	9999	1.15			
	1.000	9	1013	-3.173	0.000	-3.50	2.04	1.50	9.11	15.08	1
					-0.676	43	-9999	1.15			
			1014	-0.359	0.000	-3.50	6.01	1.50	1.00	74.07	1
					-0.894	-142	9999	1.15			
12131	0.000	9	1013	-3.174	0.000	-3.50	2.04	1.50	9.11	15.08	1
					-0.676	43	-9999	1.15			
			1014	-0.359	0.000	-3.50	6.01	1.50	1.00	74.03	1
					-0.893	-142	9999	1.15			
	1.000	9	1013	-3.892	0.000	-3.50	1.31	1.50	9.99	15.08	1
					-0.662	110	9999	1.15			
			1014	-0.365	0.000	-3.50	6.29	1.50	1.00	65.97	1
					-0.821	-149	-9999	1.15			
12132	0.000	9	1013	-3.893	0.000	-3.50	1.31	1.50	9.99	15.08	1
					-0.662	110	9999	1.15			
			1014	-0.365	0.000	-3.50	6.29	1.50	1.00	65.94	1
					-0.821	-149	-9999	1.15			
	1.000	9	1013	-4.859	0.000	-3.50	0.55	1.50	9.99	15.08	1
					-0.591	205	9999	1.15			
			1014	-0.382	0.000	-3.50	6.89	1.50	1.00	52.06	1
					-0.695	-164	-9999	1.15			
12133	0.000	9	1013	-4.859	0.000	-3.50	0.55	1.50	9.99	15.08	1
					-0.591	205	9999	1.15			
			1014	-0.382	0.000	-3.50	6.89	1.50	1.00	52.04	1
					-0.695	-164	-9999	1.15			
	1.000	9	1013	-0.410	0.000	-3.50	7.89	1.50	1.00	36.21	1
					-0.547	-184	-9999	1.15			
			1014	-5.840	0.000	-3.50	-0.06	1.50	9.99	15.08	1
					-0.452	313	9999	1.15			
12134	0.000	9	1013	-0.410	0.000	-3.50	7.89	1.50	1.00	36.20	1
					-0.547	-184	-9999	1.15			
			1014	-5.840	0.000	-3.50	-0.06	1.50	9.99	15.08	1
					-0.452	313	9999	1.15			
	1.000	9	1013	-0.427	0.000	-3.50	9.62	1.50	1.00	21.25	1
					-0.397	-213	-9999	1.15			
			1014	-6.552	0.000	-3.47	-0.04	1.50	9.99	15.08	1
					-0.303	409	9999	1.15			
12135	0.000	9	1013	-0.427	0.000	-3.50	9.62	1.50	1.00	21.24	1
					-0.397	-213	-9999	1.15			
			1014	-6.552	0.000	-3.47	-0.04	1.50	9.99	15.08	1
					-0.303	409	9999	1.15			
	1.000	9	1013	-0.639	0.000	-3.50	9.16	1.50	1.44	15.08	1
					-0.380	-206	-9999	1.15			
			1014	-7.091	0.000	-3.06	-0.58	1.50	9.99	15.08	1
					-0.168	589	9999	1.15			
12136	0.000	9	1013	-0.639	0.000	-3.50	9.16	1.50	1.44	15.08	1
					-0.380	-206	-9999	1.15			
			1014	-7.091	0.000	-3.06	-0.58	1.50	9.99	15.08	1
					-0.168	589	9999	1.15			
	1.000	9	1013	-1.693	0.000	-3.50	4.63	1.50	3.68	15.08	1
					-0.572	-98	-9999	1.15			
			1014	-7.508	0.000	-2.60	-1.20	1.50	9.99	15.08	1
					-0.063	1090	9999	1.15			
12137	0.000	9	1013	-1.693	0.000	-3.50	4.63	1.50	3.67	15.08	1
					-0.572	-98	-9999	1.15			
			1014	-7.507	0.000	-2.60	-1.20	1.50	9.99	15.08	1
					-0.063	1089	9999	1.15			
	1.000	9	1013	-4.164	0.000	-3.50	1.08	1.50	8.72	15.08	1
					-0.648	136	9999	1.15			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Required Reinforcements

Beam	x[m]	NoS	LC	Ni [MN]	Myi/Mzi [MNm]	e1/yn [o/oo]	e2/zn / mm]	nue C/S	rel tra	As [cm2]	L
12137	1.000	9	1014	-7.708	0.000	-2.14	-1.82	1.50	9.99	15.08	1
					0.008	-4940	9999	1.15			
12138	0.000	9	1013	-4.162	0.000	-3.50	1.08	1.50	8.71	15.08	1
					-0.648	136	9999	1.15			
			1014	-7.708	0.000	-2.14	-1.82	1.50	9.99	15.08	1
					0.008	-4970	9999	1.15			
	1.000	9	1013	-6.764	0.000	-3.32	-0.23	1.50	9.99	15.08	1
					-0.250	461	9999	1.15			
			1014	-7.578	0.000	-2.48	-1.35	1.50	9.99	15.08	1
					0.045	-1358	9999	1.15			
12139	0.000	9	1013	-6.763	0.000	-3.33	-0.23	1.50	9.99	15.08	1
					-0.250	461	9999	1.15			
			1014	-7.578	0.000	-2.48	-1.35	1.50	9.99	15.08	1
					0.045	-1359	9999	1.15			
	1.000	9	1013	-6.884	0.000	-3.23	-0.35	1.50	9.99	15.08	1
					0.220	-498	9999	1.15			
			1014	-7.523	0.000	-2.57	-1.23	1.50	9.99	15.08	1
					0.059	-1135	9999	1.15			
12140	0.000	9	1013	-6.886	0.000	-3.23	-0.36	1.50	9.99	15.08	1
					0.219	-498	9999	1.15			
			1014	-7.523	0.000	-2.57	-1.23	1.50	9.99	15.08	1
					0.059	-1136	9999	1.15			
	1.000	9	1013	-6.128	0.000	-3.50	-0.22	1.50	9.99	15.08	1
					0.396	-347	9999	1.15			
			1014	-7.524	0.000	-2.57	-1.23	1.50	9.99	15.08	1
					0.059	-1137	9999	1.15			
12141	0.000	9	1013	-6.129	0.000	-3.50	-0.22	1.50	9.99	15.08	1
					0.396	-347	9999	1.15			
			1014	-7.524	0.000	-2.57	-1.23	1.50	9.99	15.08	1
					0.059	-1137	9999	1.15			
	1.000	9	1013	-5.925	0.000	-3.50	-0.11	1.50	9.99	15.08	1
					0.436	-322	9999	1.15			
			1014	-7.554	0.000	-2.53	-1.30	1.50	9.99	15.08	1
					0.051	-1247	9999	1.15			
12142	0.000	9	1013	-5.925	0.000	-3.50	-0.11	1.50	9.99	15.08	1
					0.436	-322	9999	1.15			
			1014	-7.554	0.000	-2.53	-1.30	1.50	9.99	15.08	1
					0.051	-1247	9999	1.15			
	1.000	9	1013	-6.023	0.000	-3.50	-0.16	1.50	9.99	15.08	1
					0.417	-334	-9999	1.15			
			1014	-7.589	0.000	-2.46	-1.38	1.50	9.99	15.08	1
					0.042	-1418	-9999	1.15			
12143	0.000	9	1013	-6.023	0.000	-3.50	-0.16	1.50	9.99	15.08	1
					0.417	-334	-9999	1.15			
			1014	-7.589	0.000	-2.46	-1.38	1.50	9.99	15.08	1
					0.042	-1418	-9999	1.15			
	1.000	9	1013	-6.307	0.000	-3.50	-0.32	1.50	9.99	15.08	1
					0.359	-371	-9999	1.15			
			1014	-7.635	0.000	-2.37	-1.50	1.50	9.99	15.08	1
					0.030	-1778	-9999	1.15			
12144	0.000	9	1013	-6.307	0.000	-3.50	-0.32	1.50	9.99	15.08	1
					0.359	-371	-9999	1.15			
			1014	-7.635	0.000	-2.37	-1.50	1.50	9.99	15.08	1
					0.030	-1778	-9999	1.15			
	1.000	9	1013	-6.638	0.000	-3.41	-0.11	1.50	9.99	15.08	1
					0.282	-427	-9999	1.15			
			1014	-7.673	0.000	-2.28	-1.63	1.50	9.99	15.08	1
					0.020	-2415	-9999	1.15			
12145	0.000	9	1013	-6.638	0.000	-3.41	-0.11	1.50	9.99	15.08	1
					0.282	-427	-9999	1.15			
			1014	-7.673	0.000	-2.28	-1.63	1.50	9.99	15.08	1
					0.020	-2415	-9999	1.15			
	1.000	9	1013	-7.699	0.000	-2.18	-1.75	1.50	9.99	15.08	1
					0.011	-3657	-9999	1.15			
			1014	-6.970	0.000	-3.17	-0.45	1.50	9.99	15.08	1
					0.198	-531	-9999	1.15			
12146	0.000	9	1013	-7.699	0.000	-2.18	-1.75	1.50	9.99	15.08	1
					0.011	-3656	-9999	1.15			
			1014	-6.970	0.000	-3.17	-0.45	1.50	9.99	15.08	1
					0.198	-531	-9999	1.15			
	1.000	9	1013	-7.712	0.000	-2.11	-1.85	1.50	9.99	15.08	1
					0.006	-6027	-9999	1.15			
			1014	-7.258	0.000	-2.90	-0.80	1.50	9.99	15.08	1
					0.125	-703	-9999	1.15			
12147	0.000	9	1013	-7.712	0.000	-2.11	-1.85	1.50	9.99	15.08	1
					0.006	-6026	-9999	1.15			
			1014	-7.258	0.000	-2.90	-0.80	1.50	9.99	15.08	1
					0.125	-703	-9999	1.15			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Required Reinforcements

Beam	x[m]	NoS	LC	Ni [MN]	Myi/Mzi [MNm]	e1/yn [o/oo]	e2/zn / mm]	nue C/S	rel tra	As [cm2]	L
12147	1.000	9	1013	-7.720	0.000	-2.06	-1.93	1.50	9.99	15.08	1
					0.003	-9999	-9999	1.15			
			1014	-7.487	0.000	-2.63	-1.16	1.50	9.99	15.08	1
					0.068	-1034	-9999	1.15			
12148	0.000	9	1013	-7.720	0.000	-2.06	-1.93	1.50	9.99	15.08	1
					0.003	-9999	-9999	1.15			
			1014	-7.487	0.000	-2.63	-1.16	1.50	9.99	15.08	1
					0.068	-1033	-9999	1.15			
	1.000	9	1013	-7.723	0.000	-2.02	-1.97	1.50	9.99	15.08	1
					0.001	-9999	-9999	1.15			
			1014	-7.643	0.000	-2.36	-1.53	1.50	9.99	15.08	1
					0.028	-1871	-9999	1.15			
12149	0.000	9	1013	-7.723	0.000	-2.02	-1.97	1.50	9.99	15.08	1
					0.001	-9999	-9999	1.15			
			1014	-7.642	0.000	-2.36	-1.53	1.50	9.99	15.08	1
					0.028	-1870	-9999	1.15			
	1.000	9	1013	-7.724	0.000	-2.00	-2.00	1.50	9.99	15.08	1
					0.000	-9999	-9999	1.15			
			1014	-7.711	0.000	-2.12	-1.84	1.50	9.99	15.08	1
					0.007	-5650	-9999	1.15			
12150	0.000	9	1013	-7.724	0.000	-2.00	-2.00	1.50	9.99	15.08	1
					0.000	-9999	-9999	1.15			
			1014	-7.711	0.000	-2.12	-1.84	1.50	9.99	15.08	1
					0.007	-5637	-9999	1.15			
	1.000	9	1013	-7.724	0.000	-2.00	-2.00	1.50	9.99	15.08	1
					0.000	9999	9999	1.15			
			1014	-7.724	0.000	-2.00	-2.00	1.50	9.99	15.08	1
					0.000	9999	9999	1.15			

Shear Design

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Design for shear DIN 1045-1 (2003)
Minimum shear factor or tan of inclination of compressive struts 0.57 / 1.72

MNo	f-cd [MPa]	tau-rd [MPa]	sigIIQ [MPa]	sigIIT [MPa]	sigIIQ+ [MPa]	fyd [MPa]
1	14.17	0.10	10.62	7.44	10.62	
3	14.17	0.10	10.62	7.44	10.62	
4	14.17	0.10	10.62	7.44	10.62	
5	14.17	0.10	10.62	7.44	10.62	
6	14.17	0.10	10.62	7.44	10.62	
7	14.17	0.10	10.62	7.44	10.62	
8	14.17	0.10	10.62	7.44	10.62	
9	14.17	0.10	10.62	7.44	10.62	
10	14.17	0.10	10.62	7.44	10.62	
12						434.78

Tolerance for exceeding maximum shear or principal compression stress 0.0200

Longitudinal Reinforcements LCR 501
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
10001	0.000	8	0.01	0.84		0.76T	0.04		0.04	
10001	0.200	8	0.01	0.67		0.67T				
10005	0.000	8	0.03	3.65		3.02T			0.63	
10005	0.200	8	0.03	3.84		3.39T	0.31		0.14	
10006	0.000	8	0.00	0.34		0.24T	0.10			
10006	0.400	8	0.00	0.33		0.23T	0.10			
10009	0.000	8	0.00	0.34		0.24T	0.10			
10009	0.400	8	0.00	0.33		0.23T	0.10			
10010	0.000	8	0.03	3.64		3.19T	0.31		0.14	
10010	0.200	8	0.03	3.84		3.21T			0.63	
10014	0.000	8	0.01	0.67		0.67T				
10014	0.200	8	0.01	0.84		0.75T	0.04		0.04	
10016	0.000	8	0.01	0.80		0.71T	0.04		0.04	
10016	0.200	8	0.01	0.67		0.67T				
10020	0.000	8	0.03	3.84		3.21T			0.63	
10020	0.200	8	0.03	3.64		3.19T	0.31		0.14	
10021	0.000	8	0.00	0.38		0.23T	0.15			
10021	0.400	8	0.00	0.34		0.24T	0.10			
10024	0.000	8	0.00	0.34		0.24T	0.10			
10024	0.400	8	0.00	0.34		0.24T	0.10			
10025	0.000	8	0.03	3.84		3.39T	0.31		0.14	
10025	0.200	8	0.03	3.65		3.02T			0.63	
10029	0.000	8	0.01	0.67		0.67T				
10029	0.200	8	0.01	0.85		0.76T	0.04		0.04	
12001	0.000	9	0.30	15.08			15.08T			
12001	1.000	9	0.30	15.08			15.08T			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Longitudinal Reinforcements LCR 501
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
12002	0.000	9	0.30	15.08			15.08T			
12002	1.000	9	0.48	24.24			24.24T			
12003	0.000	9	0.48	24.24			24.24T			
12003	1.000	9	0.98	49.36			49.36T			
12004	0.000	9	0.30	15.08			15.08T			
12004	1.000	9	0.30	15.08			15.08T			
12005	0.000	9	0.30	15.08			15.08T			
12005	1.000	9	0.49	24.68			24.68T			
12006	0.000	9	0.49	24.68			24.68T			
12006	1.000	9	0.97	48.95			48.95T			
12007	0.000	9	0.30	15.08			15.08T			
12007	1.000	9	0.30	15.08			15.08T			
12008	0.000	9	0.30	15.08			15.08T			
12008	1.000	9	0.49	24.60			24.60T			
12009	0.000	9	0.49	24.60			24.60T			
12009	1.000	9	0.97	48.90			48.90T			
12010	0.000	9	0.30	15.08			15.08T			
12010	1.000	9	0.30	15.08			15.08T			
12011	0.000	9	0.30	15.08			15.08T			
12011	1.000	9	0.48	24.24			24.24T			
12012	0.000	9	0.48	24.24			24.24T			
12012	1.000	9	0.97	48.90			48.90T			
12013	0.000	9	0.30	15.08			15.08T			
12013	1.000	9	0.30	15.08			15.08T			
12014	0.000	9	0.30	15.08			15.08T			
12014	1.000	9	0.48	24.31			24.31T			
12015	0.000	9	0.48	24.31			24.31T			
12015	1.000	9	0.97	48.95			48.95T			
12016	0.000	9	0.30	15.08			15.08T			
12016	1.000	9	0.30	15.08			15.08T			
12017	0.000	9	0.30	15.08			15.08T			
12017	1.000	9	0.49	24.60			24.60T			
12018	0.000	9	0.49	24.60			24.60T			
12018	1.000	9	0.98	49.36			49.36T			
12019	0.000	9	0.98	49.36			49.36T			
12019	1.000	9	1.40	70.29			70.29T			
12020	0.000	9	1.40	70.23			70.23T			
12020	1.000	9	1.47	74.07			74.07T			
12021	0.000	9	1.47	74.03			74.03T			
12021	1.000	9	1.31	65.97			65.97T			
12022	0.000	9	1.31	65.94			65.94T			
12022	1.000	9	1.04	52.06			52.06T			
12023	0.000	9	1.04	52.04			52.04T			
12023	1.000	9	0.72	36.21			36.21T			
12024	0.000	9	0.72	36.20			36.20T			
12024	1.000	9	0.42	21.35			21.35T			
12025	0.000	9	0.42	21.35			21.35T			
12025	1.000	9	0.30	15.08			15.08T			
12026	0.000	9	0.30	15.08			15.08T			
12026	1.000	9	0.30	15.08			15.08T			
12027	0.000	9	0.30	15.08			15.08T			
12027	1.000	9	0.30	15.08			15.08T			
12028	0.000	9	0.30	15.08			15.08T			
12028	1.000	9	0.30	15.08			15.08T			
12029	0.000	9	0.30	15.08			15.08T			
12029	1.000	9	0.30	15.08			15.08T			
12030	0.000	9	0.30	15.08			15.08T			
12030	1.000	9	0.30	15.08			15.08T			
12031	0.000	9	0.30	15.08			15.08T			
12031	1.000	9	0.30	15.08			15.08T			
12032	0.000	9	0.30	15.08			15.08T			
12032	1.000	9	0.30	15.08			15.08T			
12033	0.000	9	0.30	15.08			15.08T			
12033	1.000	9	0.30	15.08			15.08T			
12034	0.000	9	0.30	15.08			15.08T			
12034	1.000	9	0.30	15.08			15.08T			
12035	0.000	9	0.30	15.08			15.08T			
12035	1.000	9	0.30	15.08			15.08T			
12036	0.000	9	0.30	15.08			15.08T			
12036	1.000	9	0.30	15.08			15.08T			
12037	0.000	9	0.30	15.08			15.08T			
12037	1.000	9	0.30	15.08			15.08T			
12038	0.000	9	0.30	15.08			15.08T			
12038	1.000	9	0.30	15.08			15.08T			
12039	0.000	9	0.30	15.08			15.08T			
12039	1.000	9	0.30	15.08			15.08T			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Longitudinal Reinforcements LCR 501
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
12040	0.000	9	0.30	15.08			15.08'			
12040	1.000	9	0.30	15.08			15.08'			
12041	0.000	9	0.97	48.95			48.95T			
12041	1.000	9	1.39	69.85			69.85T			
12042	0.000	9	1.39	69.79			69.79T			
12042	1.000	9	1.47	73.69			73.69T			
12043	0.000	9	1.47	73.65			73.65T			
12043	1.000	9	1.31	65.97			65.97T			
12044	0.000	9	1.31	65.94			65.94T			
12044	1.000	9	1.04	52.05			52.05T			
12045	0.000	9	1.04	52.03			52.03T			
12045	1.000	9	0.72	36.19			36.19T			
12046	0.000	9	0.72	36.19			36.19T			
12046	1.000	9	0.42	21.33			21.33T			
12047	0.000	9	0.42	21.33			21.33T			
12047	1.000	9	0.30	15.08			15.08T			
12048	0.000	9	0.30	15.08			15.08T			
12048	1.000	9	0.30	15.08			15.08T			
12049	0.000	9	0.30	15.08			15.08T			
12049	1.000	9	0.30	15.08			15.08T			
12050	0.000	9	0.30	15.08			15.08T			
12050	1.000	9	0.30	15.08			15.08T			
12051	0.000	9	0.30	15.08			15.08T			
12051	1.000	9	0.30	15.08			15.08T			
12052	0.000	9	0.30	15.08			15.08T			
12052	1.000	9	0.30	15.08			15.08T			
12053	0.000	9	0.30	15.08			15.08T			
12053	1.000	9	0.30	15.08			15.08T			
12054	0.000	9	0.30	15.08			15.08T			
12054	1.000	9	0.30	15.08			15.08T			
12055	0.000	9	0.30	15.08			15.08T			
12055	1.000	9	0.30	15.08			15.08T			
12056	0.000	9	0.30	15.08			15.08T			
12056	1.000	9	0.30	15.08			15.08T			
12057	0.000	9	0.30	15.08			15.08T			
12057	1.000	9	0.30	15.08			15.08T			
12058	0.000	9	0.30	15.08			15.08T			
12058	1.000	9	0.30	15.08			15.08T			
12059	0.000	9	0.30	15.08			15.08T			
12059	1.000	9	0.30	15.08			15.08T			
12060	0.000	9	0.30	15.08			15.08T			
12060	1.000	9	0.30	15.08			15.08T			
12061	0.000	9	0.30	15.08			15.08T			
12061	1.000	9	0.30	15.08			15.08T			
12062	0.000	9	0.30	15.08			15.08'			
12062	1.000	9	0.30	15.08			15.08'			
12063	0.000	9	0.97	48.90			48.90T			
12063	1.000	9	1.39	69.82			69.82T			
12064	0.000	9	1.39	69.76			69.76T			
12064	1.000	9	1.47	73.68			73.68T			
12065	0.000	9	1.46	73.63			73.63T			
12065	1.000	9	1.32	66.24			66.24T			
12066	0.000	9	1.32	66.21			66.21T			
12066	1.000	9	1.04	52.19			52.19T			
12067	0.000	9	1.04	52.17			52.17T			
12067	1.000	9	0.72	36.21			36.21T			
12068	0.000	9	0.72	36.20			36.20T			
12068	1.000	9	0.42	21.25			21.25T			
12069	0.000	9	0.42	21.24			21.24T			
12069	1.000	9	0.30	15.08			15.08T			
12070	0.000	9	0.30	15.08			15.08T			
12070	1.000	9	0.30	15.08			15.08T			
12071	0.000	9	0.30	15.08			15.08T			
12071	1.000	9	0.30	15.08			15.08T			
12072	0.000	9	0.30	15.08			15.08T			
12072	1.000	9	0.30	15.08			15.08T			
12073	0.000	9	0.30	15.08			15.08T			
12073	1.000	9	0.30	15.08			15.08T			
12074	0.000	9	0.30	15.08			15.08T			
12074	1.000	9	0.30	15.08			15.08T			
12075	0.000	9	0.30	15.08			15.08T			
12075	1.000	9	0.30	15.08			15.08T			
12076	0.000	9	0.30	15.08			15.08T			
12076	1.000	9	0.30	15.08			15.08T			
12077	0.000	9	0.30	15.08			15.08T			
12077	1.000	9	0.30	15.08			15.08T			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Longitudinal Reinforcements LCR 501
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
12078	0.000	9	0.30	15.08			15.08T			
12078	1.000	9	0.30	15.08			15.08T			
12079	0.000	9	0.30	15.08			15.08T			
12079	1.000	9	0.30	15.08			15.08T			
12080	0.000	9	0.30	15.08			15.08T			
12080	1.000	9	0.30	15.08			15.08T			
12081	0.000	9	0.30	15.08			15.08T			
12081	1.000	9	0.30	15.08			15.08T			
12082	0.000	9	0.30	15.08			15.08T			
12082	1.000	9	0.30	15.08			15.08T			
12083	0.000	9	0.30	15.08			15.08T			
12083	1.000	9	0.30	15.08			15.08T			
12084	0.000	9	0.30	15.08			15.08'			
12084	1.000	9	0.30	15.08			15.08'			
12085	0.000	9	0.97	48.90			48.90T			
12085	1.000	9	1.39	69.82			69.82T			
12086	0.000	9	1.39	69.76			69.76T			
12086	1.000	9	1.47	73.68			73.68T			
12087	0.000	9	1.46	73.63			73.63T			
12087	1.000	9	1.32	66.24			66.24T			
12088	0.000	9	1.32	66.21			66.21T			
12088	1.000	9	1.04	52.06			52.06T			
12089	0.000	9	1.04	52.04			52.04T			
12089	1.000	9	0.72	36.21			36.21T			
12090	0.000	9	0.72	36.20			36.20T			
12090	1.000	9	0.42	21.35			21.35T			
12091	0.000	9	0.42	21.35			21.35T			
12091	1.000	9	0.30	15.08			15.08T			
12092	0.000	9	0.30	15.08			15.08T			
12092	1.000	9	0.30	15.08			15.08T			
12093	0.000	9	0.30	15.08			15.08T			
12093	1.000	9	0.30	15.08			15.08T			
12094	0.000	9	0.30	15.08			15.08T			
12094	1.000	9	0.30	15.08			15.08T			
12095	0.000	9	0.30	15.08			15.08T			
12095	1.000	9	0.30	15.08			15.08T			
12096	0.000	9	0.30	15.08			15.08T			
12096	1.000	9	0.30	15.08			15.08T			
12097	0.000	9	0.30	15.08			15.08T			
12097	1.000	9	0.30	15.08			15.08T			
12098	0.000	9	0.30	15.08			15.08T			
12098	1.000	9	0.30	15.08			15.08T			
12099	0.000	9	0.30	15.08			15.08T			
12099	1.000	9	0.30	15.08			15.08T			
12100	0.000	9	0.30	15.08			15.08T			
12100	1.000	9	0.30	15.08			15.08T			
12101	0.000	9	0.30	15.08			15.08T			
12101	1.000	9	0.30	15.08			15.08T			
12102	0.000	9	0.30	15.08			15.08T			
12102	1.000	9	0.30	15.08			15.08T			
12103	0.000	9	0.30	15.08			15.08T			
12103	1.000	9	0.30	15.08			15.08T			
12104	0.000	9	0.30	15.08			15.08T			
12104	1.000	9	0.30	15.08			15.08T			
12105	0.000	9	0.30	15.08			15.08T			
12105	1.000	9	0.30	15.08			15.08T			
12106	0.000	9	0.30	15.08			15.08'			
12106	1.000	9	0.30	15.08			15.08'			
12107	0.000	9	0.97	48.95			48.95T			
12107	1.000	9	1.39	69.85			69.85T			
12108	0.000	9	1.39	69.79			69.79T			
12108	1.000	9	1.47	73.69			73.69T			
12109	0.000	9	1.47	73.65			73.65T			
12109	1.000	9	1.32	66.23			66.23T			
12110	0.000	9	1.32	66.21			66.21T			
12110	1.000	9	1.04	52.17			52.17T			
12111	0.000	9	1.04	52.16			52.16T			
12111	1.000	9	0.72	36.19			36.19T			
12112	0.000	9	0.72	36.19			36.19T			
12112	1.000	9	0.42	21.23			21.23T			
12113	0.000	9	0.42	21.23			21.23T			
12113	1.000	9	0.30	15.08			15.08T			
12114	0.000	9	0.30	15.08			15.08T			
12114	1.000	9	0.30	15.08			15.08T			
12115	0.000	9	0.30	15.08			15.08T			
12115	1.000	9	0.30	15.08			15.08T			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Longitudinal Reinforcements LCR 501
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
12116	0.000	9	0.30	15.08			15.08T			
12116	1.000	9	0.30	15.08			15.08T			
12117	0.000	9	0.30	15.08			15.08T			
12117	1.000	9	0.30	15.08			15.08T			
12118	0.000	9	0.30	15.08			15.08T			
12118	1.000	9	0.30	15.08			15.08T			
12119	0.000	9	0.30	15.08			15.08T			
12119	1.000	9	0.30	15.08			15.08T			
12120	0.000	9	0.30	15.08			15.08T			
12120	1.000	9	0.30	15.08			15.08T			
12121	0.000	9	0.30	15.08			15.08T			
12121	1.000	9	0.30	15.08			15.08T			
12122	0.000	9	0.30	15.08			15.08T			
12122	1.000	9	0.30	15.08			15.08T			
12123	0.000	9	0.30	15.08			15.08T			
12123	1.000	9	0.30	15.08			15.08T			
12124	0.000	9	0.30	15.08			15.08T			
12124	1.000	9	0.30	15.08			15.08T			
12125	0.000	9	0.30	15.08			15.08T			
12125	1.000	9	0.30	15.08			15.08T			
12126	0.000	9	0.30	15.08			15.08T			
12126	1.000	9	0.30	15.08			15.08T			
12127	0.000	9	0.30	15.08			15.08T			
12127	1.000	9	0.30	15.08			15.08T			
12128	0.000	9	0.30	15.08			15.08'			
12128	1.000	9	0.30	15.08			15.08'			
12129	0.000	9	0.98	49.36			49.36T			
12129	1.000	9	1.40	70.29			70.29T			
12130	0.000	9	1.40	70.23			70.23T			
12130	1.000	9	1.47	74.07			74.07T			
12131	0.000	9	1.47	74.03			74.03T			
12131	1.000	9	1.31	65.97			65.97T			
12132	0.000	9	1.31	65.94			65.94T			
12132	1.000	9	1.04	52.06			52.06T			
12133	0.000	9	1.04	52.04			52.04T			
12133	1.000	9	0.72	36.21			36.21T			
12134	0.000	9	0.72	36.20			36.20T			
12134	1.000	9	0.42	21.25			21.25T			
12135	0.000	9	0.42	21.24			21.24T			
12135	1.000	9	0.30	15.08			15.08T			
12136	0.000	9	0.30	15.08			15.08T			
12136	1.000	9	0.30	15.08			15.08T			
12137	0.000	9	0.30	15.08			15.08T			
12137	1.000	9	0.30	15.08			15.08T			
12138	0.000	9	0.30	15.08			15.08T			
12138	1.000	9	0.30	15.08			15.08T			
12139	0.000	9	0.30	15.08			15.08T			
12139	1.000	9	0.30	15.08			15.08T			
12140	0.000	9	0.30	15.08			15.08T			
12140	1.000	9	0.30	15.08			15.08T			
12141	0.000	9	0.30	15.08			15.08T			
12141	1.000	9	0.30	15.08			15.08T			
12142	0.000	9	0.30	15.08			15.08T			
12142	1.000	9	0.30	15.08			15.08T			
12143	0.000	9	0.30	15.08			15.08T			
12143	1.000	9	0.30	15.08			15.08T			
12144	0.000	9	0.30	15.08			15.08T			
12144	1.000	9	0.30	15.08			15.08T			
12145	0.000	9	0.30	15.08			15.08T			
12145	1.000	9	0.30	15.08			15.08T			
12146	0.000	9	0.30	15.08			15.08T			
12146	1.000	9	0.30	15.08			15.08T			
12147	0.000	9	0.30	15.08			15.08T			
12147	1.000	9	0.30	15.08			15.08T			
12148	0.000	9	0.30	15.08			15.08T			
12148	1.000	9	0.30	15.08			15.08T			
12149	0.000	9	0.30	15.08			15.08T			
12149	1.000	9	0.30	15.08			15.08T			
12150	0.000	9	0.30	15.08			15.08'			
12150	1.000	9	0.30	15.08			15.08'			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Maximum Degree of Utilization

		N sig-c	Vy sig-t	Vz tau	Mt sig-*	My tend.	Mz As-l	Mb As-v	Mt2 crack	Total sigdyn	lamda tau-*
Cross sect.	8	0.000	0.000	0.018	0.032	0.000	0.000	0.000	0.000	1.001	0.000
DOKOS-4		0.000	0.000	0.000	0.000	0.000	1.001	0.000	0.000	0.000	0.000
Cross sect.	9	0.000	0.000	0.248	0.000	0.000	0.000	0.000	0.000	1.000	0.000
section pile		0.000	0.000	0.000	0.000	0.000	1.000	0.000	0.000	0.000	0.000
Total System		0.000	0.000	0.248	0.032	0.000	0.000	0.000	0.000	1.001	0.000
		0.000	0.000	0.000	0.000	0.000	1.001	0.000	0.000	0.000	0.000

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_ULS_DOKOI

Selected Beam Elements

FROM	TO	INC	X-VALUE	NC	MEMBER	CS0	CS1	CS2	CS3	CS4	CS5
1000	1060	1		1	bending	10	40				

Default design code is DIN Fachbericht 102 Massivbrȯcken (2003) (Germany)
Klasse(Tab.4.118): D
Wind zone : Binnenland

Materials

No. 1 C 25/30 (DIN 1045-1)
No. 3 C 25/30 (DIN 1045-1)
No. 4 C 25/30 (DIN 1045-1)
No. 5 C 25/30 (DIN 1045-1)
No. 6 C 25/30 (DIN 1045-1)
No. 7 C 25/30 (DIN 1045-1)
No. 8 C 25/30 (DIN 1045-1)
No. 9 C 25/30 (DIN 1045-1)
No. 10 C 25/30 (DIN 1045-1)
No. 12 BSt 500 SA (DIN 1045-1)

All moments will be smoothed out between face and support
Reinforcement will be accounted for sectional values as defined in AQUA
Reinforcements saved as design case LCR 502
Reinforcements are superposed with existing minimum reinforcements

Considered Load Cases

No.	refer	act on	Title/type of load case	gam-u	gam-f	psi-0	psi-1	psi-2	psi-1'	
1	part.	CS 0	I.B. ΚΑΤΑΚ.ΣΤΟΙΧΕΙΩΝ	1.35	1.00	1.00	1.00	1.00	1.00	G perm
			G (total dead load)							
2	part.	CS 0	I.B. ΔΟΚΩΝ	1.35	1.00	1.00	1.00	1.00	1.00	G perm
			G (total dead load)							
3	part.	CS 0	I.B. ΧΥΤΗΣ ΠΛΑΚΑΣ	1.35	1.00	1.00	1.00	1.00	1.00	G perm
			G (total dead load)							
4	part.	CS 0	ΚΙΝΗΤΟ ΦΑΣΗΣ-1	1.50	0.00	0.70	0.50	0.30	0.70	Q cond
			Q_A (Pay load residential cat. A)							
7	part.	CS 0	ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝ Α1-Κ0 (Φ	1.50	0.00	0.70	0.50	0.30	0.70	Q cond
			Q_B (Pay load offices cat. B)							
8	part.	CS 0	ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝ Α2-Κ0 (Φ	1.50	0.00	0.70	0.50	0.30	0.70	Q cond
			Q_B (Pay load offices cat. B)							
11	part.	CS 0	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*Α1+0.5	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
12	part.	CS 0	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*Α1+0.5	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
13	part.	CS 0	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*Α1+1.0	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
14	part.	CS 0	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*Α1+1.0	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
5015	part.	CS 0	K creep step	1.00	1.00	1.00	1.00	1.00	1.00	P perm
			C (creep + shrinkage)							
5025	part.	CS 0	K creep step	1.00	1.00	1.00	1.00	1.00	1.00	P perm
			C (creep + shrinkage)							
6015	part.	CS 0	15 K creep step							P perm
			C (creep + shrinkage)							
6025	part.	CS 0	25 K creep step							P perm
			C (creep + shrinkage)							

Ultimate Load Design

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Design for ultimate loads DIN Fachbericht 102 Massivbrȯcken (2003)
Uniaxial bending due to symmetry

Safety factors	SC-1	SC-2	SC-S	SS-1	SS-2	PIIa
	1.50	1.50	1.50	1.15	1.15	7

Strain limits	C1	C2	S1	S2	Z1	Z2
max	-3.50	-2.00	3.00	25.00	-3.50	25.00

parameters for reinforcements

Minimum reinforcements		compression		min. reinforcem.	maximum-
Bending.	Compress.	e/d	N/Npl	requ. section	reinforc.
0.00 [cm2]	0.30 [o/o]	3.50	0.0010	0.00	0.15	9.00

Tensile forces in the longitudinal reinforcements due to shear are NOT accounted for.
Material of sections uses Ultimate Limit strain-stress law with global safety factors
Material of reinforcements uses Ultimate Limit strain-stress law with global safety factors

MNo.	temp lev.	Material-safety [-]	max.compr stress [MPa]	at strain [o/oo]	max.tens stress [MPa]	at strain [o/oo]	tension-stiffening [MPa]
1	0	1.500	-14.17	-2.00	0.00	0.00	
3	0	1.500	-14.17	-2.00	0.00	0.00	
4	0	1.500	-14.17	-2.00	0.00	0.00	
5	0	1.500	-14.17	-2.00	0.00	0.00	
6	0	1.500	-14.17	-2.00	0.00	0.00	
7	0	1.500	-14.17	-2.00	0.00	0.00	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
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MNo.	temp lev.	Material-safety [-]	max.compr stress [MPa]	at strain [o/oo]	max.tens stress [MPa]	at strain [o/oo]	tension-stiffening [MPa]
8	0	1.500	-14.17	-2.00	0.00	0.00	
9	0	1.500	-14.17	-2.00	0.00	0.00	
10	0	1.500	-14.17	-2.00	0.00	0.00	
12	0	1.150	-456.52	-25.00	456.52	25.00	

Combinations For Ultimate Design

```
 1015 ( CS 1  ) max_my-1015
       MAX  +  MY   :
               1.35 * G + 1.50 * Q_A + 1.50 * L_A + 1.35 * Q_B + 1.00 * C
 1016 ( CS 1  ) min_my-1016
       MIN  +  MY   :
               1.35 * G + 1.50 * Q_A + 1.50 * L_A + 1.35 * Q_B + 1.00 * C
```

Shear Design

==========

Design for shear DIN 1045-1 (2003)
Minimum shear factor or tan of inclination of compressive struts 0.57 / 1.72

MNo	f-cd [MPa]	tau-rd [MPa]	sigIIQ [MPa]	sigIIT [MPa]	sigIIQ+ [MPa]	fyd [MPa]
1	14.17	0.10	10.62	7.44	10.62	
3	14.17	0.10	10.62	7.44	10.62	
4	14.17	0.10	10.62	7.44	10.62	
5	14.17	0.10	10.62	7.44	10.62	
6	14.17	0.10	10.62	7.44	10.62	
7	14.17	0.10	10.62	7.44	10.62	
8	14.17	0.10	10.62	7.44	10.62	
9	14.17	0.10	10.62	7.44	10.62	
10	14.17	0.10	10.62	7.44	10.62	
12						434.78

Tolerance for exceeding maximum shear or principal compression stress 0.0200

Longitudinal Reinforcements Accumulated minimum
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
1001	0.000	1	0.05	2.37		2.37				
1001	0.883	1	0.10	4.89		2.37	2.52			
1002	0.000	1	0.10	4.89		2.37	2.52			
1002	0.883	1	0.14	7.04		2.37	4.67			
1003	0.000	1	0.14	7.04		2.37	4.67			
1003	0.883	1	0.18	8.84		2.37	6.47			
1004	0.000	1	0.18	8.84		2.37	6.47			
1004	0.883	1	0.21	10.21		2.37	7.83			
1005	0.000	1	0.21	10.21		2.37	7.83			
1005	0.883	1	0.22	11.04		2.37	8.67			
1006	0.000	1	0.22	11.04		2.37	8.67			
1006	0.883	1	0.23	11.33		2.37	8.95			
1007	0.000	1	0.23	11.33		2.37	8.95			
1007	0.883	1	0.22	11.04		2.37	8.67			
1008	0.000	1	0.22	11.04		2.37	8.67			
1008	0.883	1	0.21	10.21		2.37	7.83			
1009	0.000	1	0.21	10.21		2.37	7.83			
1009	0.883	1	0.18	8.84		2.37	6.47			
1010	0.000	1	0.18	8.84		2.37	6.47			
1010	0.883	1	0.14	7.04		2.37	4.67			
1011	0.000	1	0.14	7.04		2.37	4.67			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_ULS_DOKOI

Longitudinal Reinforcements Accumulated minimum
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
1011	0.883	1	0.10	4.89		2.37	2.52			
1012	0.000	1	0.10	4.89		2.37'	2.52			
1012	0.883	1	0.05	2.37		2.37				
1013	0.000	1	0.05	2.37		2.37				
1013	0.883	1	0.10	4.89		2.37	2.52			
1014	0.000	1	0.10	4.89		2.37	2.52			
1014	0.883	1	0.14	7.04		2.37	4.67			
1015	0.000	1	0.14	7.04		2.37	4.67			
1015	0.883	1	0.18	8.84		2.37	6.47			
1016	0.000	1	0.18	8.84		2.37	6.47			
1016	0.883	1	0.21	10.21		2.37	7.83			
1017	0.000	1	0.21	10.21		2.37	7.83			
1017	0.883	1	0.22	11.04		2.37	8.67			
1018	0.000	1	0.22	11.04		2.37	8.67			
1018	0.883	1	0.23	11.33		2.37	8.95			
1019	0.000	1	0.23	11.33		2.37	8.95			
1019	0.883	1	0.22	11.04		2.37	8.67			
1020	0.000	1	0.22	11.04		2.37	8.67			
1020	0.883	1	0.21	10.21		2.37	7.83			
1021	0.000	1	0.21	10.21		2.37	7.83			
1021	0.883	1	0.18	8.84		2.37	6.47			
1022	0.000	1	0.18	8.84		2.37	6.47			
1022	0.883	1	0.14	7.04		2.37	4.67			
1023	0.000	1	0.14	7.04		2.37	4.67			
1023	0.883	1	0.10	4.89		2.37	2.52			
1024	0.000	1	0.10	4.89		2.37'	2.52			
1024	0.883	1	0.05	2.37		2.37				
1025	0.000	1	0.05	2.37		2.37				
1025	0.883	1	0.10	4.89		2.37	2.52			
1026	0.000	1	0.10	4.89		2.37	2.52			
1026	0.883	1	0.14	7.04		2.37	4.67			
1027	0.000	1	0.14	7.04		2.37	4.67			
1027	0.883	1	0.18	8.84		2.37	6.47			
1028	0.000	1	0.18	8.84		2.37	6.47			
1028	0.883	1	0.21	10.21		2.37	7.83			
1029	0.000	1	0.21	10.21		2.37	7.83			
1029	0.883	1	0.22	11.04		2.37	8.67			
1030	0.000	1	0.22	11.04		2.37	8.67			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_ULS_DOKOI

Longitudinal Reinforcements Accumulated minimum
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
1030	0.883	1	0.23	11.33		2.37	8.95			
1031	0.000	1	0.23	11.33		2.37	8.95			
1031	0.883	1	0.22	11.04		2.37	8.67			
1032	0.000	1	0.22	11.04		2.37	8.67			
1032	0.883	1	0.21	10.21		2.37	7.83			
1033	0.000	1	0.21	10.21		2.37	7.83			
1033	0.883	1	0.18	8.84		2.37	6.47			
1034	0.000	1	0.18	8.84		2.37	6.47			
1034	0.883	1	0.14	7.04		2.37	4.67			
1035	0.000	1	0.14	7.04		2.37	4.67			
1035	0.883	1	0.10	4.89		2.37	2.52			
1036	0.000	1	0.10	4.89		2.37'	2.52			
1036	0.883	1	0.05	2.37		2.37				
1037	0.000	1	0.05	2.37		2.37				
1037	0.883	1	0.10	4.89		2.37	2.52			
1038	0.000	1	0.10	4.89		2.37	2.52			
1038	0.883	1	0.14	7.04		2.37	4.67			
1039	0.000	1	0.14	7.04		2.37	4.67			
1039	0.883	1	0.18	8.84		2.37	6.47			
1040	0.000	1	0.18	8.84		2.37	6.47			
1040	0.883	1	0.21	10.21		2.37	7.83			
1041	0.000	1	0.21	10.21		2.37	7.83			
1041	0.883	1	0.22	11.04		2.37	8.67			
1042	0.000	1	0.22	11.04		2.37	8.67			
1042	0.883	1	0.23	11.33		2.37	8.95			
1043	0.000	1	0.23	11.33		2.37	8.95			
1043	0.883	1	0.22	11.04		2.37	8.67			
1044	0.000	1	0.22	11.04		2.37	8.67			
1044	0.883	1	0.21	10.21		2.37	7.83			
1045	0.000	1	0.21	10.21		2.37	7.83			
1045	0.883	1	0.18	8.84		2.37	6.47			
1046	0.000	1	0.18	8.84		2.37	6.47			
1046	0.883	1	0.14	7.04		2.37	4.67			
1047	0.000	1	0.14	7.04		2.37	4.67			
1047	0.883	1	0.10	4.89		2.37	2.52			
1048	0.000	1	0.10	4.89		2.37'	2.52			
1048	0.883	1	0.05	2.37		2.37				
1049	0.000	1	0.05	2.37		2.37				

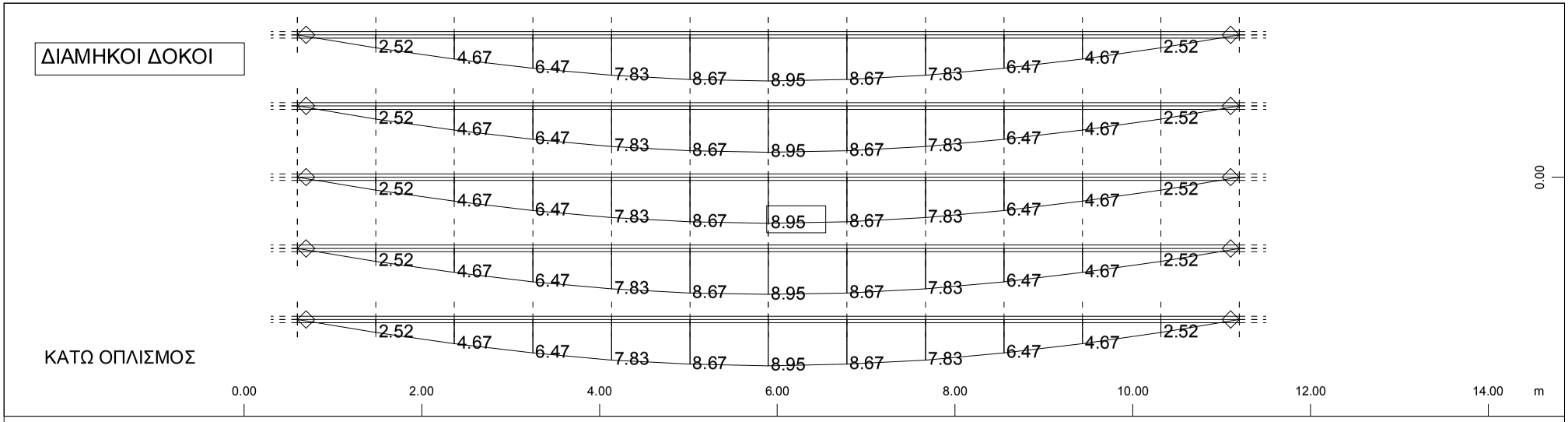
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_ULS_DOKOI

Longitudinal Reinforcements Accumulated minimum
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
1049	0.883	1	0.10	4.89		2.37	2.52			
1050	0.000	1	0.10	4.89		2.37	2.52			
1050	0.883	1	0.14	7.04		2.37	4.67			
1051	0.000	1	0.14	7.04		2.37	4.67			
1051	0.883	1	0.18	8.84		2.37	6.47			
1052	0.000	1	0.18	8.84		2.37	6.47			
1052	0.883	1	0.21	10.21		2.37	7.83			
1053	0.000	1	0.21	10.21		2.37	7.83			
1053	0.883	1	0.22	11.04		2.37	8.67			
1054	0.000	1	0.22	11.04		2.37	8.67			
1054	0.883	1	0.23	11.33		2.37	8.95			
1055	0.000	1	0.23	11.33		2.37	8.95			
1055	0.883	1	0.22	11.04		2.37	8.67			
1056	0.000	1	0.22	11.04		2.37	8.67			
1056	0.883	1	0.21	10.21		2.37	7.83			
1057	0.000	1	0.21	10.21		2.37	7.83			
1057	0.883	1	0.18	8.84		2.37	6.47			
1058	0.000	1	0.18	8.84		2.37	6.47			
1058	0.883	1	0.14	7.04		2.37	4.67			
1059	0.000	1	0.14	7.04		2.37	4.67			
1059	0.883	1	0.10	4.89		2.37	2.52			
1060	0.000	1	0.10	4.89		2.37'	2.52			
1060	0.883	1	0.05	2.37		2.37				

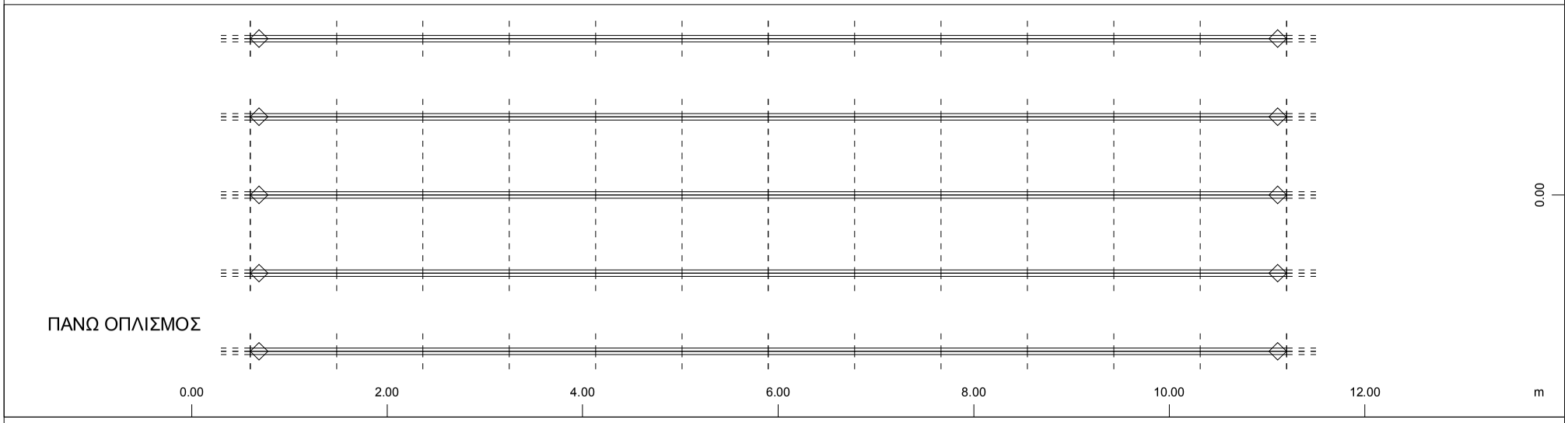
Maximum Degree of Utilization

		N sig-c	Vy sig-t	Vz tau	Mt sig-*	My tend.	Mz As-l	Mb As-v	Mt2 crack	Total sigdyn	lamda tau-*
Cross sect.	1	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	1.000	0.000
		0.000	0.000	0.000	0.000	0.000	1.000	0.000	0.000	0.000	0.000
Total System		0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	1.000	0.000
		0.000	0.000	0.000	0.000	0.000	1.000	0.000	0.000	0.000	0.000



Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case 502 , 1 cm 3D = 11.2 cm2 (Max=8.95)

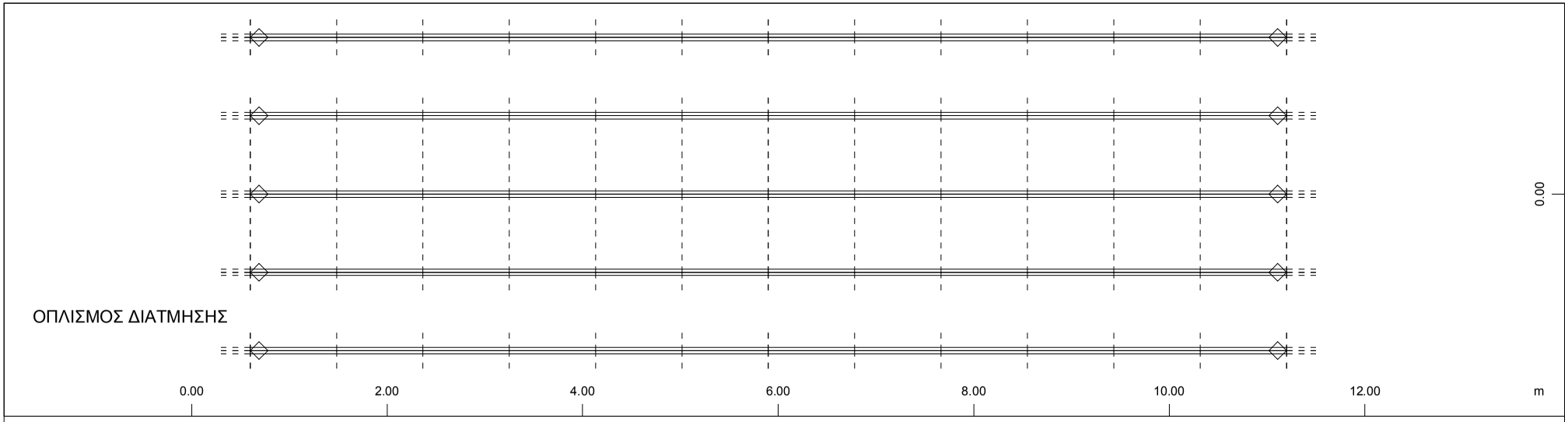
M 1 : 66



Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case 502 , 1 cm 3D = 0 cm2 (Max=0)

M 1 : 60

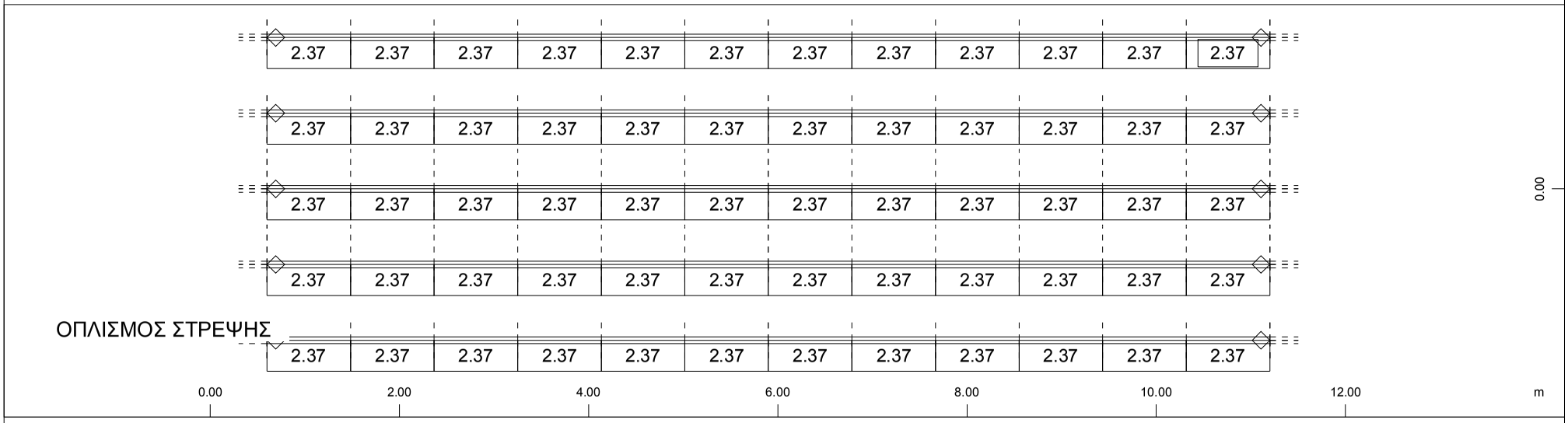
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΦΑΣΗ-1_ULS



Z X
Y

Sector of system Beam Elements Group 1
Beam Elements , Shear reinforcements (maximum), Design Case 502 , 1 cm 3D = 0 cm2/m (Max=0)

M 1 : 60



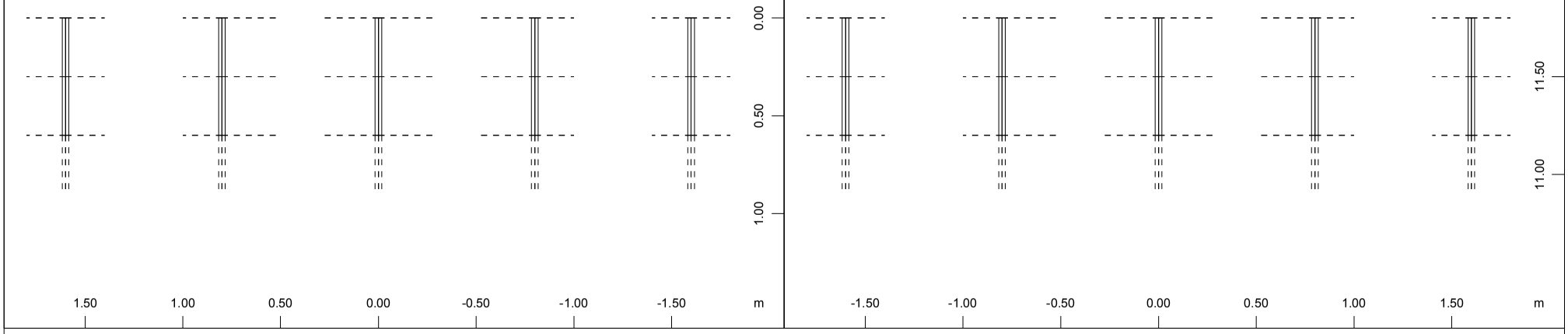
Z X
Y

Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case 502 , 1 cm 3D = 4.48 cm2 (Max=2.37)

M 1 : 62

ΔΙΑΜΗΚΗ ΔΟΚΟΙ-ΣΤΗΡΙΞΗΣ

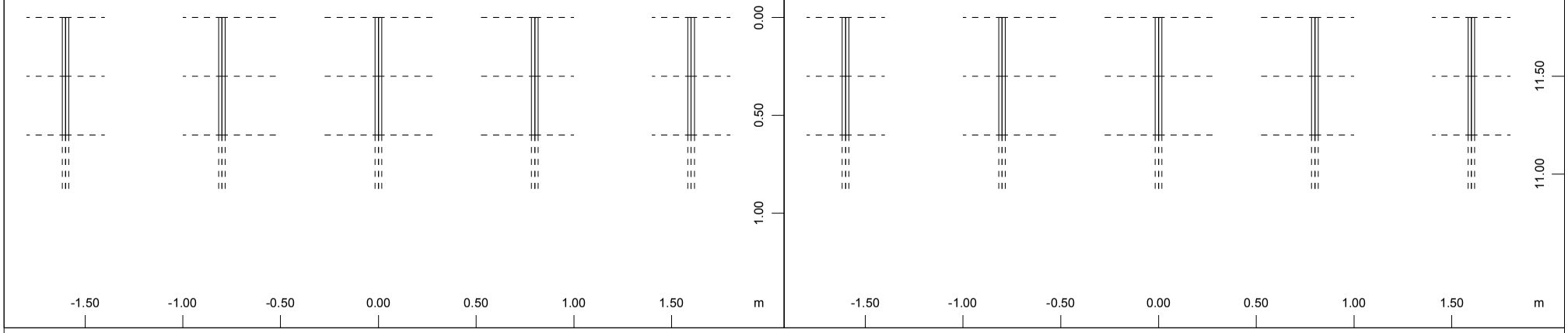
ΚΑΤΩ ΟΠΛΙΣΜΟΣ



Y Z X
Sector of system Beam Elements Group 2
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
502 , 1 cm 3D = 0 cm2 (Max=0)
M 1 : 30

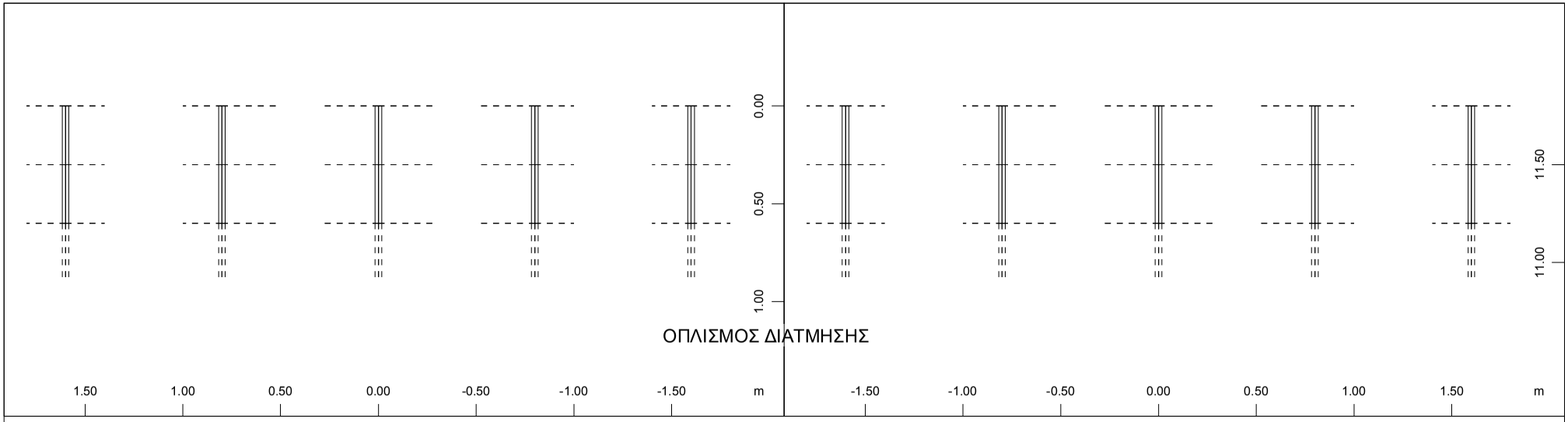
X Z Y
Sector of system Beam Elements Group 2
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
502 , 1 cm 3D = 0 cm2 (Max=0)
M 1 : 30

ΠΑΝΩ ΟΠΛΙΣΜΟΣ



Z Y X
Sector of system Beam Elements Group 2
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
502 , 1 cm 3D = 0 cm2 (Max=0)
M 1 : 30

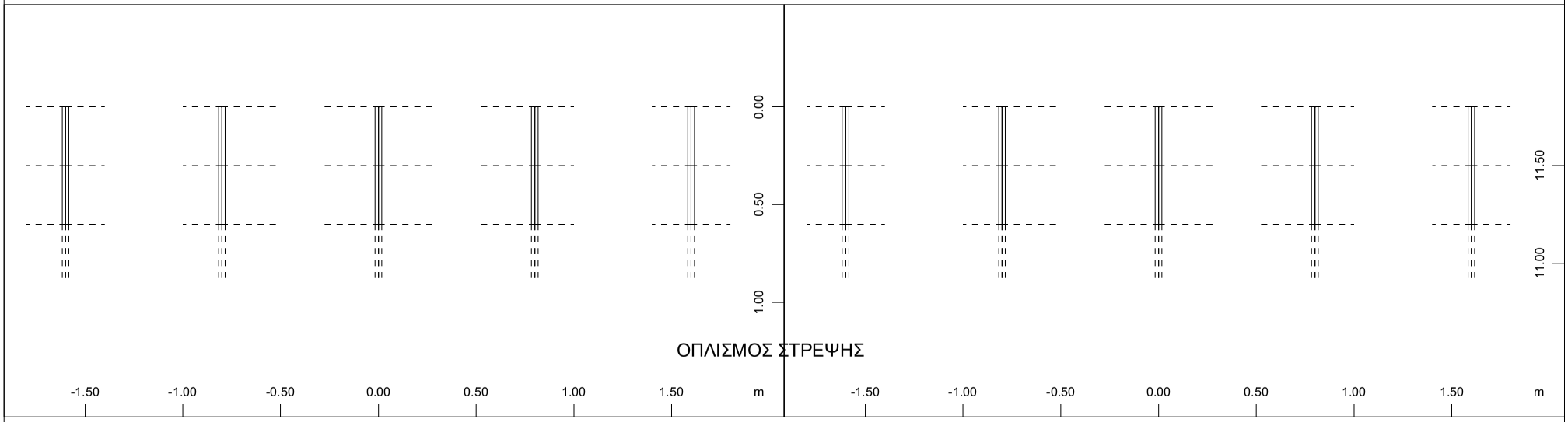
X Z Y
Sector of system Beam Elements Group 2
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
502 , 1 cm 3D = 0 cm2 (Max=0)
M 1 : 30



ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ

Y Z X
Sector of system Beam Elements Group 2
Beam Elements , Shear reinforcements (maximum), Design Case 502
, 1 cm 3D = 0 cm2/m (Max=0)
M 1 : 30

X Z Y
Sector of system Beam Elements Group 2
Beam Elements , Shear reinforcements (maximum), Design Case 502
, 1 cm 3D = 0 cm2/m (Max=0)
M 1 : 30



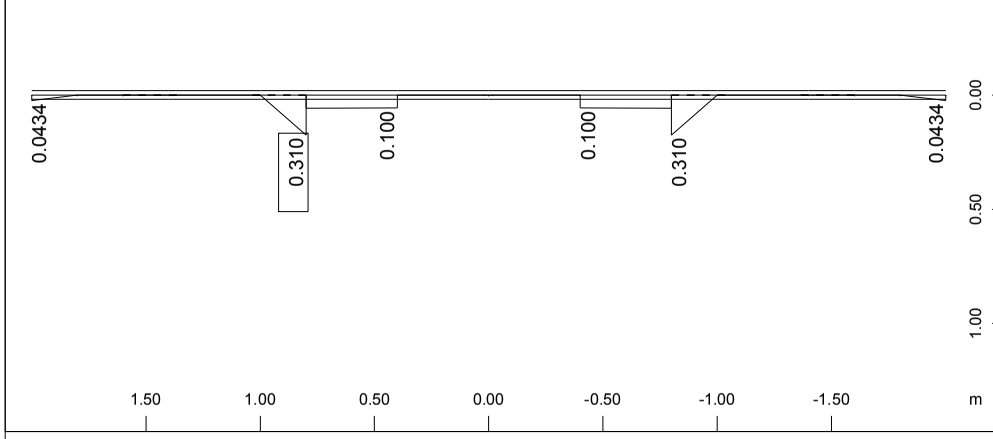
ΟΠΛΙΣΜΟΣ ΣΤΡΕΨΗΣ

Z Y X
Sector of system Beam Elements Group 2
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
502 , 1 cm 3D = 0 cm2 (Max=0)
M 1 : 30

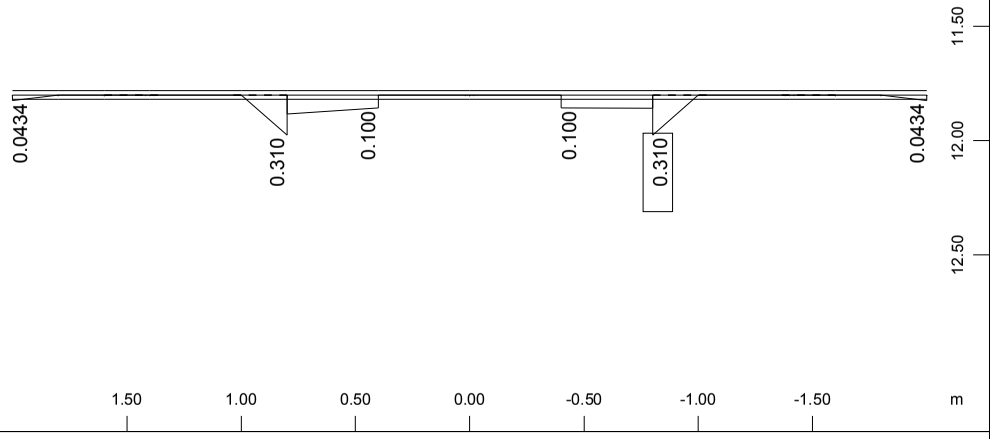
X Z Y
Sector of system Beam Elements Group 2
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
502 , 1 cm 3D = 0 cm2 (Max=0)
M 1 : 30

ΠΑΣΣΑΛΟΔΕΣΜΟΣ

ΚΑΤΩ ΟΠΛΙΣΜΟΣ

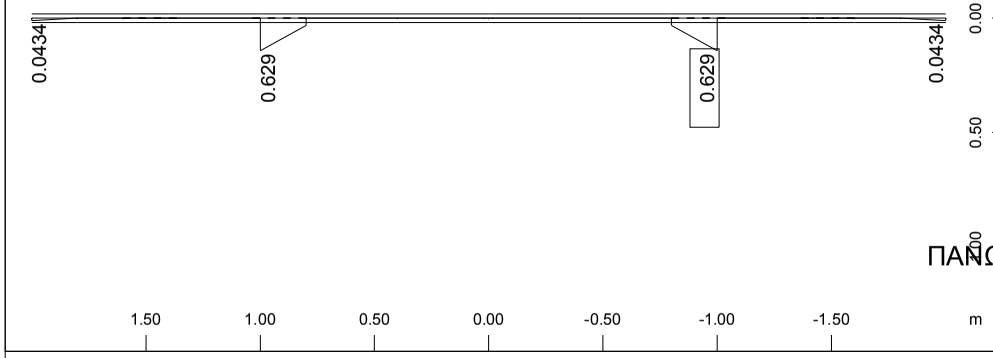


Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
501 , 1 cm 3D = 0.581 cm2 (Max=0.310)

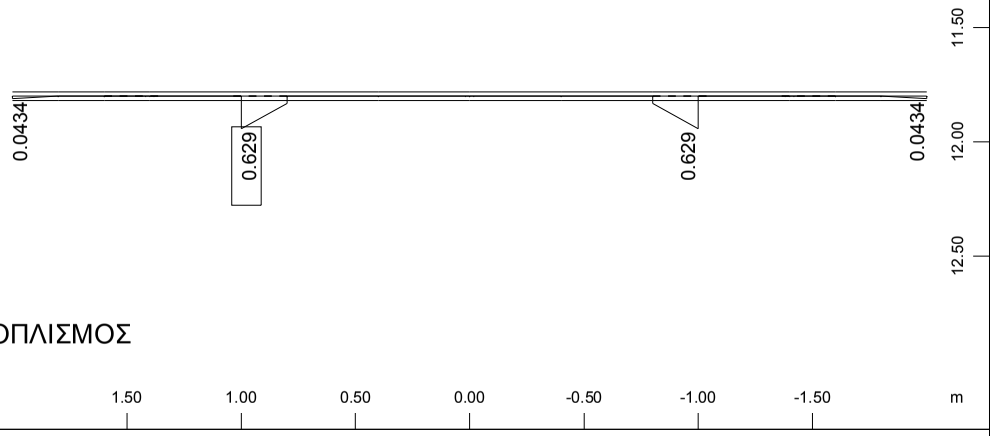


Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
501 , 1 cm 3D = 0.581 cm2 (Max=0.310)

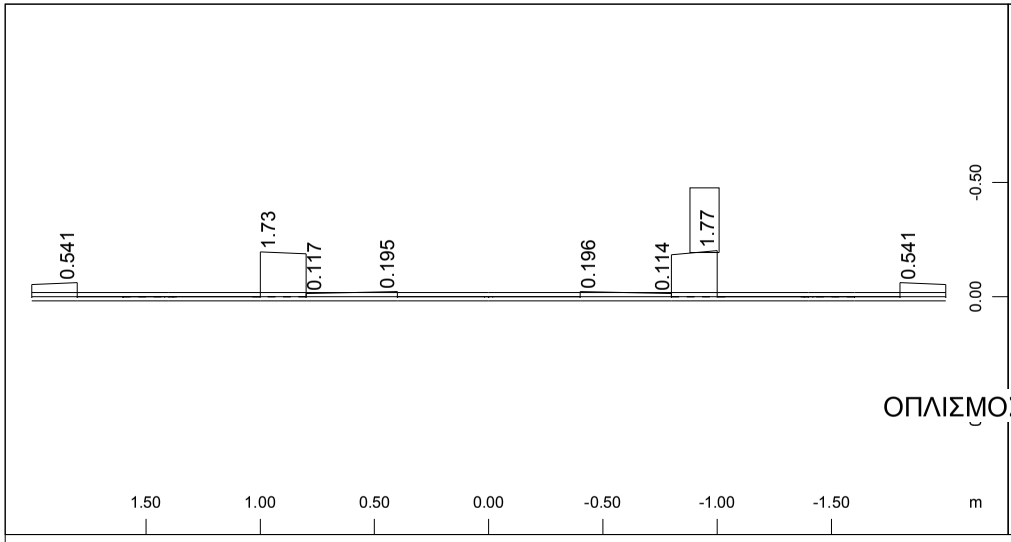
ΠΑΝΩ ΟΠΛΙΣΜΟΣ



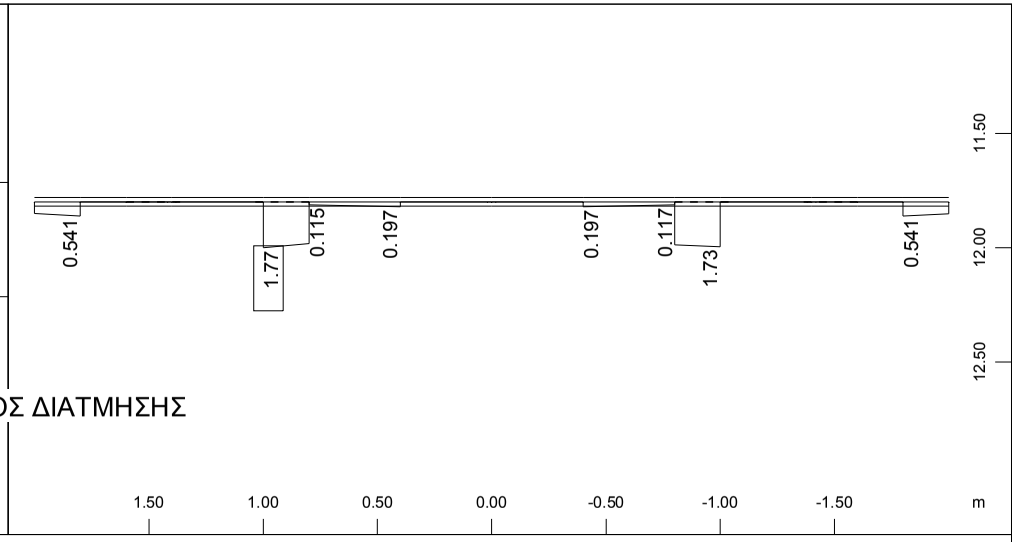
Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
501 , 1 cm 3D = 1.45 cm2 (Max=0.629)



Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
501 , 1 cm 3D = 1.45 cm2 (Max=0.629)

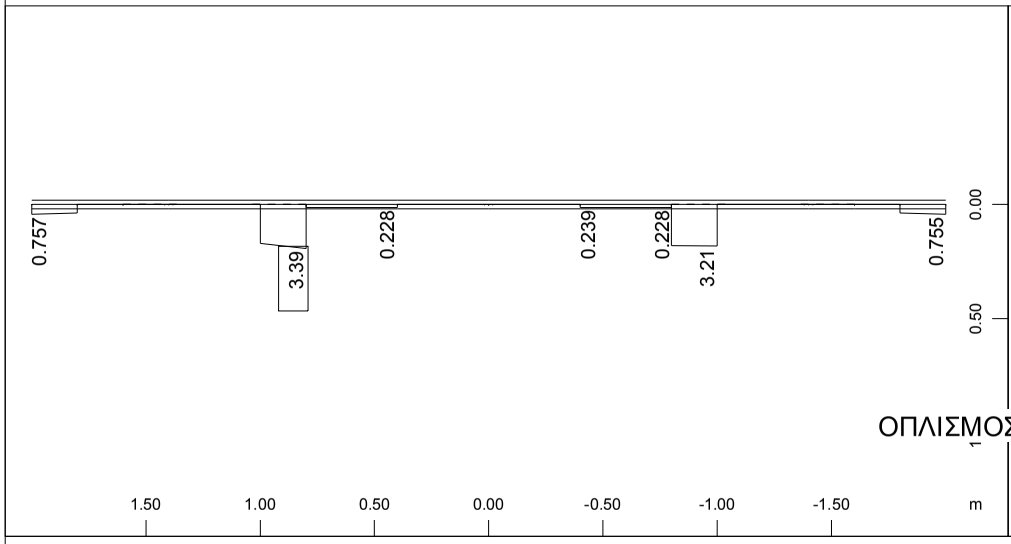


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Shear reinforcements (maximum), Design Case 501
, 1 cm 3D = 2.91 cm2/m (Max=1.77)

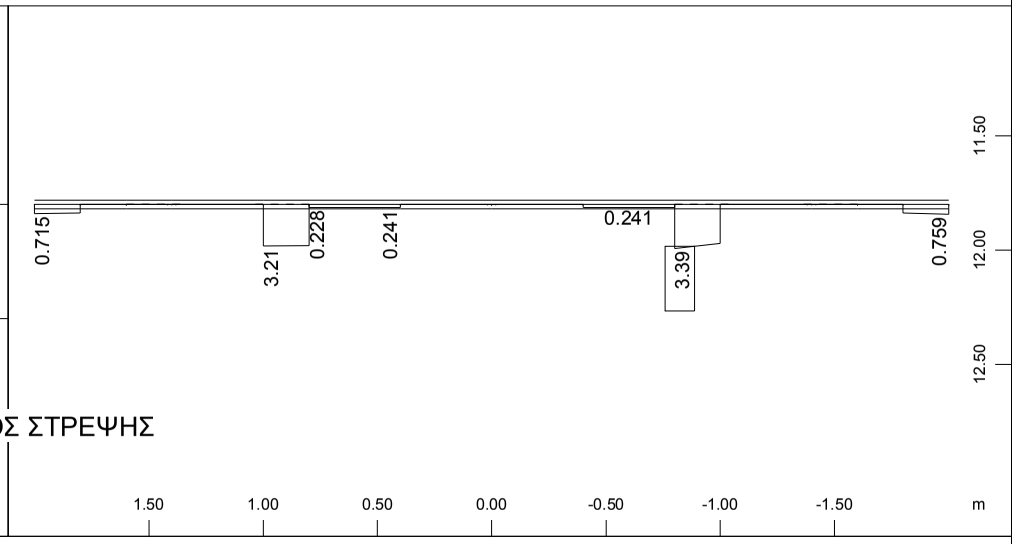


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Shear reinforcements (maximum), Design Case 501
, 1 cm 3D = 2.91 cm2/m (Max=1.77)

ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ

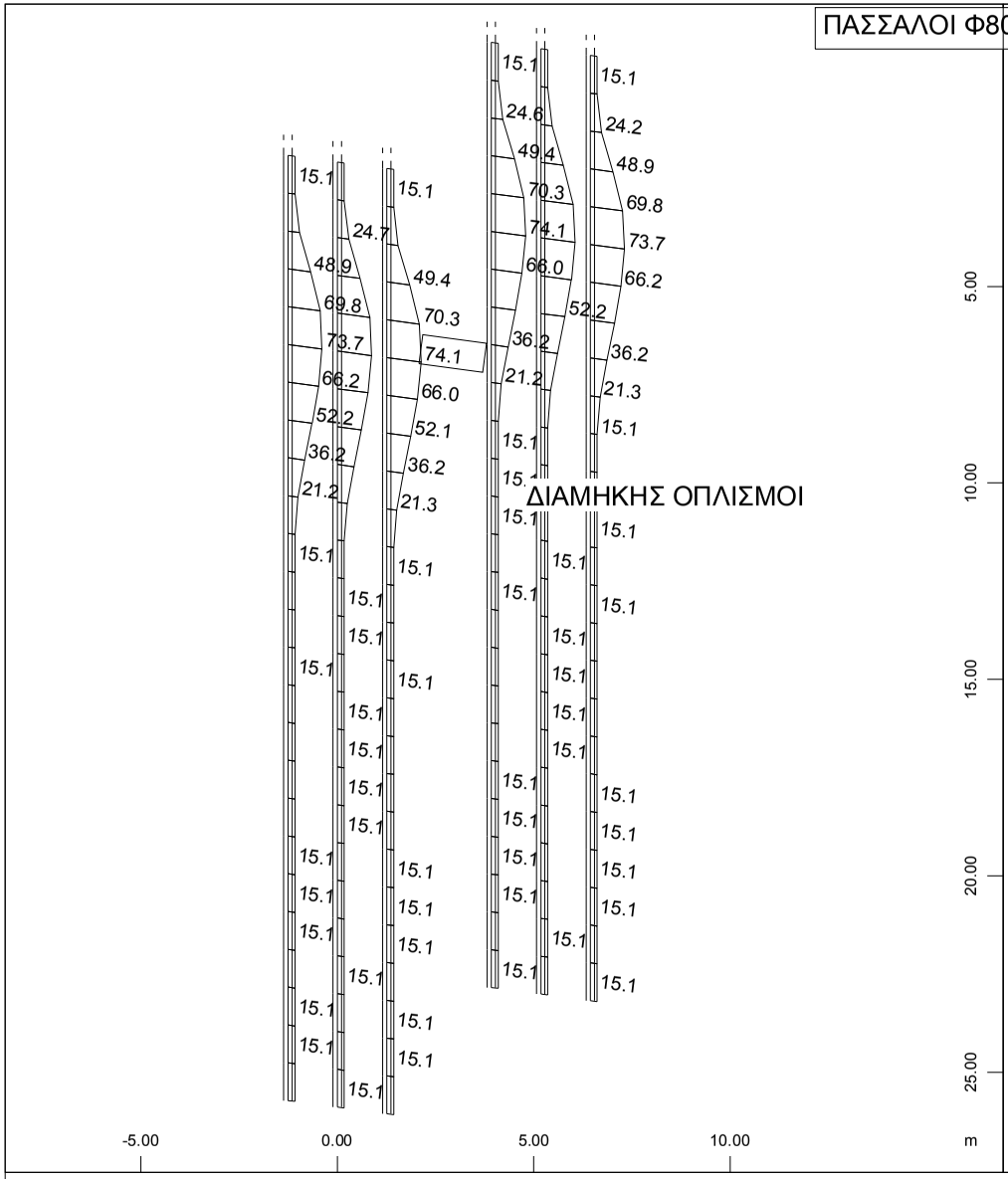


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
501 , 1 cm 3D = 5.81 cm2 (Max=3.39)



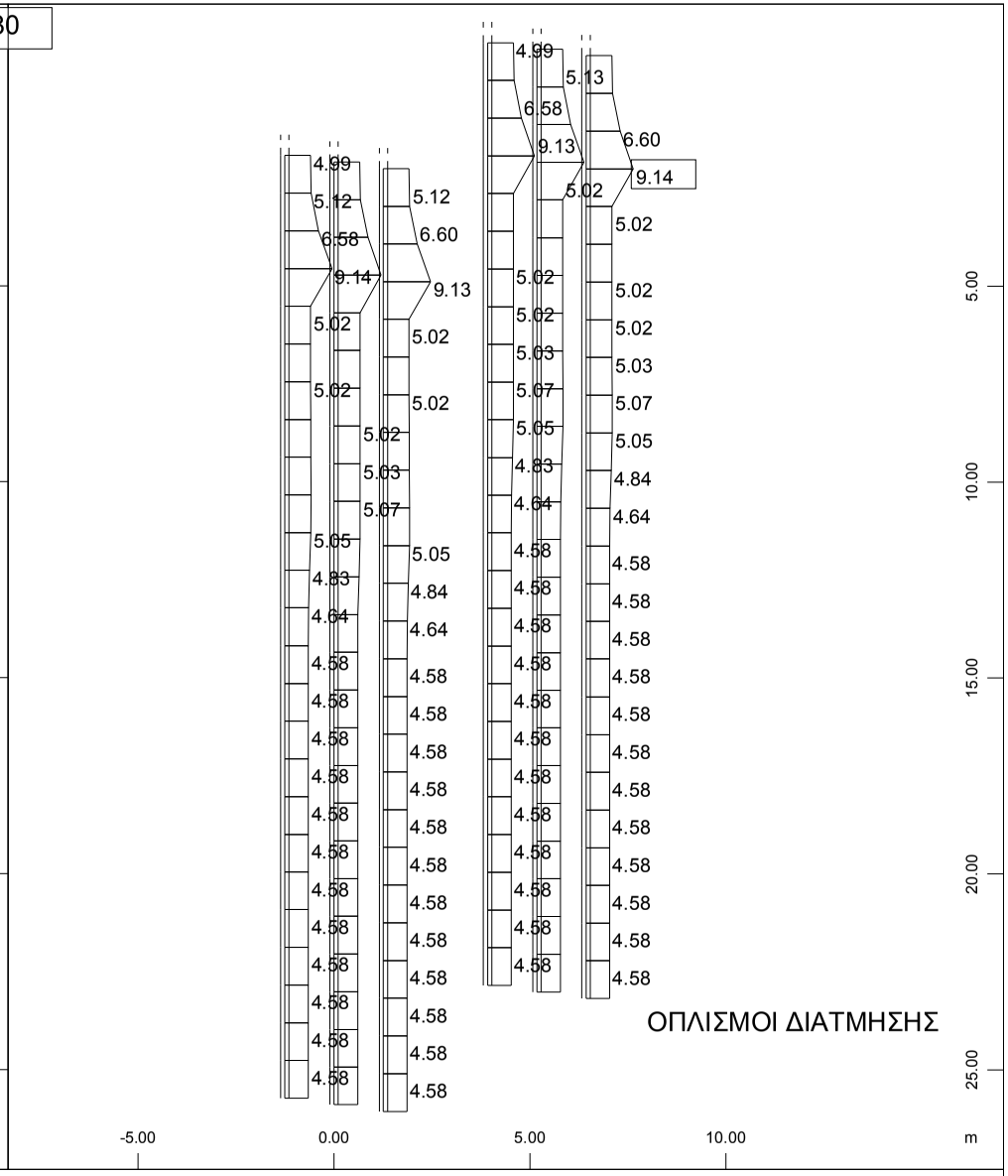
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
501 , 1 cm 3D = 5.81 cm2 (Max=3.39)

ΟΠΛΙΣΜΟΣ ΣΤΡΕΨΗΣ



Sector of system Beam Elements Group 12
Beam Elements , Longitudinal Reinforcements Lay. 1, Design
Case 501 , 1 cm 3D = 145.3 cm2 (Max=74.1)

M 1 : 191
X * 0.502
Y * 0.906
Z * 0.962



Sector of system Beam Elements Group 12
Beam Elements , Shear reinforcements (maximum), Design Case
501 , 1 cm 3D = 14.5 cm2/m (Max=9.14)

M 1 : 191
X * 0.502
Y * 0.906
Z * 0.962

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΦΑΣΗ-1_ULS

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00

6) ΦΑΣΗ-1 ΕΛΕΓΧΟΣ ΦΟΡΕΑ ΣΕ SLS

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_SLS_DOKOI

Selected Beam Elements

FROM	TO	INC	X-VALUE	NC	MEMBER	CS0	CS1	CS2	CS3	CS4	CS5
1000	1060	1		1	bending	10	40				

Default design code is DIN Fachbericht 102 Massivbrȯcken (2003) (Germany)
Klasse(Tab.4.118): D
Wind zone : Binnenland

Materials

No. 1 C 25/30 (DIN 1045-1)
No. 3 C 25/30 (DIN 1045-1)
No. 4 C 25/30 (DIN 1045-1)
No. 5 C 25/30 (DIN 1045-1)
No. 6 C 25/30 (DIN 1045-1)
No. 7 C 25/30 (DIN 1045-1)
No. 8 C 25/30 (DIN 1045-1)
No. 9 C 25/30 (DIN 1045-1)
No. 10 C 25/30 (DIN 1045-1)
No. 12 BSt 500 SA (DIN 1045-1)

All moments will be smoothed out between face and support
Reinforcement will be accounted for sectional values as defined in AQUA
Reinforcements saved as design case LCR 503
Reinforcements are superposed with existing minimum reinforcements

Considered Load Cases

No.	refer	act on	Title/type of load case	gam-u	gam-f	psi-0	psi-1	psi-2	psi-1'	
1	part.	CS 0	I.B. ΚΑΤΑΚ.ΣΤΟΙΧΕΙΩΝ G (total dead load)	1.35	1.00	1.00	1.00	1.00	1.00	G perm
2	part.	CS 0	I.B. ΔΟΚΩΝ G (total dead load)	1.35	1.00	1.00	1.00	1.00	1.00	G perm
3	part.	CS 0	I.B. ΧΥΤΗΣ ΠΛΑΚΑΣ G (total dead load)	1.35	1.00	1.00	1.00	1.00	1.00	G perm
4	part.	CS 0	ΚΙΝΗΤΟ ΦΑΣΗΣ-1 Q_A (Pay load residential cat. A)	1.50	0.00	0.70	0.50	0.30	0.70	Q cond
11	part.	CS 0	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+0.5 L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
12	part.	CS 0	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+0.5 L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
13	part.	CS 0	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+1.0 L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
14	part.	CS 0	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+1.0 L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
5015	part.	CS 0	K creep step C (creep + shrinkage)	1.00	1.00	1.00	1.00	1.00	1.00	P perm
5025	part.	CS 0	K creep step C (creep + shrinkage)	1.00	1.00	1.00	1.00	1.00	1.00	P perm
6015	part.	CS 0	15 K creep step C (creep + shrinkage)							P perm
6025	part.	CS 0	25 K creep step C (creep + shrinkage)							P perm

Combinations For Serviceability

1017 (CS 1) max_my-1017
MAX + MY :
1.00 * G + 1.00 * L_A + 1.00 * C
1018 (CS 1) min_my-1018
MIN + MY :
1.00 * G + 1.00 * L_A + 1.00 * C

Parameters for nonlinear stresses

Iteration for all forces and moments

Material of sections uses Serviceability strain-stress law without safety factors
Material of reinforcements uses Serviceability strain-stress law without safety factors

MNo.	temp lev.	Material-safety [-]	max.compr stress [MPa]	at strain [o/oo]	max.tens stress [MPa]	at strain [o/oo]	tension-stiffening [MPa]
1	0	1.000	-33.00	-2.20	0.00	0.00	
3	0	1.000	-33.00	-2.20	0.00	0.00	
4	0	1.000	-33.00	-2.20	0.00	0.00	
5	0	1.000	-33.00	-2.20	0.00	0.00	
6	0	1.000	-33.00	-2.20	0.00	0.00	
7	0	1.000	-33.00	-2.20	0.00	0.00	
8	0	1.000	-33.00	-2.20	0.00	0.00	
9	0	1.000	-33.00	-2.20	0.00	0.00	
10	0	1.000	-33.00	-2.20	0.00	0.00	
12	0	1.000	-550.00	-25.00	550.00	25.00	

Interaction thin walled normal- and shearstress via Prandtl flow rule

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_SLS_DOKOI

Nonlinear Stresses

```
  Beam     x[m] NoS   LC     e-o    ky/kz     x  zn/yn      Ni/Vi   Myi/Mzi  Ey/Ez/G-EFF
                             [o/oo]  [1/km]   [m]   [m]       [MN]    [MNm]        [MPa]
  1001    0.000   1 1017    0.000   0.000 0.750   -.-      0.000    0.000        26663
                    1018    0.000   0.000 0.750   -.-      0.000    0.000        26663
          0.883   1 1017    1.018   2.643 0.092-0.150      0.000    0.054         1417
                    1017    0.665   1.816 0.111-0.131      0.000    0.054         2063
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.05   537.35         3.81
                         ---- Check for crack width passed with additional reinforcements
                    1018    1.018   2.643 0.092-0.150      0.000    0.054         1417
                    1018    0.665   1.816 0.111-0.131      0.000    0.054         2063
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.05   537.35         3.81
                         ---- Check for crack width passed with additional reinforcements
  1002    0.000   1 1017    1.018   2.643 0.092-0.150      0.000    0.054         1417
                    1017    0.665   1.816 0.111-0.131      0.000    0.054         2063
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.05   537.35         3.81
                         ---- Check for crack width passed with additional reinforcements
                    1018    1.018   2.643 0.092-0.150      0.000    0.054         1417
                    1018    0.665   1.816 0.111-0.131      0.000    0.054         2063
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.05   537.35         3.81
                         ---- Check for crack width passed with additional reinforcements
          0.883   1 1017    0.984   2.777 0.122-0.120      0.000    0.099         2441
                    1017    0.638   1.935 0.147-0.095      0.000    0.099         3503
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.12   297.53         7.06
                         ---- Check for crack width passed with additional reinforcements
                    1018    0.984   2.777 0.122-0.120      0.000    0.099         2441
                    1018    0.638   1.935 0.147-0.095      0.000    0.099         3503
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.12   297.53         7.06
                         ---- Check for crack width passed with additional reinforcements
  1003    0.000   1 1017    0.984   2.777 0.122-0.120      0.000    0.099         2441
                    1017    0.638   1.935 0.147-0.095      0.000    0.099         3503
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.12   297.53         7.06
                         ---- Check for crack width passed with additional reinforcements
                    1018    0.984   2.777 0.122-0.120      0.000    0.099         2441
                    1018    0.638   1.935 0.147-0.095      0.000    0.099         3503
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.12   297.53         7.06
                         ---- Check for crack width passed with additional reinforcements
          0.883   1 1017    0.947   2.830 0.142-0.100      0.000    0.133         3220
                    1017    0.621   2.016 0.169-0.073      0.000    0.133         4521
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.19   221.57         9.65
                         ---- Check for crack width passed with additional reinforcements
                    1018    0.947   2.830 0.142-0.100      0.000    0.133         3220
                    1018    0.621   2.016 0.169-0.073      0.000    0.133         4521
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.19   221.57         9.65
                         ---- Check for crack width passed with additional reinforcements
  1004    0.000   1 1017    0.947   2.830 0.142-0.100      0.000    0.133         3220
                    1017    0.621   2.016 0.169-0.073      0.000    0.133         4521
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.19   221.57         9.65
                         ---- Check for crack width passed with additional reinforcements
                    1018    0.947   2.830 0.142-0.100      0.000    0.133         3220
                    1018    0.621   2.016 0.169-0.073      0.000    0.133         4521
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.19   221.57         9.65
                         ---- Check for crack width passed with additional reinforcements
          0.883   1 1017    0.918   2.856 0.155-0.087      0.000    0.158         3771
                    1017    0.609   2.070 0.183-0.059      0.000    0.158         5203
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.23   187.68        11.52
                         ---- Check for crack width passed with additional reinforcements
                    1018    0.918   2.856 0.155-0.087      0.000    0.158         3771
                    1018    0.609   2.070 0.183-0.059      0.000    0.158         5203
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.23   187.68        11.52
                         ---- Check for crack width passed with additional reinforcements
  1005    0.000   1 1017    0.918   2.856 0.155-0.087      0.000    0.158         3771
                    1017    0.609   2.070 0.183-0.059      0.000    0.158         5203
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.23   187.68        11.52
                         ---- Check for crack width passed with additional reinforcements
                    1018    0.918   2.856 0.155-0.087      0.000    0.158         3771
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ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_SLS_DOKOI

Nonlinear Stresses

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  Beam     x[m] NoS   LC     e-o    ky/kz     x  zn/yn      Ni/Vi   Myi/Mzi  Ey/Ez/G-EFF
                             [o/oo]  [1/km]   [m]   [m]       [MN]    [MNm]        [MPa]
  1005    0.000   1 1018    0.609   2.070 0.183-0.059      0.000    0.158         5203
                          --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                          T-zone h= 0.125  20.0  0.30    214.23   187.68        11.52
                          ---- Check for crack width passed with additional reinforcements
          0.883   1 1017    0.902   2.872 0.163-0.079      0.000    0.173         4096
                    1017    0.621   2.151 0.188-0.054      0.000    0.173         5468
                          --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                          T-zone h= 0.125  20.0  0.30    220.21   176.67        12.29
                          ---- Check for crack width passed with additional reinforcements
                    1018    0.902   2.872 0.163-0.079      0.000    0.173         4096
                    1018    0.621   2.151 0.188-0.054      0.000    0.173         5468
                          --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                          T-zone h= 0.125  20.0  0.30    220.21   176.67        12.29
                          ---- Check for crack width passed with additional reinforcements
  1006    0.000   1 1017    0.902   2.872 0.163-0.079      0.000    0.173         4096
                    1017    0.621   2.151 0.188-0.054      0.000    0.173         5468
                          --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                          T-zone h= 0.125  20.0  0.30    220.21   176.67        12.29
                          ---- Check for crack width passed with additional reinforcements
                    1018    0.902   2.872 0.163-0.079      0.000    0.173         4096
                    1018    0.621   2.151 0.188-0.054      0.000    0.173         5468
                          --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                          T-zone h= 0.125  20.0  0.30    220.21   176.67        12.29
                          ---- Check for crack width passed with additional reinforcements
          0.883   1 1017    0.897   2.877 0.165-0.077      0.000    0.178         4203
                    1017    0.629   2.188 0.189-0.052      0.000    0.178         5526
                          --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                          T-zone h= 0.125  20.0  0.30    223.43   174.36        12.47
                          ---- Check for crack width passed with additional reinforcements
                    1018    0.897   2.877 0.165-0.077      0.000    0.178         4203
                    1018    0.629   2.188 0.189-0.052      0.000    0.178         5526
                          --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                          T-zone h= 0.125  20.0  0.30    223.43   174.36        12.47
                          ---- Check for crack width passed with additional reinforcements
  1007    0.000   1 1017    0.897   2.877 0.165-0.077      0.000    0.178         4203
                    1017    0.629   2.188 0.189-0.052      0.000    0.178         5526
                          --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                          T-zone h= 0.125  20.0  0.30    223.43   174.36        12.47
                          ---- Check for crack width passed with additional reinforcements
                    1018    0.897   2.877 0.165-0.077      0.000    0.178         4203
                    1018    0.629   2.188 0.189-0.052      0.000    0.178         5526
                          --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                          T-zone h= 0.125  20.0  0.30    223.43   174.36        12.47
                          ---- Check for crack width passed with additional reinforcements
          0.883   1 1017    0.902   2.872 0.163-0.079      0.000    0.173         4096
                    1017    0.621   2.151 0.188-0.054      0.000    0.173         5468
                          --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                          T-zone h= 0.125  20.0  0.30    220.21   176.67        12.29
                          ---- Check for crack width passed with additional reinforcements
                    1018    0.902   2.872 0.163-0.079      0.000    0.173         4096
                    1018    0.621   2.151 0.188-0.054      0.000    0.173         5468
                          --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                          T-zone h= 0.125  20.0  0.30    220.21   176.67        12.29
                          ---- Check for crack width passed with additional reinforcements
  1008    0.000   1 1017    0.902   2.872 0.163-0.079      0.000    0.173         4096
                    1017    0.621   2.151 0.188-0.054      0.000    0.173         5468
                          --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                          T-zone h= 0.125  20.0  0.30    220.21   176.67        12.29
                          ---- Check for crack width passed with additional reinforcements
                    1018    0.902   2.872 0.163-0.079      0.000    0.173         4096
                    1018    0.621   2.151 0.188-0.054      0.000    0.173         5468
                          --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                          T-zone h= 0.125  20.0  0.30    220.21   176.67        12.29
                          ---- Check for crack width passed with additional reinforcements
          0.883   1 1017    0.918   2.856 0.155-0.087      0.000    0.158         3771
                    1017    0.609   2.070 0.183-0.059      0.000    0.158         5203
                          --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                          T-zone h= 0.125  20.0  0.30    214.23   187.68        11.52
                          ---- Check for crack width passed with additional reinforcements
                    1018    0.918   2.856 0.155-0.087      0.000    0.158         3771
                    1018    0.609   2.070 0.183-0.059      0.000    0.158         5203
                          --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                          T-zone h= 0.125  20.0  0.30    214.23   187.68        11.52
                          ---- Check for crack width passed with additional reinforcements
  1009    0.000   1 1017    0.918   2.856 0.155-0.087      0.000    0.158         3771
                    1017    0.609   2.070 0.183-0.059      0.000    0.158         5203
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ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_SLS_DOKOI

Nonlinear Stresses

```
  Beam     x[m] NoS   LC     e-o    ky/kz     x  zn/yn      Ni/Vi   Myi/Mzi  Ey/Ez/G-EFF
                           [o/oo]  [1/km]   [m]   [m]       [MN]     [MNm]        [MPa]
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.23   187.68       11.52
                                 ---- Check for crack width passed with additional reinforcements
                        1018   0.918   2.856 0.155-0.087     0.000    0.158        3771
                        1018   0.609   2.070 0.183-0.059     0.000    0.158        5203
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.23   187.68       11.52
                                 ---- Check for crack width passed with additional reinforcements
         0.883   1 1017   0.947   2.830 0.142-0.100     0.000    0.133        3220
                 1017   0.621   2.016 0.169-0.073     0.000    0.133        4521
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.19   221.57        9.65
                                 ---- Check for crack width passed with additional reinforcements
                        1018   0.947   2.830 0.142-0.100     0.000    0.133        3220
                        1018   0.621   2.016 0.169-0.073     0.000    0.133        4521
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.19   221.57        9.65
                                 ---- Check for crack width passed with additional reinforcements
  1010   0.000   1 1017   0.947   2.831 0.142-0.100     0.000    0.133        3220
                 1017   0.621   2.016 0.169-0.073     0.000    0.133        4521
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.19   221.57        9.65
                                 ---- Check for crack width passed with additional reinforcements
                        1018   0.947   2.831 0.142-0.100     0.000    0.133        3220
                        1018   0.621   2.016 0.169-0.073     0.000    0.133        4521
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.19   221.57        9.65
                                 ---- Check for crack width passed with additional reinforcements
         0.883   1 1017   0.984   2.777 0.122-0.120     0.000    0.099        2441
                 1017   0.638   1.935 0.147-0.095     0.000    0.099        3503
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.12   297.53        7.06
                                 ---- Check for crack width passed with additional reinforcements
                        1018   0.984   2.777 0.122-0.120     0.000    0.099        2441
                        1018   0.638   1.935 0.147-0.095     0.000    0.099        3503
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.12   297.53        7.06
                                 ---- Check for crack width passed with additional reinforcements
  1011   0.000   1 1017   0.984   2.777 0.122-0.120     0.000    0.099        2441
                 1017   0.638   1.935 0.147-0.095     0.000    0.099        3503
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.12   297.53        7.06
                                 ---- Check for crack width passed with additional reinforcements
                        1018   0.984   2.777 0.122-0.120     0.000    0.099        2441
                        1018   0.638   1.935 0.147-0.095     0.000    0.099        3503
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.12   297.53        7.06
                                 ---- Check for crack width passed with additional reinforcements
         0.883   1 1017   1.018   2.643 0.092-0.150     0.000    0.054        1417
                 1017   0.665   1.816 0.111-0.131     0.000    0.054        2063
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.05   537.35        3.81
                                 ---- Check for crack width passed with additional reinforcements
                        1018   1.018   2.643 0.092-0.150     0.000    0.054        1417
                        1018   0.665   1.816 0.111-0.131     0.000    0.054        2063
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.05   537.35        3.81
                                 ---- Check for crack width passed with additional reinforcements
  1012   0.000   1 1017   1.018   2.643 0.092-0.150     0.000    0.054        1417
                 1017   0.665   1.816 0.111-0.131     0.000    0.054        2063
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.05   538.88        3.81
                                 ---- Check for crack width passed with additional reinforcements
                        1018   1.018   2.643 0.092-0.150     0.000    0.054        1417
                        1018   0.665   1.816 0.111-0.131     0.000    0.054        2063
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.05   538.88        3.81
                                 ---- Check for crack width passed with additional reinforcements
         0.883   1 1017   0.000   0.000 0.477   -.-     0.000    0.000       26663
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.273  20.0            0.00                 0.00
                                 ---- Check for crack width passed with given reinforcements
                        1018   0.000   0.000 0.477   -.-     0.000    0.000       26663
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.273  20.0            0.00                 0.00
                                 ---- Check for crack width passed with given reinforcements
  1013   0.000   1 1017   0.000   0.000 0.000   -.-     0.000    0.000       26663
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ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_SLS_DOKOI

Nonlinear Stresses

```
  Beam     x[m] NoS   LC     e-o   ky/kz    x  zn/yn     Ni/Vi  Myi/Mzi Ey/Ez/G-EFF
                           [o/oo]  [1/km]   [m]   [m]      [MN]    [MNm]       [MPa]
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 C-zone h= 0.477  20.0            0.00                 0.00
                                 ---- Check for crack width passed with given reinforcements
                                 T-zone h= 0.273  20.0            0.00                 0.00
                                 ---- Check for crack width passed with given reinforcements
                          1018   0.000   0.000 0.000   -.-     0.000    0.000        26663
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 C-zone h= 0.477  20.0            0.00                 0.00
                                 ---- Check for crack width passed with given reinforcements
                                 T-zone h= 0.273  20.0            0.00                 0.00
                                 ---- Check for crack width passed with given reinforcements
         0.883    1 1017   1.018   2.643 0.092-0.150     0.000    0.054         1417
                          1017   0.665   1.816 0.111-0.131     0.000    0.054         2063
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.05   537.35         3.81
                                 ---- Check for crack width passed with additional reinforcements
                          1018   1.018   2.643 0.092-0.150     0.000    0.054         1417
                          1018   0.665   1.816 0.111-0.131     0.000    0.054         2063
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.05   537.35         3.81
                                 ---- Check for crack width passed with additional reinforcements
  1014   0.000    1 1017   1.018   2.643 0.092-0.150     0.000    0.054         1417
                          1017   0.665   1.816 0.111-0.131     0.000    0.054         2063
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.05   537.35         3.81
                                 ---- Check for crack width passed with additional reinforcements
                          1018   1.018   2.643 0.092-0.150     0.000    0.054         1417
                          1018   0.665   1.816 0.111-0.131     0.000    0.054         2063
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.05   537.35         3.81
                                 ---- Check for crack width passed with additional reinforcements
         0.883    1 1017   0.984   2.777 0.122-0.120     0.000    0.099         2441
                          1017   0.638   1.935 0.147-0.095     0.000    0.099         3503
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.12   297.53         7.06
                                 ---- Check for crack width passed with additional reinforcements
                          1018   0.984   2.777 0.122-0.120     0.000    0.099         2441
                          1018   0.638   1.935 0.147-0.095     0.000    0.099         3503
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.12   297.53         7.06
                                 ---- Check for crack width passed with additional reinforcements
  1015   0.000    1 1017   0.984   2.777 0.122-0.120     0.000    0.099         2441
                          1017   0.638   1.935 0.147-0.095     0.000    0.099         3503
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.12   297.53         7.06
                                 ---- Check for crack width passed with additional reinforcements
                          1018   0.984   2.777 0.122-0.120     0.000    0.099         2441
                          1018   0.638   1.935 0.147-0.095     0.000    0.099         3503
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.12   297.53         7.06
                                 ---- Check for crack width passed with additional reinforcements
         0.883    1 1017   0.947   2.830 0.142-0.100     0.000    0.133         3220
                          1017   0.621   2.016 0.169-0.073     0.000    0.133         4521
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.19   221.57         9.65
                                 ---- Check for crack width passed with additional reinforcements
                          1018   0.947   2.830 0.142-0.100     0.000    0.133         3220
                          1018   0.621   2.016 0.169-0.073     0.000    0.133         4521
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.19   221.57         9.65
                                 ---- Check for crack width passed with additional reinforcements
  1016   0.000    1 1017   0.947   2.830 0.142-0.100     0.000    0.133         3220
                          1017   0.621   2.016 0.169-0.073     0.000    0.133         4521
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.19   221.57         9.65
                                 ---- Check for crack width passed with additional reinforcements
                          1018   0.947   2.830 0.142-0.100     0.000    0.133         3220
                          1018   0.621   2.016 0.169-0.073     0.000    0.133         4521
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.19   221.57         9.65
                                 ---- Check for crack width passed with additional reinforcements
         0.883    1 1017   0.918   2.856 0.155-0.087     0.000    0.158         3771
                          1017   0.609   2.070 0.183-0.059     0.000    0.158         5203
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.23   187.68        11.52
                                 ---- Check for crack width passed with additional reinforcements
                          1018   0.918   2.856 0.155-0.087     0.000    0.158         3771
                          1018   0.609   2.070 0.183-0.059     0.000    0.158         5203
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ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_SLS_DOKOI

Nonlinear Stresses

```
  Beam     x[m] NoS   LC     e-o   ky/kz    x  zn/yn     Ni/Vi  Myi/Mzi Ey/Ez/G-EFF
                           [o/oo]  [1/km]   [m]   [m]      [MN]    [MNm]       [MPa]
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.23   187.68       11.52
                                 ---- Check for crack width passed with additional reinforcements
  1017   0.000   1 1017   0.918   2.856 0.155-0.087     0.000    0.158        3771
                   1017   0.609   2.070 0.183-0.059     0.000    0.158        5203
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.23   187.68       11.52
                                 ---- Check for crack width passed with additional reinforcements
                   1018   0.918   2.856 0.155-0.087     0.000    0.158        3771
                   1018   0.609   2.070 0.183-0.059     0.000    0.158        5203
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.23   187.68       11.52
                                 ---- Check for crack width passed with additional reinforcements
         0.883   1 1017   0.902   2.872 0.163-0.079     0.000    0.173        4096
                   1017   0.621   2.151 0.188-0.054     0.000    0.173        5468
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    220.21   176.67       12.29
                                 ---- Check for crack width passed with additional reinforcements
                   1018   0.902   2.872 0.163-0.079     0.000    0.173        4096
                   1018   0.621   2.151 0.188-0.054     0.000    0.173        5468
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    220.21   176.67       12.29
                                 ---- Check for crack width passed with additional reinforcements
  1018   0.000   1 1017   0.902   2.872 0.163-0.079     0.000    0.173        4096
                   1017   0.621   2.151 0.188-0.054     0.000    0.173        5468
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    220.21   176.67       12.29
                                 ---- Check for crack width passed with additional reinforcements
                   1018   0.902   2.872 0.163-0.079     0.000    0.173        4096
                   1018   0.621   2.151 0.188-0.054     0.000    0.173        5468
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    220.21   176.67       12.29
                                 ---- Check for crack width passed with additional reinforcements
         0.883   1 1017   0.897   2.877 0.165-0.077     0.000    0.178        4203
                   1017   0.629   2.188 0.189-0.052     0.000    0.178        5526
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    223.43   174.36       12.47
                                 ---- Check for crack width passed with additional reinforcements
                   1018   0.897   2.877 0.165-0.077     0.000    0.178        4203
                   1018   0.629   2.188 0.189-0.052     0.000    0.178        5526
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    223.43   174.36       12.47
                                 ---- Check for crack width passed with additional reinforcements
  1019   0.000   1 1017   0.897   2.877 0.165-0.077     0.000    0.178        4203
                   1017   0.629   2.188 0.189-0.052     0.000    0.178        5526
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    223.43   174.36       12.47
                                 ---- Check for crack width passed with additional reinforcements
                   1018   0.897   2.877 0.165-0.077     0.000    0.178        4203
                   1018   0.629   2.188 0.189-0.052     0.000    0.178        5526
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    223.43   174.36       12.47
                                 ---- Check for crack width passed with additional reinforcements
         0.883   1 1017   0.902   2.872 0.163-0.079     0.000    0.173        4096
                   1017   0.621   2.151 0.188-0.054     0.000    0.173        5468
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    220.21   176.67       12.29
                                 ---- Check for crack width passed with additional reinforcements
                   1018   0.902   2.872 0.163-0.079     0.000    0.173        4096
                   1018   0.621   2.151 0.188-0.054     0.000    0.173        5468
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    220.21   176.67       12.29
                                 ---- Check for crack width passed with additional reinforcements
  1020   0.000   1 1017   0.902   2.872 0.163-0.079     0.000    0.173        4096
                   1017   0.621   2.151 0.188-0.054     0.000    0.173        5468
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    220.21   176.67       12.29
                                 ---- Check for crack width passed with additional reinforcements
                   1018   0.902   2.872 0.163-0.079     0.000    0.173        4096
                   1018   0.621   2.151 0.188-0.054     0.000    0.173        5468
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    220.21   176.67       12.29
                                 ---- Check for crack width passed with additional reinforcements
         0.883   1 1017   0.918   2.856 0.155-0.087     0.000    0.158        3771
                   1017   0.609   2.070 0.183-0.059     0.000    0.158        5203
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.23   187.68       11.52
                                 ---- Check for crack width passed with additional reinforcements
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ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_SLS_DOKOI

Nonlinear Stresses

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  Beam     x[m] NoS   LC     e-o    ky/kz     x  zn/yn      Ni/Vi   Myi/Mzi  Ey/Ez/G-EFF
                             [o/oo]  [1/km]   [m]   [m]       [MN]    [MNm]        [MPa]
  1020    0.883   1 1018    0.918    2.856 0.155-0.087      0.000    0.158         3771
                  1018    0.609    2.070 0.183-0.059      0.000    0.158         5203
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.125  20.0  0.30    214.23   187.68        11.52
                        ---- Check for crack width passed with additional reinforcements
  1021    0.000   1 1017    0.918    2.856 0.155-0.087      0.000    0.158         3771
                  1017    0.609    2.070 0.183-0.059      0.000    0.158         5203
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.125  20.0  0.30    214.23   187.68        11.52
                        ---- Check for crack width passed with additional reinforcements
                  1018    0.918    2.856 0.155-0.087      0.000    0.158         3771
                  1018    0.609    2.070 0.183-0.059      0.000    0.158         5203
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.125  20.0  0.30    214.23   187.68        11.52
                        ---- Check for crack width passed with additional reinforcements
          0.883   1 1017    0.947    2.830 0.142-0.100      0.000    0.133         3220
                  1017    0.621    2.016 0.169-0.073      0.000    0.133         4521
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.125  20.0  0.30    214.19   221.57         9.65
                        ---- Check for crack width passed with additional reinforcements
                  1018    0.947    2.830 0.142-0.100      0.000    0.133         3220
                  1018    0.621    2.016 0.169-0.073      0.000    0.133         4521
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.125  20.0  0.30    214.19   221.57         9.65
                        ---- Check for crack width passed with additional reinforcements
  1022    0.000   1 1017    0.947    2.830 0.142-0.100      0.000    0.133         3220
                  1017    0.621    2.016 0.169-0.073      0.000    0.133         4521
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.125  20.0  0.30    214.19   221.57         9.65
                        ---- Check for crack width passed with additional reinforcements
                  1018    0.947    2.830 0.142-0.100      0.000    0.133         3220
                  1018    0.621    2.016 0.169-0.073      0.000    0.133         4521
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.125  20.0  0.30    214.19   221.57         9.65
                        ---- Check for crack width passed with additional reinforcements
          0.883   1 1017    0.984    2.777 0.122-0.120      0.000    0.099         2441
                  1017    0.638    1.935 0.147-0.095      0.000    0.099         3503
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.125  20.0  0.30    214.12   297.53         7.06
                        ---- Check for crack width passed with additional reinforcements
                  1018    0.984    2.777 0.122-0.120      0.000    0.099         2441
                  1018    0.638    1.935 0.147-0.095      0.000    0.099         3503
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.125  20.0  0.30    214.12   297.53         7.06
                        ---- Check for crack width passed with additional reinforcements
  1023    0.000   1 1017    0.984    2.777 0.122-0.120      0.000    0.099         2441
                  1017    0.638    1.935 0.147-0.095      0.000    0.099         3503
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.125  20.0  0.30    214.12   297.53         7.06
                        ---- Check for crack width passed with additional reinforcements
                  1018    0.984    2.777 0.122-0.120      0.000    0.099         2441
                  1018    0.638    1.935 0.147-0.095      0.000    0.099         3503
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.125  20.0  0.30    214.12   297.53         7.06
                        ---- Check for crack width passed with additional reinforcements
          0.883   1 1017    1.018    2.643 0.092-0.150      0.000    0.054         1417
                  1017    0.665    1.816 0.111-0.131      0.000    0.054         2063
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.125  20.0  0.30    214.05   537.35         3.81
                        ---- Check for crack width passed with additional reinforcements
                  1018    1.018    2.643 0.092-0.150      0.000    0.054         1417
                  1018    0.665    1.816 0.111-0.131      0.000    0.054         2063
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.125  20.0  0.30    214.05   537.35         3.81
                        ---- Check for crack width passed with additional reinforcements
  1024    0.000   1 1017    1.018    2.643 0.092-0.150      0.000    0.054         1417
                  1017    0.665    1.816 0.111-0.131      0.000    0.054         2063
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.125  20.0  0.30    214.05   538.88         3.81
                        ---- Check for crack width passed with additional reinforcements
                  1018    1.018    2.643 0.092-0.150      0.000    0.054         1417
                  1018    0.665    1.816 0.111-0.131      0.000    0.054         2063
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.125  20.0  0.30    214.05   538.88         3.81
                        ---- Check for crack width passed with additional reinforcements
          0.883   1 1017    0.000    0.000 0.477   -.-      0.000    0.000        26663
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ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_SLS_DOKOI

Nonlinear Stresses

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  Beam     x[m] NoS   LC     e-o   ky/kz    x  zn/yn     Ni/Vi  Myi/Mzi Ey/Ez/G-EFF
                           [o/oo]  [1/km]   [m]   [m]      [MN]    [MNm]       [MPa]
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.273  20.0            0.00                 0.00
                                 ---- Check for crack width passed with given reinforcements
                        1018   0.000   0.000 0.477   -.-     0.000    0.000       26663
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.273  20.0            0.00                 0.00
                                 ---- Check for crack width passed with given reinforcements
  1025   0.000    1 1017   0.000   0.000 0.000   -.-     0.000    0.000       26663
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 C-zone h= 0.477  20.0            0.00                 0.00
                                 ---- Check for crack width passed with given reinforcements
                                 T-zone h= 0.273  20.0            0.00                 0.00
                                 ---- Check for crack width passed with given reinforcements
                        1018   0.000   0.000 0.000   -.-     0.000    0.000       26663
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 C-zone h= 0.477  20.0            0.00                 0.00
                                 ---- Check for crack width passed with given reinforcements
                                 T-zone h= 0.273  20.0            0.00                 0.00
                                 ---- Check for crack width passed with given reinforcements
         0.883    1 1017   1.018   2.643 0.092-0.150     0.000    0.054        1417
                        1017   0.665   1.816 0.111-0.131     0.000    0.054        2063
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.05   537.35        3.81
                                 ---- Check for crack width passed with additional reinforcements
                        1018   1.018   2.643 0.092-0.150     0.000    0.054        1417
                        1018   0.665   1.816 0.111-0.131     0.000    0.054        2063
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.05   537.35        3.81
                                 ---- Check for crack width passed with additional reinforcements
  1026   0.000    1 1017   1.018   2.643 0.092-0.150     0.000    0.054        1417
                        1017   0.665   1.816 0.111-0.131     0.000    0.054        2063
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.05   537.35        3.81
                                 ---- Check for crack width passed with additional reinforcements
                        1018   1.018   2.643 0.092-0.150     0.000    0.054        1417
                        1018   0.665   1.816 0.111-0.131     0.000    0.054        2063
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.05   537.35        3.81
                                 ---- Check for crack width passed with additional reinforcements
         0.883    1 1017   0.984   2.777 0.122-0.120     0.000    0.099        2441
                        1017   0.638   1.935 0.147-0.095     0.000    0.099        3503
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.12   297.53        7.06
                                 ---- Check for crack width passed with additional reinforcements
                        1018   0.984   2.777 0.122-0.120     0.000    0.099        2441
                        1018   0.638   1.935 0.147-0.095     0.000    0.099        3503
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.12   297.53        7.06
                                 ---- Check for crack width passed with additional reinforcements
  1027   0.000    1 1017   0.984   2.777 0.122-0.120     0.000    0.099        2441
                        1017   0.638   1.935 0.147-0.095     0.000    0.099        3503
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.12   297.53        7.06
                                 ---- Check for crack width passed with additional reinforcements
                        1018   0.984   2.777 0.122-0.120     0.000    0.099        2441
                        1018   0.638   1.935 0.147-0.095     0.000    0.099        3503
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.12   297.53        7.06
                                 ---- Check for crack width passed with additional reinforcements
         0.883    1 1017   0.947   2.830 0.142-0.100     0.000    0.133        3220
                        1017   0.621   2.016 0.169-0.073     0.000    0.133        4521
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.19   221.57        9.65
                                 ---- Check for crack width passed with additional reinforcements
                        1018   0.947   2.830 0.142-0.100     0.000    0.133        3220
                        1018   0.621   2.016 0.169-0.073     0.000    0.133        4521
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.19   221.57        9.65
                                 ---- Check for crack width passed with additional reinforcements
  1028   0.000    1 1017   0.947   2.830 0.142-0.100     0.000    0.133        3220
                        1017   0.621   2.016 0.169-0.073     0.000    0.133        4521
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.19   221.57        9.65
                                 ---- Check for crack width passed with additional reinforcements
                        1018   0.947   2.830 0.142-0.100     0.000    0.133        3220
                        1018   0.621   2.016 0.169-0.073     0.000    0.133        4521
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ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_SLS_DOKOI

Nonlinear Stresses

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  Beam     x[m] NoS   LC     e-o    ky/kz     x  zn/yn      Ni/Vi   Myi/Mzi  Ey/Ez/G-EFF
                           [o/oo]  [1/km]   [m]   [m]       [MN]     [MNm]        [MPa]
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.19   221.57        9.65
                                 ---- Check for crack width passed with additional reinforcements
        0.883    1 1017   0.918   2.856 0.155-0.087     0.000    0.158        3771
                   1017   0.609   2.070 0.183-0.059     0.000    0.158        5203
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.23   187.68       11.52
                                 ---- Check for crack width passed with additional reinforcements
                   1018   0.918   2.856 0.155-0.087     0.000    0.158        3771
                   1018   0.609   2.070 0.183-0.059     0.000    0.158        5203
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.23   187.68       11.52
                                 ---- Check for crack width passed with additional reinforcements
  1029  0.000    1 1017   0.918   2.856 0.155-0.087     0.000    0.158        3771
                   1017   0.609   2.070 0.183-0.059     0.000    0.158        5203
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.23   187.68       11.52
                                 ---- Check for crack width passed with additional reinforcements
                   1018   0.918   2.856 0.155-0.087     0.000    0.158        3771
                   1018   0.609   2.070 0.183-0.059     0.000    0.158        5203
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.23   187.68       11.52
                                 ---- Check for crack width passed with additional reinforcements
        0.883    1 1017   0.902   2.872 0.163-0.079     0.000    0.173        4096
                   1017   0.621   2.151 0.188-0.054     0.000    0.173        5468
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    220.21   176.67       12.29
                                 ---- Check for crack width passed with additional reinforcements
                   1018   0.902   2.872 0.163-0.079     0.000    0.173        4096
                   1018   0.621   2.151 0.188-0.054     0.000    0.173        5468
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    220.21   176.67       12.29
                                 ---- Check for crack width passed with additional reinforcements
  1030  0.000    1 1017   0.902   2.872 0.163-0.079     0.000    0.173        4096
                   1017   0.621   2.151 0.188-0.054     0.000    0.173        5468
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    220.21   176.67       12.29
                                 ---- Check for crack width passed with additional reinforcements
                   1018   0.902   2.872 0.163-0.079     0.000    0.173        4096
                   1018   0.621   2.151 0.188-0.054     0.000    0.173        5468
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    220.21   176.67       12.29
                                 ---- Check for crack width passed with additional reinforcements
        0.883    1 1017   0.897   2.877 0.165-0.077     0.000    0.178        4203
                   1017   0.629   2.188 0.189-0.052     0.000    0.178        5526
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    223.43   174.36       12.47
                                 ---- Check for crack width passed with additional reinforcements
                   1018   0.897   2.877 0.165-0.077     0.000    0.178        4203
                   1018   0.629   2.188 0.189-0.052     0.000    0.178        5526
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    223.43   174.36       12.47
                                 ---- Check for crack width passed with additional reinforcements
  1031  0.000    1 1017   0.897   2.877 0.165-0.077     0.000    0.178        4203
                   1017   0.629   2.188 0.189-0.052     0.000    0.178        5526
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    223.43   174.36       12.47
                                 ---- Check for crack width passed with additional reinforcements
                   1018   0.897   2.877 0.165-0.077     0.000    0.178        4203
                   1018   0.629   2.188 0.189-0.052     0.000    0.178        5526
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    223.43   174.36       12.47
                                 ---- Check for crack width passed with additional reinforcements
        0.883    1 1017   0.902   2.872 0.163-0.079     0.000    0.173        4096
                   1017   0.621   2.151 0.188-0.054     0.000    0.173        5468
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    220.21   176.67       12.29
                                 ---- Check for crack width passed with additional reinforcements
                   1018   0.902   2.872 0.163-0.079     0.000    0.173        4096
                   1018   0.621   2.151 0.188-0.054     0.000    0.173        5468
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    220.21   176.67       12.29
                                 ---- Check for crack width passed with additional reinforcements
  1032  0.000    1 1017   0.902   2.872 0.163-0.079     0.000    0.173        4096
                   1017   0.621   2.151 0.188-0.054     0.000    0.173        5468
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    220.21   176.67       12.29
                                 ---- Check for crack width passed with additional reinforcements
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ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_SLS_DOKOI

Nonlinear Stresses

```
  Beam     x[m] NoS   LC     e-o    ky/kz     x  zn/yn      Ni/Vi   Myi/Mzi  Ey/Ez/G-EFF
                             [o/oo]  [1/km]   [m]   [m]       [MN]    [MNm]       [MPa]
  1032    0.000   1 1018    0.902    2.872 0.163-0.079      0.000    0.173        4096
                    1018    0.621    2.151 0.188-0.054      0.000    0.173        5468
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    220.21   176.67       12.29
                         ---- Check for crack width passed with additional reinforcements
          0.883   1 1017    0.918    2.856 0.155-0.087      0.000    0.158        3771
                    1017    0.609    2.070 0.183-0.059      0.000    0.158        5203
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.23   187.68       11.52
                         ---- Check for crack width passed with additional reinforcements
                    1018    0.918    2.856 0.155-0.087      0.000    0.158        3771
                    1018    0.609    2.070 0.183-0.059      0.000    0.158        5203
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.23   187.68       11.52
                         ---- Check for crack width passed with additional reinforcements
  1033    0.000   1 1017    0.918    2.856 0.155-0.087      0.000    0.158        3771
                    1017    0.609    2.070 0.183-0.059      0.000    0.158        5203
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.23   187.68       11.52
                         ---- Check for crack width passed with additional reinforcements
                    1018    0.918    2.856 0.155-0.087      0.000    0.158        3771
                    1018    0.609    2.070 0.183-0.059      0.000    0.158        5203
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.23   187.68       11.52
                         ---- Check for crack width passed with additional reinforcements
          0.883   1 1017    0.947    2.830 0.142-0.100      0.000    0.133        3220
                    1017    0.621    2.016 0.169-0.073      0.000    0.133        4521
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.19   221.57        9.65
                         ---- Check for crack width passed with additional reinforcements
                    1018    0.947    2.830 0.142-0.100      0.000    0.133        3220
                    1018    0.621    2.016 0.169-0.073      0.000    0.133        4521
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.19   221.57        9.65
                         ---- Check for crack width passed with additional reinforcements
  1034    0.000   1 1017    0.947    2.830 0.142-0.100      0.000    0.133        3220
                    1017    0.621    2.016 0.169-0.073      0.000    0.133        4521
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.19   221.57        9.65
                         ---- Check for crack width passed with additional reinforcements
                    1018    0.947    2.830 0.142-0.100      0.000    0.133        3220
                    1018    0.621    2.016 0.169-0.073      0.000    0.133        4521
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.19   221.57        9.65
                         ---- Check for crack width passed with additional reinforcements
          0.883   1 1017    0.984    2.777 0.122-0.120      0.000    0.099        2441
                    1017    0.638    1.935 0.147-0.095      0.000    0.099        3503
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.12   297.53        7.06
                         ---- Check for crack width passed with additional reinforcements
                    1018    0.984    2.777 0.122-0.120      0.000    0.099        2441
                    1018    0.638    1.935 0.147-0.095      0.000    0.099        3503
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.12   297.53        7.06
                         ---- Check for crack width passed with additional reinforcements
  1035    0.000   1 1017    0.984    2.777 0.122-0.120      0.000    0.099        2441
                    1017    0.638    1.935 0.147-0.095      0.000    0.099        3503
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.12   297.53        7.06
                         ---- Check for crack width passed with additional reinforcements
                    1018    0.984    2.777 0.122-0.120      0.000    0.099        2441
                    1018    0.638    1.935 0.147-0.095      0.000    0.099        3503
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.12   297.53        7.06
                         ---- Check for crack width passed with additional reinforcements
          0.883   1 1017    1.018    2.643 0.092-0.150      0.000    0.054        1417
                    1017    0.665    1.816 0.111-0.131      0.000    0.054        2063
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.05   537.35        3.81
                         ---- Check for crack width passed with additional reinforcements
                    1018    1.018    2.643 0.092-0.150      0.000    0.054        1417
                    1018    0.665    1.816 0.111-0.131      0.000    0.054        2063
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.05   537.35        3.81
                         ---- Check for crack width passed with additional reinforcements
  1036    0.000   1 1017    1.018    2.643 0.092-0.150      0.000    0.054        1417
                    1017    0.665    1.816 0.111-0.131      0.000    0.054        2063
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ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_SLS_DOKOI

Nonlinear Stresses

```
  Beam     x[m] NoS   LC     e-o   ky/kz    x  zn/yn     Ni/Vi  Myi/Mzi Ey/Ez/G-EFF
                           [o/oo]  [1/km]   [m]   [m]      [MN]    [MNm]       [MPa]
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.05   538.88        3.81
                                 ---- Check for crack width passed with additional reinforcements
                          1018   1.018   2.643 0.092-0.150     0.000    0.054        1417
                          1018   0.665   1.816 0.111-0.131     0.000    0.054        2063
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.05   538.88        3.81
                                 ---- Check for crack width passed with additional reinforcements
         0.883   1 1017   0.000   0.000 0.477   -.-     0.000    0.000       26663
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.273  20.0            0.00                 0.00
                                 ---- Check for crack width passed with given reinforcements
                          1018   0.000   0.000 0.477   -.-     0.000    0.000       26663
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.273  20.0            0.00                 0.00
                                 ---- Check for crack width passed with given reinforcements
  1037   0.000   1 1017   0.000   0.000 0.750   -.-     0.000    0.000       26663
                          1018   0.000   0.000 0.750   -.-     0.000    0.000       26663
         0.883   1 1017   1.018   2.643 0.092-0.150     0.000    0.054        1417
                          1017   0.665   1.816 0.111-0.131     0.000    0.054        2063
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.05   537.35        3.81
                                 ---- Check for crack width passed with additional reinforcements
                          1018   1.018   2.643 0.092-0.150     0.000    0.054        1417
                          1018   0.665   1.816 0.111-0.131     0.000    0.054        2063
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.05   537.35        3.81
                                 ---- Check for crack width passed with additional reinforcements
  1038   0.000   1 1017   1.018   2.643 0.092-0.150     0.000    0.054        1417
                          1017   0.665   1.816 0.111-0.131     0.000    0.054        2063
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.05   537.35        3.81
                                 ---- Check for crack width passed with additional reinforcements
                          1018   1.018   2.643 0.092-0.150     0.000    0.054        1417
                          1018   0.665   1.816 0.111-0.131     0.000    0.054        2063
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.05   537.35        3.81
                                 ---- Check for crack width passed with additional reinforcements
         0.883   1 1017   0.984   2.777 0.122-0.120     0.000    0.099        2441
                          1017   0.638   1.935 0.147-0.095     0.000    0.099        3503
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.12   297.53        7.06
                                 ---- Check for crack width passed with additional reinforcements
                          1018   0.984   2.777 0.122-0.120     0.000    0.099        2441
                          1018   0.638   1.935 0.147-0.095     0.000    0.099        3503
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.12   297.53        7.06
                                 ---- Check for crack width passed with additional reinforcements
  1039   0.000   1 1017   0.984   2.777 0.122-0.120     0.000    0.099        2441
                          1017   0.638   1.935 0.147-0.095     0.000    0.099        3503
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.12   297.53        7.06
                                 ---- Check for crack width passed with additional reinforcements
                          1018   0.984   2.777 0.122-0.120     0.000    0.099        2441
                          1018   0.638   1.935 0.147-0.095     0.000    0.099        3503
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.12   297.53        7.06
                                 ---- Check for crack width passed with additional reinforcements
         0.883   1 1017   0.947   2.831 0.142-0.100     0.000    0.133        3220
                          1017   0.621   2.016 0.169-0.073     0.000    0.133        4521
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.19   221.57        9.65
                                 ---- Check for crack width passed with additional reinforcements
                          1018   0.947   2.831 0.142-0.100     0.000    0.133        3220
                          1018   0.621   2.016 0.169-0.073     0.000    0.133        4521
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.19   221.57        9.65
                                 ---- Check for crack width passed with additional reinforcements
  1040   0.000   1 1017   0.947   2.831 0.142-0.100     0.000    0.133        3220
                          1017   0.621   2.016 0.169-0.073     0.000    0.133        4521
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.19   221.57        9.65
                                 ---- Check for crack width passed with additional reinforcements
                          1018   0.947   2.831 0.142-0.100     0.000    0.133        3220
                          1018   0.621   2.016 0.169-0.073     0.000    0.133        4521
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.19   221.57        9.65
                                 ---- Check for crack width passed with additional reinforcements
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ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_SLS_DOKOI

Nonlinear Stresses

```
  Beam     x[m] NoS    LC     e-o    ky/kz     x  zn/yn      Ni/Vi   Myi/Mzi  Ey/Ez/G-EFF
                             [o/oo]  [1/km]   [m]   [m]       [MN]    [MNm]        [MPa]
  1040    0.883   1 1017    0.918    2.856 0.155-0.087      0.000    0.158         3771
                    1017    0.609    2.070 0.183-0.059      0.000    0.158         5203
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.23   187.68        11.52
                         ---- Check for crack width passed with additional reinforcements
                    1018    0.918    2.856 0.155-0.087      0.000    0.158         3771
                    1018    0.609    2.070 0.183-0.059      0.000    0.158         5203
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.23   187.68        11.52
                         ---- Check for crack width passed with additional reinforcements
  1041    0.000   1 1017    0.918    2.856 0.155-0.087      0.000    0.158         3771
                    1017    0.609    2.070 0.183-0.059      0.000    0.158         5203
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.23   187.68        11.52
                         ---- Check for crack width passed with additional reinforcements
                    1018    0.918    2.856 0.155-0.087      0.000    0.158         3771
                    1018    0.609    2.070 0.183-0.059      0.000    0.158         5203
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.23   187.68        11.52
                         ---- Check for crack width passed with additional reinforcements
          0.883   1 1017    0.902    2.872 0.163-0.079      0.000    0.173         4096
                    1017    0.621    2.151 0.188-0.054      0.000    0.173         5468
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    220.21   176.67        12.29
                         ---- Check for crack width passed with additional reinforcements
                    1018    0.902    2.872 0.163-0.079      0.000    0.173         4096
                    1018    0.621    2.151 0.188-0.054      0.000    0.173         5468
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    220.21   176.67        12.29
                         ---- Check for crack width passed with additional reinforcements
  1042    0.000   1 1017    0.902    2.872 0.163-0.079      0.000    0.173         4096
                    1017    0.621    2.151 0.188-0.054      0.000    0.173         5468
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    220.21   176.67        12.29
                         ---- Check for crack width passed with additional reinforcements
                    1018    0.902    2.872 0.163-0.079      0.000    0.173         4096
                    1018    0.621    2.151 0.188-0.054      0.000    0.173         5468
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    220.21   176.67        12.29
                         ---- Check for crack width passed with additional reinforcements
          0.883   1 1017    0.897    2.877 0.165-0.077      0.000    0.178         4203
                    1017    0.629    2.188 0.189-0.052      0.000    0.178         5526
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    223.43   174.36        12.47
                         ---- Check for crack width passed with additional reinforcements
                    1018    0.897    2.877 0.165-0.077      0.000    0.178         4203
                    1018    0.629    2.188 0.189-0.052      0.000    0.178         5526
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    223.43   174.36        12.47
                         ---- Check for crack width passed with additional reinforcements
  1043    0.000   1 1017    0.897    2.877 0.165-0.077      0.000    0.178         4203
                    1017    0.629    2.188 0.189-0.052      0.000    0.178         5526
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    223.43   174.36        12.47
                         ---- Check for crack width passed with additional reinforcements
                    1018    0.897    2.877 0.165-0.077      0.000    0.178         4203
                    1018    0.629    2.188 0.189-0.052      0.000    0.178         5526
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    223.43   174.36        12.47
                         ---- Check for crack width passed with additional reinforcements
          0.883   1 1017    0.902    2.872 0.163-0.079      0.000    0.173         4096
                    1017    0.621    2.151 0.188-0.054      0.000    0.173         5468
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    220.21   176.67        12.29
                         ---- Check for crack width passed with additional reinforcements
                    1018    0.902    2.872 0.163-0.079      0.000    0.173         4096
                    1018    0.621    2.151 0.188-0.054      0.000    0.173         5468
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    220.21   176.67        12.29
                         ---- Check for crack width passed with additional reinforcements
  1044    0.000   1 1017    0.902    2.872 0.163-0.079      0.000    0.173         4096
                    1017    0.621    2.151 0.188-0.054      0.000    0.173         5468
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    220.21   176.67        12.29
                         ---- Check for crack width passed with additional reinforcements
                    1018    0.902    2.872 0.163-0.079      0.000    0.173         4096
                    1018    0.621    2.151 0.188-0.054      0.000    0.173         5468
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ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_SLS_DOKOI

Nonlinear Stresses

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  Beam     x[m] NoS   LC     e-o   ky/kz     x  zn/yn      Ni/Vi   Myi/Mzi Ey/Ez/G-EFF
                           [o/oo]  [1/km]   [m]   [m]       [MN]    [MNm]       [MPa]
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    220.21   176.67       12.29
                                 ---- Check for crack width passed with additional reinforcements
        0.883   1 1017   0.918   2.856 0.155-0.087     0.000    0.158        3771
                  1017   0.609   2.070 0.183-0.059     0.000    0.158        5203
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.23   187.68       11.52
                                 ---- Check for crack width passed with additional reinforcements
                  1018   0.918   2.856 0.155-0.087     0.000    0.158        3771
                  1018   0.609   2.070 0.183-0.059     0.000    0.158        5203
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.23   187.68       11.52
                                 ---- Check for crack width passed with additional reinforcements
  1045  0.000   1 1017   0.918   2.856 0.155-0.087     0.000    0.158        3771
                  1017   0.609   2.070 0.183-0.059     0.000    0.158        5203
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.23   187.68       11.52
                                 ---- Check for crack width passed with additional reinforcements
                  1018   0.918   2.856 0.155-0.087     0.000    0.158        3771
                  1018   0.609   2.070 0.183-0.059     0.000    0.158        5203
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.23   187.68       11.52
                                 ---- Check for crack width passed with additional reinforcements
        0.883   1 1017   0.947   2.831 0.142-0.100     0.000    0.133        3220
                  1017   0.621   2.016 0.169-0.073     0.000    0.133        4521
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.19   221.57        9.65
                                 ---- Check for crack width passed with additional reinforcements
                  1018   0.947   2.831 0.142-0.100     0.000    0.133        3220
                  1018   0.621   2.016 0.169-0.073     0.000    0.133        4521
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.19   221.57        9.65
                                 ---- Check for crack width passed with additional reinforcements
  1046  0.000   1 1017   0.947   2.831 0.142-0.100     0.000    0.133        3220
                  1017   0.621   2.016 0.169-0.073     0.000    0.133        4521
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.19   221.57        9.65
                                 ---- Check for crack width passed with additional reinforcements
                  1018   0.947   2.831 0.142-0.100     0.000    0.133        3220
                  1018   0.621   2.016 0.169-0.073     0.000    0.133        4521
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.19   221.57        9.65
                                 ---- Check for crack width passed with additional reinforcements
        0.883   1 1017   0.984   2.777 0.122-0.120     0.000    0.099        2441
                  1017   0.638   1.935 0.147-0.095     0.000    0.099        3503
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.12   297.53        7.06
                                 ---- Check for crack width passed with additional reinforcements
                  1018   0.984   2.777 0.122-0.120     0.000    0.099        2441
                  1018   0.638   1.935 0.147-0.095     0.000    0.099        3503
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.12   297.53        7.06
                                 ---- Check for crack width passed with additional reinforcements
  1047  0.000   1 1017   0.984   2.777 0.122-0.120     0.000    0.099        2441
                  1017   0.638   1.935 0.147-0.095     0.000    0.099        3503
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.12   297.53        7.06
                                 ---- Check for crack width passed with additional reinforcements
                  1018   0.984   2.777 0.122-0.120     0.000    0.099        2441
                  1018   0.638   1.935 0.147-0.095     0.000    0.099        3503
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.12   297.53        7.06
                                 ---- Check for crack width passed with additional reinforcements
        0.883   1 1017   1.018   2.643 0.092-0.150     0.000    0.054        1417
                  1017   0.665   1.816 0.111-0.131     0.000    0.054        2063
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.05   537.35        3.81
                                 ---- Check for crack width passed with additional reinforcements
                  1018   1.018   2.643 0.092-0.150     0.000    0.054        1417
                  1018   0.665   1.816 0.111-0.131     0.000    0.054        2063
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.05   537.35        3.81
                                 ---- Check for crack width passed with additional reinforcements
  1048  0.000   1 1017   1.018   2.643 0.092-0.150     0.000    0.054        1417
                  1017   0.665   1.816 0.111-0.131     0.000    0.054        2063
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.05   538.88        3.81
                                 ---- Check for crack width passed with additional reinforcements
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ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_SLS_DOKOI

Nonlinear Stresses

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  Beam     x[m] NoS   LC     e-o   ky/kz     x  zn/yn      Ni/Vi   Myi/Mzi Ey/Ez/G-EFF
                             [o/oo]  [1/km]   [m]   [m]       [MN]    [MNm]       [MPa]
  1048    0.000   1 1018    1.018   2.643 0.092-0.150      0.000    0.054        1417
                  1018    0.665   1.816 0.111-0.131      0.000    0.054        2063
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.125  20.0  0.30    214.05   538.88        3.81
                        ---- Check for crack width passed with additional reinforcements
          0.883   1 1017    0.000   0.000 0.273   -.-      0.000    0.000       26663
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.477  20.0            0.00                    0.00
                        ---- Check for crack width passed with given reinforcements
                  1018    0.000   0.000 0.273   -.-      0.000    0.000       26663
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.477  20.0            0.00                    0.00
                        ---- Check for crack width passed with given reinforcements
  1049    0.000   1 1017    0.000   0.000 0.750   -.-      0.000    0.000       26663
                  1018    0.000   0.000 0.750   -.-      0.000    0.000       26663
          0.883   1 1017    1.018   2.643 0.092-0.150      0.000    0.054        1417
                  1017    0.665   1.816 0.111-0.131      0.000    0.054        2063
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.125  20.0  0.30    214.05   537.35        3.81
                        ---- Check for crack width passed with additional reinforcements
                  1018    1.018   2.643 0.092-0.150      0.000    0.054        1417
                  1018    0.665   1.816 0.111-0.131      0.000    0.054        2063
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.125  20.0  0.30    214.05   537.35        3.81
                        ---- Check for crack width passed with additional reinforcements
  1050    0.000   1 1017    1.018   2.643 0.092-0.150      0.000    0.054        1417
                  1017    0.665   1.816 0.111-0.131      0.000    0.054        2063
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.125  20.0  0.30    214.05   537.35        3.81
                        ---- Check for crack width passed with additional reinforcements
                  1018    1.018   2.643 0.092-0.150      0.000    0.054        1417
                  1018    0.665   1.816 0.111-0.131      0.000    0.054        2063
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.125  20.0  0.30    214.05   537.35        3.81
                        ---- Check for crack width passed with additional reinforcements
          0.883   1 1017    0.984   2.777 0.122-0.120      0.000    0.099        2441
                  1017    0.638   1.935 0.147-0.095      0.000    0.099        3503
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.125  20.0  0.30    214.12   297.53        7.06
                        ---- Check for crack width passed with additional reinforcements
                  1018    0.984   2.777 0.122-0.120      0.000    0.099        2441
                  1018    0.638   1.935 0.147-0.095      0.000    0.099        3503
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.125  20.0  0.30    214.12   297.53        7.06
                        ---- Check for crack width passed with additional reinforcements
  1051    0.000   1 1017    0.984   2.777 0.122-0.120      0.000    0.099        2441
                  1017    0.638   1.935 0.147-0.095      0.000    0.099        3503
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.125  20.0  0.30    214.12   297.53        7.06
                        ---- Check for crack width passed with additional reinforcements
                  1018    0.984   2.777 0.122-0.120      0.000    0.099        2441
                  1018    0.638   1.935 0.147-0.095      0.000    0.099        3503
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.125  20.0  0.30    214.12   297.53        7.06
                        ---- Check for crack width passed with additional reinforcements
          0.883   1 1017    0.947   2.831 0.142-0.100      0.000    0.133        3220
                  1017    0.621   2.016 0.169-0.073      0.000    0.133        4521
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.125  20.0  0.30    214.19   221.57        9.65
                        ---- Check for crack width passed with additional reinforcements
                  1018    0.947   2.831 0.142-0.100      0.000    0.133        3220
                  1018    0.621   2.016 0.169-0.073      0.000    0.133        4521
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.125  20.0  0.30    214.19   221.57        9.65
                        ---- Check for crack width passed with additional reinforcements
  1052    0.000   1 1017    0.947   2.830 0.142-0.100      0.000    0.133        3220
                  1017    0.621   2.016 0.169-0.073      0.000    0.133        4521
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.125  20.0  0.30    214.19   221.57        9.65
                        ---- Check for crack width passed with additional reinforcements
                  1018    0.947   2.830 0.142-0.100      0.000    0.133        3220
                  1018    0.621   2.016 0.169-0.073      0.000    0.133        4521
                        --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                        T-zone h= 0.125  20.0  0.30    214.19   221.57        9.65
                        ---- Check for crack width passed with additional reinforcements
          0.883   1 1017    0.918   2.856 0.155-0.087      0.000    0.158        3771
                  1017    0.609   2.070 0.183-0.059      0.000    0.158        5203
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ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_SLS_DOKOI

Nonlinear Stresses

```
  Beam     x[m] NoS   LC     e-o   ky/kz    x  zn/yn     Ni/Vi  Myi/Mzi Ey/Ez/G-EFF
                           [o/oo]  [1/km]   [m]   [m]      [MN]    [MNm]       [MPa]
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.23   187.68       11.52
                                 ---- Check for crack width passed with additional reinforcements
                        1018   0.918   2.856 0.155-0.087     0.000    0.158        3771
                        1018   0.609   2.070 0.183-0.059     0.000    0.158        5203
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.23   187.68       11.52
                                 ---- Check for crack width passed with additional reinforcements
  1053   0.000   1 1017   0.918   2.856 0.155-0.087     0.000    0.158        3771
                        1017   0.609   2.070 0.183-0.059     0.000    0.158        5203
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.23   187.68       11.52
                                 ---- Check for crack width passed with additional reinforcements
                        1018   0.918   2.856 0.155-0.087     0.000    0.158        3771
                        1018   0.609   2.070 0.183-0.059     0.000    0.158        5203
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    214.23   187.68       11.52
                                 ---- Check for crack width passed with additional reinforcements
         0.883   1 1017   0.902   2.872 0.163-0.079     0.000    0.173        4096
                        1017   0.621   2.151 0.188-0.054     0.000    0.173        5468
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    220.21   176.67       12.29
                                 ---- Check for crack width passed with additional reinforcements
                        1018   0.902   2.872 0.163-0.079     0.000    0.173        4096
                        1018   0.621   2.151 0.188-0.054     0.000    0.173        5468
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    220.21   176.67       12.29
                                 ---- Check for crack width passed with additional reinforcements
  1054   0.000   1 1017   0.902   2.872 0.163-0.079     0.000    0.173        4096
                        1017   0.621   2.151 0.188-0.054     0.000    0.173        5468
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    220.21   176.67       12.29
                                 ---- Check for crack width passed with additional reinforcements
                        1018   0.902   2.872 0.163-0.079     0.000    0.173        4096
                        1018   0.621   2.151 0.188-0.054     0.000    0.173        5468
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    220.21   176.67       12.29
                                 ---- Check for crack width passed with additional reinforcements
         0.883   1 1017   0.897   2.877 0.165-0.077     0.000    0.178        4203
                        1017   0.629   2.188 0.189-0.052     0.000    0.178        5526
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    223.43   174.36       12.47
                                 ---- Check for crack width passed with additional reinforcements
                        1018   0.897   2.877 0.165-0.077     0.000    0.178        4203
                        1018   0.629   2.188 0.189-0.052     0.000    0.178        5526
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    223.43   174.36       12.47
                                 ---- Check for crack width passed with additional reinforcements
  1055   0.000   1 1017   0.897   2.877 0.165-0.077     0.000    0.178        4203
                        1017   0.629   2.188 0.189-0.052     0.000    0.178        5526
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    223.43   174.36       12.47
                                 ---- Check for crack width passed with additional reinforcements
                        1018   0.897   2.877 0.165-0.077     0.000    0.178        4203
                        1018   0.629   2.188 0.189-0.052     0.000    0.178        5526
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    223.43   174.36       12.47
                                 ---- Check for crack width passed with additional reinforcements
         0.883   1 1017   0.902   2.872 0.163-0.079     0.000    0.173        4096
                        1017   0.621   2.151 0.188-0.054     0.000    0.173        5468
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    220.21   176.67       12.29
                                 ---- Check for crack width passed with additional reinforcements
                        1018   0.902   2.872 0.163-0.079     0.000    0.173        4096
                        1018   0.621   2.151 0.188-0.054     0.000    0.173        5468
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    220.21   176.67       12.29
                                 ---- Check for crack width passed with additional reinforcements
  1056   0.000   1 1017   0.902   2.872 0.163-0.079     0.000    0.173        4096
                        1017   0.621   2.151 0.188-0.054     0.000    0.173        5468
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    220.21   176.67       12.29
                                 ---- Check for crack width passed with additional reinforcements
                        1018   0.902   2.872 0.163-0.079     0.000    0.173        4096
                        1018   0.621   2.151 0.188-0.054     0.000    0.173        5468
                                 --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                                 T-zone h= 0.125  20.0  0.30    220.21   176.67       12.29
                                 ---- Check for crack width passed with additional reinforcements
```

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_SLS_DOKOI

Nonlinear Stresses

```
  Beam     x[m] NoS   LC     e-o    ky/kz     x  zn/yn      Ni/Vi   Myi/Mzi  Ey/Ez/G-EFF
                             [o/oo]  [1/km]   [m]   [m]       [MN]    [MNm]       [MPa]
  1056    0.883   1 1017    0.918    2.856 0.155-0.087      0.000    0.158        3771
                    1017    0.609    2.070 0.183-0.059      0.000    0.158        5203
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.23   187.68       11.52
                         ---- Check for crack width passed with additional reinforcements
                    1018    0.918    2.856 0.155-0.087      0.000    0.158        3771
                    1018    0.609    2.070 0.183-0.059      0.000    0.158        5203
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.23   187.68       11.52
                         ---- Check for crack width passed with additional reinforcements
  1057    0.000   1 1017    0.918    2.856 0.155-0.087      0.000    0.158        3771
                    1017    0.609    2.070 0.183-0.059      0.000    0.158        5203
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.23   187.68       11.52
                         ---- Check for crack width passed with additional reinforcements
                    1018    0.918    2.856 0.155-0.087      0.000    0.158        3771
                    1018    0.609    2.070 0.183-0.059      0.000    0.158        5203
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.23   187.68       11.52
                         ---- Check for crack width passed with additional reinforcements
          0.883   1 1017    0.947    2.831 0.142-0.100      0.000    0.133        3220
                    1017    0.621    2.016 0.169-0.073      0.000    0.133        4521
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.19   221.57        9.65
                         ---- Check for crack width passed with additional reinforcements
                    1018    0.947    2.831 0.142-0.100      0.000    0.133        3220
                    1018    0.621    2.016 0.169-0.073      0.000    0.133        4521
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.19   221.57        9.65
                         ---- Check for crack width passed with additional reinforcements
  1058    0.000   1 1017    0.947    2.831 0.142-0.100      0.000    0.133        3220
                    1017    0.621    2.016 0.169-0.073      0.000    0.133        4521
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.19   221.57        9.65
                         ---- Check for crack width passed with additional reinforcements
                    1018    0.947    2.831 0.142-0.100      0.000    0.133        3220
                    1018    0.621    2.016 0.169-0.073      0.000    0.133        4521
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.19   221.57        9.65
                         ---- Check for crack width passed with additional reinforcements
          0.883   1 1017    0.984    2.777 0.122-0.120      0.000    0.099        2441
                    1017    0.638    1.935 0.147-0.095      0.000    0.099        3503
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.12   297.53        7.06
                         ---- Check for crack width passed with additional reinforcements
                    1018    0.984    2.777 0.122-0.120      0.000    0.099        2441
                    1018    0.638    1.935 0.147-0.095      0.000    0.099        3503
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.12   297.53        7.06
                         ---- Check for crack width passed with additional reinforcements
  1059    0.000   1 1017    0.984    2.777 0.122-0.120      0.000    0.099        2441
                    1017    0.638    1.935 0.147-0.095      0.000    0.099        3503
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.12   297.53        7.06
                         ---- Check for crack width passed with additional reinforcements
                    1018    0.984    2.777 0.122-0.120      0.000    0.099        2441
                    1018    0.638    1.935 0.147-0.095      0.000    0.099        3503
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.12   297.53        7.06
                         ---- Check for crack width passed with additional reinforcements
          0.883   1 1017    1.018    2.643 0.092-0.150      0.000    0.054        1417
                    1017    0.665    1.816 0.111-0.131      0.000    0.054        2063
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.05   537.35        3.81
                         ---- Check for crack width passed with additional reinforcements
                    1018    1.018    2.643 0.092-0.150      0.000    0.054        1417
                    1018    0.665    1.816 0.111-0.131      0.000    0.054        2063
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.05   537.35        3.81
                         ---- Check for crack width passed with additional reinforcements
  1060    0.000   1 1017    1.018    2.643 0.092-0.150      0.000    0.054        1417
                    1017    0.665    1.816 0.111-0.131      0.000    0.054        2063
                         --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                         T-zone h= 0.125  20.0  0.30    214.05   538.88        3.81
                         ---- Check for crack width passed with additional reinforcements
                    1018    1.018    2.643 0.092-0.150      0.000    0.054        1417
                    1018    0.665    1.816 0.111-0.131      0.000    0.054        2063
```

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_SLS_DOKOI

Nonlinear Stresses

```
  Beam     x[m] NoS   LC     e-o   ky/kz    x  zn/yn     Ni/Vi  Myi/Mzi Ey/Ez/G-EFF
                           [o/oo]  [1/km]   [m]   [m]      [MN]    [MNm]       [MPa]
                          --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                          T-zone h= 0.125  20.0  0.30    214.05   538.88        3.81
                          ---- Check for crack width passed with additional reinforcements
         0.883   1 1017   0.000   0.000 0.273   -.-     0.000    0.000       26663
                          --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                          T-zone h= 0.477  20.0            0.00                 0.00
                          ---- Check for crack width passed with given reinforcements
                     1018   0.000   0.000 0.273   -.-     0.000    0.000       26663
                          --------------- D[mm] w[mm]  sig[MPa] ssr[MPa] As-eff[cm2]
                          T-zone h= 0.477  20.0            0.00                 0.00
                          ---- Check for crack width passed with given reinforcements
```

Parameters for nonlinear stress / Crackwidth DIN 1045-1 (neu)

MNo	design width [mm]	[mm]	bond [-]	load [-]	h-max [m]
12	0.300	0.300	0.80	0.25	0.800

Check for crack width passed with additional reinforcements

Stiffness is not saved in database

Longitudinal Reinforcements LCR 503
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
1001	0.000	1	0.05	2.37		2.37				
1001	0.883	1	0.12	6.19		2.37	3.81			
1002	0.000	1	0.12	6.19		2.37	3.81			
1002	0.883	1	0.19	9.44		2.37	7.06			
1003	0.000	1	0.19	9.44		2.37	7.06			
1003	0.883	1	0.24	12.02		2.37	9.65			
1004	0.000	1	0.24	12.02		2.37	9.65			
1004	0.883	1	0.28	13.89		2.37	11.52			
1005	0.000	1	0.28	13.89		2.37	11.52			
1005	0.883	1	0.29	14.66		2.37	12.29			
1006	0.000	1	0.29	14.66		2.37	12.29			
1006	0.883	1	0.30	14.84		2.37	12.47			
1007	0.000	1	0.30	14.84		2.37	12.47			
1007	0.883	1	0.29	14.66		2.37	12.29			
1008	0.000	1	0.29	14.66		2.37	12.29			
1008	0.883	1	0.28	13.89		2.37	11.52			
1009	0.000	1	0.28	13.89		2.37	11.52			
1009	0.883	1	0.24	12.02		2.37	9.65			
1010	0.000	1	0.24	12.02		2.37	9.65			
1010	0.883	1	0.19	9.44		2.37	7.06			
1011	0.000	1	0.19	9.44		2.37	7.06			
1011	0.883	1	0.12	6.19		2.37	3.81			
1012	0.000	1	0.12	6.19		2.37	3.81			
1012	0.883	1	0.05	2.37		2.37				
1013	0.000	1	0.05	2.37		2.37				
1013	0.883	1	0.12	6.19		2.37	3.81			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_SLS_DOKOI

Longitudinal Reinforcements LCR 503
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
1014	0.000	1	0.12	6.19		2.37	3.81			
1014	0.883	1	0.19	9.44		2.37	7.06			
1015	0.000	1	0.19	9.44		2.37	7.06			
1015	0.883	1	0.24	12.02		2.37	9.65			
1016	0.000	1	0.24	12.02		2.37	9.65			
1016	0.883	1	0.28	13.89		2.37	11.52			
1017	0.000	1	0.28	13.89		2.37	11.52			
1017	0.883	1	0.29	14.66		2.37	12.29			
1018	0.000	1	0.29	14.66		2.37	12.29			
1018	0.883	1	0.30	14.84		2.37	12.47			
1019	0.000	1	0.30	14.84		2.37	12.47			
1019	0.883	1	0.29	14.66		2.37	12.29			
1020	0.000	1	0.29	14.66		2.37	12.29			
1020	0.883	1	0.28	13.89		2.37	11.52			
1021	0.000	1	0.28	13.89		2.37	11.52			
1021	0.883	1	0.24	12.02		2.37	9.65			
1022	0.000	1	0.24	12.02		2.37	9.65			
1022	0.883	1	0.19	9.44		2.37	7.06			
1023	0.000	1	0.19	9.44		2.37	7.06			
1023	0.883	1	0.12	6.19		2.37	3.81			
1024	0.000	1	0.12	6.19		2.37	3.81			
1024	0.883	1	0.05	2.37		2.37				
1025	0.000	1	0.05	2.37		2.37				
1025	0.883	1	0.12	6.19		2.37	3.81			
1026	0.000	1	0.12	6.19		2.37	3.81			
1026	0.883	1	0.19	9.44		2.37	7.06			
1027	0.000	1	0.19	9.44		2.37	7.06			
1027	0.883	1	0.24	12.02		2.37	9.65			
1028	0.000	1	0.24	12.02		2.37	9.65			
1028	0.883	1	0.28	13.89		2.37	11.52			
1029	0.000	1	0.28	13.89		2.37	11.52			
1029	0.883	1	0.29	14.66		2.37	12.29			
1030	0.000	1	0.29	14.66		2.37	12.29			
1030	0.883	1	0.30	14.84		2.37	12.47			
1031	0.000	1	0.30	14.84		2.37	12.47			
1031	0.883	1	0.29	14.66		2.37	12.29			
1032	0.000	1	0.29	14.66		2.37	12.29			
1032	0.883	1	0.28	13.89		2.37	11.52			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_SLS_DOKOI

Longitudinal Reinforcements LCR 503
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
1033	0.000	1	0.28	13.89		2.37	11.52			
1033	0.883	1	0.24	12.02		2.37	9.65			
1034	0.000	1	0.24	12.02		2.37	9.65			
1034	0.883	1	0.19	9.44		2.37	7.06			
1035	0.000	1	0.19	9.44		2.37	7.06			
1035	0.883	1	0.12	6.19		2.37	3.81			
1036	0.000	1	0.12	6.19		2.37	3.81			
1036	0.883	1	0.05	2.37		2.37				
1037	0.000	1	0.05	2.37		2.37				
1037	0.883	1	0.12	6.19		2.37	3.81			
1038	0.000	1	0.12	6.19		2.37	3.81			
1038	0.883	1	0.19	9.44		2.37	7.06			
1039	0.000	1	0.19	9.44		2.37	7.06			
1039	0.883	1	0.24	12.02		2.37	9.65			
1040	0.000	1	0.24	12.02		2.37	9.65			
1040	0.883	1	0.28	13.89		2.37	11.52			
1041	0.000	1	0.28	13.89		2.37	11.52			
1041	0.883	1	0.29	14.66		2.37	12.29			
1042	0.000	1	0.29	14.66		2.37	12.29			
1042	0.883	1	0.30	14.84		2.37	12.47			
1043	0.000	1	0.30	14.84		2.37	12.47			
1043	0.883	1	0.29	14.66		2.37	12.29			
1044	0.000	1	0.29	14.66		2.37	12.29			
1044	0.883	1	0.28	13.89		2.37	11.52			
1045	0.000	1	0.28	13.89		2.37	11.52			
1045	0.883	1	0.24	12.02		2.37	9.65			
1046	0.000	1	0.24	12.02		2.37	9.65			
1046	0.883	1	0.19	9.44		2.37	7.06			
1047	0.000	1	0.19	9.44		2.37	7.06			
1047	0.883	1	0.12	6.19		2.37	3.81			
1048	0.000	1	0.12	6.19		2.37	3.81			
1048	0.883	1	0.05	2.37		2.37				
1049	0.000	1	0.05	2.37		2.37				
1049	0.883	1	0.12	6.19		2.37	3.81			
1050	0.000	1	0.12	6.19		2.37	3.81			
1050	0.883	1	0.19	9.44		2.37	7.06			
1051	0.000	1	0.19	9.44		2.37	7.06			
1051	0.883	1	0.24	12.02		2.37	9.65			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-1_SLS_DOKOI

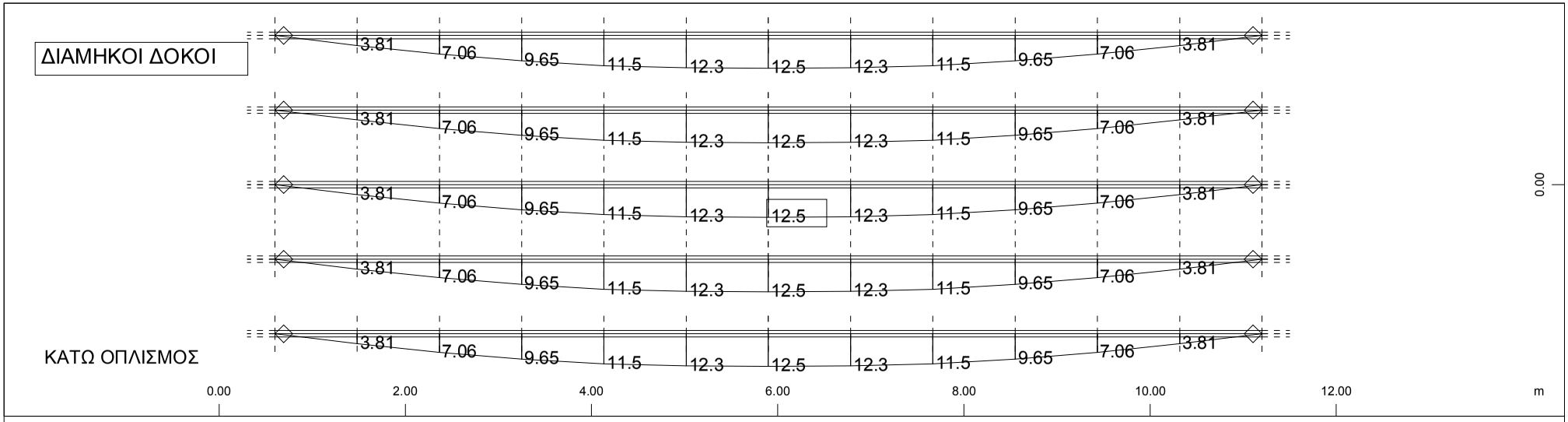
Longitudinal Reinforcements LCR 503
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
1052	0.000	1	0.24	12.02		2.37	9.65			
1052	0.883	1	0.28	13.89		2.37	11.52			
1053	0.000	1	0.28	13.89		2.37	11.52			
1053	0.883	1	0.29	14.66		2.37	12.29			
1054	0.000	1	0.29	14.66		2.37	12.29			
1054	0.883	1	0.30	14.84		2.37	12.47			
1055	0.000	1	0.30	14.84		2.37	12.47			
1055	0.883	1	0.29	14.66		2.37	12.29			
1056	0.000	1	0.29	14.66		2.37	12.29			
1056	0.883	1	0.28	13.89		2.37	11.52			
1057	0.000	1	0.28	13.89		2.37	11.52			
1057	0.883	1	0.24	12.02		2.37	9.65			
1058	0.000	1	0.24	12.02		2.37	9.65			
1058	0.883	1	0.19	9.44		2.37	7.06			
1059	0.000	1	0.19	9.44		2.37	7.06			
1059	0.883	1	0.12	6.19		2.37	3.81			
1060	0.000	1	0.12	6.19		2.37	3.81			
1060	0.883	1	0.05	2.37		2.37				

Reinforcements saved as design case LCR 503
Reinforcements are superposed with existing minimum reinforcements

Maximum Degree of Utilization

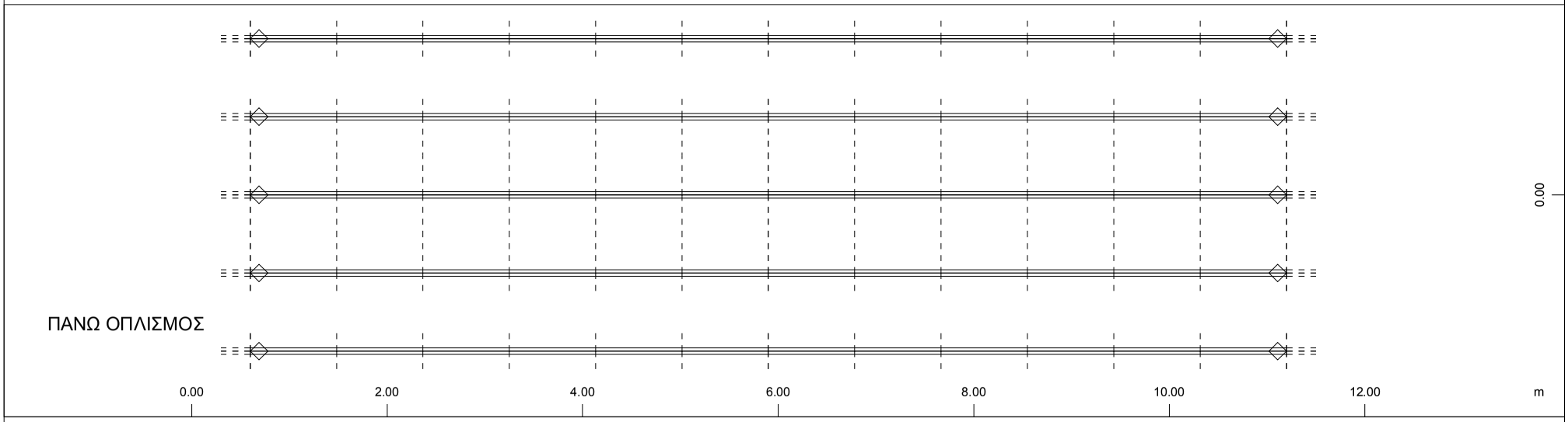
		N sig-c	Vy sig-t	Vz tau	Mt sig-*	My tend.	Mz As-l	Mb As-v	Mt2 crack	Total sigdyn	lamda tau-*
Cross sect.	1	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
		0.000	0.000	0.000	0.000	0.000	0.000	0.000	2.241	0.000	0.000
Total System		0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
		0.000	0.000	0.000	0.000	0.000	0.000	0.000	2.241	0.000	0.000



Z—X
Y

Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case 503 , 1 cm 3D = 22.4 cm2 (Max=12.5)

M 1 : 63

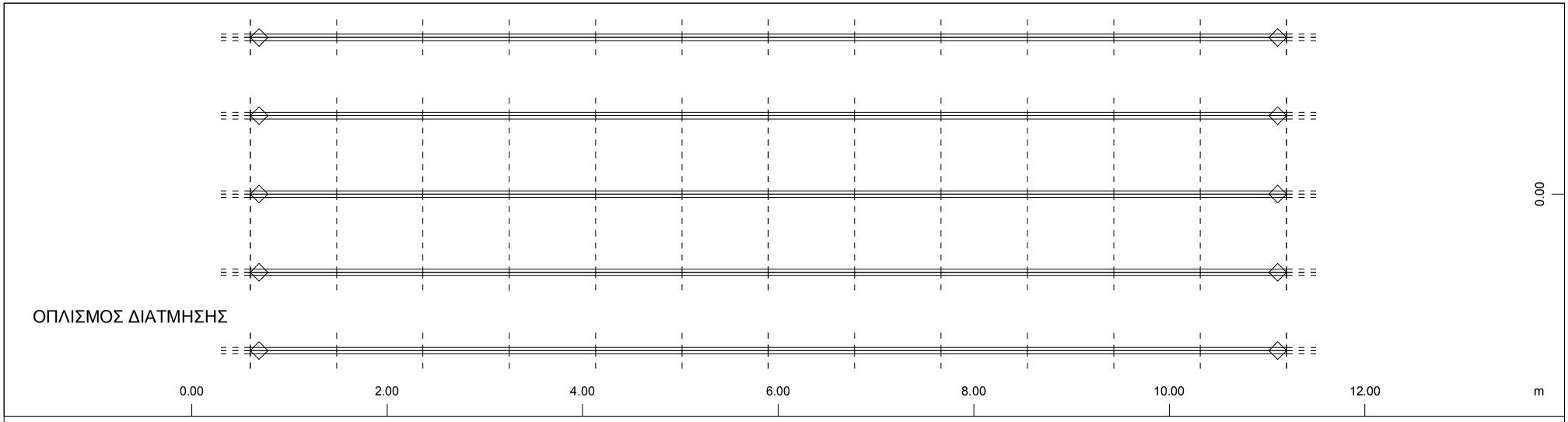


Z—X
Y

Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case 503 , 1 cm 3D = 0 cm2 (Max=0)

M 1 : 60

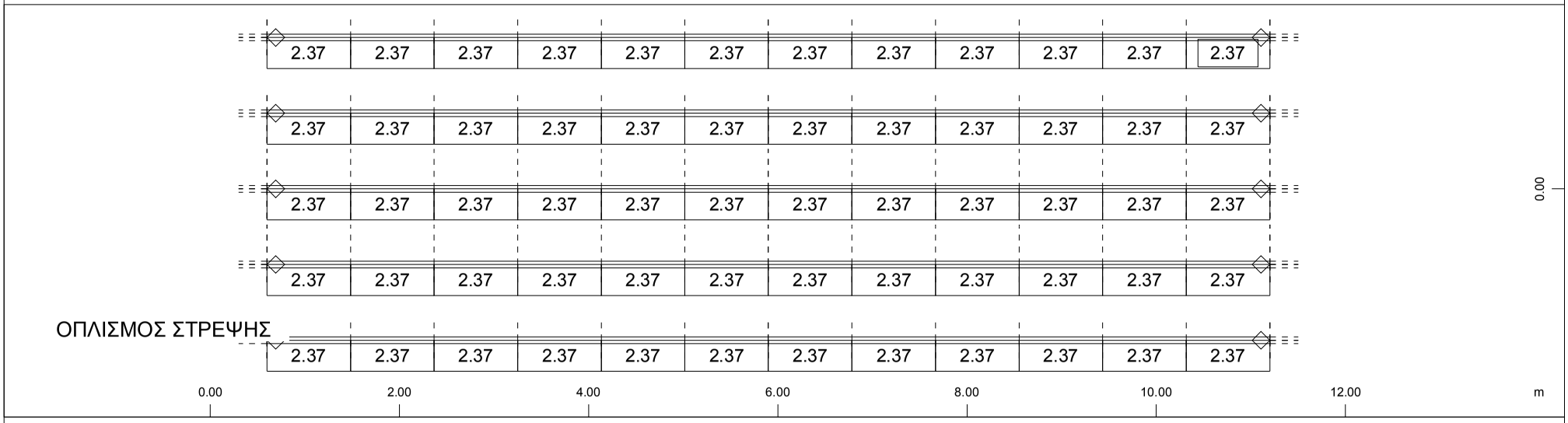
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΦΑΣΗ-1_SLS



Z X
Y

Sector of system Beam Elements Group 1
Beam Elements , Shear reinforcements (maximum), Design Case 503 , 1 cm 3D = 0 cm2/m (Max=0)

M 1 : 60



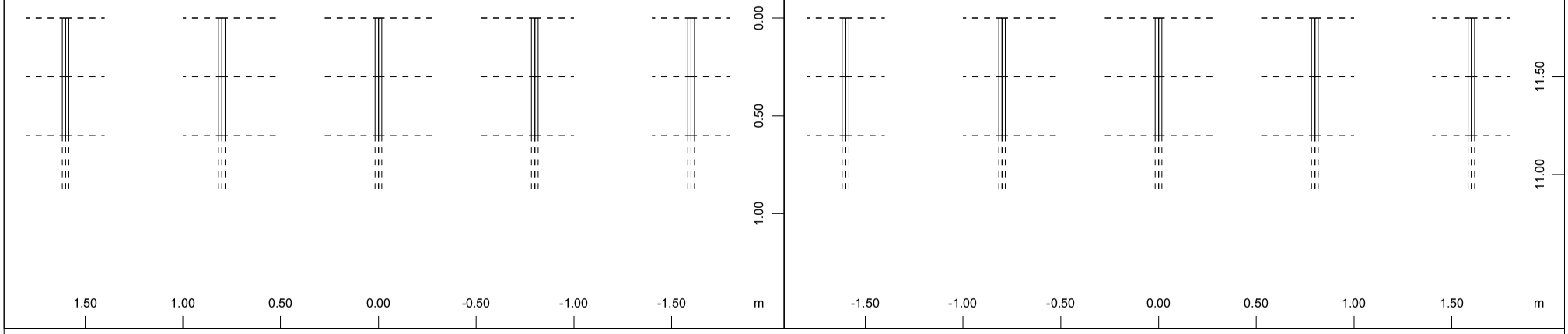
Z X
Y

Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case 503 , 1 cm 3D = 4.48 cm2 (Max=2.37)

M 1 : 62

ΔΙΑΜΗΚΗ ΔΟΚΟΙ-ΣΤΗΡΙΞΗΣ

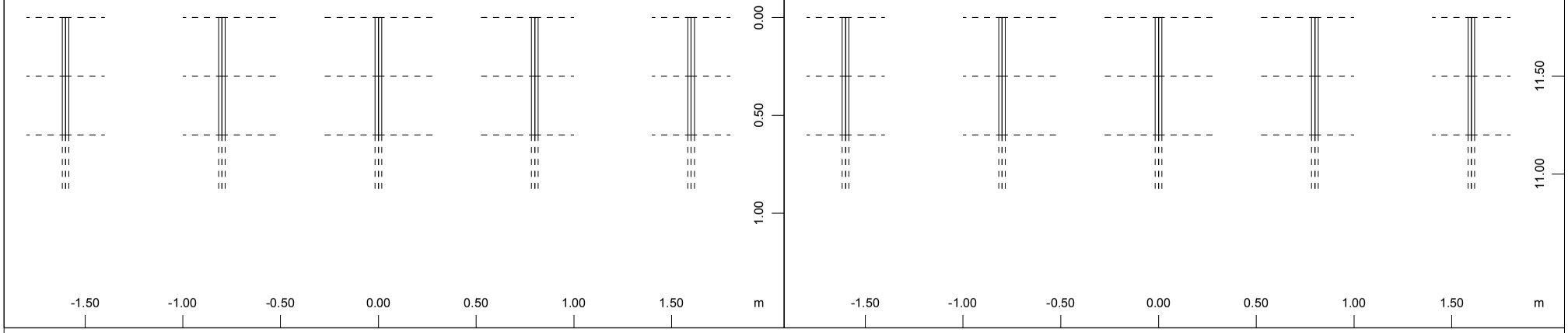
ΚΑΤΩ ΟΠΛΙΣΜΟΣ



Y Z X
Sector of system Beam Elements Group 2
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
503 , 1 cm 3D = 0 cm2 (Max=0)
M 1 : 30

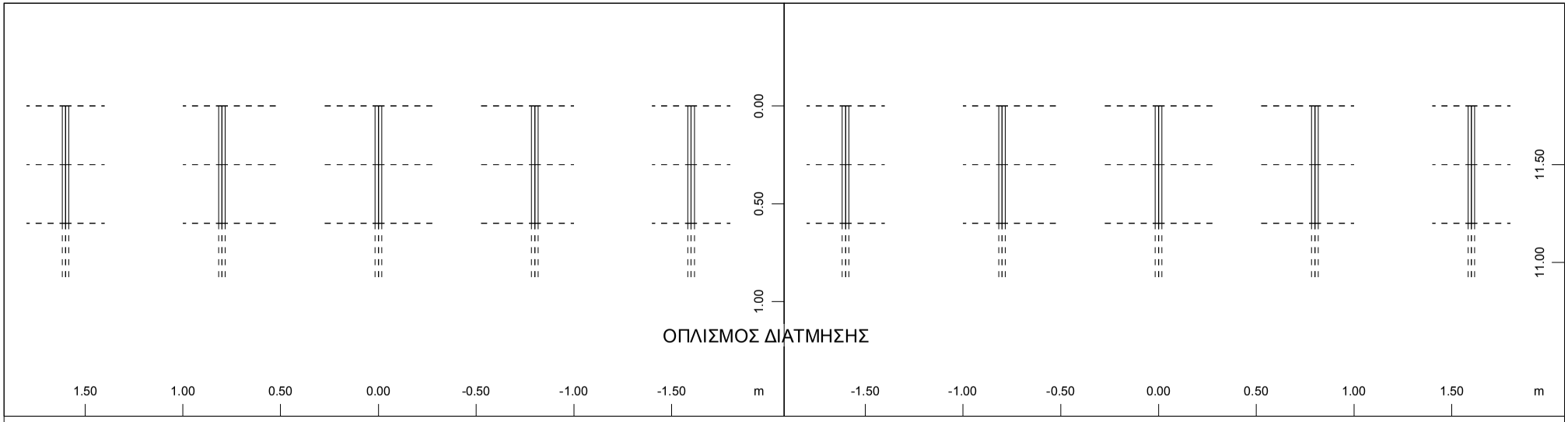
X Z Y
Sector of system Beam Elements Group 2
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
503 , 1 cm 3D = 0 cm2 (Max=0)
M 1 : 30

ΠΑΝΩ ΟΠΛΙΣΜΟΣ



Z Y X
Sector of system Beam Elements Group 2
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
503 , 1 cm 3D = 0 cm2 (Max=0)
M 1 : 30

X Z Y
Sector of system Beam Elements Group 2
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
503 , 1 cm 3D = 0 cm2 (Max=0)
M 1 : 30



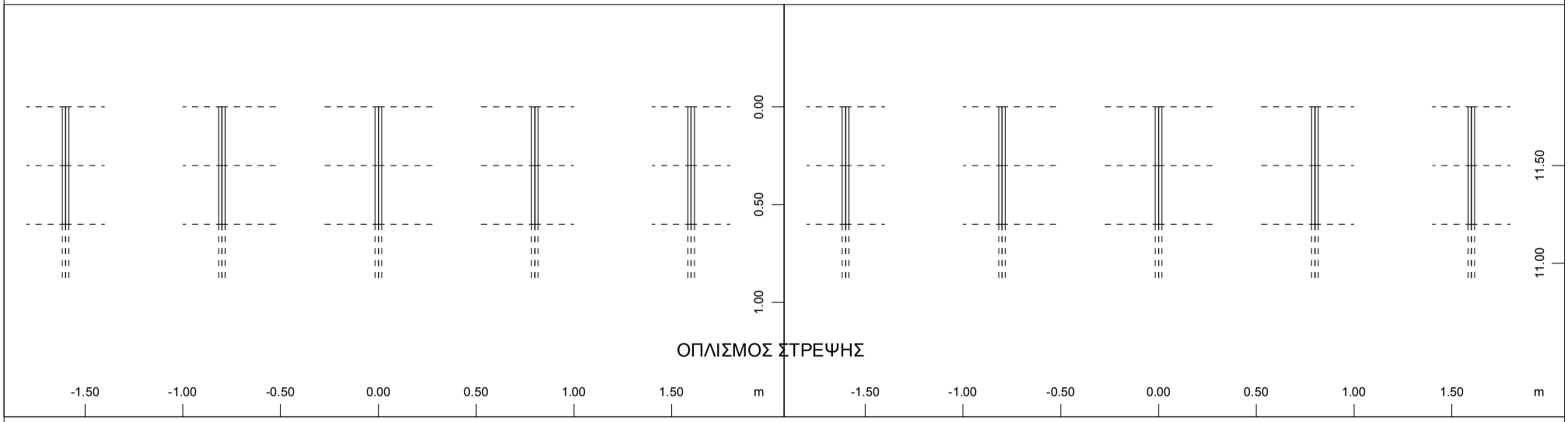
ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ

Y Z X

Sector of system Beam Elements Group 2 M 1 : 30
Beam Elements , Shear reinforcements (maximum), Design Case 503
, 1 cm 3D = 0 cm2/m (Max=0)

X Z Y

Sector of system Beam Elements Group 2 M 1 : 30
Beam Elements , Shear reinforcements (maximum), Design Case 503
, 1 cm 3D = 0 cm2/m (Max=0)



ΟΠΛΙΣΜΟΣ ΣΤΡΕΨΗΣ

Z Y X

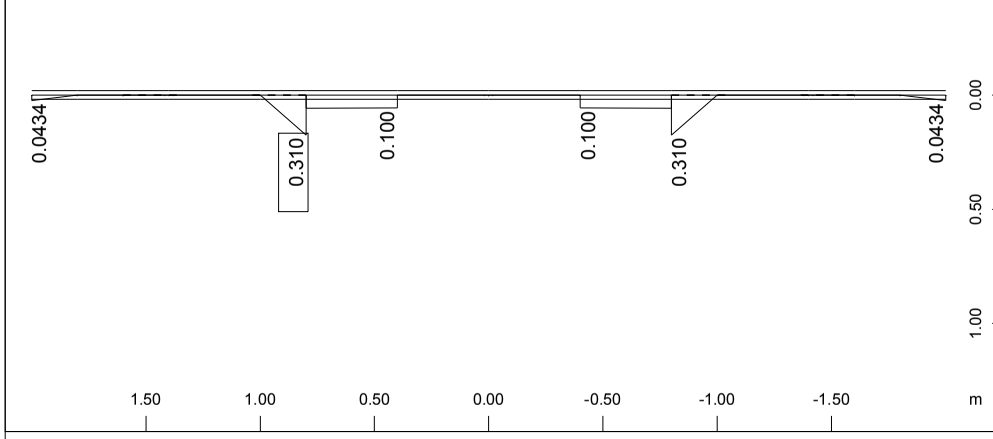
Sector of system Beam Elements Group 2 M 1 : 30
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
503 , 1 cm 3D = 0 cm2 (Max=0)

X Z Y

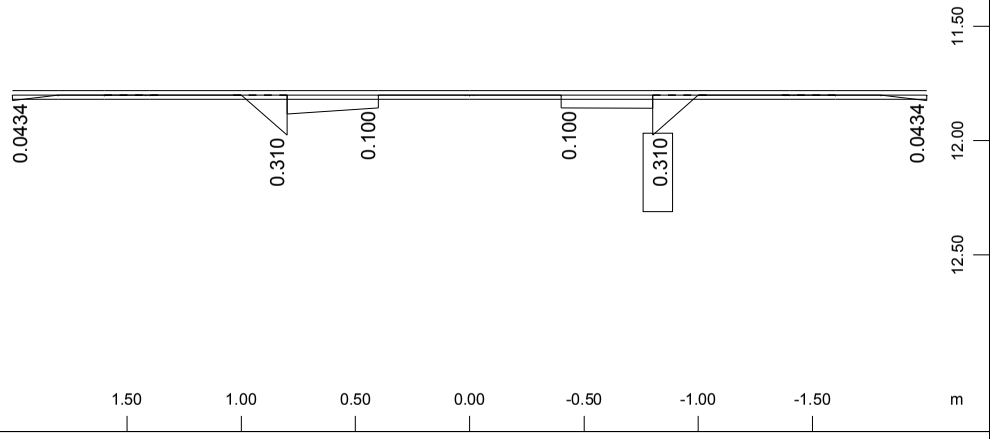
Sector of system Beam Elements Group 2 M 1 : 30
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
503 , 1 cm 3D = 0 cm2 (Max=0)

ΠΑΣΣΑΛΟΔΕΣΜΟΣ

ΚΑΤΩ ΟΠΛΙΣΜΟΣ

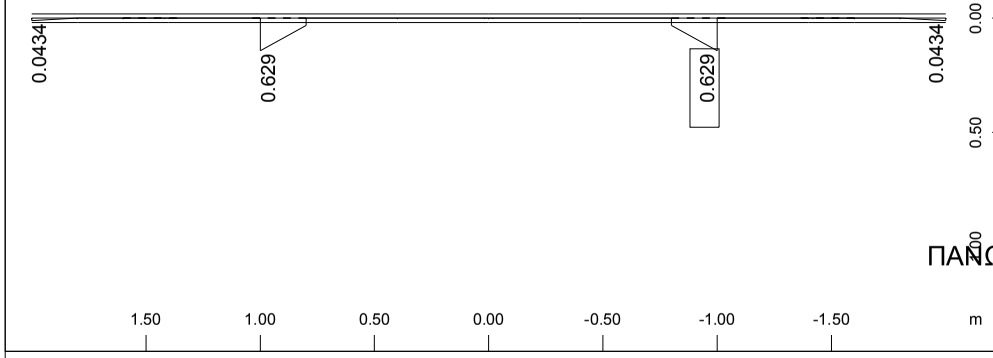


Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
503 , 1 cm 3D = 0.581 cm2 (Max=0.310)

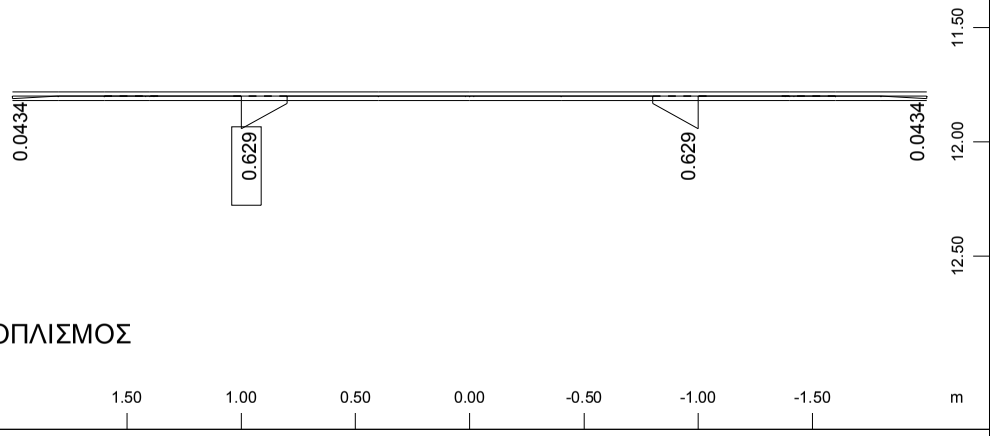


Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
503 , 1 cm 3D = 0.581 cm2 (Max=0.310)

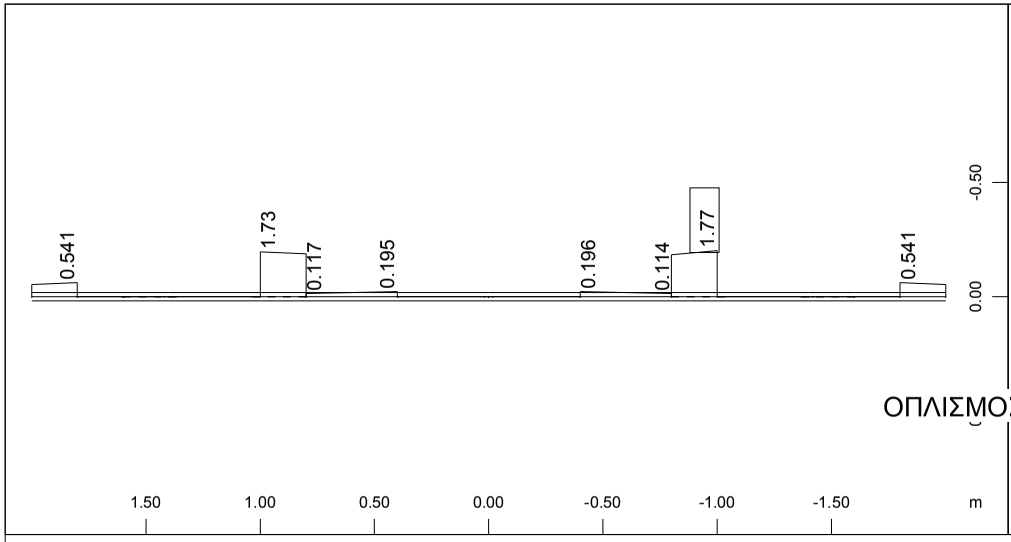
ΠΑΝΩ ΟΠΛΙΣΜΟΣ



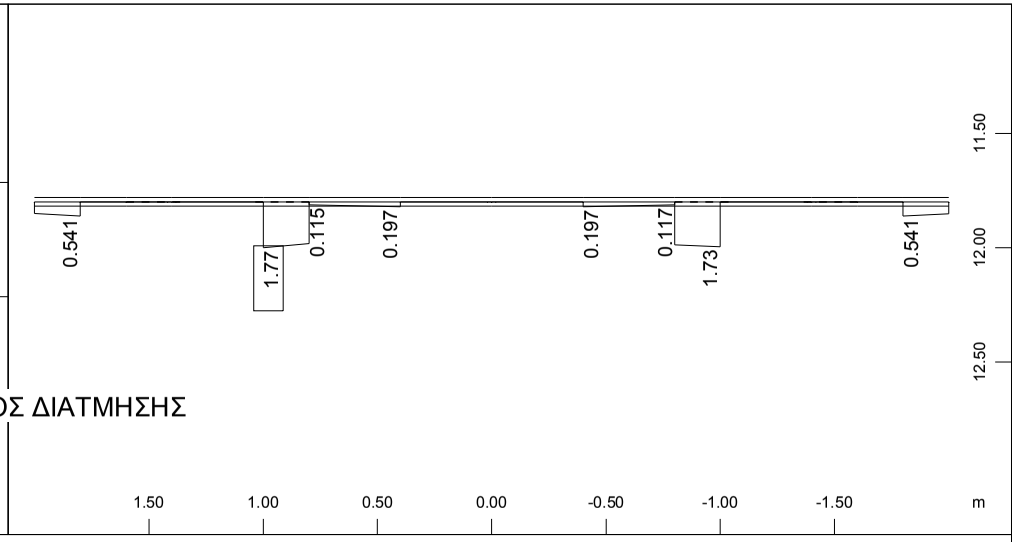
Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
503 , 1 cm 3D = 1.45 cm2 (Max=0.629)



Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
503 , 1 cm 3D = 1.45 cm2 (Max=0.629)

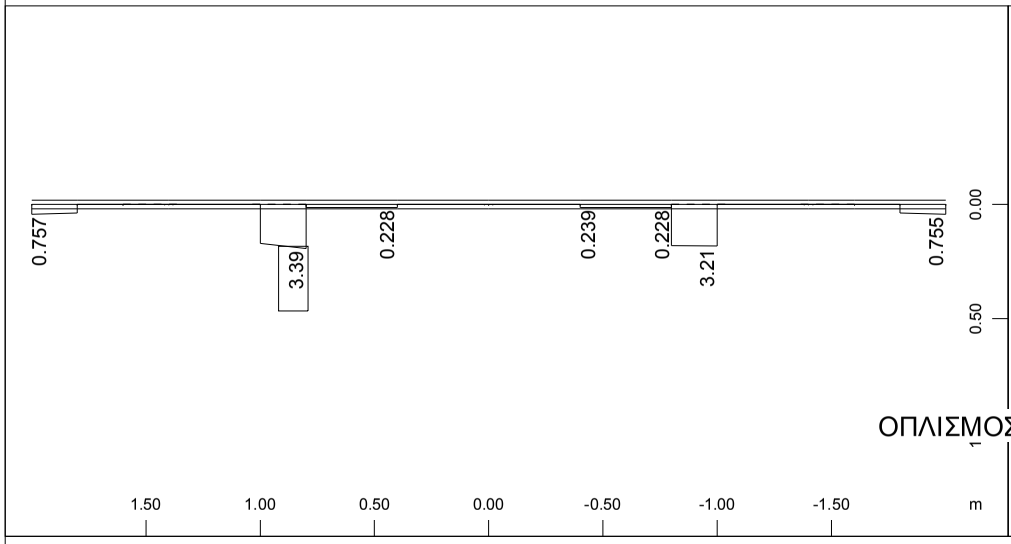


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Shear reinforcements (maximum), Design Case 503
, 1 cm 3D = 2.91 cm2/m (Max=1.77)

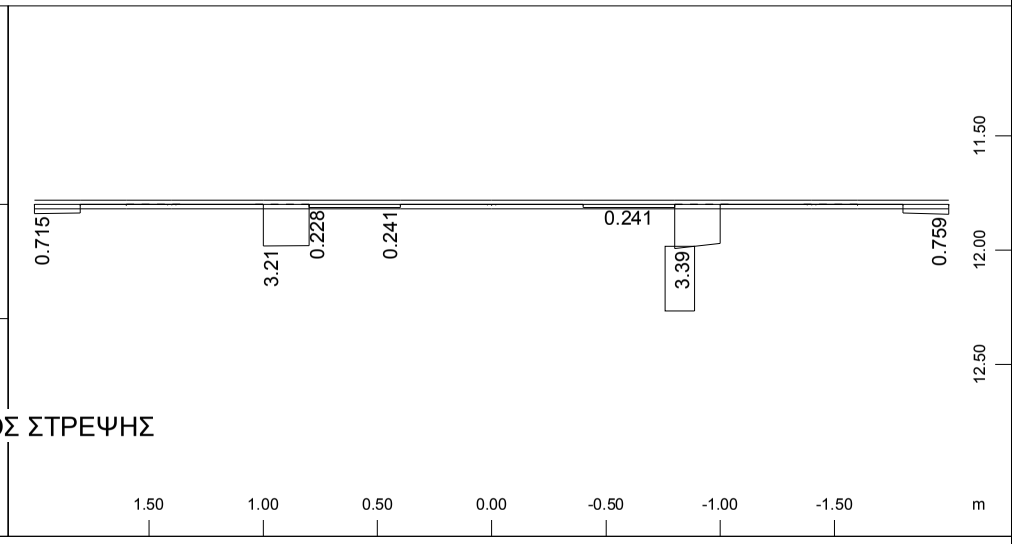


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Shear reinforcements (maximum), Design Case 503
, 1 cm 3D = 2.91 cm2/m (Max=1.77)

ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ

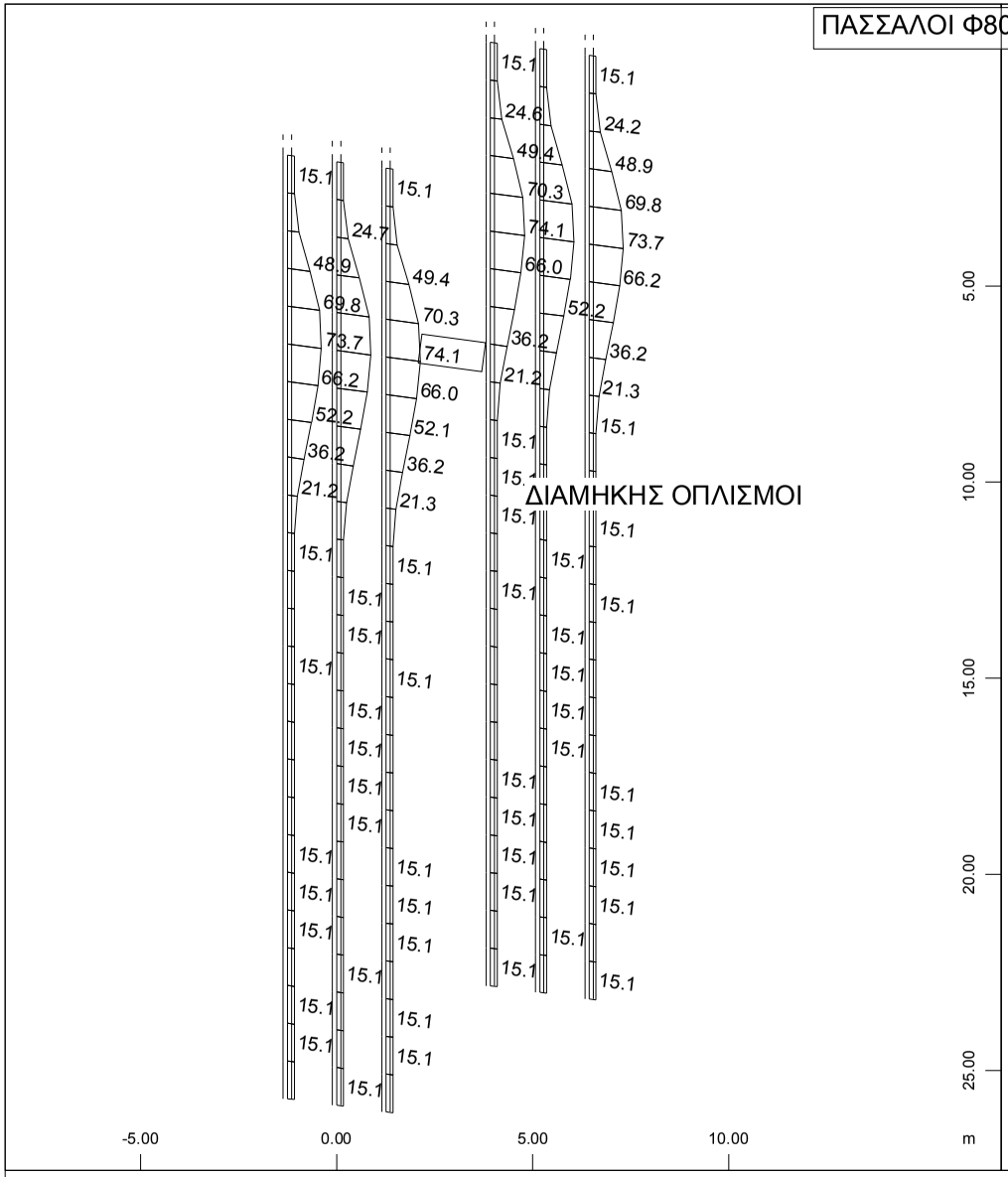


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
503 , 1 cm 3D = 5.81 cm2 (Max=3.39)



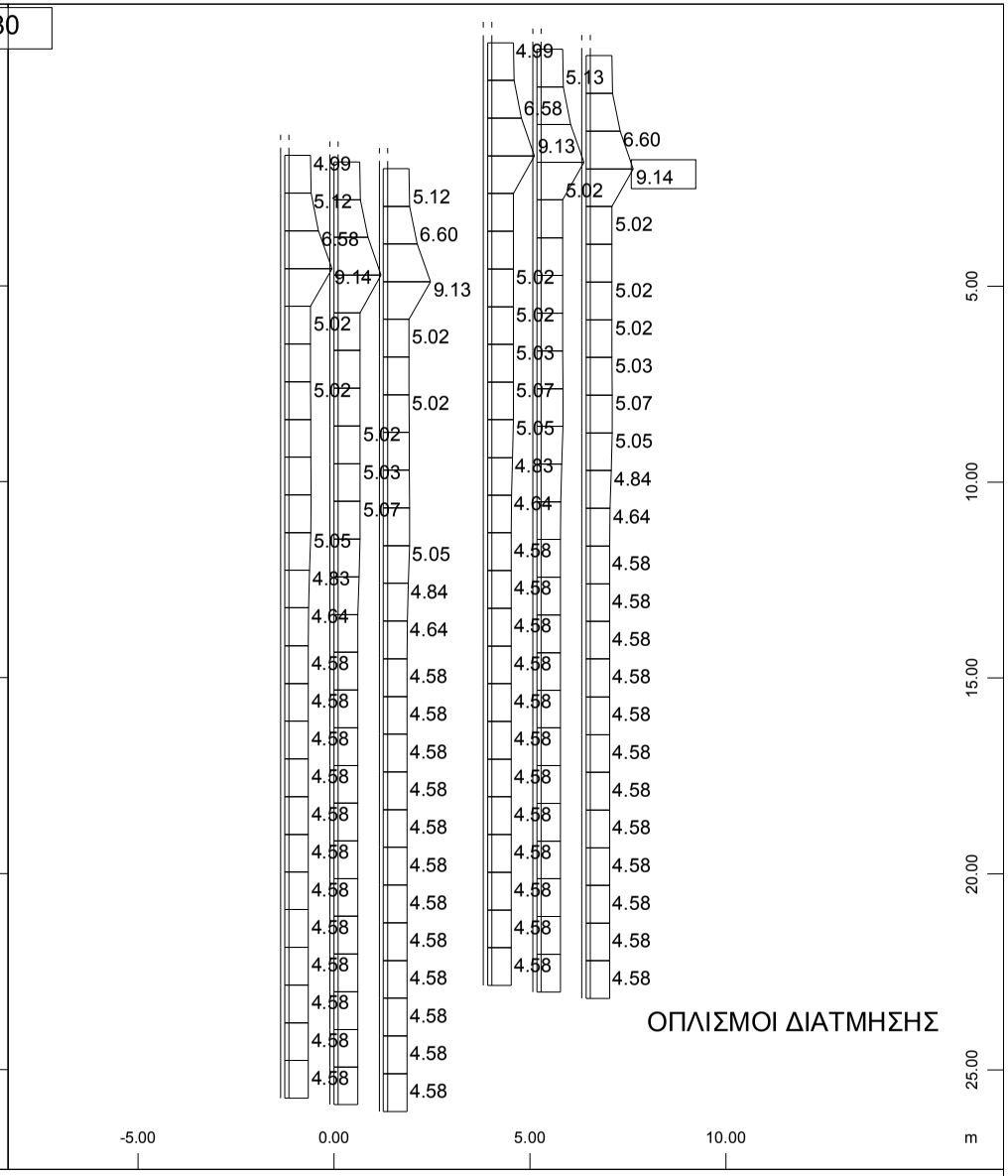
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
503 , 1 cm 3D = 5.81 cm2 (Max=3.39)

ΟΠΛΙΣΜΟΣ ΣΤΡΕΨΗΣ



Sector of system Beam Elements Group 12
Beam Elements , Longitudinal Reinforcements Lay. 1, Design
Case 503 , 1 cm 3D = 145.3 cm2 (Max=74.1)

M 1 : 191
X * 0.502
Y * 0.906
Z * 0.962



Sector of system Beam Elements Group 12
Beam Elements , Shear reinforcements (maximum), Design Case
503 , 1 cm 3D = 14.5 cm2/m (Max=9.14)

M 1 : 191
X * 0.502
Y * 0.906
Z * 0.962

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΦΑΣΗ-1_SLS

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00

7) ΦΑΣΗ-2 ΕΛΕΓΧΟΣ ΦΟΡΕΑ ΣΕ ULS-STATIKA

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
COMBINATION-LL-characteristik

Combination rule Number 1
COMB.LL-TS_ON DECK
Resulting loadcases type Design Combination
Loadcase selection

Number	factor	type		Title
41	1.00	Conditional LC		L.L.UDL_2.50KN/m2
42	1.00	Conditional LC		L.L.UDL_6.50KN/m2
50	1.00	Exclusive LC	A 1	TS_RIGHT_Posit.1
51	1.00	Exclusive LC	A 1	TS_RIGHT_Posit.2
52	1.00	Exclusive LC	A 1	TS_RIGHT_Posit.3
53	1.00	Exclusive LC	A 1	TS_RIGHT_Posit.4
54	1.00	Exclusive LC	A 1	TS_RIGHT_Posit.5
55	1.00	Exclusive LC	A 1	TS_RIGHT_Posit.6
56	1.00	Exclusive LC	A 1	TS_RIGHT_Posit.7
57	1.00	Exclusive LC	A 1	TS_RIGHT_Posit.8
58	1.00	Exclusive LC	A 1	TS_RIGHT_Posit.9
59	1.00	Exclusive LC	A 1	TS_RIGHT_Posit.10
60	1.00	Exclusive LC	A 1	TS_RIGHT_Posit.11
61	1.00	Exclusive LC	A 1	TS_RIGHT_Posit.12
70	1.00	Conditional LC		ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A1 2.50
71	1.00	Conditional LC		ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A2 2.50
72	1.00	Conditional LC		ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A1 6.50
73	1.00	Conditional LC		ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A2 6.50

Combination rule Number 2
COMB.LL-TS_OUT OF DECK
Resulting loadcases type Design Combination
Loadcase selection

Number	factor	type		Title
41	1.00	Conditional LC		L.L.UDL_2.50KN/m2
42	1.00	Conditional LC		L.L.UDL_6.50KN/m2
70	1.00	Conditional LC		ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A1 2.50
71	1.00	Conditional LC		ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A2 2.50
72	1.00	Conditional LC		ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A1 6.50
73	1.00	Conditional LC		ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A2 6.50
74	1.00	Conditional LC		ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A1 TS1-
75	1.00	Conditional LC		ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A2 TS1-

Generated Loadcases

Number	Comb	Title
2001	1	MAX-MX QUAD LL-ON
2002	1	MIN-MX QUAD LL-ON
2003	1	MAX-MY QUAD LL-ON
2004	1	MIN-MY QUAD LL-ON
2005	1	MAX-MXY QUAD LL-ON
2006	1	MIN-MXY QUAD LL-ON
2001	1	MAX-MX QUAK LL-ON
2002	1	MIN-MX QUAK LL-ON
2003	1	MAX-MY QUAK LL-ON
2004	1	MIN-MY QUAK LL-ON
2005	1	MAX-MXY QUAK LL-ON
2006	1	MIN-MXY QUAK LL-ON
2007	1	MAX-VX QUAD LL-ON
2008	1	MIN-VX QUAD LL-ON
2007	1	MAX-VX QUAK LL-ON
2008	1	MIN-VX QUAK LL-ON
2009	1	MAX-VY QUAD LL-ON
2010	1	MIN-VY QUAD LL-ON
2009	1	MAX-VY QUAK LL-ON
2010	1	MIN-VY QUAK LL-ON
2011	1	MAX-NXX QUAD LL-ON
2012	1	MIN-NXX QUAD LL-ON
2013	1	MAX-NYY QUAD LL-ON
2014	1	MIN-NYY QUAD LL-ON
2015	1	MAX-NXY QUAD LL-ON
2016	1	MIN-NXY QUAD LL-ON
2011	1	MAX-NXX QUAK LL-ON
2012	1	MIN-NXX QUAK LL-ON
2013	1	MAX-NYY QUAK LL-ON
2014	1	MIN-NYY QUAK LL-ON
2015	1	MAX-NXY QUAK LL-ON
2016	1	MIN-NXY QUAK LL-ON
1101	1	MAX-MY BEAM
1102	1	MIN-MY BEAM
1103	1	MAX-VZ BEAM
1104	1	MIN-VZ BEAM
1105	1	MAX-MZ BEAM
1106	1	MIN-MZ BEAM
1107	1	MAX-VY BEAM
1108	1	MIN-VY BEAM

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
COMBINATION-LL-characteristik

Generated Loadcases

Number	Comb	Title
1109	1	MAX-N BEAM
1110	1	MIN-N BEAM
1111	1	MAX-MT BEAM
1112	1	MIN-MT BEAM
2021	2	MAX-MX QUAD LL-OFF
2022	2	MIN-MX QUAD LL-OFF
2023	2	MAX-MY QUAD LL-OFF
2024	2	MIN-MY QUAD LL-OFF
2025	2	MAX-MXY QUAD LL-OFF
2026	2	MIN-MXY QUAD LL-OFF
2021	2	MAX-MX QUAK LL-OFF
2022	2	MIN-MX QUAK LL-OFF
2023	2	MAX-MY QUAK LL-OFF
2024	2	MIN-MY QUAK LL-OFF
2025	2	MAX-MXY QUAK LL-OFF
2026	2	MIN-MXY QUAK LL-OFF
2027	2	MAX-VX QUAD LL-OFF
2028	2	MIN-VX QUAD LL-OFF
2027	2	MAX-VX QUAK LL-OFF
2028	2	MIN-VX QUAK LL-OFF
2029	2	MAX-VY QUAD LL-OFF
2030	2	MIN-VY QUAD LL-OFF
2029	2	MAX-VY QUAK LL-OFF
2030	2	MIN-VY QUAK LL-OFF
2031	2	MAX-NXX QUAD LL-OFF
2032	2	MIN-NXX QUAD LL-OFF
2033	2	MAX-NYY QUAD LL-OFF
2034	2	MIN-NYY QUAD LL-OFF
2035	2	MAX-NXY QUAD LL-OFF
2036	2	MIN-NXY QUAD LL-OFF
2031	2	MAX-NXX QUAK LL-OFF
2032	2	MIN-NXX QUAK LL-OFF
2033	2	MAX-NYY QUAK LL-OFF
2034	2	MIN-NYY QUAK LL-OFF
2035	2	MAX-NXY QUAK LL-OFF
2036	2	MIN-NXY QUAK LL-OFF
1121	2	MAX-MY BEAM
1122	2	MIN-MY BEAM
1123	2	MAX-VZ BEAM
1124	2	MIN-VZ BEAM
1125	2	MAX-MZ BEAM
1126	2	MIN-MZ BEAM
1127	2	MAX-VY BEAM
1128	2	MIN-VY BEAM
1129	2	MAX-N BEAM
1130	2	MIN-N BEAM
1131	2	MAX-MT BEAM
1132	2	MIN-MT BEAM

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
COMBINATION ULD

Combination rule Number 1
COMB.ULS-2-BEAM
Resulting loadcases type Design Combination
Loadcase selection

Number	factor	type		Title
31	1.35	Exclusive LC	AG	ΡΥΣΕΙΣ-ΟΔΟΣΤΡΩΣΙΑ
32	1.35	Combined with LC		ΡΕΖΟΔΡΟΜΙΟ
33	1.35	Combined with LC		ΠΡΟΣΘΕΤΗ ΟΔΟΣΤΡΩΣΙΑ 0.50
31	1.00	Exclusive LC	AG	ΡΥΣΕΙΣ-ΟΔΟΣΤΡΩΣΙΑ
32	1.00	Combined with LC		ΡΕΖΟΔΡΟΜΙΟ
33	1.00	Combined with LC		ΠΡΟΣΘΕΤΗ ΟΔΟΣΤΡΩΣΙΑ 0.50
36	1.50	Exclusive LC	A 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+0.5
37	1.50	Exclusive LC	A 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+0.5
38	1.50	Exclusive LC	A 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+1.0
39	1.50	Exclusive LC	A 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+1.0
1101	1.35	Exclusive LC	A 2	MAX-MY BEAM
1102	1.35	Exclusive LC	A 2	MIN-MY BEAM
1103	1.35	Exclusive LC	A 2	MAX-VZ BEAM
1104	1.35	Exclusive LC	A 2	MIN-VZ BEAM
1105	1.35	Exclusive LC	A 2	MAX-MZ BEAM
1106	1.35	Exclusive LC	A 2	MIN-MZ BEAM
1107	1.35	Exclusive LC	A 2	MAX-VY BEAM
1108	1.35	Exclusive LC	A 2	MIN-VY BEAM
1109	1.35	Exclusive LC	A 2	MAX-N BEAM
1110	1.35	Exclusive LC	A 2	MIN-N BEAM
1111	1.35	Exclusive LC	A 2	MAX-MT BEAM
1112	1.35	Exclusive LC	A 2	MIN-MT BEAM
1121	1.35	Exclusive LC	A 2	MAX-MY BEAM
1122	1.35	Exclusive LC	A 2	MIN-MY BEAM
1123	1.35	Exclusive LC	A 2	MAX-VZ BEAM
1124	1.35	Exclusive LC	A 2	MIN-VZ BEAM
1125	1.35	Exclusive LC	A 2	MAX-MZ BEAM
1126	1.35	Exclusive LC	A 2	MIN-MZ BEAM
1127	1.35	Exclusive LC	A 2	MAX-VY BEAM
1128	1.35	Exclusive LC	A 2	MIN-VY BEAM
1129	1.35	Exclusive LC	A 2	MAX-N BEAM
1130	1.35	Exclusive LC	A 2	MIN-N BEAM
1131	1.35	Exclusive LC	A 2	MAX-MT BEAM
1132	1.35	Exclusive LC	A 2	MIN-MT BEAM
80	1.20	Conditional LC		ΠΙΘΑΝΕΣ ΚΑΘΙΖΗΣΕΙΣ A1
81	1.20	Conditional LC		ΠΙΘΑΝΕΣ ΚΑΘΙΖΗΣΕΙΣ A2

Combination rule Number 2
COMB.ULS-2-QUAD
Resulting loadcases type Design Combination
Loadcase selection

Number	factor	type		Title
1	1.35	Exclusive LC	AG	I.B. ΚΑΤΑΚ.ΣΤΟΙΧΕΙΩΝ
2	1.35	Combined with LC		I.B. ΔΟΚΩΝ
3	1.35	Combined with LC		I.B. ΧΥΤΗΣ ΠΛΑΚΑΣ
31	1.35	Combined with LC		ΡΥΣΕΙΣ-ΟΔΟΣΤΡΩΣΙΑ
32	1.35	Combined with LC		ΡΕΖΟΔΡΟΜΙΟ
33	1.35	Combined with LC		ΠΡΟΣΘΕΤΗ ΟΔΟΣΤΡΩΣΙΑ 0.50
1	1.00	Exclusive LC	AG	I.B. ΚΑΤΑΚ.ΣΤΟΙΧΕΙΩΝ
2	1.00	Combined with LC		I.B. ΔΟΚΩΝ
3	1.00	Combined with LC		I.B. ΧΥΤΗΣ ΠΛΑΚΑΣ
31	1.00	Combined with LC		ΡΥΣΕΙΣ-ΟΔΟΣΤΡΩΣΙΑ
32	1.00	Combined with LC		ΡΕΖΟΔΡΟΜΙΟ
33	1.00	Combined with LC		ΠΡΟΣΘΕΤΗ ΟΔΟΣΤΡΩΣΙΑ 0.50
11	1.50	Exclusive LC	A 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+0.5
12	1.50	Exclusive LC	A 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+0.5
13	1.50	Exclusive LC	A 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+1.0
14	1.50	Exclusive LC	A 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+1.0
36	1.50	Exclusive LC	A 2	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+0.5
37	1.50	Exclusive LC	A 2	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+0.5
38	1.50	Exclusive LC	A 2	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+1.0
39	1.50	Exclusive LC	A 2	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+1.0
2001	1.35	Exclusive LC	A 3	MAX-MX QUAK LL-ON
2002	1.35	Exclusive LC	A 3	MIN-MX QUAK LL-ON
2003	1.35	Exclusive LC	A 3	MAX-MY QUAK LL-ON
2004	1.35	Exclusive LC	A 3	MIN-MY QUAK LL-ON
2005	1.35	Exclusive LC	A 3	MAX-MXY QUAK LL-ON
2006	1.35	Exclusive LC	A 3	MIN-MXY QUAK LL-ON
2007	1.35	Exclusive LC	A 3	MAX-VX QUAK LL-ON
2008	1.35	Exclusive LC	A 3	MIN-VX QUAK LL-ON
2009	1.35	Exclusive LC	A 3	MAX-VY QUAK LL-ON
2010	1.35	Exclusive LC	A 3	MIN-VY QUAK LL-ON
2011	1.35	Exclusive LC	A 3	MAX-NXX QUAK LL-ON
2012	1.35	Exclusive LC	A 3	MIN-NXX QUAK LL-ON
2013	1.35	Exclusive LC	A 3	MAX-NYY QUAK LL-ON

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
COMBINATION ULD

Combination rule Number 2
COMB.ULS-2-QUAD
Resulting loadcases type Design Combination
Loadcase selection

Number	factor	type		Title
2014	1.35	Exclusive LC	A 3	MIN-NYY QUAK LL-ON
2015	1.35	Exclusive LC	A 3	MAX-NXY QUAK LL-ON
2016	1.35	Exclusive LC	A 3	MIN-NXY QUAK LL-ON
2021	1.35	Exclusive LC	A 3	MAX-MX QUAK LL-OFF
2022	1.35	Exclusive LC	A 3	MIN-MX QUAK LL-OFF
2023	1.35	Exclusive LC	A 3	MAX-MY QUAK LL-OFF
2024	1.35	Exclusive LC	A 3	MIN-MY QUAK LL-OFF
2025	1.35	Exclusive LC	A 3	MAX-MXY QUAK LL-OFF
2026	1.35	Exclusive LC	A 3	MIN-MXY QUAK LL-OFF
2027	1.35	Exclusive LC	A 3	MAX-VX QUAK LL-OFF
2028	1.35	Exclusive LC	A 3	MIN-VX QUAK LL-OFF
2029	1.35	Exclusive LC	A 3	MAX-VY QUAK LL-OFF
2030	1.35	Exclusive LC	A 3	MIN-VY QUAK LL-OFF
2031	1.35	Exclusive LC	A 3	MAX-NXX QUAK LL-OFF
2032	1.35	Exclusive LC	A 3	MIN-NXX QUAK LL-OFF
2033	1.35	Exclusive LC	A 3	MAX-NYY QUAK LL-OFF
2034	1.35	Exclusive LC	A 3	MIN-NYY QUAK LL-OFF
2035	1.35	Exclusive LC	A 3	MAX-NXY QUAK LL-OFF
2036	1.35	Exclusive LC	A 3	MIN-NXY QUAK LL-OFF
80	1.20	Conditional LC		ΠΙΘΑΝΕΣ ΚΑΘΙΖΗΣΕΙΣ A1
81	1.20	Conditional LC		ΠΙΘΑΝΕΣ ΚΑΘΙΖΗΣΕΙΣ A2
6015	1.00	Conditional LC		15 K creep step
6025	1.00	Conditional LC		25 K creep step
6055	1.00	Conditional LC		55 K creep step
6060	1.00	Conditional LC		60 K creep step
6061	1.00	Conditional LC		61 K creep step
6062	1.00	Conditional LC		62 K creep step
6063	1.00	Conditional LC		63 K creep step
6064	1.00	Conditional LC		64 K creep step

Combination rule Number 3
COMB.ULS_1+2_piles
Resulting loadcases type Design Combination
Loadcase selection

Number	factor	type		Title
1	1.35	Exclusive LC	AG	Ι.Β. ΚΑΤΑΚ.ΣΤΟΙΧΕΙΩΝ
2	1.35	Combined with LC		Ι.Β. ΔΟΚΩΝ
3	1.35	Combined with LC		Ι.Β. ΧΥΤΗΣ ΠΛΑΚΑΣ
31	1.35	Combined with LC		ΡΥΣΕΙΣ-ΟΔΟΣΤΡΩΣΙΑ
32	1.35	Combined with LC		PEZODROMIO
33	1.35	Combined with LC		ΠΡΟΣΘΕΤΗ ΟΔΟΣΤΡΩΣΙΑ 0.50
1	1.00	Exclusive LC	AG	Ι.Β. ΚΑΤΑΚ.ΣΤΟΙΧΕΙΩΝ
2	1.00	Combined with LC		Ι.Β. ΔΟΚΩΝ
3	1.00	Combined with LC		Ι.Β. ΧΥΤΗΣ ΠΛΑΚΑΣ
31	1.00	Combined with LC		ΡΥΣΕΙΣ-ΟΔΟΣΤΡΩΣΙΑ
32	1.00	Combined with LC		PEZODROMIO
33	1.00	Combined with LC		ΠΡΟΣΘΕΤΗ ΟΔΟΣΤΡΩΣΙΑ 0.50
11	1.50	Exclusive LC	A 3	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+0.5
12	1.50	Exclusive LC	A 3	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+0.5
13	1.50	Exclusive LC	A 3	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+1.0
14	1.50	Exclusive LC	A 3	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+1.0
36	1.50	Exclusive LC	A 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+0.5
37	1.50	Exclusive LC	A 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+0.5
38	1.50	Exclusive LC	A 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+1.0
39	1.50	Exclusive LC	A 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+1.0
1101	1.35	Exclusive LC	A 2	MAX-MY BEAM
1102	1.35	Exclusive LC	A 2	MIN-MY BEAM
1103	1.35	Exclusive LC	A 2	MAX-VZ BEAM
1104	1.35	Exclusive LC	A 2	MIN-VZ BEAM
1105	1.35	Exclusive LC	A 2	MAX-MZ BEAM
1106	1.35	Exclusive LC	A 2	MIN-MZ BEAM
1107	1.35	Exclusive LC	A 2	MAX-VY BEAM
1108	1.35	Exclusive LC	A 2	MIN-VY BEAM
1109	1.35	Exclusive LC	A 2	MAX-N BEAM
1110	1.35	Exclusive LC	A 2	MIN-N BEAM
1111	1.35	Exclusive LC	A 2	MAX-MT BEAM
1112	1.35	Exclusive LC	A 2	MIN-MT BEAM
1121	1.35	Exclusive LC	A 2	MAX-MY BEAM
1122	1.35	Exclusive LC	A 2	MIN-MY BEAM
1123	1.35	Exclusive LC	A 2	MAX-VZ BEAM
1124	1.35	Exclusive LC	A 2	MIN-VZ BEAM
1125	1.35	Exclusive LC	A 2	MAX-MZ BEAM
1126	1.35	Exclusive LC	A 2	MIN-MZ BEAM
1127	1.35	Exclusive LC	A 2	MAX-VY BEAM
1128	1.35	Exclusive LC	A 2	MIN-VY BEAM

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
COMBINATION ULD

Combination rule Number 3
COMB.ULS_1+2_piles
Resulting loadcases type Design Combination
Loadcase selection

Number	factor	type		Title
1129	1.35	Exclusive LC	A 2	MAX-N BEAM
1130	1.35	Exclusive LC	A 2	MIN-N BEAM
1131	1.35	Exclusive LC	A 2	MAX-MT BEAM
1132	1.35	Exclusive LC	A 2	MIN-MT BEAM
80	1.20	Conditional LC		ΠΙΘΑΝΕΣ ΚΑΘΙΖΗΣΕΙΣ A1
81	1.20	Conditional LC		ΠΙΘΑΝΕΣ ΚΑΘΙΖΗΣΕΙΣ A2

Generated Loadcases

Number	Comb	Title
1201	1	MAX-MY BEAM
1202	1	MIN-MY BEAM
1203	1	MAX-VZ BEAM
1204	1	MIN-VZ BEAM
1205	1	MAX-MZ BEAM
1206	1	MIN-MZ BEAM
1207	1	MAX-VY BEAM
1208	1	MIN-VY BEAM
1209	1	MAX-N BEAM
1210	1	MIN-N BEAM
1211	1	MAX-MT BEAM
1212	1	MIN-MT BEAM
2101	2	MAX-MX QUAD ULS-B1
2102	2	MIN-MX QUAD ULS-B1
2103	2	MAX-MY QUAD ULS-B1
2104	2	MIN-MY QUAD ULS-B1
2105	2	MAX-MXY QUAD ULS-B1
2106	2	MIN-MXY QUAD ULS-B1
2101	2	MAX-MX QUAK ULS-B1
2102	2	MIN-MX QUAK ULS-B1
2103	2	MAX-MY QUAK ULS-B1
2104	2	MIN-MY QUAK ULS-B1
2105	2	MAX-MXY QUAK ULS-B1
2106	2	MIN-MXY QUAK ULS-B1
2107	2	MAX-VX QUAD ULS-B1
2108	2	MIN-VX QUAD ULS-B1
2107	2	MAX-VX QUAK ULS-B1
2108	2	MIN-VX QUAK ULS-B1
2109	2	MAX-VY QUAD ULS-B1
2110	2	MIN-VY QUAD ULS-B1
2109	2	MAX-VY QUAK ULS-B1
2110	2	MIN-VY QUAK ULS-B1
2111	2	MAX-NXX QUAD ULS-B1
2112	2	MIN-NXX QUAD ULS-B1
2113	2	MAX-NYY QUAD ULS-B1
2114	2	MIN-NYY QUAD ULS-B1
2115	2	MAX-NXY QUAD ULS-B1
2116	2	MIN-NXY QUAD ULS-B1
2111	2	MAX-NXX QUAK ULS-B1
2112	2	MIN-NXX QUAK ULS-B1
2113	2	MAX-NYY QUAK ULS-B1
2114	2	MIN-NYY QUAK ULS-B1
2115	2	MAX-NXY QUAK ULS-B1
2116	2	MIN-NXY QUAK ULS-B1
1281	3	MAX-MY BEAM
1282	3	MIN-MY BEAM
1283	3	MAX-VZ BEAM
1284	3	MIN-VZ BEAM
1285	3	MAX-MZ BEAM
1286	3	MIN-MZ BEAM
1287	3	MAX-VY BEAM
1288	3	MIN-VY BEAM
1289	3	MAX-N BEAM
1290	3	MIN-N BEAM
1291	3	MAX-MT BEAM
1292	3	MIN-MT BEAM

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΔΙΑΣΤΑΣΙΟΛΟΓΗΣΗ ΦΟΡΕΑ ΤΕΧΝΙΚΟΥ ΓΙΑ ΟΚΑ ΣΤΑΤΙΚΩΝ ΦΟΡΤΙΣΕΩΝ

Design according to DIN1045-1 2008
Loadcases have been calculated in the Ultimate Limit State
In BEMESS no additional load safety factor is applied.

Load Cases for the Design

Loadcase 2101 MAX-MX QUAK ULS-B1
Loadcase 2102 MIN-MX QUAK ULS-B1
Loadcase 2103 MAX-MY QUAK ULS-B1
Loadcase 2104 MIN-MY QUAK ULS-B1
Loadcase 2105 MAX-MXY QUAK ULS-B1
Loadcase 2106 MIN-MXY QUAK ULS-B1
Loadcase 2107 MAX-VX QUAK ULS-B1
Loadcase 2108 MIN-VX QUAK ULS-B1
Loadcase 2109 MAX-VY QUAK ULS-B1
Loadcase 2110 MIN-VY QUAK ULS-B1
Loadcase 2111 MAX-NXX QUAK ULS-B1
Loadcase 2112 MIN-NXX QUAK ULS-B1
Loadcase 2113 MAX-NYY QUAK ULS-B1
Loadcase 2114 MIN-NYY QUAK ULS-B1
Loadcase 2115 MAX-NXY QUAK ULS-B1
Loadcase 2116 MIN-NXY QUAK ULS-B1

Material (DIN1045-1 2008)

Mat	f-ck [MPa]	f-cr [MPa]	f-yk [MPa]	f-tk [MPa]	f-ctm [MPa]	N [-]	minQ [-]	type
1	25.0	21.2			2.565	7.5	0.20	mainly static
3	25.0	21.2			2.565	7.5	0.20	mainly static
4	25.0	21.2			2.565	7.5	0.20	mainly static
5	25.0	21.2			2.565	7.5	0.20	mainly static
6	25.0	21.2			2.565	7.5	0.20	mainly static
7	25.0	21.2			2.565	7.5	0.20	mainly static
8	25.0	21.2			2.565	7.5	0.20	mainly static
9	25.0	21.2			2.565	7.5	0.20	mainly static
10	25.0	21.2			2.565	7.5	0.20	mainly static

Minimum reinforcement: 0.00 p.c. of stat. req. section
12 500.0 525.0

Reduction of FC in case of transvers tension = 25.0 [o/o]

Material-safety-factors:

Mat	concr SC1	SC2	steel SS1	SS2
1	1.50	1.50		
3	1.50	1.50		
4	1.50	1.50		
5	1.50	1.50		
6	1.50	1.50		
7	1.50	1.50		
8	1.50	1.50		
9	1.50	1.50		
10	1.50	1.50		
12			1.15	1.15

Acc. the german DIN Fachberichten a minimum concrete shear capacity VRd,ct
is taken into account in the shear design without shear reinforcement.

In shear design the cotangens theta is limited to 1.750 .

At direct supports from the face of the support up to 1.0*d the shear force is reduced.
The maximum shear capacity is checked at the face of the support without reduction.

The punching design has been switched off and must be done separately.
Outside the punching area, the normal slab shear design may increase the,
longitudinal reinforcement up to 0.20% [input CTRL...RO_V].

Geometry (axial covers)

No	he-upper [mm]	hi-upper [mm]	he-lower [mm]	hi-lower [mm]	Elem. height [mm]
1	50	70	35	55	As saved

Selection of elements

	from	to	inc	group	GEOMETRY
Element	3001	3999	1	-	1
Element	4001	4999	1	-	1
Element	8001	8999	1	-	1

Reinforcement is saved in the data base file
Number of stored reinforcement-distribution: 521

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΔΙΑΣΤΑΣΙΟΛΟΓΗΣΗ ΦΟΡΕΑ ΤΕΧΝΙΚΟΥ ΓΙΑ ΟΚΑ ΣΤΑΤΙΚΩΝ ΦΟΡΤΙΣΕΩΝ

Maximum Reinforcement [cm2/m]
(stored in data base file with reinforcement-distribution-no. 521)

Grp	Element	upper:As	Ast	dir	lower:As	Ast	dir	Ass[cm2/m2]	AssE[cm2]
3	3001	0.77	0.28	0	0.50	0.25	0		
3	3002	0.67	0.28	0	0.79	0.17	0		
3	3003	0.56	0.20	0	0.97	0.20	0		
3	3004	0.46	0.11	0	1.11	0.22	0		
3	3005	0.37	0.07	0	1.18	0.24	0		
3	3006	0.33	0.07	0	1.20	0.24	0		
3	3007	0.82	0.68	0	0.34	0.37	0		
3	3008	0.67	0.82	0	0.65	0.24	0		
3	3009	0.56	0.66	0	0.84	0.29	0		
3	3010	0.46	0.62	0	1.01	0.27	0		
3	3011	0.37	0.56	0	1.05	0.24	0		
3	3012	0.33	0.53	0	1.07	0.21	0		
3	3013	0.77	0.66	0	0.42	0.17	0		
3	3014	0.67	0.64	0	0.78	0.33	0		
3	3015	0.57	0.50	0	1.02	0.41	0		
3	3016	0.46	0.36	0	1.17	0.44	0		
3	3017	0.37	0.24	0	1.24	0.42	0		
3	3018	0.33	0.24	0	1.18	0.41	0		
3	3019	0.78	0.63	0	0.45	0.38	0		
3	3020	0.69	0.70	0	0.75	0.33	0		
3	3021	0.58	0.66	0	0.96	0.26	0		
3	3022	0.47	0.60	0	1.08	0.22	0		
3	3023	0.38	0.53	0	1.15	0.23	0		
3	3024	0.33	0.41	0	1.14	0.23	0		
3	3025	0.85	0.87	0	0.37	0.27	0		
3	3026	0.71	1.20	0	0.58	0.23	0		
3	3027	0.58	1.35	0	0.72	0.19	0		
3	3028	0.48	1.32	0	0.80	0.16	0		
3	3029	0.38	1.27	0	0.79	0.16	0		
3	3030	0.33	1.22	0	0.78	0.16	0		
3	3031	0.81	0.72	0	0.38	0.13	0		
3	3032	0.70	0.97	0	0.41	0.16	0		
3	3033	0.59	1.10	0	0.63	0.24	0		
3	3034	0.48	1.05	0	0.75	0.18	0		
3	3035	0.38	0.98	0	0.76	0.25	0		
3	3036	0.33	0.94	0	0.78	0.22	0		
3	3037	0.85	0.52	0	0.41	0.28	0		
3	3038	0.72	0.60	0	0.45	0.30	0		
3	3039	0.59	0.67	0	0.62	0.28	0		
3	3040	0.48	0.65	0	0.73	0.26	0		
3	3041	0.38	0.63	0	0.79	0.22	0		
3	3042	0.33	0.61	0	0.80	0.20	0		
3	3043	0.81	0.35	0	0.42	0.46	0		
3	3044	0.72	0.37	0	0.70	0.69	0		
3	3045	0.59	0.22	0	0.91	0.83	0		
3	3046	0.48	0.13	0	1.02	0.88	0		
3	3047	0.38	0.18	0	1.04	0.85	0		
3	3048	0.33	0.09	0	1.02	0.83	0		
3	3049	0.78	0.22	0	0.58	0.84	0		
3	3050	0.70	0.16	0	0.79	1.08	0		
3	3051	0.59	0.19	0	1.00	1.26	0		
3	3052	0.48	0.15	0	1.08	1.28	0		
3	3053	0.38	0.11	0	1.10	1.22	0		
3	3054	0.33	0.10	0	1.00	1.09	0		
3	3055	0.81	0.44	0	0.55	0.54	0		
3	3056	0.71	0.40	0	0.82	0.91	0		
3	3057	0.58	0.27	0	0.91	0.99	0		
3	3058	0.48	0.21	0	1.00	1.08	0		
3	3059	0.38	0.16	0	1.03	1.06	0		
3	3060	0.33	0.07	0	1.01	1.02	0		
3	3061	0.74	0.27	0	0.59	0.52	0		
3	3062	0.69	0.23	0	0.75	0.61	0		
3	3063	0.58	0.19	0	0.86	0.59	0		
3	3064	0.47	0.17	0	0.83	0.56	0		
3	3065	0.38	0.13	0	0.85	0.47	0		
3	3066	0.33	0.07	0	0.83	0.40	0		
3	3067	0.79	0.25	0	0.43	0.57	0		
3	3068	0.67	0.45	0	0.69	0.48	0		
3	3069	0.57	0.61	0	0.70	0.36	0		
3	3070	0.46	0.62	0	0.71	0.27	0		
3	3071	0.37	0.58	0	0.57	0.22	0		
3	3072	0.33	0.54	0	0.55	0.18	0		
3	3073	0.75	0.24	0	0.47	0.26	0		
3	3074	0.67	0.28	0	0.55	0.25	0		
3	3075	0.56	0.29	0	0.64	0.25	0		
3	3076	0.46	0.28	0	0.61	0.21	0		
3	3077	0.37	0.18	0	0.64	0.17	0		
3	3078	0.33	0.12	0	0.62	0.14	0		

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΔΙΑΣΤΑΣΙΟΛΟΓΗΣΗ ΦΟΡΕΑ ΤΕΧΝΙΚΟΥ ΓΙΑ ΟΚΑ ΣΤΑΤΙΚΩΝ ΦΟΡΤΙΣΕΩΝ

Maximum Reinforcement [cm2/m]
(stored in data base file with reinforcement-distribution-no. 521)

Grp	Element	upper:As	Ast	dir	lower:As	Ast	dir	Ass[cm2/m2]	AssE[cm2]
3	3079	0.74	0.15	0	0.44	0.16	0		
3	3080	0.67	0.18	0	0.56	0.17	0		
3	3081	0.56	0.18	0	0.58	0.17	0		
3	3082	0.46	0.12	0	0.58	0.15	0		
3	3083	0.37	0.08	0	0.61	0.13	0		
3	3084	0.33	0.07	0	0.61	0.12	0		
3	3085	0.33	0.07	0	1.20	0.24	0		
3	3086	0.37	0.08	0	1.19	0.24	0		
3	3087	0.46	0.10	0	1.11	0.22	0		
3	3088	0.56	0.23	0	0.96	0.19	0		
3	3089	0.67	0.26	0	0.71	0.17	0		
3	3090	0.77	0.25	0	0.62	0.26	0		
3	3091	0.33	0.58	0	1.07	0.22	0		
3	3092	0.37	0.62	0	1.04	0.25	0		
3	3093	0.46	0.66	0	0.98	0.27	0		
3	3094	0.56	0.67	0	0.78	0.24	0		
3	3095	0.67	0.79	0	0.62	0.18	0		
3	3096	0.82	0.69	0	0.34	0.33	0		
3	3097	0.33	0.24	0	1.23	0.39	0		
3	3098	0.37	0.26	0	1.23	0.45	0		
3	3099	0.46	0.32	0	1.15	0.46	0		
3	3100	0.57	0.51	0	0.99	0.43	0		
3	3101	0.67	0.65	0	0.76	0.34	0		
3	3102	0.81	0.66	0	0.39	0.16	0		
3	3103	0.33	0.46	0	1.14	0.23	0		
3	3104	0.38	0.55	0	1.14	0.23	0		
3	3105	0.47	0.62	0	1.07	0.23	0		
3	3106	0.58	0.66	0	0.93	0.25	0		
3	3107	0.69	0.68	0	0.77	0.31	0		
3	3108	0.78	0.64	0	0.41	0.35	0		
3	3109	0.33	1.26	0	0.78	0.16	0		
3	3110	0.38	1.29	0	0.80	0.16	0		
3	3111	0.48	1.32	0	0.79	0.16	0		
3	3112	0.58	1.34	0	0.70	0.20	0		
3	3113	0.71	1.21	0	0.55	0.23	0		
3	3114	0.85	0.82	0	0.38	0.27	0		
3	3115	0.33	0.96	0	0.79	0.24	0		
3	3116	0.38	1.00	0	0.76	0.24	0		
3	3117	0.48	1.06	0	0.71	0.25	0		
3	3118	0.59	1.08	0	0.64	0.23	0		
3	3119	0.70	0.98	0	0.41	0.16	0		
3	3120	0.81	0.71	0	0.38	0.13	0		
3	3121	0.33	0.62	0	0.80	0.22	0		
3	3122	0.38	0.64	0	0.79	0.21	0		
3	3123	0.48	0.66	0	0.73	0.25	0		
3	3124	0.59	0.67	0	0.63	0.27	0		
3	3125	0.72	0.58	0	0.46	0.30	0		
3	3126	0.85	0.50	0	0.41	0.28	0		
3	3127	0.33	0.14	0	1.03	0.82	0		
3	3128	0.38	0.18	0	1.06	0.88	0		
3	3129	0.48	0.13	0	1.03	0.89	0		
3	3130	0.59	0.20	0	0.91	0.83	0		
3	3131	0.72	0.37	0	0.68	0.67	0		
3	3132	0.81	0.40	0	0.42	0.42	0		
3	3133	0.33	0.15	0	1.07	1.16	0		
3	3134	0.38	0.17	0	1.11	1.24	0		
3	3135	0.48	0.14	0	1.09	1.29	0		
3	3136	0.59	0.22	0	0.98	1.25	0		
3	3137	0.70	0.16	0	0.80	1.10	0		
3	3138	0.78	0.21	0	0.58	0.84	0		
3	3139	0.33	0.07	0	0.99	1.00	0		
3	3140	0.38	0.17	0	1.01	1.04	0		
3	3141	0.48	0.22	0	0.98	1.04	0		
3	3142	0.58	0.27	0	0.94	1.08	0		
3	3143	0.71	0.40	0	0.83	0.89	0		
3	3144	0.81	0.44	0	0.49	0.49	0		
3	3145	0.33	0.07	0	0.81	0.42	0		
3	3146	0.38	0.14	0	0.82	0.49	0		
3	3147	0.47	0.17	0	0.80	0.55	0		
3	3148	0.58	0.22	0	0.85	0.59	0		
3	3149	0.69	0.22	0	0.76	0.61	0		
3	3150	0.74	0.26	0	0.59	0.51	0		
3	3151	0.33	0.56	0	0.54	0.19	0		
3	3152	0.37	0.57	0	0.58	0.19	0		
3	3153	0.46	0.59	0	0.67	0.28	0		
3	3154	0.57	0.59	0	0.74	0.36	0		
3	3155	0.67	0.45	0	0.68	0.48	0		
3	3156	0.79	0.23	0	0.43	0.57	0		

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΔΙΑΣΤΑΣΙΟΛΟΓΗΣΗ ΦΟΡΕΑ ΤΕΧΝΙΚΟΥ ΓΙΑ ΟΚΑ ΣΤΑΤΙΚΩΝ ΦΟΡΤΙΣΕΩΝ

Maximum Reinforcement [cm2/m]
(stored in data base file with reinforcement-distribution-no. 521)

Grp	Element	upper:As	Ast	dir	lower:As	Ast	dir	Ass[cm2/m2]	AssE[cm2]
3	3157	0.33	0.13	0	0.60	0.13	0		
3	3158	0.37	0.17	0	0.62	0.18	0		
3	3159	0.46	0.28	0	0.67	0.22	0		
3	3160	0.56	0.29	0	0.62	0.25	0		
3	3161	0.67	0.28	0	0.58	0.25	0		
3	3162	0.75	0.24	0	0.47	0.25	0		
3	3163	0.33	0.07	0	0.60	0.12	0		
3	3164	0.37	0.09	0	0.60	0.13	0		
3	3165	0.46	0.11	0	0.62	0.16	0		
3	3166	0.56	0.18	0	0.57	0.17	0		
3	3167	0.67	0.18	0	0.59	0.17	0		
3	3168	0.74	0.15	0	0.44	0.16	0		
4	4001	2.91	0.58	0	1.03	0.21	0		
4	4002	1.00	0.20	0	0.63	0.47	0		
4	4003	1.95	0.46	0	0.80	1.00	0	10.80	0.65
4	4004	0.85	0.38	0	0.50	0.85	0		
4	4005	2.24	1.52	0	0.91	0.21	0		
4	4006	0.97	0.75	0	0.39	0.08	0		
4	4007	2.43	0.99	0	0.97	0.27	0		
4	4008	1.04	0.65	0	0.47	0.35	0		
4	4009	1.90	0.53	0	0.79	0.60	0		
4	4010	0.96	0.40	0	0.42	0.52	0		
4	4011	2.07	1.13	0	0.85	0.30	0		
4	4012	0.91	0.60	0	0.41	0.11	0		
4	4013	1.97	0.77	0	0.84	0.32	0		
4	4014	0.97	0.51	0	0.43	0.34	0		
4	4015	2.01	0.97	0	0.84	0.43	0		
4	4016	0.91	0.47	0	0.49	0.51	0		
4	4017	1.92	0.66	0	0.85	0.60	0		
4	4018	0.91	0.22	0	0.52	0.71	0		
4	4019	1.86	1.03	0	0.79	0.22	0		
4	4020	0.88	0.51	0	0.43	0.09	0		
4	4021	2.17	0.58	0	0.97	0.27	0		
4	4022	0.90	0.27	0	0.48	0.39	0		
4	4023	1.77	0.50	0	0.91	1.07	0		
4	4024	0.82	0.16	0	0.53	0.79	0		
4	4025	1.77	1.04	0	0.80	0.18	0		
4	4026	0.78	0.51	0	0.39	0.18	0		
4	4027	2.24	0.45	0	1.07	0.21	0		
4	4028	0.96	0.28	0	0.46	0.11	0		
4	4029	1.00	0.20	0	0.44	0.47	0		
4	4030	2.89	0.58	0	1.03	0.21	0		
4	4031	0.85	0.40	0	0.44	0.85	0		
4	4032	1.95	0.46	0	0.80	1.03	0	10.94	0.66
4	4033	0.97	0.75	0	0.39	0.08	0		
4	4034	2.24	1.52	0	0.91	0.21	0		
4	4035	1.02	0.66	0	0.42	0.34	0		
4	4036	2.42	0.99	0	0.97	0.27	0		
4	4037	0.96	0.35	0	0.42	0.52	0		
4	4038	1.90	0.53	0	0.79	0.60	0		
4	4039	0.91	0.61	0	0.41	0.11	0		
4	4040	2.07	1.12	0	0.85	0.30	0		
4	4041	0.97	0.46	0	0.43	0.33	0		
4	4042	1.97	0.72	0	0.84	0.32	0		
4	4043	0.91	0.48	0	0.43	0.44	0		
4	4044	2.01	0.97	0	0.84	0.40	0		
4	4045	0.91	0.22	0	0.45	0.67	0		
4	4046	1.92	0.66	0	0.85	0.58	0		
4	4047	0.88	0.51	0	0.43	0.09	0		
4	4048	1.86	1.03	0	0.79	0.22	0		
4	4049	0.90	0.27	0	0.48	0.39	0		
4	4050	2.17	0.58	0	0.97	0.27	0		
4	4051	0.82	0.16	0	0.51	0.80	0		
4	4052	1.77	0.50	0	0.91	1.07	0		
4	4053	0.78	0.51	0	0.39	0.18	0		
4	4054	1.77	1.04	0	0.80	0.18	0		
4	4055	0.96	0.28	0	0.46	0.11	0		
4	4056	2.24	0.46	0	1.07	0.21	0		
8	8001	0.28	1.39	0	0.85	4.25	0		
8	8002	0.53	2.62	0	1.73	8.67	0		
8	8003	0.67	3.20	0	7.25	18.76	0		
8	8004	3.02	2.86	0	1.69	8.44	0		
8	8005	2.37	3.18	0	8.92	20.87	0		
8	8006	0.95	2.62	0	1.55	7.73	0		
8	8007	0.44	1.53	0	0.85	4.26	0		
8	8008	0.47	2.33	0	1.88	7.58	0		
8	8009	1.37	3.33	0	7.98	18.78	0		
8	8010	0.49	2.44	0	1.51	7.55	0		

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΔΙΑΣΤΑΣΙΟΛΟΓΗΣΗ ΦΟΡΕΑ ΤΕΧΝΙΚΟΥ ΓΙΑ ΟΚΑ ΣΤΑΤΙΚΩΝ ΦΟΡΤΙΣΕΩΝ

Maximum Reinforcement [cm2/m]
(stored in data base file with reinforcement-distribution-no. 521)

Grp	Element	upper:As	Ast	dir	lower:As	Ast	dir	Ass[cm2/m2]	AssE[cm2]
8	8011	2.02	3.00	0	5.84	15.03	0		
8	8012	0.57	2.85	0	1.69	8.44	0		
8	8013	1.85	2.90	0	5.60	14.05	0		
8	8014	0.59	2.96	0	1.74	8.69	0		
8	8015	1.47	3.06	0	5.63	14.07	0		
8	8016	0.59	2.95	0	1.73	8.67	0		
8	8017	1.41	2.20	0	5.81	14.94	0		
8	8018	0.48	2.42	0	1.69	8.46	0		
8	8019	1.38	2.56	0	7.98	18.71	0		
8	8020	0.46	2.28	0	1.51	7.57	0		
8	8021	0.37	1.34	0	0.88	4.41	0		
8	8022	0.60	1.99	0	1.78	7.58	0		
8	8023	1.76	0.65	0	9.08	21.11	0		
8	8024	0.70	1.93	0	1.58	7.88	0		
8	8025	0.52	0.54	0	8.18	21.25	0		
8	8026	2.57	1.93	0	1.68	8.42	0		
8	8027	0.15	0.76	0	0.85	4.24	0		
8	8028	0.45	1.39	0	1.73	8.64	0		
8	8029	0.28	1.39	0	0.85	4.25	0		
8	8030	0.67	3.20	0	7.20	18.62	0		
8	8031	0.56	2.62	0	1.73	8.67	0		
8	8032	3.06	2.86	0	1.69	8.44	0		
8	8033	2.39	3.18	0	8.92	20.87	0		
8	8034	0.50	1.53	0	0.85	4.26	0		
8	8035	1.37	3.33	0	7.98	18.78	0		
8	8036	0.99	2.62	0	1.55	7.73	0		
8	8037	0.47	2.33	0	1.88	7.58	0		
8	8038	0.49	2.44	0	1.51	7.56	0		
8	8039	2.02	3.00	0	5.84	15.03	0		
8	8040	1.66	2.90	0	5.60	14.05	0		
8	8041	0.57	2.85	0	1.69	8.44	0		
8	8042	0.59	2.96	0	1.74	8.69	0		
8	8043	1.47	3.06	0	5.63	14.07	0		
8	8044	1.41	2.20	0	5.81	14.94	0		
8	8045	0.59	2.95	0	1.73	8.67	0		
8	8046	0.48	2.42	0	1.69	8.46	0		
8	8047	1.38	2.56	0	7.98	18.71	0		
8	8048	0.37	1.34	0	0.88	4.41	0		
8	8049	1.76	0.65	0	9.08	21.11	0		
8	8050	0.46	2.28	0	1.51	7.57	0		
8	8051	0.60	1.99	0	1.81	7.58	0		
8	8052	0.70	1.93	0	1.58	7.88	0		
8	8053	0.52	0.54	0	8.18	21.25	0		
8	8054	0.15	0.76	0	0.85	4.24	0		
8	8055	2.57	1.93	0	1.68	8.42	0		
8	8056	0.45	1.39	0	1.73	8.64	0		

REINFORCEMENT INDEX [kg netto]: 0.058 (Upper)
0.116 (Lower)
0.001 (Shear)

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΔΙΑΣΤΑΣΙΟΛΟΓΗΣΗ ΦΟΡΕΑ ΤΕΧΝΙΚΟΥ ΓΙΑ ΟΚΑ ΣΤΑΤΙΚΩΝ ΦΟΡΤΙΣΕΩΝ

Maximal values of the shear design
Only elements with shear reinforcement are printed.
At punching points punching reinforcement is printed.

element	ass [cm2/m2]	tau [MPa]	acc.VED/VRDmax	acc.cot_theta	min_z [m]
4003	10.80	0.56	0.180	1.75	0.186
4032	10.94	0.57	0.182	1.75	0.186

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Selected Beam Elements

FROM	TO	INC	X-VALUE	NC	MEMBER	CS0	CS1	CS2	CS3	CS4	CS5
10001											
10005											
10006											
10009											
10010											
10014											
10016											
10020											
10021											
10024											
10025											
10029											
12000	12150	1									

Default design code is DIN Fachbericht 102 Massivbrȯcken (2003) (Germany)
Klasse(Tab.4.118): D
Wind zone : Binnenland

Materials

No. 1 C 25/30 (DIN 1045-1)
No. 3 C 25/30 (DIN 1045-1)
No. 4 C 25/30 (DIN 1045-1)
No. 5 C 25/30 (DIN 1045-1)
No. 6 C 25/30 (DIN 1045-1)
No. 7 C 25/30 (DIN 1045-1)
No. 8 C 25/30 (DIN 1045-1)
No. 9 C 25/30 (DIN 1045-1)
No. 10 C 25/30 (DIN 1045-1)
No. 12 BSt 500 SA (DIN 1045-1)

Reinforcement will be accounted for sectional values as defined in AQUA
Reinforcements saved as design case LCR 506

Considered Load Cases

No.	refer	act on	Title/type of load case	gam-u	gam-f	psi-0	psi-1	psi-2	psi-1'	
1	part.	CS 1	I.B. ΚΑΤΑΚ.ΣΤΟΙΧΕΙΩΝ	1.35	1.00	1.00	1.00	1.00	1.00	G perm
			G (total dead load)							
2	part.	CS 1	I.B. ΔΟΚΩΝ	1.35	1.00	1.00	1.00	1.00	1.00	G perm
			G (total dead load)							
3	part.	CS 1	I.B. ΧΥΤΗΣ ΠΛΑΚΑΣ	1.35	1.00	1.00	1.00	1.00	1.00	G perm
			G (total dead load)							
11	part.	CS 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+0.5	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
12	part.	CS 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+0.5	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
13	part.	CS 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+1.0	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
14	part.	CS 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+1.0	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
1201	part.	CS 1	MAX-MY BEAM	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
1202	part.	CS 1	MIN-MY BEAM	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
1203	part.	CS 1	MAX-VZ BEAM	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
1204	part.	CS 1	MIN-VZ BEAM	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
1205	part.	CS 1	MAX-MZ BEAM	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
1206	part.	CS 1	MIN-MZ BEAM	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
1207	part.	CS 1	MAX-VY BEAM	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
1208	part.	CS 1	MIN-VY BEAM	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
1209	part.	CS 1	MAX-N BEAM	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
1210	part.	CS 1	MIN-N BEAM	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
1211	part.	CS 1	MAX-MT BEAM	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
1212	part.	CS 1	MIN-MT BEAM	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Ultimate Load Design

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==============================================================================
Design for ultimate loads DIN Fachbericht 102 Massivbrücken (2003)
Biaxial bending, uniaxial stress calculated in y-z axis
Safety factors SC-1  SC-2  SC-S  SS-1  SS-2  PIIa
               1.50  1.50  1.50  1.15  1.15     7
Strain limits    C1    C2    S1    S2    Z1    Z2
          max -3.50 -2.00  3.00 25.00 -3.50 25.00

parameters for reinforcements
Minimum reinforcements    compression  min. reinforcem.  maximum-
   Bending.  Compress.    e/d   N/Npl    requ. section   reinforc.
 0.00 [cm2] 0.30 [o/o]   3.50  0.0010     0.00     0.15       9.00
Tensile forces in the longitudinal reinforcements due to shear are NOT accounted for.
Material of sections uses Ultimate Limit strain-stress law with global safety factors
Material of reinforcements uses Ultimate Limit strain-stress law with global safety factors
```

MNo.	temp lev.	Material-safety [-]	max.compr stress [MPa]	at strain [o/oo]	max.tens stress [MPa]	at strain [o/oo]	tension-stiffening [MPa]
1	0	1.500	-14.17	-2.00	0.00	0.00	
3	0	1.500	-14.17	-2.00	0.00	0.00	
4	0	1.500	-14.17	-2.00	0.00	0.00	
5	0	1.500	-14.17	-2.00	0.00	0.00	
6	0	1.500	-14.17	-2.00	0.00	0.00	
7	0	1.500	-14.17	-2.00	0.00	0.00	
8	0	1.500	-14.17	-2.00	0.00	0.00	
9	0	1.500	-14.17	-2.00	0.00	0.00	
10	0	1.500	-14.17	-2.00	0.00	0.00	
12	0	1.150	-456.52	-25.00	456.52	25.00	

Combinations For Ultimate Design

```
 1213 ( gross ) max_my-1213
       MAX  +  MY   :
               1.35 * G + 1.50 * L_A + 1.00 * L_B
 1214 ( gross ) min_my-1214
       MIN  +  MY   :
               1.35 * G + 1.50 * L_A + 1.00 * L_B
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Shear Design

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==============================================================================
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Design for shear DIN 1045-1 (2003)
Minimum shear factor or tan of inclination of compressive struts 0.57 / 1.72

MNo	f-cd [MPa]	tau-rd [MPa]	sigIIQ [MPa]	sigIIT [MPa]	sigIIQ+ [MPa]	fyd [MPa]
1	14.17	0.10	10.62	7.44	10.62	
3	14.17	0.10	10.62	7.44	10.62	
4	14.17	0.10	10.62	7.44	10.62	
5	14.17	0.10	10.62	7.44	10.62	
6	14.17	0.10	10.62	7.44	10.62	
7	14.17	0.10	10.62	7.44	10.62	
8	14.17	0.10	10.62	7.44	10.62	
9	14.17	0.10	10.62	7.44	10.62	
10	14.17	0.10	10.62	7.44	10.62	
12						434.78

Tolerance for exceeding maximum shear or principal compression stress 0.0200

Longitudinal Reinforcements Accumulated minimum
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
10001	0.000	8	0.03	4.25		3.98T	0.14		0.14	
10001	0.200	8	0.02	3.29		3.29T				
10005	0.000	8	0.12	15.75		13.82T			1.93	
10005	0.200	8	0.13	16.90		15.67T	0.83		0.40	
10006	0.000	8	0.00	0.46		0.35T	0.10			
10006	0.400	8	0.01	1.43		1.24T	0.19			
10009	0.000	8	0.02	2.00		1.60T	0.40			
10009	0.400	8	0.02	2.01		1.67T	0.34			
10010	0.000	8	0.13	16.58		15.37T	0.78		0.43	
10010	0.200	8	0.13	17.19		15.31T			1.88	
10014	0.000	8	0.02	3.28		3.28T				
10014	0.200	8	0.03	3.60		3.39T	0.11		0.11	
10016	0.000	8	0.03	4.22		3.94T	0.14		0.14	
10016	0.200	8	0.03	3.56		3.56T				
10020	0.000	8	0.12	15.59		13.74T			1.85	
10020	0.200	8	0.13	16.80		15.56T	0.83		0.40	
10021	0.000	8	0.00	0.51		0.27T	0.25			
10021	0.400	8	0.01	1.43		1.25T	0.19			
10024	0.000	8	0.01	1.87		1.50T	0.37			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
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Longitudinal Reinforcements Accumulated minimum
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
10024	0.400	8	0.01	1.87		1.53T	0.34			
10025	0.000	8	0.12	16.21		15.05T	0.75		0.41	
10025	0.200	8	0.14	18.37		16.54T			1.83	
10029	0.000	8	0.03	3.43		3.43T				
10029	0.200	8	0.03	3.55		3.35T	0.10		0.10	
12001	0.000	9	0.69	34.65			34.65T			
12001	1.000	9	0.41	20.40			20.40T			
12002	0.000	9	0.41	20.40			20.40T			
12002	1.000	9	0.48	24.24			24.24T			
12003	0.000	9	0.48	24.24			24.24T			
12003	1.000	9	0.98	49.36			49.36T			
12004	0.000	9	0.30	15.08			15.08T			
12004	1.000	9	0.30	15.08			15.08T			
12005	0.000	9	0.30	15.08			15.08T			
12005	1.000	9	0.49	24.68			24.68T			
12006	0.000	9	0.49	24.68			24.68T			
12006	1.000	9	0.97	48.95			48.95T			
12007	0.000	9	0.40	20.22			20.22T			
12007	1.000	9	0.30	15.08			15.08T			
12008	0.000	9	0.30	15.08			15.08T			
12008	1.000	9	0.49	24.60			24.60T			
12009	0.000	9	0.49	24.60			24.60T			
12009	1.000	9	0.97	48.90			48.90T			
12010	0.000	9	0.69	34.65			34.65T			
12010	1.000	9	0.41	20.40			20.40T			
12011	0.000	9	0.41	20.40			20.40T			
12011	1.000	9	0.48	24.24			24.24T			
12012	0.000	9	0.48	24.24			24.24T			
12012	1.000	9	0.97	48.90			48.90T			
12013	0.000	9	0.31	15.65			15.65T			
12013	1.000	9	0.30	15.08			15.08T			
12014	0.000	9	0.30	15.08			15.08T			
12014	1.000	9	0.48	24.31			24.31T			
12015	0.000	9	0.48	24.31			24.31T			
12015	1.000	9	0.97	48.95			48.95T			
12016	0.000	9	0.46	22.97			22.97T			
12016	1.000	9	0.31	15.70			15.70T			
12017	0.000	9	0.31	15.70			15.70T			
12017	1.000	9	0.49	24.60			24.60T			
12018	0.000	9	0.49	24.60			24.60T			
12018	1.000	9	0.98	49.36			49.36T			
12019	0.000	9	0.98	49.36			49.36T			
12019	1.000	9	1.40	70.29			70.29T			
12020	0.000	9	1.40	70.23			70.23T			
12020	1.000	9	1.47	74.07			74.07T			
12021	0.000	9	1.47	74.03			74.03T			
12021	1.000	9	1.31	65.97			65.97T			
12022	0.000	9	1.31	65.94			65.94T			
12022	1.000	9	1.04	52.06			52.06T			
12023	0.000	9	1.04	52.04			52.04T			
12023	1.000	9	0.72	36.21			36.21T			
12024	0.000	9	0.72	36.20			36.20T			
12024	1.000	9	0.42	21.35			21.35T			
12025	0.000	9	0.42	21.35			21.35T			
12025	1.000	9	0.30	15.08			15.08T			
12026	0.000	9	0.30	15.08			15.08T			
12026	1.000	9	0.30	15.08			15.08T			
12027	0.000	9	0.30	15.08			15.08T			
12027	1.000	9	0.30	15.08			15.08T			
12028	0.000	9	0.30	15.08			15.08T			
12028	1.000	9	0.30	15.08			15.08T			
12029	0.000	9	0.30	15.08			15.08T			
12029	1.000	9	0.30	15.08			15.08T			
12030	0.000	9	0.30	15.08			15.08T			
12030	1.000	9	0.30	15.08			15.08T			
12031	0.000	9	0.30	15.08			15.08T			
12031	1.000	9	0.30	15.08			15.08T			
12032	0.000	9	0.30	15.08			15.08T			
12032	1.000	9	0.30	15.08			15.08T			
12033	0.000	9	0.30	15.08			15.08T			
12033	1.000	9	0.30	15.08			15.08T			
12034	0.000	9	0.30	15.08			15.08T			
12034	1.000	9	0.30	15.08			15.08T			
12035	0.000	9	0.30	15.08			15.08T			
12035	1.000	9	0.30	15.08			15.08T			
12036	0.000	9	0.30	15.08			15.08T			

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Longitudinal Reinforcements Accumulated minimum
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
12036	1.000	9	0.30	15.08			15.08T			
12037	0.000	9	0.30	15.08			15.08T			
12037	1.000	9	0.30	15.08			15.08T			
12038	0.000	9	0.30	15.08			15.08T			
12038	1.000	9	0.30	15.08			15.08T			
12039	0.000	9	0.30	15.08			15.08T			
12039	1.000	9	0.30	15.08			15.08T			
12040	0.000	9	0.30	15.08			15.08'			
12040	1.000	9	0.30	15.08			15.08'			
12041	0.000	9	0.97	48.95			48.95T			
12041	1.000	9	1.39	69.85			69.85T			
12042	0.000	9	1.39	69.79			69.79T			
12042	1.000	9	1.47	73.69			73.69T			
12043	0.000	9	1.47	73.65			73.65T			
12043	1.000	9	1.31	65.97			65.97T			
12044	0.000	9	1.31	65.94			65.94T			
12044	1.000	9	1.04	52.05			52.05T			
12045	0.000	9	1.04	52.03			52.03T			
12045	1.000	9	0.72	36.19			36.19T			
12046	0.000	9	0.72	36.19			36.19T			
12046	1.000	9	0.42	21.33			21.33T			
12047	0.000	9	0.42	21.33			21.33T			
12047	1.000	9	0.30	15.08			15.08T			
12048	0.000	9	0.30	15.08			15.08T			
12048	1.000	9	0.30	15.08			15.08T			
12049	0.000	9	0.30	15.08			15.08T			
12049	1.000	9	0.30	15.08			15.08T			
12050	0.000	9	0.30	15.08			15.08T			
12050	1.000	9	0.30	15.08			15.08T			
12051	0.000	9	0.30	15.08			15.08T			
12051	1.000	9	0.30	15.08			15.08T			
12052	0.000	9	0.30	15.08			15.08T			
12052	1.000	9	0.30	15.08			15.08T			
12053	0.000	9	0.30	15.08			15.08T			
12053	1.000	9	0.30	15.08			15.08T			
12054	0.000	9	0.30	15.08			15.08T			
12054	1.000	9	0.30	15.08			15.08T			
12055	0.000	9	0.30	15.08			15.08T			
12055	1.000	9	0.30	15.08			15.08T			
12056	0.000	9	0.30	15.08			15.08T			
12056	1.000	9	0.30	15.08			15.08T			
12057	0.000	9	0.30	15.08			15.08T			
12057	1.000	9	0.30	15.08			15.08T			
12058	0.000	9	0.30	15.08			15.08T			
12058	1.000	9	0.30	15.08			15.08T			
12059	0.000	9	0.30	15.08			15.08T			
12059	1.000	9	0.30	15.08			15.08T			
12060	0.000	9	0.30	15.08			15.08T			
12060	1.000	9	0.30	15.08			15.08T			
12061	0.000	9	0.30	15.08			15.08T			
12061	1.000	9	0.30	15.08			15.08T			
12062	0.000	9	0.30	15.08			15.08'			
12062	1.000	9	0.30	15.08			15.08'			
12063	0.000	9	0.97	48.90			48.90T			
12063	1.000	9	1.39	69.82			69.82T			
12064	0.000	9	1.39	69.76			69.76T			
12064	1.000	9	1.47	73.68			73.68T			
12065	0.000	9	1.46	73.63			73.63T			
12065	1.000	9	1.32	66.24			66.24T			
12066	0.000	9	1.32	66.21			66.21T			
12066	1.000	9	1.04	52.19			52.19T			
12067	0.000	9	1.04	52.17			52.17T			
12067	1.000	9	0.72	36.21			36.21T			
12068	0.000	9	0.72	36.20			36.20T			
12068	1.000	9	0.42	21.25			21.25T			
12069	0.000	9	0.42	21.24			21.24T			
12069	1.000	9	0.30	15.08			15.08T			
12070	0.000	9	0.30	15.08			15.08T			
12070	1.000	9	0.30	15.08			15.08T			
12071	0.000	9	0.30	15.08			15.08T			
12071	1.000	9	0.30	15.08			15.08T			
12072	0.000	9	0.30	15.08			15.08T			
12072	1.000	9	0.30	15.08			15.08T			
12073	0.000	9	0.30	15.08			15.08T			
12073	1.000	9	0.30	15.08			15.08T			
12074	0.000	9	0.30	15.08			15.08T			

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Longitudinal Reinforcements Accumulated minimum
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
12074	1.000	9	0.30	15.08			15.08T			
12075	0.000	9	0.30	15.08			15.08T			
12075	1.000	9	0.30	15.08			15.08T			
12076	0.000	9	0.30	15.08			15.08T			
12076	1.000	9	0.30	15.08			15.08T			
12077	0.000	9	0.30	15.08			15.08T			
12077	1.000	9	0.30	15.08			15.08T			
12078	0.000	9	0.30	15.08			15.08T			
12078	1.000	9	0.30	15.08			15.08T			
12079	0.000	9	0.30	15.08			15.08T			
12079	1.000	9	0.30	15.08			15.08T			
12080	0.000	9	0.30	15.08			15.08T			
12080	1.000	9	0.30	15.08			15.08T			
12081	0.000	9	0.30	15.08			15.08T			
12081	1.000	9	0.30	15.08			15.08T			
12082	0.000	9	0.30	15.08			15.08T			
12082	1.000	9	0.30	15.08			15.08T			
12083	0.000	9	0.30	15.08			15.08T			
12083	1.000	9	0.30	15.08			15.08T			
12084	0.000	9	0.30	15.08			15.08'			
12084	1.000	9	0.30	15.08			15.08'			
12085	0.000	9	0.97	48.90			48.90T			
12085	1.000	9	1.39	69.82			69.82T			
12086	0.000	9	1.39	69.76			69.76T			
12086	1.000	9	1.47	73.68			73.68T			
12087	0.000	9	1.46	73.63			73.63T			
12087	1.000	9	1.32	66.24			66.24T			
12088	0.000	9	1.32	66.21			66.21T			
12088	1.000	9	1.04	52.06			52.06T			
12089	0.000	9	1.04	52.04			52.04T			
12089	1.000	9	0.72	36.21			36.21T			
12090	0.000	9	0.72	36.20			36.20T			
12090	1.000	9	0.42	21.35			21.35T			
12091	0.000	9	0.42	21.35			21.35T			
12091	1.000	9	0.30	15.08			15.08T			
12092	0.000	9	0.30	15.08			15.08T			
12092	1.000	9	0.30	15.08			15.08T			
12093	0.000	9	0.30	15.08			15.08T			
12093	1.000	9	0.30	15.08			15.08T			
12094	0.000	9	0.30	15.08			15.08T			
12094	1.000	9	0.30	15.08			15.08T			
12095	0.000	9	0.30	15.08			15.08T			
12095	1.000	9	0.30	15.08			15.08T			
12096	0.000	9	0.30	15.08			15.08T			
12096	1.000	9	0.30	15.08			15.08T			
12097	0.000	9	0.30	15.08			15.08T			
12097	1.000	9	0.30	15.08			15.08T			
12098	0.000	9	0.30	15.08			15.08T			
12098	1.000	9	0.30	15.08			15.08T			
12099	0.000	9	0.30	15.08			15.08T			
12099	1.000	9	0.30	15.08			15.08T			
12100	0.000	9	0.30	15.08			15.08T			
12100	1.000	9	0.30	15.08			15.08T			
12101	0.000	9	0.30	15.08			15.08T			
12101	1.000	9	0.30	15.08			15.08T			
12102	0.000	9	0.30	15.08			15.08T			
12102	1.000	9	0.30	15.08			15.08T			
12103	0.000	9	0.30	15.08			15.08T			
12103	1.000	9	0.30	15.08			15.08T			
12104	0.000	9	0.30	15.08			15.08T			
12104	1.000	9	0.30	15.08			15.08T			
12105	0.000	9	0.30	15.08			15.08T			
12105	1.000	9	0.30	15.08			15.08T			
12106	0.000	9	0.30	15.08			15.08'			
12106	1.000	9	0.30	15.08			15.08'			
12107	0.000	9	0.97	48.95			48.95T			
12107	1.000	9	1.39	69.85			69.85T			
12108	0.000	9	1.39	69.79			69.79T			
12108	1.000	9	1.47	73.69			73.69T			
12109	0.000	9	1.47	73.65			73.65T			
12109	1.000	9	1.32	66.23			66.23T			
12110	0.000	9	1.32	66.21			66.21T			
12110	1.000	9	1.04	52.17			52.17T			
12111	0.000	9	1.04	52.16			52.16T			
12111	1.000	9	0.72	36.19			36.19T			
12112	0.000	9	0.72	36.19			36.19T			

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Longitudinal Reinforcements Accumulated minimum
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
12112	1.000	9	0.42	21.23			21.23T			
12113	0.000	9	0.42	21.23			21.23T			
12113	1.000	9	0.30	15.08			15.08T			
12114	0.000	9	0.30	15.08			15.08T			
12114	1.000	9	0.30	15.08			15.08T			
12115	0.000	9	0.30	15.08			15.08T			
12115	1.000	9	0.30	15.08			15.08T			
12116	0.000	9	0.30	15.08			15.08T			
12116	1.000	9	0.30	15.08			15.08T			
12117	0.000	9	0.30	15.08			15.08T			
12117	1.000	9	0.30	15.08			15.08T			
12118	0.000	9	0.30	15.08			15.08T			
12118	1.000	9	0.30	15.08			15.08T			
12119	0.000	9	0.30	15.08			15.08T			
12119	1.000	9	0.30	15.08			15.08T			
12120	0.000	9	0.30	15.08			15.08T			
12120	1.000	9	0.30	15.08			15.08T			
12121	0.000	9	0.30	15.08			15.08T			
12121	1.000	9	0.30	15.08			15.08T			
12122	0.000	9	0.30	15.08			15.08T			
12122	1.000	9	0.30	15.08			15.08T			
12123	0.000	9	0.30	15.08			15.08T			
12123	1.000	9	0.30	15.08			15.08T			
12124	0.000	9	0.30	15.08			15.08T			
12124	1.000	9	0.30	15.08			15.08T			
12125	0.000	9	0.30	15.08			15.08T			
12125	1.000	9	0.30	15.08			15.08T			
12126	0.000	9	0.30	15.08			15.08T			
12126	1.000	9	0.30	15.08			15.08T			
12127	0.000	9	0.30	15.08			15.08T			
12127	1.000	9	0.30	15.08			15.08T			
12128	0.000	9	0.30	15.08			15.08'			
12128	1.000	9	0.30	15.08			15.08'			
12129	0.000	9	0.98	49.36			49.36T			
12129	1.000	9	1.40	70.29			70.29T			
12130	0.000	9	1.40	70.23			70.23T			
12130	1.000	9	1.47	74.07			74.07T			
12131	0.000	9	1.47	74.03			74.03T			
12131	1.000	9	1.31	65.97			65.97T			
12132	0.000	9	1.31	65.94			65.94T			
12132	1.000	9	1.04	52.06			52.06T			
12133	0.000	9	1.04	52.04			52.04T			
12133	1.000	9	0.72	36.21			36.21T			
12134	0.000	9	0.72	36.20			36.20T			
12134	1.000	9	0.42	21.25			21.25T			
12135	0.000	9	0.42	21.24			21.24T			
12135	1.000	9	0.30	15.08			15.08T			
12136	0.000	9	0.30	15.08			15.08T			
12136	1.000	9	0.30	15.08			15.08T			
12137	0.000	9	0.30	15.08			15.08T			
12137	1.000	9	0.30	15.08			15.08T			
12138	0.000	9	0.30	15.08			15.08T			
12138	1.000	9	0.30	15.08			15.08T			
12139	0.000	9	0.30	15.08			15.08T			
12139	1.000	9	0.30	15.08			15.08T			
12140	0.000	9	0.30	15.08			15.08T			
12140	1.000	9	0.30	15.08			15.08T			
12141	0.000	9	0.30	15.08			15.08T			
12141	1.000	9	0.30	15.08			15.08T			
12142	0.000	9	0.30	15.08			15.08T			
12142	1.000	9	0.30	15.08			15.08T			
12143	0.000	9	0.30	15.08			15.08T			
12143	1.000	9	0.30	15.08			15.08T			
12144	0.000	9	0.30	15.08			15.08T			
12144	1.000	9	0.30	15.08			15.08T			
12145	0.000	9	0.30	15.08			15.08T			
12145	1.000	9	0.30	15.08			15.08T			
12146	0.000	9	0.30	15.08			15.08T			
12146	1.000	9	0.30	15.08			15.08T			
12147	0.000	9	0.30	15.08			15.08T			
12147	1.000	9	0.30	15.08			15.08T			
12148	0.000	9	0.30	15.08			15.08T			
12148	1.000	9	0.30	15.08			15.08T			
12149	0.000	9	0.30	15.08			15.08T			
12149	1.000	9	0.30	15.08			15.08T			
12150	0.000	9	0.30	15.08			15.08'			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Longitudinal Reinforcements Accumulated minimum
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
12150	1.000	9	0.30	15.08			15.08'			

Shear Reinforcements per Cutted Part of Section Accumulated minimum

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
10001	0.000	8	0.93	1.76				
10001	0.200	8	0.80	1.89				
10005	0.000	8	3.35	5.65				
10005	0.200	8	3.65	5.34				
10006	0.000	8	0.09	0.13				
10006	0.400	8	0.30	0.35				
10009	0.000	8	0.39	0.38				
10009	0.400	8	0.39	0.54				
10010	0.000	8	3.58	5.53				
10010	0.200	8	3.71	5.87				
10014	0.000	8	0.80	1.45				
10014	0.200	8	0.80	1.33				
10016	0.000	8	0.92	1.76				
10016	0.200	8	0.86	1.89				
10020	0.000	8	3.34	5.50				
10020	0.200	8	3.63	5.34				
10021	0.000	8	0.09	0.14				
10021	0.400	8	0.30	0.35				
10024	0.000	8	0.36	0.38				
10024	0.400	8	0.36	0.50				
10025	0.000	8	3.51	5.35				
10025	0.200	8	4.02	5.96				
10029	0.000	8	0.83	1.46				
10029	0.200	8	0.79	1.31				
12001	0.000	9	0.00	5.03				
12001	1.000	9	0.00	5.12				
12002	0.000	9	0.00	5.12				
12002	1.000	9	0.00	6.60				
12003	0.000	9	0.00	6.60				
12003	1.000	9	0.00	9.13				
12004	0.000	9	0.00	5.06				
12004	1.000	9	0.00	5.12				
12005	0.000	9	0.00	5.12				
12005	1.000	9	0.00	6.57				
12006	0.000	9	0.00	6.57				
12006	1.000	9	0.00	10.21				
12007	0.000	9	0.00	5.13				
12007	1.000	9	0.00	5.12				
12008	0.000	9	0.00	5.12				
12008	1.000	9	0.00	6.58				
12009	0.000	9	0.00	6.58				
12009	1.000	9	0.00	9.68				
12010	0.000	9	0.00	5.03				
12010	1.000	9	0.00	5.12				
12011	0.000	9	0.00	5.12				
12011	1.000	9	0.00	6.60				
12012	0.000	9	0.00	6.60				
12012	1.000	9	0.00	9.14				
12013	0.000	9	0.00	5.11				
12013	1.000	9	0.00	5.13				
12014	0.000	9	0.00	5.13				
12014	1.000	9	0.00	6.59				
12015	0.000	9	0.00	6.59				
12015	1.000	9	0.00	9.13				
12016	0.000	9	0.00	5.13				
12016	1.000	9	0.00	5.12				
12017	0.000	9	0.00	5.12				
12017	1.000	9	0.00	6.58				
12018	0.000	9	0.00	6.58				
12018	1.000	9	0.00	9.13				
12019	0.000	9	0.00	9.13				
12019	1.000	9	0.00	5.04				
12020	0.000	9	0.00	5.04				
12020	1.000	9	0.00	5.05				
12021	0.000	9	0.00	5.05				
12021	1.000	9	0.00	5.04				
12022	0.000	9	0.00	5.04				
12022	1.000	9	0.00	5.04				
12023	0.000	9	0.00	5.04				
12023	1.000	9	0.00	5.06				
12024	0.000	9	0.00	5.06				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Shear Reinforcements per Cutted Part of Section Accumulated minimum

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
12024	1.000	9	0.00	5.10				
12025	0.000	9	0.00	5.10				
12025	1.000	9	0.00	5.05				
12026	0.000	9	0.00	5.05				
12026	1.000	9	0.00	4.84				
12027	0.000	9	0.00	4.84				
12027	1.000	9	0.00	4.64				
12028	0.000	9	0.00	4.64				
12028	1.000	9	0.00	4.58				
12029	0.000	9	0.00	4.58				
12029	1.000	9	0.00	4.58				
12030	0.000	9	0.00	4.58				
12030	1.000	9	0.00	4.58				
12031	0.000	9	0.00	4.58				
12031	1.000	9	0.00	4.58				
12032	0.000	9	0.00	4.58				
12032	1.000	9	0.00	4.58				
12033	0.000	9	0.00	4.58				
12033	1.000	9	0.00	4.58				
12034	0.000	9	0.00	4.58				
12034	1.000	9	0.00	4.58				
12035	0.000	9	0.00	4.58				
12035	1.000	9	0.00	4.58				
12036	0.000	9	0.00	4.58				
12036	1.000	9	0.00	4.58				
12037	0.000	9	0.00	4.58				
12037	1.000	9	0.00	4.58				
12038	0.000	9	0.00	4.58				
12038	1.000	9	0.00	4.58				
12039	0.000	9	0.00	4.58				
12039	1.000	9	0.00	4.58				
12040	0.000	9	0.00	4.58				
12040	1.000	9	0.00	4.58				
12041	0.000	9	0.00	10.21				
12041	1.000	9	0.00	5.04				
12042	0.000	9	0.00	5.04				
12042	1.000	9	0.00	5.03				
12043	0.000	9	0.00	5.03				
12043	1.000	9	0.00	5.04				
12044	0.000	9	0.00	5.04				
12044	1.000	9	0.00	5.07				
12045	0.000	9	0.00	5.07				
12045	1.000	9	0.00	5.04				
12046	0.000	9	0.00	5.04				
12046	1.000	9	0.00	5.07				
12047	0.000	9	0.00	5.07				
12047	1.000	9	0.00	5.05				
12048	0.000	9	0.00	5.05				
12048	1.000	9	0.00	4.84				
12049	0.000	9	0.00	4.84				
12049	1.000	9	0.00	4.64				
12050	0.000	9	0.00	4.64				
12050	1.000	9	0.00	4.58				
12051	0.000	9	0.00	4.58				
12051	1.000	9	0.00	4.58				
12052	0.000	9	0.00	4.58				
12052	1.000	9	0.00	4.58				
12053	0.000	9	0.00	4.58				
12053	1.000	9	0.00	4.58				
12054	0.000	9	0.00	4.58				
12054	1.000	9	0.00	4.58				
12055	0.000	9	0.00	4.58				
12055	1.000	9	0.00	4.58				
12056	0.000	9	0.00	4.58				
12056	1.000	9	0.00	4.58				
12057	0.000	9	0.00	4.58				
12057	1.000	9	0.00	4.58				
12058	0.000	9	0.00	4.58				
12058	1.000	9	0.00	4.58				
12059	0.000	9	0.00	4.58				
12059	1.000	9	0.00	4.58				
12060	0.000	9	0.00	4.58				
12060	1.000	9	0.00	4.58				
12061	0.000	9	0.00	4.58				
12061	1.000	9	0.00	4.58				
12062	0.000	9	0.00	4.58				
12062	1.000	9	0.00	4.58				
12063	0.000	9	0.00	9.68				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Shear Reinforcements per Cutted Part of Section Accumulated minimum

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
12063	1.000	9	0.00	5.05				
12064	0.000	9	0.00	5.05				
12064	1.000	9	0.00	5.03				
12065	0.000	9	0.00	5.03				
12065	1.000	9	0.00	5.05				
12066	0.000	9	0.00	5.05				
12066	1.000	9	0.00	5.02				
12067	0.000	9	0.00	5.02				
12067	1.000	9	0.00	5.03				
12068	0.000	9	0.00	5.03				
12068	1.000	9	0.00	5.07				
12069	0.000	9	0.00	5.07				
12069	1.000	9	0.00	5.05				
12070	0.000	9	0.00	5.05				
12070	1.000	9	0.00	4.83				
12071	0.000	9	0.00	4.83				
12071	1.000	9	0.00	4.64				
12072	0.000	9	0.00	4.64				
12072	1.000	9	0.00	4.58				
12073	0.000	9	0.00	4.58				
12073	1.000	9	0.00	4.58				
12074	0.000	9	0.00	4.58				
12074	1.000	9	0.00	4.58				
12075	0.000	9	0.00	4.58				
12075	1.000	9	0.00	4.58				
12076	0.000	9	0.00	4.58				
12076	1.000	9	0.00	4.58				
12077	0.000	9	0.00	4.58				
12077	1.000	9	0.00	4.58				
12078	0.000	9	0.00	4.58				
12078	1.000	9	0.00	4.58				
12079	0.000	9	0.00	4.58				
12079	1.000	9	0.00	4.58				
12080	0.000	9	0.00	4.58				
12080	1.000	9	0.00	4.58				
12081	0.000	9	0.00	4.58				
12081	1.000	9	0.00	4.58				
12082	0.000	9	0.00	4.58				
12082	1.000	9	0.00	4.58				
12083	0.000	9	0.00	4.58				
12083	1.000	9	0.00	4.58				
12084	0.000	9	0.00	4.58				
12084	1.000	9	0.00	4.58				
12085	0.000	9	0.00	9.14				
12085	1.000	9	0.00	5.03				
12086	0.000	9	0.00	5.03				
12086	1.000	9	0.00	5.03				
12087	0.000	9	0.00	5.03				
12087	1.000	9	0.00	5.03				
12088	0.000	9	0.00	5.03				
12088	1.000	9	0.00	5.04				
12089	0.000	9	0.00	5.04				
12089	1.000	9	0.00	5.06				
12090	0.000	9	0.00	5.06				
12090	1.000	9	0.00	5.10				
12091	0.000	9	0.00	5.10				
12091	1.000	9	0.00	5.05				
12092	0.000	9	0.00	5.05				
12092	1.000	9	0.00	4.84				
12093	0.000	9	0.00	4.84				
12093	1.000	9	0.00	4.64				
12094	0.000	9	0.00	4.64				
12094	1.000	9	0.00	4.58				
12095	0.000	9	0.00	4.58				
12095	1.000	9	0.00	4.58				
12096	0.000	9	0.00	4.58				
12096	1.000	9	0.00	4.58				
12097	0.000	9	0.00	4.58				
12097	1.000	9	0.00	4.58				
12098	0.000	9	0.00	4.58				
12098	1.000	9	0.00	4.58				
12099	0.000	9	0.00	4.58				
12099	1.000	9	0.00	4.58				
12100	0.000	9	0.00	4.58				
12100	1.000	9	0.00	4.58				
12101	0.000	9	0.00	4.58				
12101	1.000	9	0.00	4.58				
12102	0.000	9	0.00	4.58				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Shear Reinforcements per Cutted Part of Section Accumulated minimum

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
12102	1.000	9	0.00	4.58				
12103	0.000	9	0.00	4.58				
12103	1.000	9	0.00	4.58				
12104	0.000	9	0.00	4.58				
12104	1.000	9	0.00	4.58				
12105	0.000	9	0.00	4.58				
12105	1.000	9	0.00	4.58				
12106	0.000	9	0.00	4.58				
12106	1.000	9	0.00	4.58				
12107	0.000	9	0.00	9.13				
12107	1.000	9	0.00	5.04				
12108	0.000	9	0.00	5.04				
12108	1.000	9	0.00	5.04				
12109	0.000	9	0.00	5.04				
12109	1.000	9	0.00	5.02				
12110	0.000	9	0.00	5.02				
12110	1.000	9	0.00	5.02				
12111	0.000	9	0.00	5.02				
12111	1.000	9	0.00	5.03				
12112	0.000	9	0.00	5.03				
12112	1.000	9	0.00	5.07				
12113	0.000	9	0.00	5.07				
12113	1.000	9	0.00	5.05				
12114	0.000	9	0.00	5.05				
12114	1.000	9	0.00	4.83				
12115	0.000	9	0.00	4.83				
12115	1.000	9	0.00	4.64				
12116	0.000	9	0.00	4.64				
12116	1.000	9	0.00	4.58				
12117	0.000	9	0.00	4.58				
12117	1.000	9	0.00	4.58				
12118	0.000	9	0.00	4.58				
12118	1.000	9	0.00	4.58				
12119	0.000	9	0.00	4.58				
12119	1.000	9	0.00	4.58				
12120	0.000	9	0.00	4.58				
12120	1.000	9	0.00	4.58				
12121	0.000	9	0.00	4.58				
12121	1.000	9	0.00	4.58				
12122	0.000	9	0.00	4.58				
12122	1.000	9	0.00	4.58				
12123	0.000	9	0.00	4.58				
12123	1.000	9	0.00	4.58				
12124	0.000	9	0.00	4.58				
12124	1.000	9	0.00	4.58				
12125	0.000	9	0.00	4.58				
12125	1.000	9	0.00	4.58				
12126	0.000	9	0.00	4.58				
12126	1.000	9	0.00	4.58				
12127	0.000	9	0.00	4.58				
12127	1.000	9	0.00	4.58				
12128	0.000	9	0.00	4.58				
12128	1.000	9	0.00	4.58				
12129	0.000	9	0.00	9.13				
12129	1.000	9	0.00	5.03				
12130	0.000	9	0.00	5.03				
12130	1.000	9	0.00	5.03				
12131	0.000	9	0.00	5.03				
12131	1.000	9	0.00	5.04				
12132	0.000	9	0.00	5.04				
12132	1.000	9	0.00	5.04				
12133	0.000	9	0.00	5.04				
12133	1.000	9	0.00	5.03				
12134	0.000	9	0.00	5.03				
12134	1.000	9	0.00	5.07				
12135	0.000	9	0.00	5.07				
12135	1.000	9	0.00	5.05				
12136	0.000	9	0.00	5.05				
12136	1.000	9	0.00	4.83				
12137	0.000	9	0.00	4.83				
12137	1.000	9	0.00	4.64				
12138	0.000	9	0.00	4.64				
12138	1.000	9	0.00	4.58				
12139	0.000	9	0.00	4.58				
12139	1.000	9	0.00	4.58				
12140	0.000	9	0.00	4.58				
12140	1.000	9	0.00	4.58				
12141	0.000	9	0.00	4.58				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_ULS_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Shear Reinforcements per Cutted Part of Section Accumulated minimum

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
12141	1.000	9	0.00	4.58				
12142	0.000	9	0.00	4.58				
12142	1.000	9	0.00	4.58				
12143	0.000	9	0.00	4.58				
12143	1.000	9	0.00	4.58				
12144	0.000	9	0.00	4.58				
12144	1.000	9	0.00	4.58				
12145	0.000	9	0.00	4.58				
12145	1.000	9	0.00	4.58				
12146	0.000	9	0.00	4.58				
12146	1.000	9	0.00	4.58				
12147	0.000	9	0.00	4.58				
12147	1.000	9	0.00	4.58				
12148	0.000	9	0.00	4.58				
12148	1.000	9	0.00	4.58				
12149	0.000	9	0.00	4.58				
12149	1.000	9	0.00	4.58				
12150	0.000	9	0.00	4.58				
12150	1.000	9	0.00	4.58				

Maximum Degree of Utilization

		N sig-c	Vy sig-t	Vz tau	Mt sig-*	My tend.	Mz As-l	Mb As-v	Mt2 crack	Total sigdyn	lamda tau-*
Cross sect.	8	0.000	0.000	0.048	0.170	0.000	0.000	0.000	0.000	1.001	0.000
DOKOS-4		0.000	0.000	0.000	0.000	0.000	1.001	2.368	0.000	0.000	0.000
Cross sect.	9	0.000	0.000	0.273	0.000	0.000	0.000	0.000	0.000	1.000	0.000
section pile		0.000	0.000	0.000	0.000	0.000	1.000	1.119	0.000	0.000	0.000
Total System		0.000	0.000	0.273	0.170	0.000	0.000	0.000	0.000	1.001	0.000
		0.000	0.000	0.000	0.000	0.000	1.001	2.368	0.000	0.000	0.000

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_ULS_DOKOI

Selected Beam Elements

FROM	TO	INC	X-VALUE	NC	MEMBER	CS0	CS1	CS2	CS3	CS4	CS5
1000	1060	1		1	bending	10	40				
2000	2020	1									

Default design code is DIN Fachbericht 102 Massivbrȯcken (2003) (Germany)
Klasse(Tab.4.118): D
Wind zone : Binnenland

Materials

No. 1 C 25/30 (DIN 1045-1)
No. 3 C 25/30 (DIN 1045-1)
No. 4 C 25/30 (DIN 1045-1)
No. 5 C 25/30 (DIN 1045-1)
No. 6 C 25/30 (DIN 1045-1)
No. 7 C 25/30 (DIN 1045-1)
No. 8 C 25/30 (DIN 1045-1)
No. 9 C 25/30 (DIN 1045-1)
No. 10 C 25/30 (DIN 1045-1)
No. 12 BSt 500 SA (DIN 1045-1)

Reinforcement will be accounted for sectional values as defined in AQUA
Reinforcements saved as design case LCR 507

Considered Load Cases

No.	refer	act on	Title/type of load case	gam-u	gam-f	psi-0	psi-1	psi-2	psi-1'	
1	part.	CS 0	I.B. ΚΑΤΑΚ.ΣΤΟΙΧΕΙΩΝ G (total dead load)	1.35	1.00	1.00	1.00	1.00	1.00	G perm
2	part.	CS 0	I.B. ΔΟΚΩΝ G (total dead load)	1.35	1.00	1.00	1.00	1.00	1.00	G perm
3	part.	CS 0	I.B. ΧΥΤΗΣ ΠΛΑΚΑΣ G (total dead load)	1.35	1.00	1.00	1.00	1.00	1.00	G perm
11	part.	CS 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+0.5 L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
12	part.	CS 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+0.5 L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
13	part.	CS 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+1.0 L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
14	part.	CS 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+1.0 L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
1201	part.	CS 1	MAX-MY BEAM L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
1202	part.	CS 1	MIN-MY BEAM L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
1203	part.	CS 1	MAX-VZ BEAM L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
1204	part.	CS 1	MIN-VZ BEAM L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
1205	part.	CS 1	MAX-MZ BEAM L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
1206	part.	CS 1	MIN-MZ BEAM L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
1207	part.	CS 1	MAX-VY BEAM L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
1208	part.	CS 1	MIN-VY BEAM L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
1209	part.	CS 1	MAX-N BEAM L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
1210	part.	CS 1	MIN-N BEAM L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
1211	part.	CS 1	MAX-MT BEAM L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
1212	part.	CS 1	MIN-MT BEAM L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
5015	part.	CS 0	K creep step C (creep + shrinkage)	1.00	1.00	1.00	1.00	1.00	1.00	P perm
5025	part.	CS 0	K creep step C (creep + shrinkage)	1.00	1.00	1.00	1.00	1.00	1.00	P perm
5055	part.	CS 1	K creep step C (creep + shrinkage)	1.00	1.00	1.00	1.00	1.00	1.00	P perm
5060	part.	CS 1	K creep step C (creep + shrinkage)	1.00	1.00	1.00	1.00	1.00	1.00	P perm
5061	part.	CS 1	K creep step C (creep + shrinkage)	1.00	1.00	1.00	1.00	1.00	1.00	P perm
5062	part.	CS 1	K creep step C (creep + shrinkage)	1.00	1.00	1.00	1.00	1.00	1.00	P perm
5063	part.	CS 1	K creep step C (creep + shrinkage)	1.00	1.00	1.00	1.00	1.00	1.00	P perm
5064	part.	CS 1	K creep step C (creep + shrinkage)	1.00	1.00	1.00	1.00	1.00	1.00	P perm
6015	part.	CS 0	15 K creep step C (creep + shrinkage)							P perm

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_ULS_DOKOI

Considered Load Cases

```
 No. refer  act on  Title/type of load case  gam-u gam-f psi-0 psi-1 psi-2 psi-1'
6025 part.  CS 0    25 K  creep step                                          P perm
                    C    (creep + shrinkage )
6055 part.  CS 1    55 K  creep step                                          P perm
                    C    (creep + shrinkage )
6060 part.  CS 1    60 K  creep step                                          P perm
                    C    (creep + shrinkage )
6061 part.  CS 1    61 K  creep step                                          P perm
                    C    (creep + shrinkage )
6062 part.  CS 1    62 K  creep step                                          P perm
                    C    (creep + shrinkage )
6063 part.  CS 1    63 K  creep step                                          P perm
                    C    (creep + shrinkage )
6064 part.  CS 1    64 K  creep step                                          P perm
                    C    (creep + shrinkage )
```

Ultimate Load Design

```
==========================================================================
Design for ultimate loads DIN Fachbericht 102 Massivbrȯcken (2003)
Uniaxial bending due to symmetry
Safety factors SC-1  SC-2  SC-S  SS-1  SS-2  PIIa
               1.50  1.50  1.50  1.15  1.15     7
Strain limits    C1    C2    S1    S2    Z1    Z2
          max -3.50 -2.00  3.00 25.00 -3.50 25.00

parameters for reinforcements
Minimum reinforcements    compression  min. reinforcem.  maximum-
   Bending.  Compress.    e/d   N/Npl    requ. section   reinforc.
 0.00 [cm2] 0.30 [o/o]   3.50  0.0010     0.00     0.15       9.00
Tensile forces in the longitudinal reinforcements due to shear are NOT accounted for.
Material of sections uses Ultimate Limit strain-stress law with global safety factors
Material of reinforcements uses Ultimate Limit strain-stress law with global safety factors
```

MNo.	temp lev.	Material-safety [-]	max.compr stress [MPa]	at strain [o/oo]	max.tens stress [MPa]	at strain [o/oo]	tension-stiffening [MPa]
1	0	1.500	-14.17	-2.00	0.00	0.00	
3	0	1.500	-14.17	-2.00	0.00	0.00	
4	0	1.500	-14.17	-2.00	0.00	0.00	
5	0	1.500	-14.17	-2.00	0.00	0.00	
6	0	1.500	-14.17	-2.00	0.00	0.00	
7	0	1.500	-14.17	-2.00	0.00	0.00	
8	0	1.500	-14.17	-2.00	0.00	0.00	
9	0	1.500	-14.17	-2.00	0.00	0.00	
10	0	1.500	-14.17	-2.00	0.00	0.00	
12	0	1.150	-456.52	-25.00	456.52	25.00	

Combinations For Ultimate Design

```
 1215 (CS-1: 1) max_my-1215
       MAX  +  MY   :
               1.35 * G + 1.50 * L_A + 1.00 * L_B + 1.00 * C
 1216 (CS-1: 1) min_my-1216
       MIN  +  MY   :
               1.35 * G + 1.50 * L_A + 1.00 * L_B + 1.00 * C
 1221 (CS-1: 1) max_vz-1221
       MAX  +  VZ   :
               1.35 * G + 1.50 * L_A + 1.00 * L_B + 1.00 * C
 1222 (CS-1: 1) min_vz-1222
       MIN  +  VY   :
               1.35 * G + 1.50 * L_A + 1.00 * L_B + 1.00 * C
```

Shear Design

```
==========================================================================
Design for shear DIN 1045-1 (2003)
Minimum shear factor or tan of inclination of compressive struts  0.57 /   1.72
```

MNo	f-cd [MPa]	tau-rd [MPa]	sigIIQ [MPa]	sigIIT [MPa]	sigIIQ+ [MPa]	fyd [MPa]
1	14.17	0.10	10.62	7.44	10.62	
3	14.17	0.10	10.62	7.44	10.62	
4	14.17	0.10	10.62	7.44	10.62	
5	14.17	0.10	10.62	7.44	10.62	
6	14.17	0.10	10.62	7.44	10.62	
7	14.17	0.10	10.62	7.44	10.62	
8	14.17	0.10	10.62	7.44	10.62	
9	14.17	0.10	10.62	7.44	10.62	
10	14.17	0.10	10.62	7.44	10.62	
12						434.78

```
Tolerance for exceeding maximum shear or principal compression stress  0.0200
```

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_ULS_DOKOI

Longitudinal Reinforcements LCR 507
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
1001	0.000	1	0.26	13.17		3.28T	0.57		9.32	
1001	0.883	1	0.28	13.94		3.38T	5.32		5.23	
1002	0.000	1	0.30	14.75		4.21T	5.31		5.23	
1002	0.883	1	0.32	15.98		3.58T	10.15		2.24	
1003	0.000	1	0.32	15.85		3.46T	10.14		2.25	
1003	0.883	1	0.35	17.18		3.37T	13.80			
1004	0.000	1	0.34	16.83		3.04T	13.79			
1004	0.883	1	0.39	19.52		2.98T	16.54			
1005	0.000	1	0.38	19.04		2.52T	16.53			
1005	0.883	1	0.42	20.77		2.47T	18.30			
1006	0.000	1	0.42	20.67		2.37T	18.29			
1006	0.883	1	0.42	21.11		2.37T	18.74			
1007	0.000	1	0.42	21.12		2.37T	18.74			
1007	0.883	1	0.41	20.64		2.37T	18.27			
1008	0.000	1	0.42	21.06		2.79T	18.28			
1008	0.883	1	0.39	19.27		2.82T	16.44			
1009	0.000	1	0.40	19.83		3.36T	16.46			
1009	0.883	1	0.35	17.38		3.42T	13.96			
1010	0.000	1	0.36	17.84		3.88T	13.97			
1010	0.883	1	0.33	16.45		3.96T	10.24		2.25	
1011	0.000	1	0.34	16.69		4.19T	10.26		2.24	
1011	0.883	1	0.31	15.46		4.90T	5.33		5.23	
1012	0.000	1	0.30	14.70		4.13T	5.34		5.23	
1012	0.883	1	0.28	13.92		4.02T	0.57		9.32	
1013	0.000	1	0.24	12.08		2.37T	0.58		9.12	
1013	0.883	1	0.23	11.45		2.37T	3.81		5.26	
1014	0.000	1	0.24	11.89		2.81T	3.81		5.26	
1014	0.883	1	0.26	13.06		2.78T	8.03		2.25	
1015	0.000	1	0.26	13.07		2.77T	8.05		2.25	
1015	0.883	1	0.29	14.19		2.71T	11.47			
1016	0.000	1	0.28	13.94		2.45T	11.49			
1016	0.883	1	0.33	16.33		2.41T	13.92			
1017	0.000	1	0.33	16.29		2.37T	13.92			
1017	0.883	1	0.36	17.98		2.37T	15.61			
1018	0.000	1	0.36	17.98		2.37T	15.61			
1018	0.883	1	0.37	18.39		2.37T	16.02			
1019	0.000	1	0.37	18.40		2.37T	16.02			
1019	0.883	1	0.36	17.94		2.37T	15.56			
1020	0.000	1	0.36	17.94		2.37T	15.57			
1020	0.883	1	0.33	16.33		2.37T	13.96			
1021	0.000	1	0.33	16.39		2.45T	13.94			
1021	0.883	1	0.28	14.03		2.48T	11.55			
1022	0.000	1	0.29	14.31		2.77T	11.54			
1022	0.883	1	0.26	13.12		2.82T	8.06		2.25	
1023	0.000	1	0.26	13.14		2.85T	8.04		2.25	
1023	0.883	1	0.24	11.99		2.92T	3.81		5.26	
1024	0.000	1	0.23	11.64		2.57T	3.81		5.26	
1024	0.883	1	0.25	12.20		2.50T	0.58		9.12	
1025	0.000	1	0.24	12.14		2.37T	0.54		9.23	
1025	0.883	1	0.23	11.63		2.37T	3.91		5.34	
1026	0.000	1	0.23	11.63		2.37T	3.91		5.34	
1026	0.883	1	0.25	12.61		2.37T	7.99		2.24	
1027	0.000	1	0.25	12.61		2.37T	7.99		2.24	
1027	0.883	1	0.27	13.48		2.37T	11.10			
1028	0.000	1	0.27	13.48		2.37T	11.10			
1028	0.883	1	0.32	15.73		2.37T	13.36			
1029	0.000	1	0.32	15.73		2.37T	13.36			
1029	0.883	1	0.35	17.27		2.37T	14.90			
1030	0.000	1	0.35	17.27		2.37T	14.90			
1030	0.883	1	0.35	17.64		2.37T	15.27			
1031	0.000	1	0.35	17.64		2.37T	15.27			
1031	0.883	1	0.35	17.25		2.37T	14.87			
1032	0.000	1	0.35	17.25		2.37T	14.87			
1032	0.883	1	0.32	15.74		2.37T	13.36			
1033	0.000	1	0.32	15.73		2.37T	13.36			
1033	0.883	1	0.27	13.57		2.37T	11.20			
1034	0.000	1	0.27	13.57		2.37T	11.20			
1034	0.883	1	0.25	12.64		2.37T	8.02		2.24	
1035	0.000	1	0.25	12.63		2.37T	8.02		2.24	
1035	0.883	1	0.23	11.59		2.37T	3.88		5.34	
1036	0.000	1	0.23	11.59		2.37T	3.87		5.34	
1036	0.883	1	0.24	12.14		2.37T	0.53		9.23	
1037	0.000	1	0.24	11.78		2.37T	0.36		9.05	
1037	0.883	1	0.24	11.92		2.37T	4.22		5.32	
1038	0.000	1	0.24	12.13		2.60T	4.20		5.32	
1038	0.883	1	0.26	12.89		2.58T	8.11		2.20	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_ULS_DOKOI

Longitudinal Reinforcements LCR 507
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
1039	0.000	1	0.26	12.95		2.66T	8.10		2.20	
1039	0.883	1	0.27	13.62		2.64T	10.98			
1040	0.000	1	0.27	13.34		2.37T	10.97			
1040	0.883	1	0.31	15.46		2.37T	13.09			
1041	0.000	1	0.31	15.47		2.37T	13.09			
1041	0.883	1	0.34	16.88		2.37T	14.50			
1042	0.000	1	0.34	16.88		2.37T	14.50			
1042	0.883	1	0.35	17.21		2.37T	14.84			
1043	0.000	1	0.35	17.21		2.37T	14.84			
1043	0.883	1	0.34	16.87		2.37T	14.50			
1044	0.000	1	0.34	16.87		2.37T	14.49			
1044	0.883	1	0.31	15.44		2.37T	13.07			
1045	0.000	1	0.31	15.45		2.37T	13.08			
1045	0.883	1	0.27	13.47		2.37T	11.10			
1046	0.000	1	0.27	13.48		2.37T	11.11			
1046	0.883	1	0.26	12.72		2.37T	8.15		2.20	
1047	0.000	1	0.26	12.73		2.37T	8.16		2.20	
1047	0.883	1	0.24	11.85		2.37T	4.16		5.32	
1048	0.000	1	0.24	11.87		2.37T	4.17		5.32	
1048	0.883	1	0.24	11.78		2.37T	0.36		9.05	
1049	0.000	1	0.24	12.11		2.42T	0.32		9.36	
1049	0.883	1	0.23	11.58		2.47T	3.81		5.30	
1050	0.000	1	0.24	11.85		2.74T	3.81		5.30	
1050	0.883	1	0.24	11.84		2.68T	7.06		2.10	
1051	0.000	1	0.24	12.11		2.94T	7.06		2.11	
1051	0.883	1	0.25	12.64		2.90T	9.73			
1052	0.000	1	0.25	12.48		2.74T	9.74			
1052	0.883	1	0.29	14.32		2.72T	11.61			
1053	0.000	1	0.28	13.99		2.37T	11.61			
1053	0.883	1	0.31	15.26		2.37T	12.88			
1054	0.000	1	0.31	15.26		2.37T	12.88			
1054	0.883	1	0.31	15.55		2.37T	13.18			
1055	0.000	1	0.31	15.55		2.37T	13.18			
1055	0.883	1	0.31	15.22		2.37T	12.85			
1056	0.000	1	0.31	15.22		2.37T	12.85			
1056	0.883	1	0.28	14.01		2.37T	11.64			
1057	0.000	1	0.28	14.01		2.37T	11.63			
1057	0.883	1	0.24	12.11		2.37T	9.73			
1058	0.000	1	0.24	12.10		2.37T	9.73			
1058	0.883	1	0.23	11.55		2.37T	7.06		2.11	
1059	0.000	1	0.23	11.54		2.37T	7.06		2.10	
1059	0.883	1	0.23	11.49		2.37T	3.81		5.30	
1060	0.000	1	0.23	11.48		2.37T	3.81		5.29	
1060	0.883	1	0.24	12.06		2.37T	0.33		9.36	
2001	0.000	2	0.17	13.75		0.35T			13.41	
2001	0.300	2	0.14	11.34		0.24T			11.09	
2002	0.000	2	0.15	11.93		0.81T			11.12	
2002	0.300	2	0.14	10.89		0.81T	0.62		9.46	
2003	0.000	2	0.14	11.01		0.93T	0.63		9.45	
2003	0.300	2	0.15	12.04		0.93T			11.12	
2004	0.000	2	0.14	11.50		0.41T			11.09	
2004	0.300	2	0.17	13.81		0.41T			13.40	
2005	0.000	2	0.15	12.39		0.22T			12.17	
2005	0.300	2	0.14	11.42		0.22T	0.36		10.84	
2006	0.000	2	0.14	11.52		0.54T	0.11		10.87	
2006	0.300	2	0.13	10.44		0.54T	0.64		9.26	
2007	0.000	2	0.13	10.46		0.56T	0.64		9.26	
2007	0.300	2	0.14	11.54		0.55T	0.11		10.87	
2008	0.000	2	0.14	11.17		0.23T	0.10		10.84	
2008	0.300	2	0.15	12.40		0.23T			12.17	
2009	0.000	2	0.16	12.58		0.16T			12.42	
2009	0.300	2	0.14	11.22		0.17T	0.06		11.00	
2010	0.000	2	0.14	11.39		0.29T	0.06		11.03	
2010	0.300	2	0.13	10.27		0.30T	0.60		9.37	
2011	0.000	2	0.13	10.27		0.30T	0.60		9.37	
2011	0.300	2	0.14	11.38		0.29T	0.05		11.04	
2012	0.000	2	0.14	11.18		0.12T	0.05		11.00	
2012	0.300	2	0.16	12.54		0.12T			12.42	
2013	0.000	2	0.15	12.12		0.08T			12.04	
2013	0.300	2	0.14	10.81		0.08T			10.74	
2014	0.000	2	0.14	11.11		0.34T			10.77	
2014	0.300	2	0.12	9.95		0.34T	0.42		9.19	
2015	0.000	2	0.12	9.82		0.22T	0.42		9.19	
2015	0.300	2	0.14	10.99		0.22T			10.77	
2016	0.000	2	0.14	10.82		0.09T			10.74	
2016	0.300	2	0.15	12.12		0.08T			12.04	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_ULS_DOKOI

Longitudinal Reinforcements LCR 507
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
2017	0.000	2	0.16	12.55		0.19T			12.36	
2017	0.300	2	0.14	11.16		0.11T			11.05	
2018	0.000	2	0.14	11.55		0.47T			11.08	
2018	0.300	2	0.13	10.41		0.55T	0.37		9.50	
2019	0.000	2	0.13	10.34		0.48T	0.37		9.49	
2019	0.300	2	0.14	11.55		0.47T			11.07	
2020	0.000	2	0.14	11.16		0.11T			11.05	
2020	0.300	2	0.16	12.55		0.19T			12.36	

Longitudinal Reinforcements Accumulated minimum
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
1001	0.000	1	0.26	13.17		3.28T	0.57		9.32	
1001	0.883	1	0.28	13.94		3.38T	5.32		5.23	
1002	0.000	1	0.30	14.75		4.21T	5.31		5.23	
1002	0.883	1	0.32	15.98		3.58T	10.15		2.24	
1003	0.000	1	0.32	15.85		3.46T	10.14		2.25	
1003	0.883	1	0.35	17.18		3.37T	13.80			
1004	0.000	1	0.34	16.83		3.04T	13.79			
1004	0.883	1	0.39	19.52		2.98T	16.54			
1005	0.000	1	0.38	19.04		2.52T	16.53			
1005	0.883	1	0.42	20.77		2.47T	18.30			
1006	0.000	1	0.42	20.67		2.37T	18.29			
1006	0.883	1	0.42	21.11		2.37T	18.74			
1007	0.000	1	0.42	21.12		2.37T	18.74			
1007	0.883	1	0.41	20.64		2.37T	18.27			
1008	0.000	1	0.42	21.06		2.79T	18.28			
1008	0.883	1	0.39	19.27		2.82T	16.44			
1009	0.000	1	0.40	19.83		3.36T	16.46			
1009	0.883	1	0.35	17.38		3.42T	13.96			
1010	0.000	1	0.36	17.84		3.88T	13.97			
1010	0.883	1	0.33	16.45		3.96T	10.24		2.25	
1011	0.000	1	0.34	16.69		4.19T	10.26		2.24	
1011	0.883	1	0.31	15.46		4.90T	5.33		5.23	
1012	0.000	1	0.30	14.70		4.13T	5.34		5.23	
1012	0.883	1	0.28	13.92		4.02T	0.57		9.32	
1013	0.000	1	0.24	12.08		2.37T	0.58		9.12	
1013	0.883	1	0.23	11.45		2.37T	3.81		5.26	
1014	0.000	1	0.24	11.89		2.81T	3.81		5.26	
1014	0.883	1	0.26	13.06		2.78T	8.03		2.25	
1015	0.000	1	0.26	13.07		2.77T	8.05		2.25	
1015	0.883	1	0.29	14.19		2.71T	11.47			
1016	0.000	1	0.28	13.94		2.45T	11.49			
1016	0.883	1	0.33	16.33		2.41T	13.92			
1017	0.000	1	0.33	16.29		2.37T	13.92			
1017	0.883	1	0.36	17.98		2.37T	15.61			
1018	0.000	1	0.36	17.98		2.37T	15.61			
1018	0.883	1	0.37	18.39		2.37T	16.02			
1019	0.000	1	0.37	18.40		2.37T	16.02			
1019	0.883	1	0.36	17.94		2.37T	15.56			
1020	0.000	1	0.36	17.94		2.37T	15.57			
1020	0.883	1	0.33	16.33		2.37T	13.96			
1021	0.000	1	0.33	16.39		2.45T	13.94			
1021	0.883	1	0.28	14.03		2.48T	11.55			
1022	0.000	1	0.29	14.31		2.77T	11.54			
1022	0.883	1	0.26	13.12		2.82T	8.06		2.25	
1023	0.000	1	0.26	13.14		2.85T	8.04		2.25	
1023	0.883	1	0.24	11.99		2.92T	3.81		5.26	
1024	0.000	1	0.23	11.64		2.57T	3.81		5.26	
1024	0.883	1	0.25	12.20		2.50T	0.58		9.12	
1025	0.000	1	0.24	12.14		2.37T	0.54		9.23	
1025	0.883	1	0.23	11.63		2.37T	3.91		5.34	
1026	0.000	1	0.23	11.63		2.37T	3.91		5.34	
1026	0.883	1	0.25	12.61		2.37T	7.99		2.24	
1027	0.000	1	0.25	12.61		2.37T	7.99		2.24	
1027	0.883	1	0.27	13.48		2.37T	11.10			
1028	0.000	1	0.27	13.48		2.37T	11.10			
1028	0.883	1	0.32	15.73		2.37T	13.36			
1029	0.000	1	0.32	15.73		2.37T	13.36			
1029	0.883	1	0.35	17.27		2.37T	14.90			
1030	0.000	1	0.35	17.27		2.37T	14.90			
1030	0.883	1	0.35	17.64		2.37T	15.27			
1031	0.000	1	0.35	17.64		2.37T	15.27			
1031	0.883	1	0.35	17.25		2.37T	14.87			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_ULS_DOKOI

Longitudinal Reinforcements Accumulated minimum
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
1032	0.000	1	0.35	17.25		2.37T	14.87			
1032	0.883	1	0.32	15.74		2.37T	13.36			
1033	0.000	1	0.32	15.73		2.37T	13.36			
1033	0.883	1	0.27	13.57		2.37T	11.20			
1034	0.000	1	0.27	13.57		2.37T	11.20			
1034	0.883	1	0.25	12.64		2.37T	8.02		2.24	
1035	0.000	1	0.25	12.63		2.37T	8.02		2.24	
1035	0.883	1	0.23	11.59		2.37T	3.88		5.34	
1036	0.000	1	0.23	11.59		2.37T	3.87		5.34	
1036	0.883	1	0.24	12.14		2.37T	0.53		9.23	
1037	0.000	1	0.24	11.78		2.37T	0.36		9.05	
1037	0.883	1	0.24	11.92		2.37T	4.22		5.32	
1038	0.000	1	0.24	12.13		2.60T	4.20		5.32	
1038	0.883	1	0.26	12.89		2.58T	8.11		2.20	
1039	0.000	1	0.26	12.95		2.66T	8.10		2.20	
1039	0.883	1	0.27	13.62		2.64T	10.98			
1040	0.000	1	0.27	13.34		2.37T	10.97			
1040	0.883	1	0.31	15.46		2.37T	13.09			
1041	0.000	1	0.31	15.47		2.37T	13.09			
1041	0.883	1	0.34	16.88		2.37T	14.50			
1042	0.000	1	0.34	16.88		2.37T	14.50			
1042	0.883	1	0.35	17.21		2.37T	14.84			
1043	0.000	1	0.35	17.21		2.37T	14.84			
1043	0.883	1	0.34	16.87		2.37T	14.50			
1044	0.000	1	0.34	16.87		2.37T	14.49			
1044	0.883	1	0.31	15.44		2.37T	13.07			
1045	0.000	1	0.31	15.45		2.37T	13.08			
1045	0.883	1	0.27	13.47		2.37T	11.10			
1046	0.000	1	0.27	13.48		2.37T	11.11			
1046	0.883	1	0.26	12.72		2.37T	8.15		2.20	
1047	0.000	1	0.26	12.73		2.37T	8.16		2.20	
1047	0.883	1	0.24	11.85		2.37T	4.16		5.32	
1048	0.000	1	0.24	11.87		2.37T	4.17		5.32	
1048	0.883	1	0.24	11.78		2.37T	0.36		9.05	
1049	0.000	1	0.24	12.11		2.42T	0.32		9.36	
1049	0.883	1	0.23	11.58		2.47T	3.81		5.30	
1050	0.000	1	0.24	11.85		2.74T	3.81		5.30	
1050	0.883	1	0.24	11.84		2.68T	7.06		2.10	
1051	0.000	1	0.24	12.11		2.94T	7.06		2.11	
1051	0.883	1	0.25	12.64		2.90T	9.73			
1052	0.000	1	0.25	12.48		2.74T	9.74			
1052	0.883	1	0.29	14.32		2.72T	11.61			
1053	0.000	1	0.28	13.99		2.37T	11.61			
1053	0.883	1	0.31	15.26		2.37T	12.88			
1054	0.000	1	0.31	15.26		2.37T	12.88			
1054	0.883	1	0.31	15.55		2.37T	13.18			
1055	0.000	1	0.31	15.55		2.37T	13.18			
1055	0.883	1	0.31	15.22		2.37T	12.85			
1056	0.000	1	0.31	15.22		2.37T	12.85			
1056	0.883	1	0.28	14.01		2.37T	11.64			
1057	0.000	1	0.28	14.01		2.37T	11.63			
1057	0.883	1	0.24	12.11		2.37T	9.73			
1058	0.000	1	0.24	12.10		2.37T	9.73			
1058	0.883	1	0.23	11.55		2.37T	7.06		2.11	
1059	0.000	1	0.23	11.54		2.37T	7.06		2.10	
1059	0.883	1	0.23	11.49		2.37T	3.81		5.30	
1060	0.000	1	0.23	11.48		2.37T	3.81		5.29	
1060	0.883	1	0.24	12.06		2.37T	0.33		9.36	
2001	0.000	2	0.17	13.75		0.35T			13.41	
2001	0.300	2	0.14	11.34		0.24T			11.09	
2002	0.000	2	0.15	11.93		0.81T			11.12	
2002	0.300	2	0.14	10.89		0.81T	0.62		9.46	
2003	0.000	2	0.14	11.01		0.93T	0.63		9.45	
2003	0.300	2	0.15	12.04		0.93T			11.12	
2004	0.000	2	0.14	11.50		0.41T			11.09	
2004	0.300	2	0.17	13.81		0.41T			13.40	
2005	0.000	2	0.15	12.39		0.22T			12.17	
2005	0.300	2	0.14	11.42		0.22T	0.36		10.84	
2006	0.000	2	0.14	11.52		0.54T	0.11		10.87	
2006	0.300	2	0.13	10.44		0.54T	0.64		9.26	
2007	0.000	2	0.13	10.46		0.56T	0.64		9.26	
2007	0.300	2	0.14	11.54		0.55T	0.11		10.87	
2008	0.000	2	0.14	11.17		0.23T	0.10		10.84	
2008	0.300	2	0.15	12.40		0.23T			12.17	
2009	0.000	2	0.16	12.58		0.16T			12.42	
2009	0.300	2	0.14	11.22		0.17T	0.06		11.00	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_ULS_DOKOI

Longitudinal Reinforcements Accumulated minimum
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
2010	0.000	2	0.14	11.39		0.29T	0.06		11.03	
2010	0.300	2	0.13	10.27		0.30T	0.60		9.37	
2011	0.000	2	0.13	10.27		0.30T	0.60		9.37	
2011	0.300	2	0.14	11.38		0.29T	0.05		11.04	
2012	0.000	2	0.14	11.18		0.12T	0.05		11.00	
2012	0.300	2	0.16	12.54		0.12T			12.42	
2013	0.000	2	0.15	12.12		0.08T			12.04	
2013	0.300	2	0.14	10.81		0.08T			10.74	
2014	0.000	2	0.14	11.11		0.34T			10.77	
2014	0.300	2	0.12	9.95		0.34T	0.42		9.19	
2015	0.000	2	0.12	9.82		0.22T	0.42		9.19	
2015	0.300	2	0.14	10.99		0.22T			10.77	
2016	0.000	2	0.14	10.82		0.09T			10.74	
2016	0.300	2	0.15	12.12		0.08T			12.04	
2017	0.000	2	0.16	12.55		0.19T			12.36	
2017	0.300	2	0.14	11.16		0.11T			11.05	
2018	0.000	2	0.14	11.55		0.47T			11.08	
2018	0.300	2	0.13	10.41		0.55T	0.37		9.50	
2019	0.000	2	0.13	10.34		0.48T	0.37		9.49	
2019	0.300	2	0.14	11.55		0.47T			11.07	
2020	0.000	2	0.14	11.16		0.11T			11.05	
2020	0.300	2	0.16	12.55		0.19T			12.36	

Shear Reinforcements per Cutted Part of Section Accumulated minimum

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
1001	0.000	1	1.84	7.22				
1001	0.883	1	1.84	6.93				
1002	0.000	1	2.00	6.46				
1002	0.883	1	2.00	6.23				
1003	0.000	1	1.92	5.68				
1003	0.883	1	1.92	5.44				
1004	0.000	1	1.72	4.81				
1004	0.883	1	1.72	4.57				
1005	0.000	1	1.44	3.87				
1005	0.883	1	1.44	3.64				
1006	0.000	1	1.11	2.89				
1006	0.883	1	1.11	2.66				
1007	0.000	1	1.25	2.42				
1007	0.883	1	1.25	2.64				
1008	0.000	1	1.61	3.28				
1008	0.883	1	1.61	3.51				
1009	0.000	1	1.94	4.18				
1009	0.883	1	1.94	4.41				
1010	0.000	1	2.19	4.75				
1010	0.883	1	2.19	4.97				
1011	0.000	1	2.33	5.66				
1011	0.883	1	2.33	5.88				
1012	0.000	1	2.24	6.51				
1012	0.883	1	2.24	6.75				
1013	0.000	1	1.33	5.60				
1013	0.883	1	1.33	5.35				
1014	0.000	1	1.54	5.17				
1014	0.883	1	1.54	4.95				
1015	0.000	1	1.53	4.52				
1015	0.883	1	1.53	4.29				
1016	0.000	1	1.38	3.74				
1016	0.883	1	1.38	3.51				
1017	0.000	1	1.10	2.87				
1017	0.883	1	1.14	2.66				
1018	0.000	1	0.84	2.02				
1018	0.883	1	0.84	1.79				
1019	0.000	1	0.89	1.28				
1019	0.883	1	0.89	1.37				
1020	0.000	1	1.17	2.12				
1020	0.883	1	1.17	2.35				
1021	0.000	1	1.41	2.93				
1021	0.883	1	1.41	3.16				
1022	0.000	1	1.57	3.59				
1022	0.883	1	1.57	3.82				
1023	0.000	1	1.59	4.33				
1023	0.883	1	1.59	4.56				
1024	0.000	1	1.41	5.10				
1024	0.883	1	1.41	5.36				
1025	0.000	1	0.64	4.69				
1025	0.883	1	0.64	4.43				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_ULS_DOKOI

Shear Reinforcements per Cutted Part of Section Accumulated minimum

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
1026	0.000	1	0.48	4.18				
1026	0.883	1	0.48	3.95				
1027	0.000	1	0.46	3.48				
1027	0.883	1	0.46	3.25				
1028	0.000	1	0.54	2.84				
1028	0.883	1	0.54	2.61				
1029	0.000	1	0.58	2.21				
1029	0.883	1	0.58	1.98				
1030	0.000	1	0.56	1.60				
1030	0.883	1	0.56	1.37				
1031	0.000	1	0.54	1.03				
1031	0.883	1	0.54	0.85				
1032	0.000	1	0.59	1.41				
1032	0.883	1	0.59	1.64				
1033	0.000	1	0.70	2.06				
1033	0.883	1	0.70	2.29				
1034	0.000	1	0.78	2.64				
1034	0.883	1	0.78	2.74				
1035	0.000	1	0.81	3.28				
1035	0.883	1	0.81	3.78				
1036	0.000	1	0.72	3.93				
1036	0.883	1	0.72	3.68				
1037	0.000	1	0.97	4.81				
1037	0.883	1	0.97	4.54				
1038	0.000	1	1.43	4.21				
1038	0.883	1	1.43	3.98				
1039	0.000	1	1.48	3.75				
1039	0.883	1	1.48	3.51				
1040	0.000	1	1.30	3.25				
1040	0.883	1	1.30	3.02				
1041	0.000	1	1.06	2.69				
1041	0.883	1	1.06	2.45				
1042	0.000	1	0.83	2.08				
1042	0.883	1	0.83	1.85				
1043	0.000	1	0.80	1.90				
1043	0.883	1	0.80	2.13				
1044	0.000	1	0.97	2.10				
1044	0.883	1	0.97	2.32				
1045	0.000	1	0.98	2.63				
1045	0.883	1	0.98	2.85				
1046	0.000	1	1.22	3.14				
1046	0.883	1	1.22	3.36				
1047	0.000	1	0.96	3.26				
1047	0.883	1	0.96	3.32				
1048	0.000	1	0.84	3.79				
1048	0.883	1	0.78	3.81				
1049	0.000	1	1.36	4.20				
1049	0.883	1	1.36	3.76				
1050	0.000	1	1.48	3.35				
1050	0.883	1	1.48	3.13				
1051	0.000	1	1.62	2.88				
1051	0.883	1	1.62	2.65				
1052	0.000	1	1.52	2.45				
1052	0.883	1	1.52	2.22				
1053	0.000	1	1.30	2.01				
1053	0.883	1	1.30	1.78				
1054	0.000	1	1.03	1.16				
1054	0.883	1	1.03	0.94				
1055	0.000	1	0.84	0.75				
1055	0.883	1	0.84	0.89				
1056	0.000	1	0.72	1.21				
1056	0.883	1	0.72	1.67				
1057	0.000	1	0.84	1.99				
1057	0.883	1	0.84	2.22				
1058	0.000	1	1.08	2.50				
1058	0.883	1	1.08	2.73				
1059	0.000	1	1.23	2.97				
1059	0.883	1	1.23	3.19				
1060	0.000	1	1.16	3.34				
1060	0.883	1	1.34	4.20				
2001	0.000	2	0.15	6.31				
2001	0.300	2	0.11	6.05				
2002	0.000	2	0.36	6.76				
2002	0.300	2	0.36	6.55				
2003	0.000	2	0.40	5.34				
2003	0.300	2	0.40	5.47				
2004	0.000	2	0.18	5.38				
2004	0.300	2	0.18	5.52				

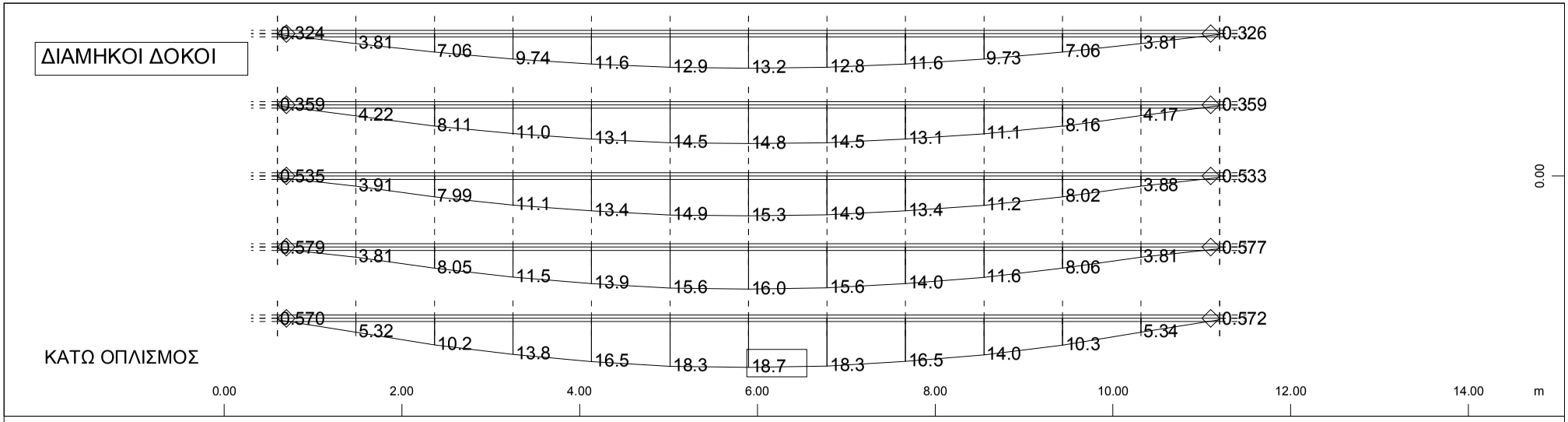
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_ULS_DOKOI

Shear Reinforcements per Cutted Part of Section Accumulated minimum

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
2005	0.000	2	0.10	4.61				
2005	0.300	2	0.10	4.47				
2006	0.000	2	0.24	5.03				
2006	0.300	2	0.24	4.85				
2007	0.000	2	0.24	4.66				
2007	0.300	2	0.24	4.83				
2008	0.000	2	0.10	4.26				
2008	0.300	2	0.10	4.41				
2009	0.000	2	0.07	4.86				
2009	0.300	2	0.07	4.73				
2010	0.000	2	0.13	5.06				
2010	0.300	2	0.13	4.89				
2011	0.000	2	0.13	3.48				
2011	0.300	2	0.13	3.67				
2012	0.000	2	0.05	3.33				
2012	0.300	2	0.05	3.40				
2013	0.000	2	0.03	5.15				
2013	0.300	2	0.03	4.94				
2014	0.000	2	0.15	5.11				
2014	0.300	2	0.15	4.95				
2015	0.000	2	0.10	3.38				
2015	0.300	2	0.10	3.56				
2016	0.000	2	0.04	4.00				
2016	0.300	2	0.04	4.04				
2017	0.000	2	0.08	3.68				
2017	0.300	2	0.06	3.57				
2018	0.000	2	0.23	3.96				
2018	0.300	2	0.23	3.82				
2019	0.000	2	0.21	3.45				
2019	0.300	2	0.21	3.64				
2020	0.000	2	0.05					
2020	0.300	2	0.08	3.26				

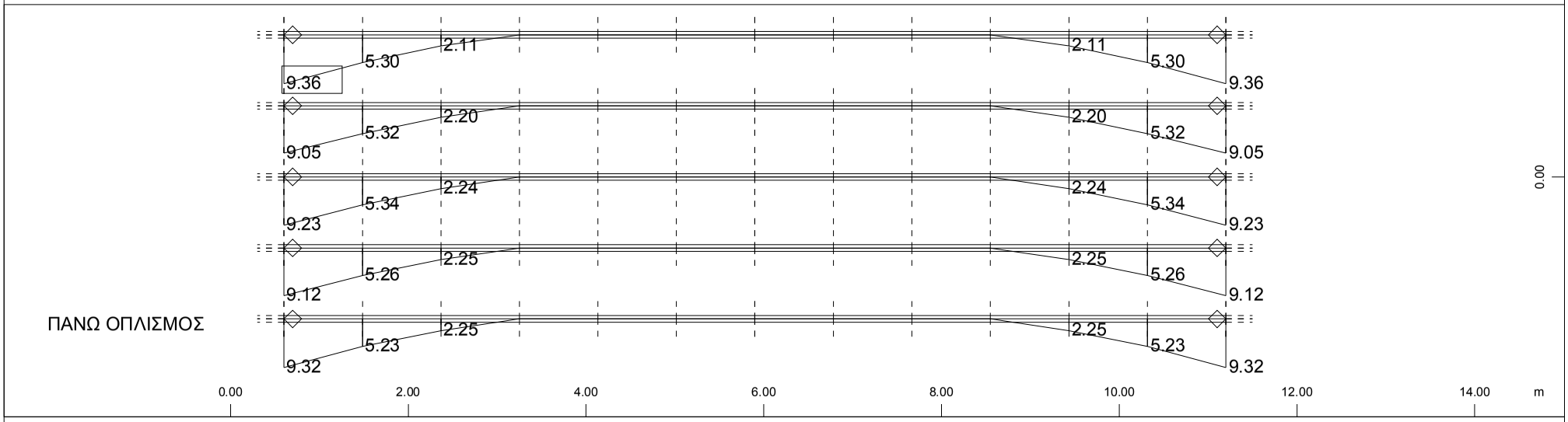
Maximum Degree of Utilization

		N sig-c	Vy sig-t	Vz tau	Mt sig-*	My tend.	Mz As-l	Mb As-v	Mt2 crack	Total sigdyn	lamda tau-*
Cross sect.	1	0.000	0.000	0.398	0.182	0.000	0.000	0.000	0.000	1.001	0.000
		0.000	0.000	0.000	0.000	0.000	1.001	0.000	0.000	0.000	0.000
Cross sect.	2	0.000	0.000	0.136	0.023	0.000	0.000	0.000	0.000	1.001	0.000
DOKOS-2		0.000	0.000	0.000	0.000	0.000	1.001	0.000	0.000	0.000	0.000
Total System		0.000	0.000	0.398	0.182	0.000	0.000	0.000	0.000	1.001	0.000
		0.000	0.000	0.000	0.000	0.000	1.001	0.000	0.000	0.000	0.000



Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case 507 , 1 cm 3D = 22.4 cm2 (Max=18.7)

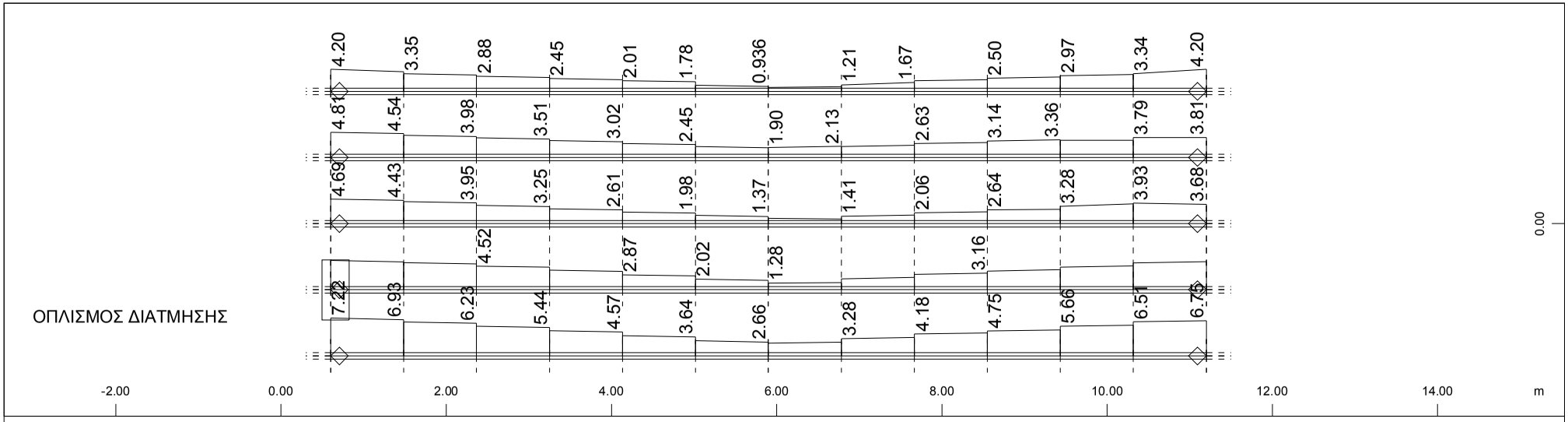
M 1 : 66



Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case 507 , 1 cm 3D = 11.2 cm2 (Max=9.36)

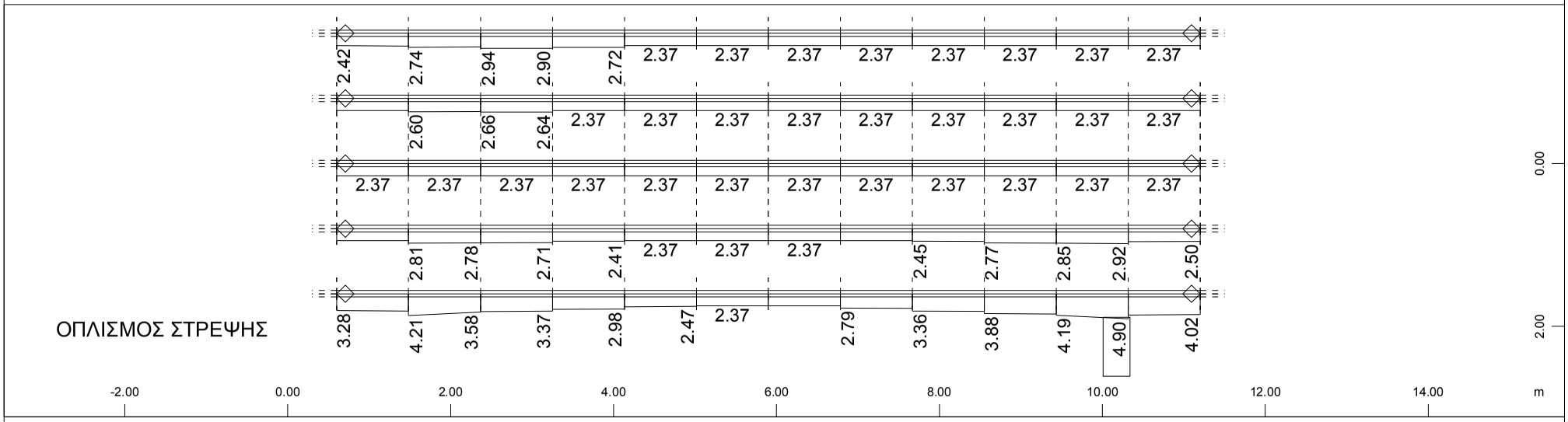
M 1 : 66

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΦΑΣΗ-2_ULS



Sector of system Beam Elements Group 1
Beam Elements , Shear reinforcements (maximum), Design Case 507 , 1 cm 3D = 11.2 cm2/m (Max=7.22)

M 1 : 71

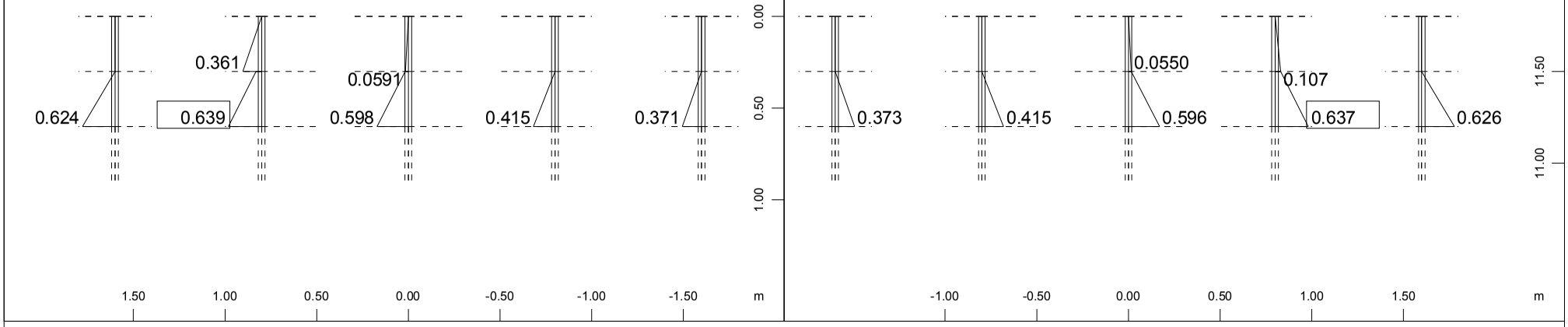


Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case 507 , 1 cm 3D = 11.2 cm2 (Max=4.90)

M 1 : 72

ΔΙΑΜΗΚΗ ΔΟΚΟΙ-ΣΤΗΡΙΞΗΣ

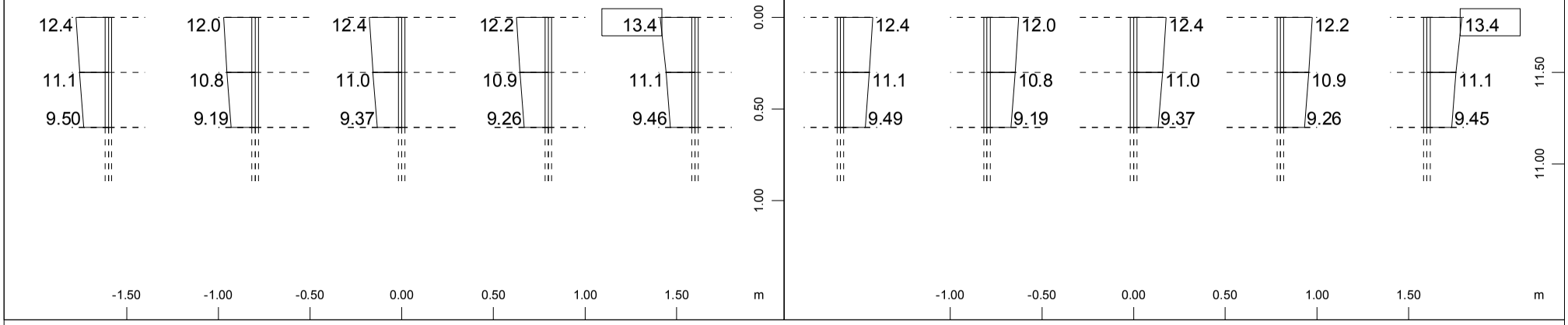
ΚΑΤΩ ΟΠΛΙΣΜΟΣ



Y Z X
Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
507 , 1 cm 3D = 1.12 cm2 (Max=0.639)

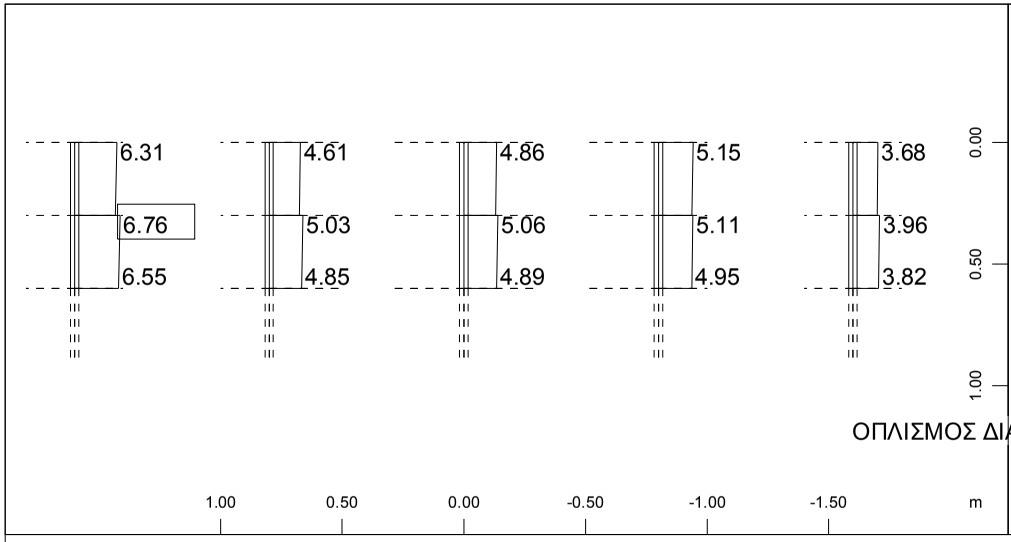
X Z Y
Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
507 , 1 cm 3D = 1.12 cm2 (Max=0.637)

ΠΑΝΩ ΟΠΛΙΣΜΟΣ

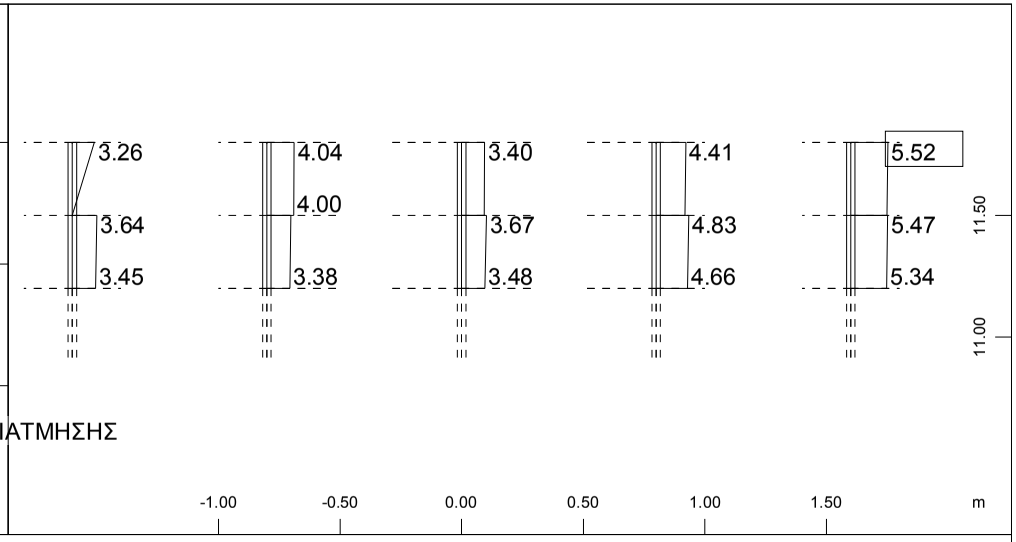


Z Y X
Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
507 , 1 cm 3D = 22.4 cm2 (Max=13.4)

X Z Y
Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
507 , 1 cm 3D = 22.4 cm2 (Max=13.4)

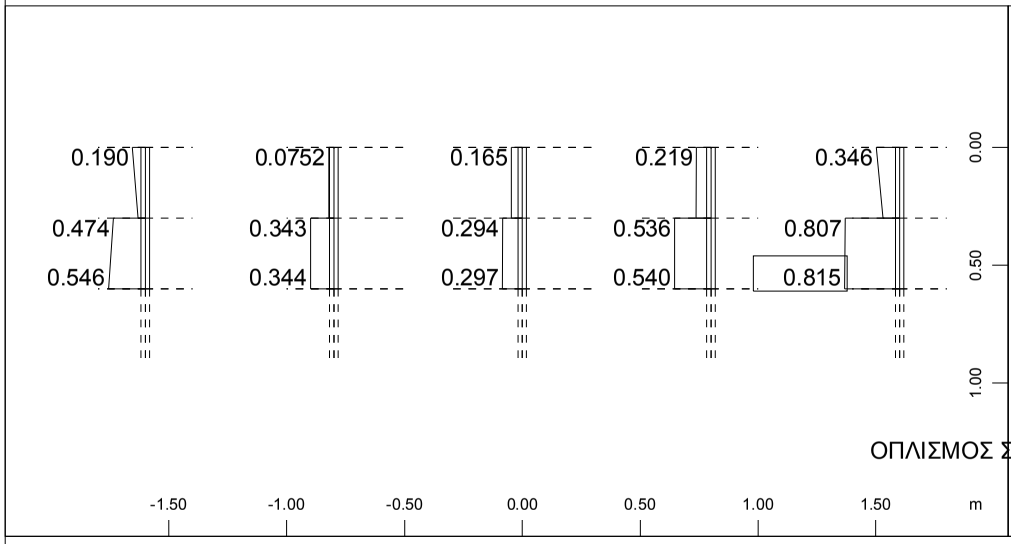


Sector of system Beam Elements Group 2 M 1 : 31
Beam Elements , Shear reinforcements (maximum), Design Case 507
, 1 cm 3D = 11.2 cm2/m (Max=6.76)

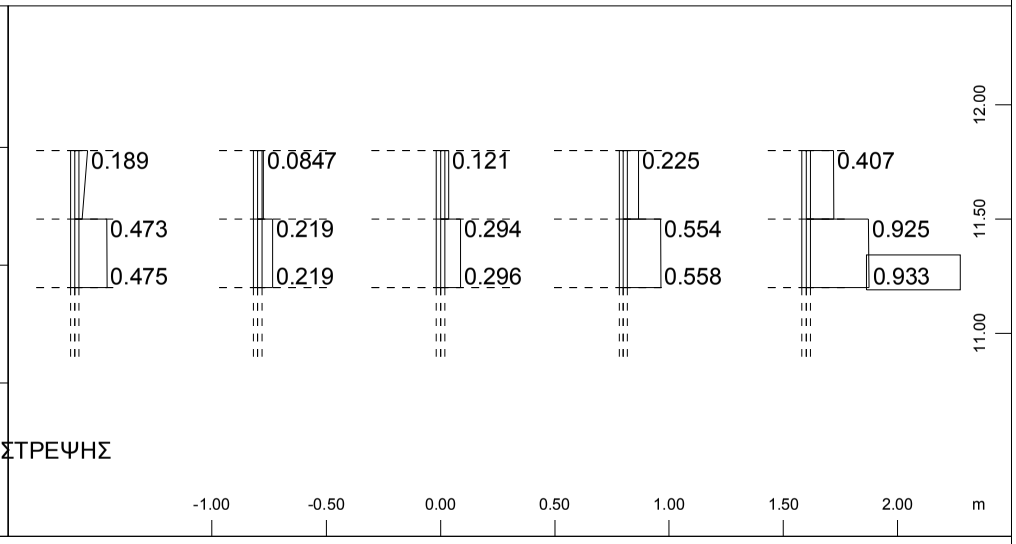


Sector of system Beam Elements Group 2 M 1 : 31
Beam Elements , Shear reinforcements (maximum), Design Case 507
, 1 cm 3D = 11.2 cm2/m (Max=5.52)

ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ



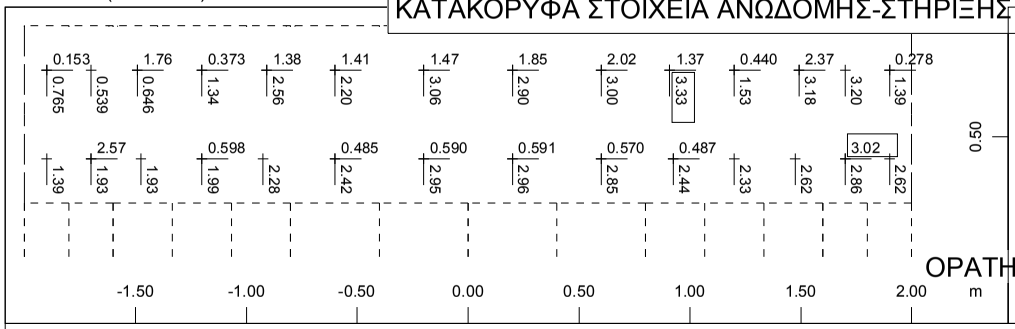
Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
507 , 1 cm 3D = 1.12 cm2 (Max=0.815)



Sector of system Beam Elements Group 2 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
507 , 1 cm 3D = 1.12 cm2 (Max=0.933)

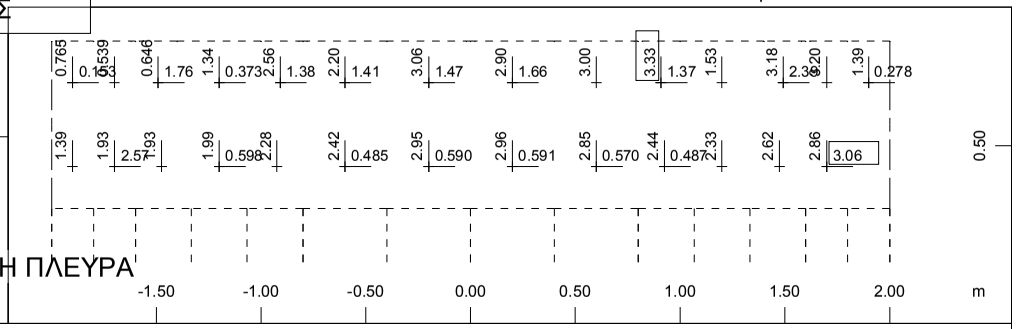
ΟΠΛΙΣΜΟΣ ΣΤΡΕΨΗΣ

ΚΑΤΑΚΟΡΥΦΑ ΣΤΟΙΧΕΙΑ ΑΝΩΔΟΜΗΣ-ΣΤΗΡΙΞΗΣ

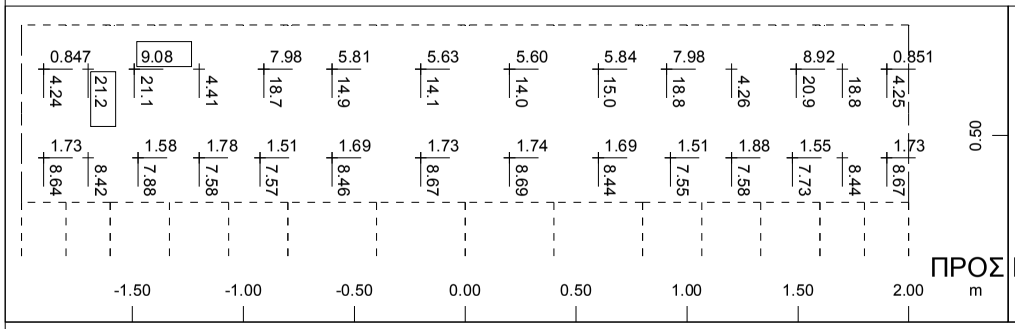


Sector of system Quadrilateral Elements Group 8 M 1 : 34
upper Reinforcements in Elements in cm2/m, Design Case 521 ULS
design (Max=3.33)

ΟΡΑΤΗ ΠΛΕΥΡΑ

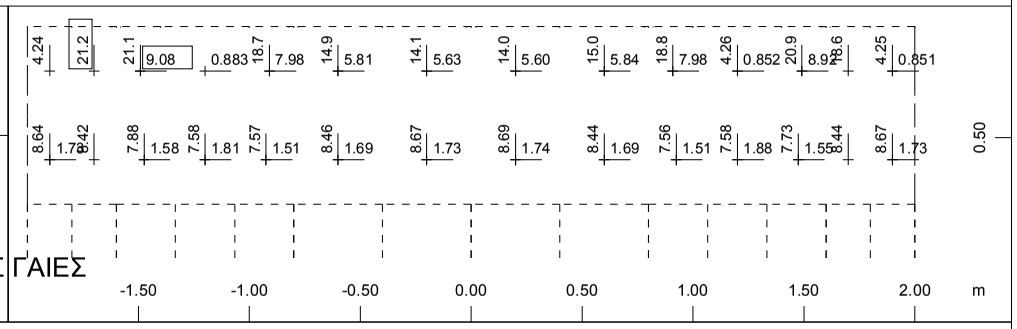


Sector of system Quadrilateral Elements Group 8 M 1 : 36
upper Reinforcements in Elements in cm2/m, Design Case 521 ULS
design (Max=3.33)

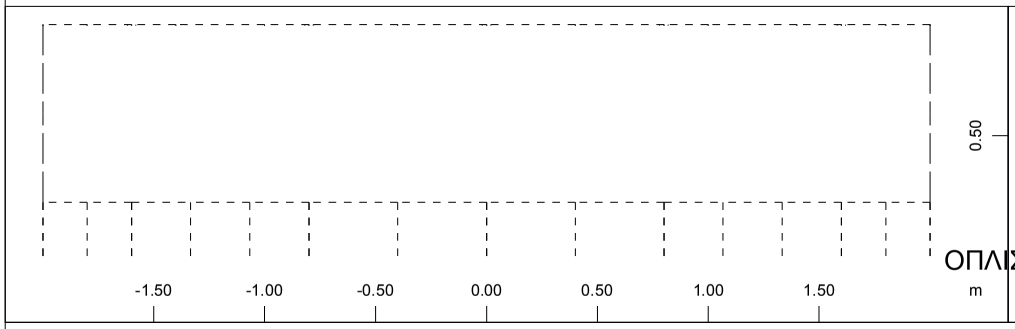


Sector of system Quadrilateral Elements Group 8 M 1 : 34
lower Reinforcements in Elements in cm2/m, Design Case 521 ULS
design (Max=21.2)

ΠΡΟΣ ΓΑΙΕΣ

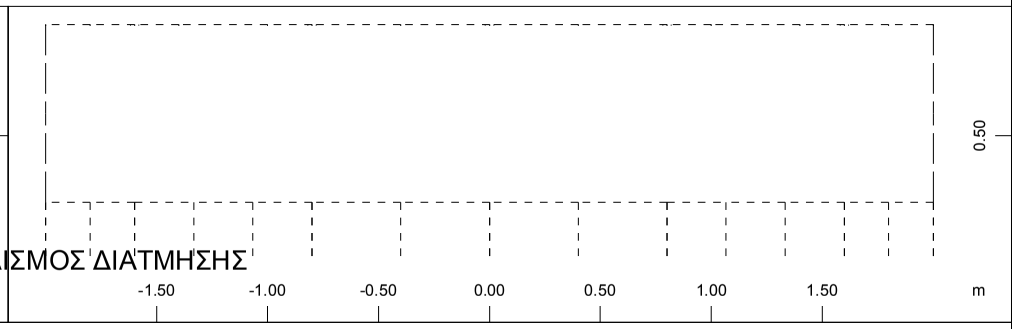


Sector of system Quadrilateral Elements Group 8 M 1 : 34
lower Reinforcements in Elements in cm2/m, Design Case 521 ULS
design (Max=21.2)



Sector of system Quadrilateral Elements Group 8 M 1 : 34
Shear reinforcement from middle of element in cm2/m2, Design
Case 521 ULS design (Max=0)

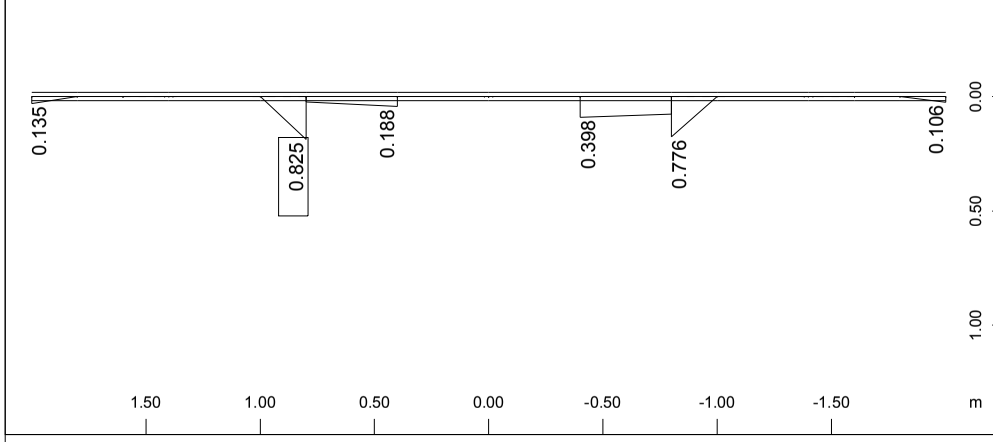
ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ



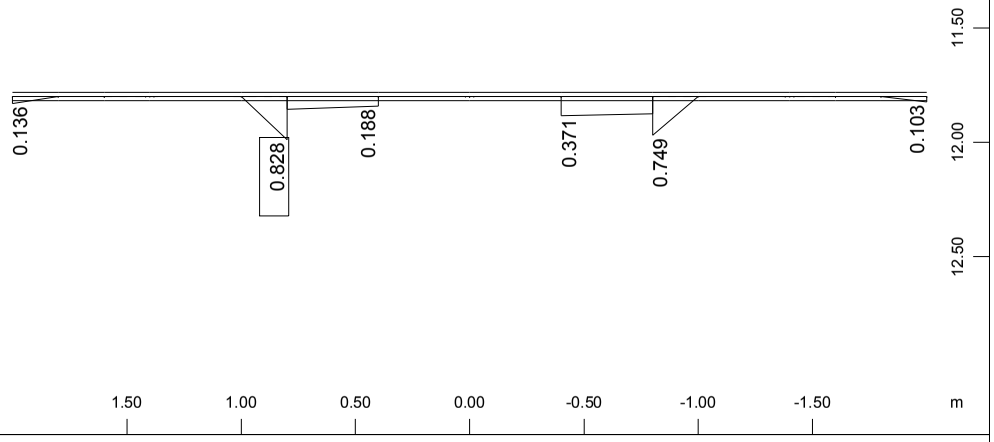
Sector of system Quadrilateral Elements Group 8 M 1 : 34
Shear reinforcement from middle of element in cm2/m2, Design
Case 521 ULS design (Max=0)

ΠΑΣΣΑΛΟΔΕΣΜΟΣ

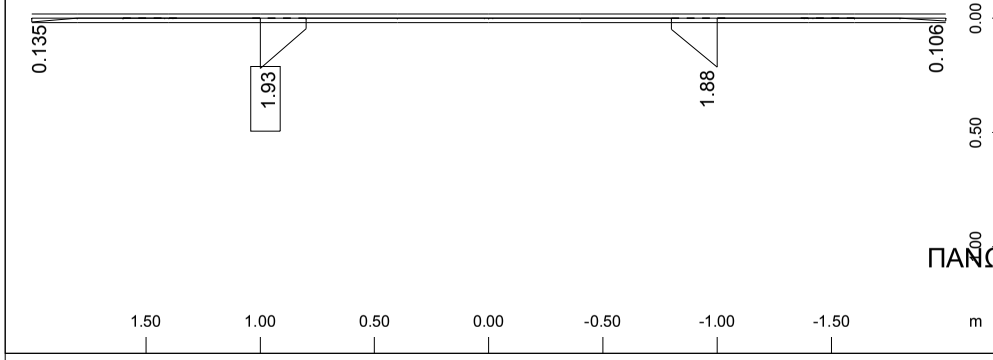
ΚΑΤΩ ΟΠΛΙΣΜΟΣ



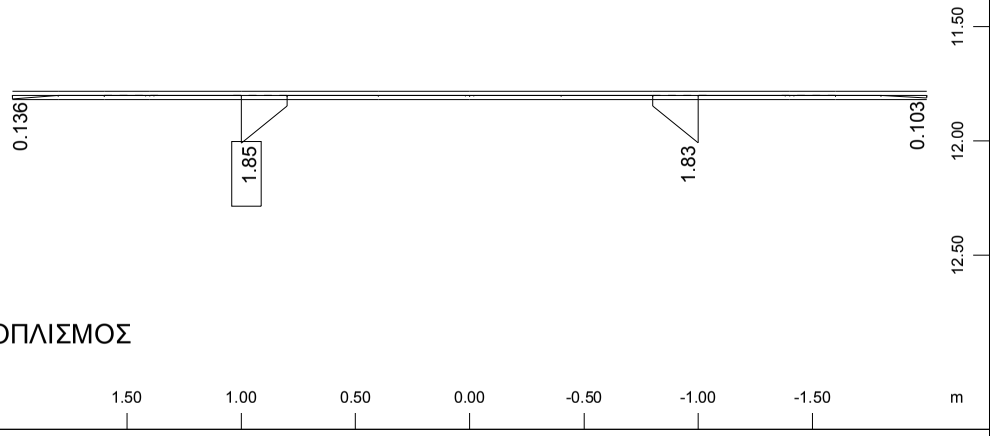
Y Z X
Sector of system Beam Elements Group 10
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
506 , 1 cm 3D = 1.45 cm2 (Max=0.825)
M 1 : 33



Y Z X
Sector of system Beam Elements Group 10
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
506 , 1 cm 3D = 1.45 cm2 (Max=0.828)
M 1 : 33

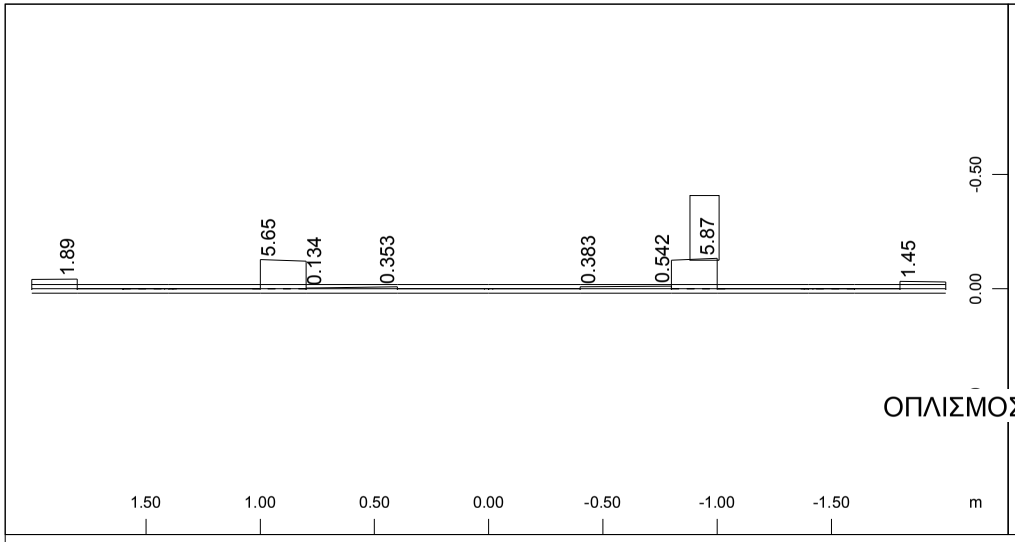


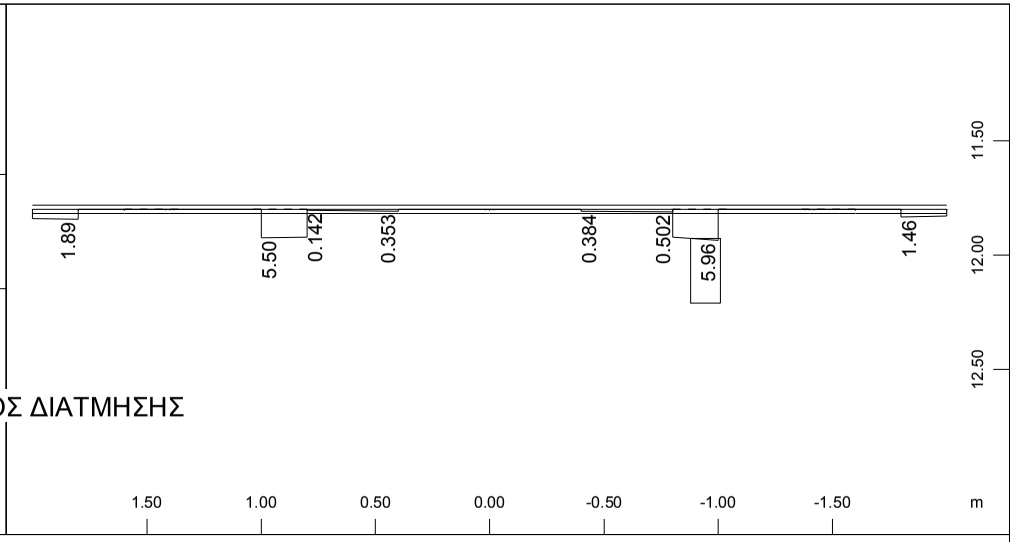
Y Z X
Sector of system Beam Elements Group 10
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
506 , 1 cm 3D = 2.91 cm2 (Max=1.93)
M 1 : 33



Y Z X
Sector of system Beam Elements Group 10
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
506 , 1 cm 3D = 2.91 cm2 (Max=1.85)
M 1 : 33

ΠΑΝΩ ΟΠΛΙΣΜΟΣ

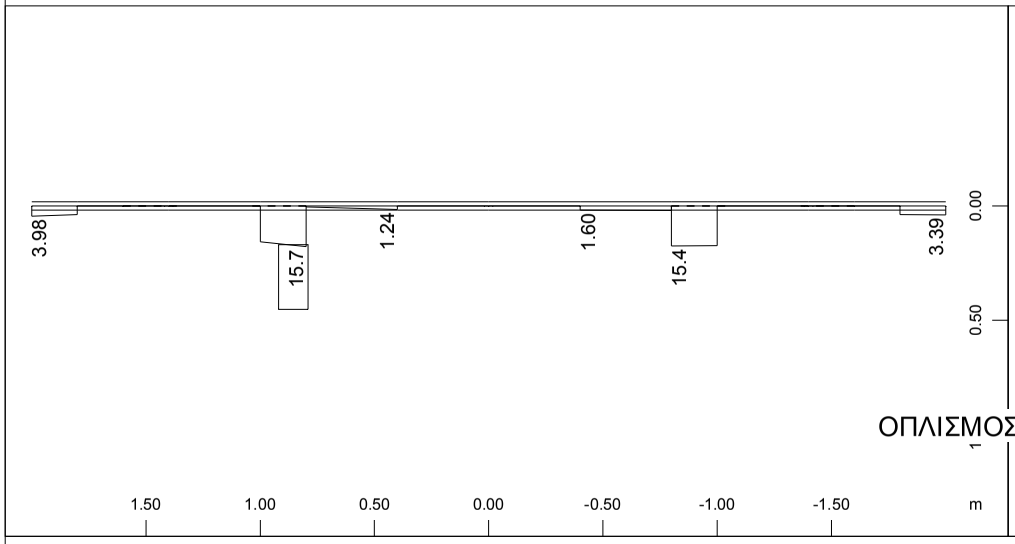


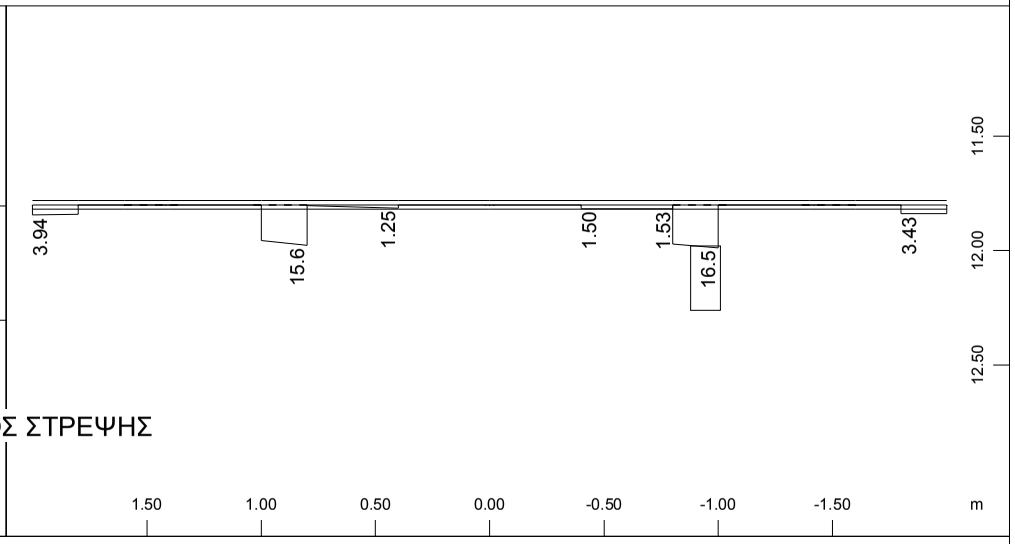


ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ

Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Shear reinforcements (maximum), Design Case 506
, 1 cm 3D = 14.5 cm2/m (Max=5.87)

Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Shear reinforcements (maximum), Design Case 506
, 1 cm 3D = 14.5 cm2/m (Max=5.96)

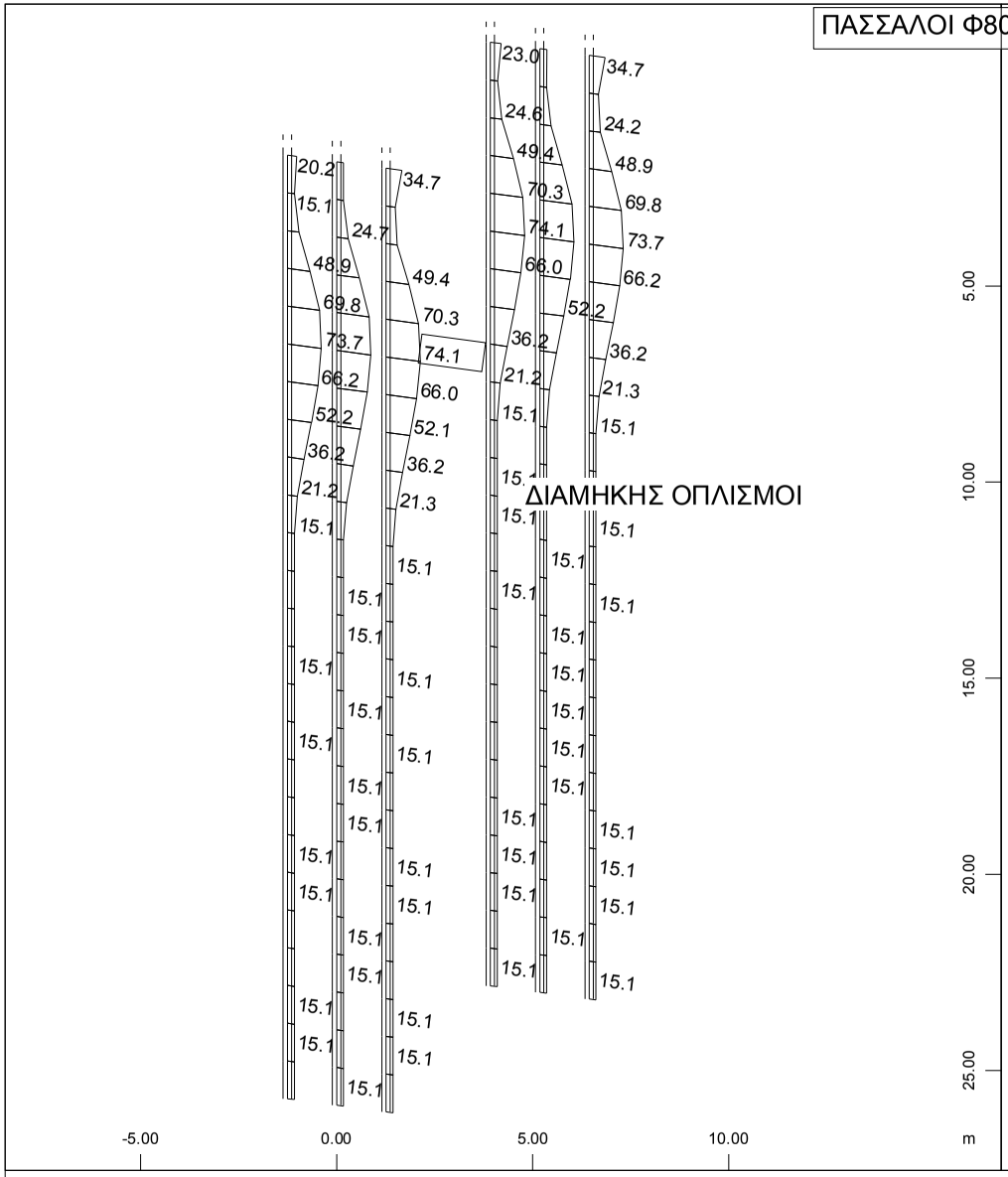




ΟΠΛΙΣΜΟΣ ΣΤΡΕΨΗΣ

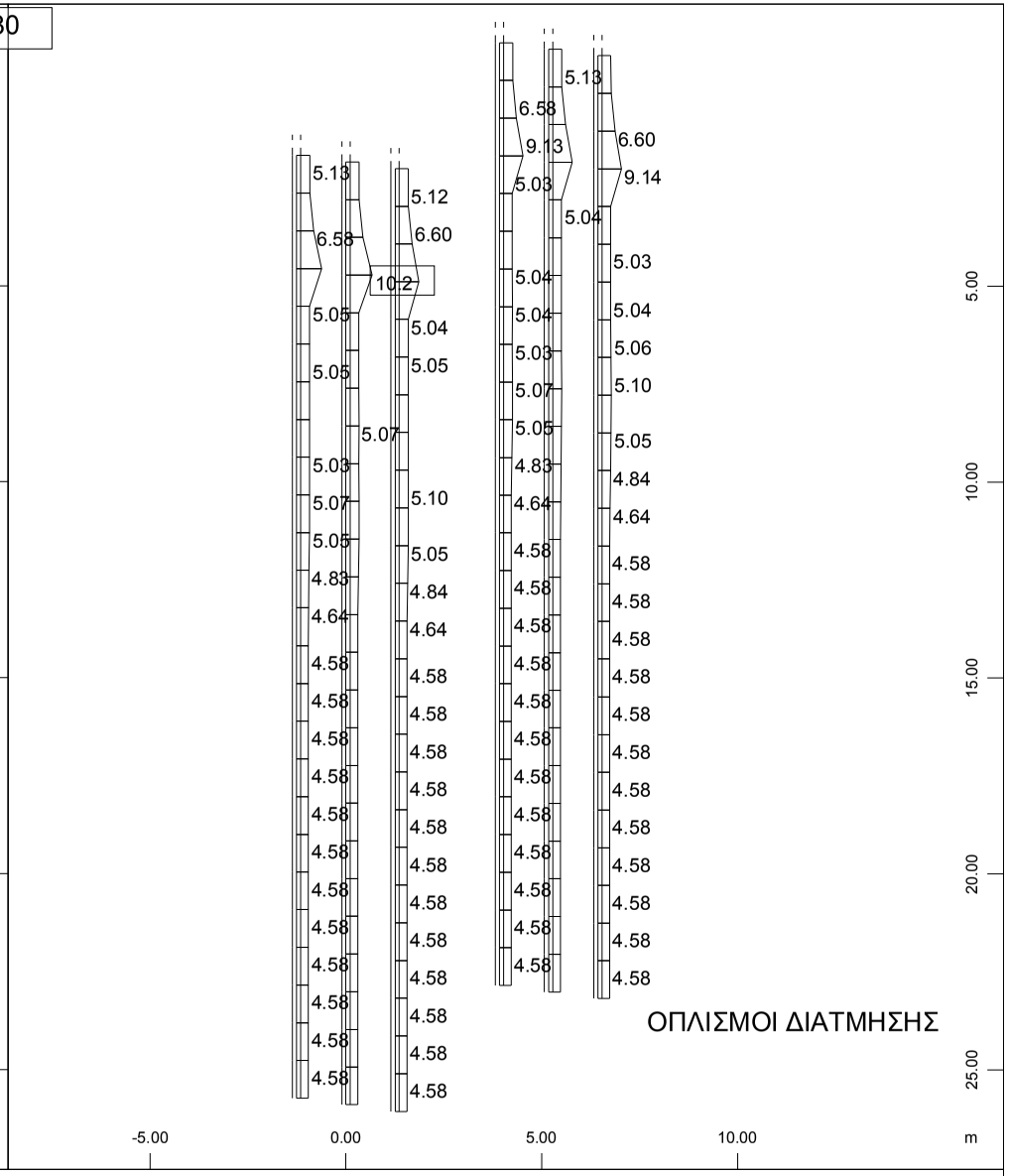
Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
506 , 1 cm 3D = 29.1 cm2 (Max=15.7)

Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
506 , 1 cm 3D = 29.1 cm2 (Max=16.5)



Sector of system Beam Elements Group 12
Beam Elements , Longitudinal Reinforcements Lay. 1, Design
Case 506 , 1 cm 3D = 145.3 cm2 (Max=74.1)

M 1 : 191
X * 0.502
Y * 0.906
Z * 0.962

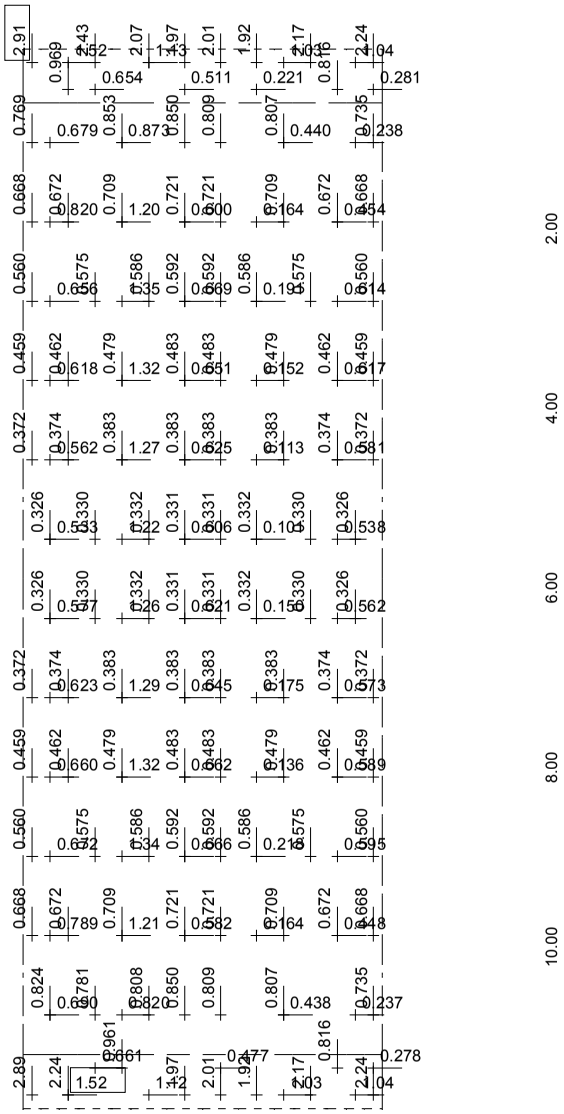


Sector of system Beam Elements Group 12
Beam Elements , Shear reinforcements (maximum), Design Case
506 , 1 cm 3D = 29.1 cm2/m (Max=10.2)

M 1 : 191
X * 0.502
Y * 0.906
Z * 0.962

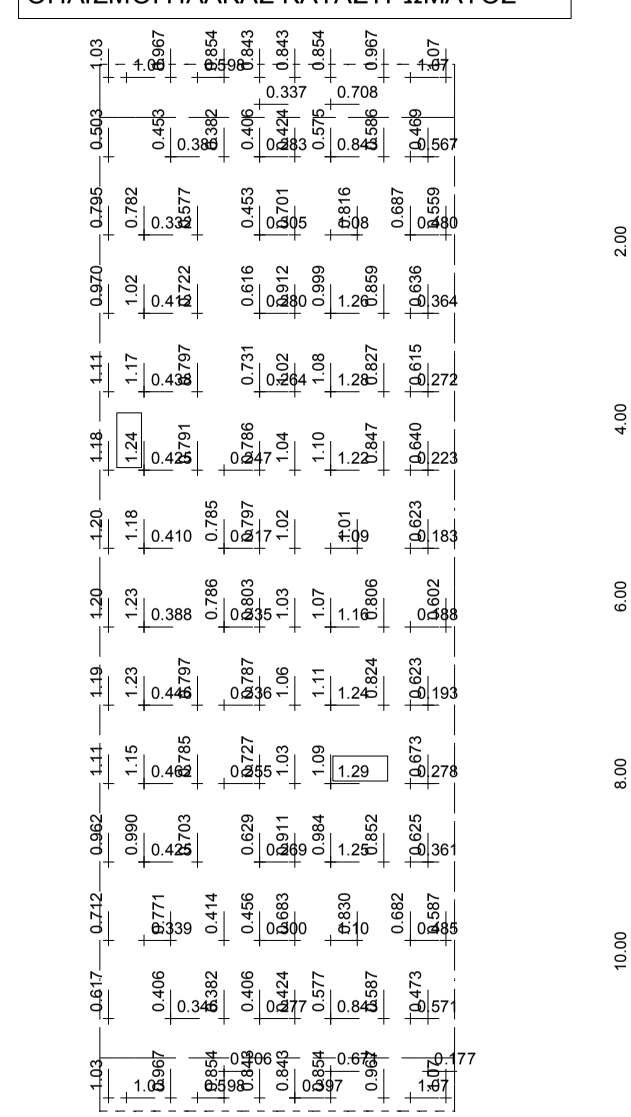
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΦΑΣΗ-2_ULS

ΟΠΛΙΣΜΟΙ ΠΛΑΚΑΣ ΚΑΤΑΣΤΡΩΜΑΤΟΣ



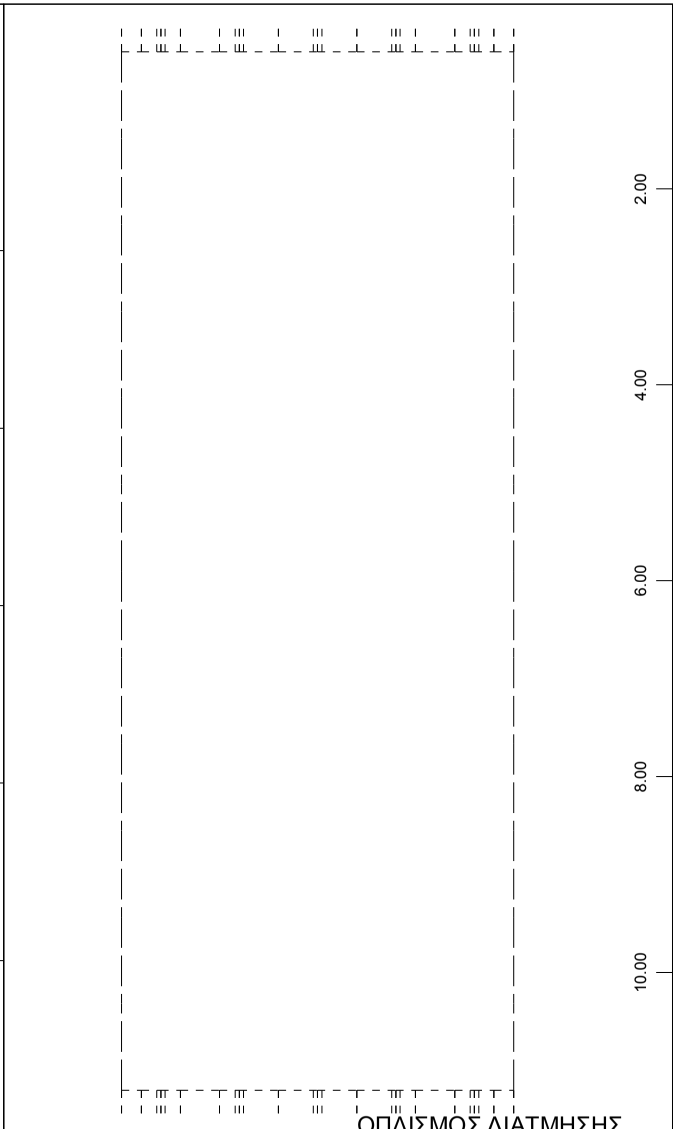
Sector of system Quadrilateral Elements Group 3 4
upper Reinforcements in Elements in cm2/m, Design Case
521 ULS design (Max=2.91)

M 1 : 84



Sector of system Quadrilateral Elements Group 3 4
lower Reinforcements in Elements in cm2/m, Design Case
521 ULS design (Max=1.29)

M 1 : 85



Sector of system Quadrilateral Elements Group 3
Shear reinforcement from middle of element in cm2/m2,
Design Case 521 ULS design (Max=0)

M 1 : 77

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00

8) ΦΑΣΗ-2 ΕΛΕΓΧΟΣ ΦΟΡΕΑ ΣΕ ULS-ΠΡΟΣΚΡΟΥΣΗΣ

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
COMBINATION-LL-frequent

Combination rule Number 1
COMB.LL-TS_ON DECK
Resulting loadcases type Design Combination
Loadcase selection

Number	factor	type		Title
41	0.40	Conditional LC		L.L.UDL_2.50KN/m2
42	0.40	Conditional LC		L.L.UDL_6.50KN/m2
50	0.75	Exclusive LC	A 1	TS_RIGHT_Posit.1
51	0.75	Exclusive LC	A 1	TS_RIGHT_Posit.2
52	0.75	Exclusive LC	A 1	TS_RIGHT_Posit.3
53	0.75	Exclusive LC	A 1	TS_RIGHT_Posit.4
54	0.75	Exclusive LC	A 1	TS_RIGHT_Posit.5
55	0.75	Exclusive LC	A 1	TS_RIGHT_Posit.6
56	0.75	Exclusive LC	A 1	TS_RIGHT_Posit.7
57	0.75	Exclusive LC	A 1	TS_RIGHT_Posit.8
58	0.75	Exclusive LC	A 1	TS_RIGHT_Posit.9
59	0.75	Exclusive LC	A 1	TS_RIGHT_Posit.10
60	0.75	Exclusive LC	A 1	TS_RIGHT_Posit.11
61	0.75	Exclusive LC	A 1	TS_RIGHT_Posit.12
70	0.40	Conditional LC		ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A1 2.50
71	0.40	Conditional LC		ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A2 2.50
72	0.40	Conditional LC		ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A1 6.50
73	0.40	Conditional LC		ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A2 6.50

Combination rule Number 2
COMB.LL-TS_OUT OF DECK
Resulting loadcases type Design Combination
Loadcase selection

Number	factor	type		Title
41	0.40	Conditional LC		L.L.UDL_2.50KN/m2
42	0.40	Conditional LC		L.L.UDL_6.50KN/m2
70	0.40	Conditional LC		ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A1 2.50
71	0.40	Conditional LC		ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A2 2.50
72	0.40	Conditional LC		ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A1 6.50
73	0.40	Conditional LC		ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A2 6.50
74	0.75	Conditional LC		ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A1 TS1-
75	0.75	Conditional LC		ΩΘΗΣΕΙΣ ΚΙΝΗΤΩΝΝ A2 TS1-

Generated Loadcases

Number	Comb	Title
2201	1	MAX-MX QUAD LL-ON
2202	1	MIN-MX QUAD LL-ON
2203	1	MAX-MY QUAD LL-ON
2204	1	MIN-MY QUAD LL-ON
2205	1	MAX-MXY QUAD LL-ON
2206	1	MIN-MXY QUAD LL-ON
2201	1	MAX-MX QUAK LL-ON
2202	1	MIN-MX QUAK LL-ON
2203	1	MAX-MY QUAK LL-ON
2204	1	MIN-MY QUAK LL-ON
2205	1	MAX-MXY QUAK LL-ON
2206	1	MIN-MXY QUAK LL-ON
2207	1	MAX-VX QUAD LL-ON
2208	1	MIN-VX QUAD LL-ON
2207	1	MAX-VX QUAK LL-ON
2208	1	MIN-VX QUAK LL-ON
2209	1	MAX-VY QUAD LL-ON
2210	1	MIN-VY QUAD LL-ON
2209	1	MAX-VY QUAK LL-ON
2210	1	MIN-VY QUAK LL-ON
2211	1	MAX-NXX QUAD LL-ON
2212	1	MIN-NXX QUAD LL-ON
2213	1	MAX-NYY QUAD LL-ON
2214	1	MIN-NYY QUAD LL-ON
2215	1	MAX-NXY QUAD LL-ON
2216	1	MIN-NXY QUAD LL-ON
2211	1	MAX-NXX QUAK LL-ON
2212	1	MIN-NXX QUAK LL-ON
2213	1	MAX-NYY QUAK LL-ON
2214	1	MIN-NYY QUAK LL-ON
2215	1	MAX-NXY QUAK LL-ON
2216	1	MIN-NXY QUAK LL-ON
1401	1	MAX-MY BEAM
1402	1	MIN-MY BEAM
1403	1	MAX-VZ BEAM
1404	1	MIN-VZ BEAM
1405	1	MAX-MZ BEAM
1406	1	MIN-MZ BEAM
1407	1	MAX-VY BEAM
1408	1	MIN-VY BEAM

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
COMBINATION-LL-frequent

Generated Loadcases

Number	Comb	Title
1409	1	MAX-N BEAM
1410	1	MIN-N BEAM
1411	1	MAX-MT BEAM
1412	1	MIN-MT BEAM
2221	2	MAX-MX QUAD LL-OFF
2222	2	MIN-MX QUAD LL-OFF
2223	2	MAX-MY QUAD LL-OFF
2224	2	MIN-MY QUAD LL-OFF
2225	2	MAX-MXY QUAD LL-OFF
2226	2	MIN-MXY QUAD LL-OFF
2221	2	MAX-MX QUAK LL-OFF
2222	2	MIN-MX QUAK LL-OFF
2223	2	MAX-MY QUAK LL-OFF
2224	2	MIN-MY QUAK LL-OFF
2225	2	MAX-MXY QUAK LL-OFF
2226	2	MIN-MXY QUAK LL-OFF
2227	2	MAX-VX QUAD LL-OFF
2228	2	MIN-VX QUAD LL-OFF
2227	2	MAX-VX QUAK LL-OFF
2228	2	MIN-VX QUAK LL-OFF
2229	2	MAX-VY QUAD LL-OFF
2230	2	MIN-VY QUAD LL-OFF
2229	2	MAX-VY QUAK LL-OFF
2230	2	MIN-VY QUAK LL-OFF
2231	2	MAX-NXX QUAD LL-OFF
2232	2	MIN-NXX QUAD LL-OFF
2233	2	MAX-NYY QUAD LL-OFF
2234	2	MIN-NYY QUAD LL-OFF
2235	2	MAX-NXY QUAD LL-OFF
2236	2	MIN-NXY QUAD LL-OFF
2231	2	MAX-NXX QUAK LL-OFF
2232	2	MIN-NXX QUAK LL-OFF
2233	2	MAX-NYY QUAK LL-OFF
2234	2	MIN-NYY QUAK LL-OFF
2235	2	MAX-NXY QUAK LL-OFF
2236	2	MIN-NXY QUAK LL-OFF
1421	2	MAX-MY BEAM
1422	2	MIN-MY BEAM
1423	2	MAX-VZ BEAM
1424	2	MIN-VZ BEAM
1425	2	MAX-MZ BEAM
1426	2	MIN-MZ BEAM
1427	2	MAX-VY BEAM
1428	2	MIN-VY BEAM
1429	2	MAX-N BEAM
1430	2	MIN-N BEAM
1431	2	MAX-MT BEAM
1432	2	MIN-MT BEAM

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
COMBINATION ULD-CRASH

Combination rule Number 1
COMB.ULS-1-BEAM
Resulting loadcases type Design Combination
Loadcase selection

Number	factor	type		Title
31	1.00	Exclusive LC	AG	ΡΥΣΕΙΣ-ΟΔΟΣΤΡΩΣΙΑ
32	1.00	Combined with LC		ΡΕΖΟDROMIO
33	1.00	Combined with LC		ΠΡΟΣΘΕΤΗ ΟΔΟΣΤΡΩΣΙΑ 0.50
36	1.00	Exclusive LC	A 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+0.5
37	1.00	Exclusive LC	A 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+0.5
38	1.00	Exclusive LC	A 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+1.0
39	1.00	Exclusive LC	A 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+1.0
1401	1.00	Exclusive LC	A 2	MAX-MY BEAM
1402	1.00	Exclusive LC	A 2	MIN-MY BEAM
1403	1.00	Exclusive LC	A 2	MAX-VZ BEAM
1404	1.00	Exclusive LC	A 2	MIN-VZ BEAM
1405	1.00	Exclusive LC	A 2	MAX-MZ BEAM
1406	1.00	Exclusive LC	A 2	MIN-MZ BEAM
1407	1.00	Exclusive LC	A 2	MAX-VY BEAM
1408	1.00	Exclusive LC	A 2	MIN-VY BEAM
1409	1.00	Exclusive LC	A 2	MAX-N BEAM
1410	1.00	Exclusive LC	A 2	MIN-N BEAM
1411	1.00	Exclusive LC	A 2	MAX-MT BEAM
1412	1.00	Exclusive LC	A 2	MIN-MT BEAM
1421	1.00	Exclusive LC	A 2	MAX-MY BEAM
1422	1.00	Exclusive LC	A 2	MIN-MY BEAM
1423	1.00	Exclusive LC	A 2	MAX-VZ BEAM
1424	1.00	Exclusive LC	A 2	MIN-VZ BEAM
1425	1.00	Exclusive LC	A 2	MAX-MZ BEAM
1426	1.00	Exclusive LC	A 2	MIN-MZ BEAM
1427	1.00	Exclusive LC	A 2	MAX-VY BEAM
1428	1.00	Exclusive LC	A 2	MIN-VY BEAM
1429	1.00	Exclusive LC	A 2	MAX-N BEAM
1430	1.00	Exclusive LC	A 2	MIN-N BEAM
1431	1.00	Exclusive LC	A 2	MAX-MT BEAM
1432	1.00	Exclusive LC	A 2	MIN-MT BEAM
80	1.00	Conditional LC		ΠΙΘΑΝΕΣ ΚΑΘΙΖΗΣΕΙΣ A1
81	1.00	Conditional LC		ΠΙΘΑΝΕΣ ΚΑΘΙΖΗΣΕΙΣ A2
100	1.00	Exclusive LC	A 3	CRASH_-Y_Pos.1 _DECK
101	1.00	Exclusive LC	A 3	CRASH_-Y_Pos.2 _DECK
102	1.00	Exclusive LC	A 3	CRASH_-Y_Pos.3 _DECK
103	1.00	Exclusive LC	A 3	CRASH_-Y_Pos.4 _DECK
104	1.00	Exclusive LC	A 3	CRASH_-Y_Pos.5 _DECK
105	1.00	Exclusive LC	A 3	CRASH_-Y_Pos.6 _DECK
106	1.00	Exclusive LC	A 3	CRASH_-Y_Pos.7 _DECK
107	1.00	Exclusive LC	A 3	CRASH_-Y_Pos.8 _DECK
108	1.00	Exclusive LC	A 3	CRASH_-Y_Pos.9 _DECK
109	1.00	Exclusive LC	A 3	CRASH_-Y_Pos.10 _DECK
110	1.00	Exclusive LC	A 3	CRASH_-Y_Pos.11 _DECK
111	1.00	Exclusive LC	A 3	CRASH_-Y_Pos.12 _DECK

Combination rule Number 2
COMB.ULS-1-QUAD
Resulting loadcases type Design Combination
Loadcase selection

Number	factor	type		Title
1	1.00	Exclusive LC	AG	Ι.Β. ΚΑΤΑΚ.ΣΤΟΙΧΕΙΩΝ
2	1.00	Combined with LC		Ι.Β. ΔΟΚΩΝ
3	1.00	Combined with LC		Ι.Β. ΧΥΤΗΣ ΠΛΑΚΑΣ
31	1.00	Combined with LC		ΡΥΣΕΙΣ-ΟΔΟΣΤΡΩΣΙΑ
32	1.00	Combined with LC		ΡΕΖΟDROMIO
33	1.00	Combined with LC		ΠΡΟΣΘΕΤΗ ΟΔΟΣΤΡΩΣΙΑ 0.50
11	1.00	Exclusive LC	A 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+0.5
12	1.00	Exclusive LC	A 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+0.5
13	1.00	Exclusive LC	A 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+1.0
14	1.00	Exclusive LC	A 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+1.0
36	1.00	Exclusive LC	A 2	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+0.5
37	1.00	Exclusive LC	A 2	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+0.5
38	1.00	Exclusive LC	A 2	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+1.0
39	1.00	Exclusive LC	A 2	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+1.0
2201	1.00	Exclusive LC	A 3	MAX-MX QUAK LL-ON
2202	1.00	Exclusive LC	A 3	MIN-MX QUAK LL-ON
2203	1.00	Exclusive LC	A 3	MAX-MY QUAK LL-ON
2204	1.00	Exclusive LC	A 3	MIN-MY QUAK LL-ON
2205	1.00	Exclusive LC	A 3	MAX-MXY QUAK LL-ON
2206	1.00	Exclusive LC	A 3	MIN-MXY QUAK LL-ON
2207	1.00	Exclusive LC	A 3	MAX-VX QUAK LL-ON
2208	1.00	Exclusive LC	A 3	MIN-VX QUAK LL-ON
2209	1.00	Exclusive LC	A 3	MAX-VY QUAK LL-ON
2210	1.00	Exclusive LC	A 3	MIN-VY QUAK LL-ON

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
COMBINATION ULD-CRASH

Combination rule Number 2
COMB.ULS-1-QUAD
Resulting loadcases type Design Combination
Loadcase selection

Number	factor	type	Title
2211	1.00	Exclusive LC	A 3 MAX-NXX QUAK LL-ON
2212	1.00	Exclusive LC	A 3 MIN-NXX QUAK LL-ON
2213	1.00	Exclusive LC	A 3 MAX-NYY QUAK LL-ON
2214	1.00	Exclusive LC	A 3 MIN-NYY QUAK LL-ON
2215	1.00	Exclusive LC	A 3 MAX-NXY QUAK LL-ON
2216	1.00	Exclusive LC	A 3 MIN-NXY QUAK LL-ON
2221	1.00	Exclusive LC	A 3 MAX-MX QUAK LL-OFF
2222	1.00	Exclusive LC	A 3 MIN-MX QUAK LL-OFF
2223	1.00	Exclusive LC	A 3 MAX-MY QUAK LL-OFF
2224	1.00	Exclusive LC	A 3 MIN-MY QUAK LL-OFF
2225	1.00	Exclusive LC	A 3 MAX-MXY QUAK LL-OFF
2226	1.00	Exclusive LC	A 3 MIN-MXY QUAK LL-OFF
2227	1.00	Exclusive LC	A 3 MAX-VX QUAK LL-OFF
2228	1.00	Exclusive LC	A 3 MIN-VX QUAK LL-OFF
2229	1.00	Exclusive LC	A 3 MAX-VY QUAK LL-OFF
2230	1.00	Exclusive LC	A 3 MIN-VY QUAK LL-OFF
2231	1.00	Exclusive LC	A 3 MAX-NXX QUAK LL-OFF
2232	1.00	Exclusive LC	A 3 MIN-NXX QUAK LL-OFF
2233	1.00	Exclusive LC	A 3 MAX-NYY QUAK LL-OFF
2234	1.00	Exclusive LC	A 3 MIN-NYY QUAK LL-OFF
2235	1.00	Exclusive LC	A 3 MAX-NXY QUAK LL-OFF
2236	1.00	Exclusive LC	A 3 MIN-NXY QUAK LL-OFF
80	1.00	Conditional LC	ΠΙΘΑΝΕΣ ΚΑΘΙΖΗΣΕΙΣ A1
81	1.00	Conditional LC	ΠΙΘΑΝΕΣ ΚΑΘΙΖΗΣΕΙΣ A2
100	1.00	Exclusive LC	A 4 CRASH_-Y_Pos.1 _DECK
101	1.00	Exclusive LC	A 4 CRASH_-Y_Pos.2 _DECK
102	1.00	Exclusive LC	A 4 CRASH_-Y_Pos.3 _DECK
103	1.00	Exclusive LC	A 4 CRASH_-Y_Pos.4 _DECK
104	1.00	Exclusive LC	A 4 CRASH_-Y_Pos.5 _DECK
105	1.00	Exclusive LC	A 4 CRASH_-Y_Pos.6 _DECK
106	1.00	Exclusive LC	A 4 CRASH_-Y_Pos.7 _DECK
107	1.00	Exclusive LC	A 4 CRASH_-Y_Pos.8 _DECK
108	1.00	Exclusive LC	A 4 CRASH_-Y_Pos.9 _DECK
109	1.00	Exclusive LC	A 4 CRASH_-Y_Pos.10 _DECK
110	1.00	Exclusive LC	A 4 CRASH_-Y_Pos.11 _DECK
111	1.00	Exclusive LC	A 4 CRASH_-Y_Pos.12 _DECK
6015	1.00	Conditional LC	15 K creep step
6025	1.00	Conditional LC	25 K creep step
6055	1.00	Conditional LC	55 K creep step
6060	1.00	Conditional LC	60 K creep step
6061	1.00	Conditional LC	61 K creep step
6062	1.00	Conditional LC	62 K creep step
6063	1.00	Conditional LC	63 K creep step
6064	1.00	Conditional LC	64 K creep step

Generated Loadcases

Number	Comb	Title
1501	1	MAX-MY BEAM
1502	1	MIN-MY BEAM
1503	1	MAX-VZ BEAM
1504	1	MIN-VZ BEAM
1505	1	MAX-MZ BEAM
1506	1	MIN-MZ BEAM
1507	1	MAX-VY BEAM
1508	1	MIN-VY BEAM
1509	1	MAX-N BEAM
1510	1	MIN-N BEAM
1511	1	MAX-MT BEAM
1512	1	MIN-MT BEAM
2301	2	MAX-MX QUAD ULS-B1
2302	2	MIN-MX QUAD ULS-B1
2303	2	MAX-MY QUAD ULS-B1
2304	2	MIN-MY QUAD ULS-B1
2305	2	MAX-MXY QUAD ULS-B1
2306	2	MIN-MXY QUAD ULS-B1
2301	2	MAX-MX QUAK ULS-B1
2302	2	MIN-MX QUAK ULS-B1
2303	2	MAX-MY QUAK ULS-B1
2304	2	MIN-MY QUAK ULS-B1
2305	2	MAX-MXY QUAK ULS-B1
2306	2	MIN-MXY QUAK ULS-B1
2307	2	MAX-VX QUAD ULS-B1
2308	2	MIN-VX QUAD ULS-B1
2307	2	MAX-VX QUAK ULS-B1
2308	2	MIN-VX QUAK ULS-B1
2309	2	MAX-VY QUAD ULS-B1

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
COMBINATION ULD-CRASH

Generated Loadcases

Number	Comb	Title
2310	2	MIN-VY QUAD ULS-B1
2309	2	MAX-VY QUAK ULS-B1
2310	2	MIN-VY QUAK ULS-B1
2311	2	MAX-NXX QUAD ULS-B1
2312	2	MIN-NXX QUAD ULS-B1
2313	2	MAX-NYY QUAD ULS-B1
2314	2	MIN-NYY QUAD ULS-B1
2315	2	MAX-NXY QUAD ULS-B1
2316	2	MIN-NXY QUAD ULS-B1
2311	2	MAX-NXX QUAK ULS-B1
2312	2	MIN-NXX QUAK ULS-B1
2313	2	MAX-NYY QUAK ULS-B1
2314	2	MIN-NYY QUAK ULS-B1
2315	2	MAX-NXY QUAK ULS-B1
2316	2	MIN-NXY QUAK ULS-B1

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN CRASH

Design according to DIN1045-1 2008
Loadcases have been calculated in the Ultimate Limit State
In BEMESS no additional load safety factor is applied.

Load Cases for the Design

Loadcase 2301 MAX-MX QUAK ULS-B1
Loadcase 2302 MIN-MX QUAK ULS-B1
Loadcase 2303 MAX-MY QUAK ULS-B1
Loadcase 2304 MIN-MY QUAK ULS-B1
Loadcase 2305 MAX-MXY QUAK ULS-B1
Loadcase 2306 MIN-MXY QUAK ULS-B1
Loadcase 2307 MAX-VX QUAK ULS-B1
Loadcase 2308 MIN-VX QUAK ULS-B1
Loadcase 2309 MAX-VY QUAK ULS-B1
Loadcase 2310 MIN-VY QUAK ULS-B1
Loadcase 2311 MAX-NXX QUAK ULS-B1
Loadcase 2312 MIN-NXX QUAK ULS-B1
Loadcase 2313 MAX-NYY QUAK ULS-B1
Loadcase 2314 MIN-NYY QUAK ULS-B1
Loadcase 2315 MAX-NXY QUAK ULS-B1
Loadcase 2316 MIN-NXY QUAK ULS-B1

Material (DIN1045-1 2008)

Mat	f-ck [MPa]	f-cr [MPa]	f-yk [MPa]	f-tk [MPa]	f-ctm [MPa]	N [-]	minQ [-]	type
B1	25.0	21.2	500.0	500.0	2.565	10.5	0.20	mainly static

Minimum reinforcement: 0.00 p.c. of stat. req. section

Reduction of FC in case of transvers tension = 25.0 [o/o]

Material-safety-factors:

Mat	concr SC1	SC2	steel SS1	SS2
B1	1.30	1.30	1.00	1.00

Acc. the german DIN Fachberichten a minimum concrete shear capacity VRd,ct
is taken into account in the shear design without shear reinforcement.

In shear design the cotangens theta is limited to 1.750 .

At direct supports from the face of the support up to 1.0*d the shear force is reduced.
The maximum shear capacity is checked at the face of the support without reduction.

The punching design has been switched off and must be done separately.
Outside the punching area, the normal slab shear design may increase the,
longitudinal reinforcement up to 0.20% [input CTRL...RO_V].

Geometry (axial covers)

No	he-upper [mm]	hi-upper [mm]	he-lower [mm]	hi-lower [mm]	Elem. height [mm]
1	50	70	35	55	As saved

Selection of elements

	from	to	inc	group	GEOMETRY
Element	3001	3999	1	-	1
Element	8001	8999	1	-	1

Reinforcement is saved in the data base file
Number of stored reinforcement-distribution: 524

REINFORCEMENT ACC. TO DIN1045-1 2008 in [cm2/m] upper/lower
General load safety factor - as defined in BEMESS: Gamma-f = 1.00
Shear: stresses VEd/d and VRd,ct/d with d=effective depth = h-hm
Shear index 2m = minimum shear reinforcement

Grp	ELEM No	LC No	MAT No	GEO No	h [m]	Reinforcement main	cross	dir	dphi deg	Shr zon	VEd/d [MPa] VRd,ct/d	Ass [cm2/m2]
3	3001	maximum			0.25	1.40	0.36	0		1	0.063	
						0.77	0.36	0			0.529	
3	3002	maximum			0.25	1.23	0.40	0		1	0.089	
						1.04	0.33	0			0.525	
3	3003	maximum			0.25	1.02	0.41	0		1	0.093	
						0.96	0.37	0			0.518	
3	3004	maximum			0.25	0.82	0.37	0		1	0.093	
						0.88	0.32	0			0.517	
3	3005	maximum			0.25	0.62	0.34	0		1	0.090	
						0.81	0.30	0			0.518	
3	3006	maximum			0.25	0.43	0.25	0		1	0.083	
						0.71	0.28	0			0.517	
3	3007	maximum			0.25	1.44	0.77	0		1	0.126	
						0.96	0.57	0			0.526	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN CRASH

REINFORCEMENT ACC. TO DIN1045-1 2008 in [cm2/m] upper/lower
General load safety factor - as defined in BEMESS: Gamma-f = 1.00
Shear: stresses VEd/d and VRd,ct/d with d=effective depth = h-hm
Shear index 2m = minimum shear reinforcement

Grp	ELEM No	LC No	MAT No	GEO No	h [m]	Reinforcement main	Reinforcement cross	dphi dir	dphi deg	Shr zon	VEd/d [MPa] VRd,ct/d	Ass [cm2/m2]
3	3008	maximum			0.25	1.20	0.77	0		1	0.140	
						1.04	0.39	0			0.516	
3	3009	maximum			0.25	1.03	0.79	0		1	0.128	
						1.07	0.36	0			0.515	
3	3010	maximum			0.25	0.84	0.73	0		1	0.121	
						0.94	0.38	0			0.507	
3	3011	maximum			0.25	0.74	0.66	0		1	0.110	
						0.91	0.41	0			0.512	
3	3012	maximum			0.25	0.48	0.47	0		1	0.107	
						0.88	0.35	0			0.510	
3	3013	maximum			0.25	1.55	1.04	0		1	0.163	
						0.78	0.36	0			0.497	
3	3014	maximum			0.25	1.41	1.04	0		1	0.145	
						0.96	0.64	0			0.500	
3	3015	maximum			0.25	1.25	0.94	0		1	0.125	
						1.08	0.60	0			0.499	
3	3016	maximum			0.25	1.06	0.85	0		1	0.108	
						1.16	0.56	0			0.498	
3	3017	maximum			0.25	0.87	0.72	0		1	0.090	
						1.19	0.55	0			0.498	
3	3018	maximum			0.25	0.70	0.65	0		1	0.082	
						1.06	0.52	0			0.498	
3	3019	maximum			0.25	1.35	0.92	0		1	0.065	
						0.68	0.72	0			0.500	
3	3020	maximum			0.25	1.28	1.01	0		1	0.074	
						0.87	0.57	0			0.499	
3	3021	maximum			0.25	1.14	0.97	0		1	0.083	
						0.99	0.53	0			0.496	
3	3022	maximum			0.25	0.98	0.89	0		1	0.085	
						1.03	0.42	0			0.497	
3	3023	maximum			0.25	0.81	0.81	0		1	0.089	
						0.80	0.36	0			0.498	
3	3024	maximum			0.25	0.59	0.70	0		1	0.091	
						0.72	0.36	0			0.502	
3	3025	maximum			0.25	1.19	0.87	0		1	0.136	
						0.79	0.71	0			0.498	
3	3026	maximum			0.25	1.15	1.20	0		1	0.153	
						0.83	0.47	0			0.496	
3	3027	maximum			0.25	1.04	1.21	0		1	0.159	
						0.95	0.39	0			0.496	
3	3028	maximum			0.25	0.90	1.14	0		1	0.157	
						1.00	0.38	0			0.497	
3	3029	maximum			0.25	0.74	1.07	0		1	0.156	
						1.04	0.38	0			0.497	
3	3030	maximum			0.25	0.61	1.00	0		1	0.156	
						0.68	0.36	0			0.497	
3	3031	maximum			0.25	1.15	1.20	0		1	0.038	
						0.56	0.68	0			0.496	
3	3032	maximum			0.25	1.20	1.26	0		1	0.032	
						0.69	0.61	0			0.496	
3	3033	maximum			0.25	1.07	1.19	0		1	0.037	
						0.58	0.64	0			0.500	
3	3034	maximum			0.25	0.92	1.17	0		1	0.033	
						0.64	0.57	0			0.501	
3	3035	maximum			0.25	0.79	1.08	0		1	0.036	
						0.62	0.37	0			0.506	
3	3036	maximum			0.25	0.68	1.14	0		1	0.039	
						0.49	0.27	0			0.500	
3	3037	maximum			0.25	1.07	1.09	0		1	0.034	
						0.85	0.94	0			0.503	
3	3038	maximum			0.25	1.06	1.01	0		1	0.027	
						0.69	0.58	0			0.510	
3	3039	maximum			0.25	0.97	1.01	0		1	0.032	
						0.73	0.46	0			0.500	
3	3040	maximum			0.25	0.88	1.28	0		1	0.033	
						0.57	0.33	0			0.506	
3	3041	maximum			0.25	0.78	1.34	0		1	0.040	
						0.57	0.30	0			0.500	
3	3042	maximum			0.25	0.69	1.36	0		1	0.043	
						0.42	0.30	0			0.498	
3	3043	maximum			0.25	1.00	1.32	0		1	0.131	
						0.85	1.76	0			0.495	
3	3044	maximum			0.25	1.23	1.25	0		1	0.149	
						0.94	1.37	0			0.505	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN CRASH

REINFORCEMENT ACC. TO DIN1045-1 2008 in [cm2/m] upper/lower
General load safety factor - as defined in BEMESS: Gamma-f = 1.00
Shear: stresses VEd/d and VRd,ct/d with d=effective depth = h-hm
Shear index 2m = minimum shear reinforcement

Grp	ELEM No	LC No	MAT No	GEO No	h [m]	Reinforcement main	Reinforcement cross	dphi dir	dphi deg	Shr zon	VEd/d [MPa] VRd,ct/d	Ass [cm2/m2]
3	3045	maximum			0.25	1.22	1.50	0		1	0.146	
						0.80	0.82	0			0.502	
3	3046	maximum			0.25	1.13	1.65	0		1	0.137	
						0.76	0.40	0			0.512	
3	3047	maximum			0.25	1.02	1.64	0		1	0.125	
						0.94	0.73	0			0.508	
3	3048	maximum			0.25	0.94	1.81	0		1	0.121	
						0.94	0.74	0			0.504	
3	3049	maximum			0.25	1.25	1.76	0		1	0.139	
						0.91	1.21	0			0.499	
3	3050	maximum			0.25	1.28	1.99	0		1	0.163	
						1.11	1.38	0			0.498	
3	3051	maximum			0.25	1.30	2.58	0		1	0.169	
						1.17	0.91	0			0.500	
3	3052	maximum			0.25	1.20	2.79	0		1	0.167	
						1.10	0.65	0			0.499	
3	3053	maximum			0.25	1.16	2.81	0		1	0.165	
						0.95	1.00	0			0.499	
3	3054	maximum			0.25	1.08	2.74	0		1	0.166	
						0.93	0.97	0			0.498	
3	3055	maximum			0.25	1.53	2.51	0		1	0.388	
						1.30	2.97	0			0.496	
3	3056	maximum			0.25	2.12	3.47	0		1	0.332	
						1.42	1.72	0			0.518	
3	3057	maximum			0.25	2.00	4.21	0		1	0.302	
						1.33	0.73	0			0.518	
3	3058	maximum			0.25	1.85	4.38	0		1	0.298	
						0.75	0.66	0			0.512	
3	3059	maximum			0.25	1.76	4.37	0		1	0.302	
						0.66	0.58	0			0.510	
3	3060	maximum			0.25	1.71	4.18	0		1	0.304	
						0.74	0.71	0			0.513	
3	3061	maximum			0.25	1.83	6.09	0		1	0.387	
						0.64	0.24	0			0.496	
3	3062	maximum			0.25	2.01	6.72	0		1	0.346	
						0.70	0.29	0			0.511	
3	3063	maximum			0.25	2.17	6.90	0		1	0.319	
						0.50	0.28	0			0.509	
3	3064	maximum			0.25	2.22	6.78	0		1	0.313	
						0.44	0.28	0			0.517	
3	3065	maximum			0.25	1.94	6.60	0		1	0.315	
						0.64	0.42	0			0.512	
3	3066	maximum			0.25	1.88	6.53	0		1	0.315	
						0.65	0.46	0			0.508	
3	3067	maximum			0.25	2.55	9.52	0		2	0.398	6.65
						0.20	0.26	0			0.499	
3	3068	maximum			0.25	2.75	10.18	0		2	0.387	6.46
						0.33	0.23	0			0.518	
3	3069	maximum			0.25	3.00	10.10	0		1	0.359	
						0.25	0.17	0			0.514	
3	3070	maximum			0.25	2.75	9.67	0		1	0.353	
						0.39	0.16	0			0.510	
3	3071	maximum			0.25	2.88	9.92	0		1	0.358	
						0.47	0.13	0			0.507	
3	3072	maximum			0.25	2.56	9.64	0		1	0.359	
						0.55	0.12	0			0.506	
3	3073	maximum			0.25	3.84	15.96	0		1	0.323	
						0.68	0.60	0			0.544	
3	3074	maximum			0.25	4.45	15.54	0		1	0.139	
						0.61	1.30	0			0.498	
3	3075	maximum			0.25	4.89	15.22	0		1	0.139	
						0.95	1.91	0			0.522	
3	3076	maximum			0.25	4.86	15.17	0		1	0.140	
						0.96	1.88	0			0.510	
3	3077	maximum			0.25	4.64	15.59	0		1	0.147	
						1.15	1.52	0			0.503	
3	3078	maximum			0.25	4.04	15.11	0		1	0.144	
						1.13	1.25	0			0.534	
3	3079	maximum			0.25	1.38	1.62	0		1	0.371	
						2.63	1.56	0			0.531	
3	3080	maximum			0.25	2.94	2.84	0		2	0.548	9.16
						3.51	1.98	0			0.560	
3	3081	maximum			0.25	3.23	3.03	0		2	0.629	10.50
						3.92	2.16	0			0.504	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN CRASH

REINFORCEMENT ACC. TO DIN1045-1 2008 in [cm2/m] upper/lower
General load safety factor - as defined in BEMESS: Gamma-f = 1.00
Shear: stresses VEd/d and VRd,ct/d with d=effective depth = h-hm
Shear index 2m = minimum shear reinforcement

Grp	ELEM No	LC No	MAT No	GEO No	h [m]	Reinforcement main	Reinforcement cross	dir	dphi deg	Shr zon	VEd/d [MPa] VRd,ct/d	Ass [cm2/m2]
3	3082	maximum			0.25	3.38	3.14	0		2	0.631	10.54
						4.13	2.18	0			0.503	
3	3083	maximum			0.25	3.09	2.92	0		2	0.572	9.56
						4.07	2.62	0			0.510	
3	3084	maximum			0.25	2.42	2.45	0		1	0.470	
						3.68	1.98	0			0.512	
3	3085	maximum			0.25	0.37	0.20	0		1	0.085	
						0.69	0.27	0			0.515	
3	3086	maximum			0.25	0.54	0.32	0		1	0.089	
						0.77	0.28	0			0.521	
3	3087	maximum			0.25	0.72	0.35	0		1	0.090	
						0.79	0.29	0			0.516	
3	3088	maximum			0.25	0.90	0.38	0		1	0.087	
						0.88	0.36	0			0.518	
3	3089	maximum			0.25	1.10	0.36	0		1	0.082	
						0.82	0.32	0			0.526	
3	3090	maximum			0.25	1.29	0.33	0		1	0.058	
						0.73	0.36	0			0.530	
3	3091	maximum			0.25	0.43	0.49	0		1	0.116	
						0.79	0.38	0			0.506	
3	3092	maximum			0.25	0.65	0.65	0		1	0.125	
						0.86	0.39	0			0.511	
3	3093	maximum			0.25	0.76	0.71	0		1	0.132	
						0.87	0.37	0			0.511	
3	3094	maximum			0.25	0.93	0.75	0		1	0.133	
						0.91	0.46	0			0.515	
3	3095	maximum			0.25	1.09	0.74	0		1	0.138	
						0.90	0.39	0			0.517	
3	3096	maximum			0.25	1.33	0.70	0		1	0.115	
						0.87	0.54	0			0.527	
3	3097	maximum			0.25	0.66	0.64	0		1	0.093	
						1.09	0.57	0			0.499	
3	3098	maximum			0.25	0.81	0.71	0		1	0.109	
						1.11	0.55	0			0.499	
3	3099	maximum			0.25	0.99	0.79	0		1	0.122	
						1.08	0.65	0			0.499	
3	3100	maximum			0.25	1.16	0.92	0		1	0.130	
						1.00	0.63	0			0.500	
3	3101	maximum			0.25	1.30	0.97	0		1	0.141	
						0.89	0.66	0			0.501	
3	3102	maximum			0.25	1.41	0.94	0		1	0.148	
						0.74	0.36	0			0.498	
3	3103	maximum			0.25	0.56	0.70	0		1	0.093	
						0.70	0.35	0			0.496	
3	3104	maximum			0.25	0.77	0.81	0		1	0.092	
						0.75	0.35	0			0.498	
3	3105	maximum			0.25	0.93	0.88	0		1	0.088	
						0.82	0.45	0			0.498	
3	3106	maximum			0.25	1.07	0.94	0		1	0.084	
						0.95	0.48	0			0.496	
3	3107	maximum			0.25	1.18	0.95	0		1	0.073	
						0.85	0.47	0			0.498	
3	3108	maximum			0.25	1.23	0.85	0		1	0.063	
						0.67	0.71	0			0.498	
3	3109	maximum			0.25	0.59	1.04	0		1	0.164	
						0.61	0.32	0			0.497	
3	3110	maximum			0.25	0.72	1.10	0		1	0.167	
						0.73	0.35	0			0.502	
3	3111	maximum			0.25	0.87	1.17	0		1	0.167	
						0.75	0.35	0			0.498	
3	3112	maximum			0.25	1.00	1.23	0		1	0.163	
						0.73	0.38	0			0.496	
3	3113	maximum			0.25	1.07	1.19	0		1	0.151	
						0.82	0.54	0			0.496	
3	3114	maximum			0.25	1.08	0.85	0		1	0.127	
						0.70	0.72	0			0.502	
3	3115	maximum			0.25	0.67	1.10	0		1	0.039	
						0.40	0.27	0			0.499	
3	3116	maximum			0.25	0.77	1.08	0		1	0.035	
						0.59	0.32	0			0.508	
3	3117	maximum			0.25	0.91	1.13	0		1	0.032	
						0.57	0.55	0			0.501	
3	3118	maximum			0.25	1.05	1.19	0		1	0.035	
						0.56	0.62	0			0.500	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN CRASH

REINFORCEMENT ACC. TO DIN1045-1 2008 in [cm2/m] upper/lower
General load safety factor - as defined in BEMESS: Gamma-f = 1.00
Shear: stresses VEd/d and VRd,ct/d with d=effective depth = h-hm
Shear index 2m = minimum shear reinforcement

Grp	ELEM No	LC No	MAT No	GEO No	h [m]	Reinforcement main	cross	dir	dphi deg	Shr zon	VEd/d [MPa] VRd,ct/d	Ass [cm2/m2]
3	3119	maximum			0.25	1.13	1.26	0		1	0.030	
						0.55	0.55	0			0.498	
3	3120	maximum			0.25	1.03	1.12	0		1	0.038	
						0.56	0.64	0			0.496	
3	3121	maximum			0.25	0.71	1.27	0		1	0.043	
						0.38	0.27	0			0.498	
3	3122	maximum			0.25	0.78	1.32	0		1	0.039	
						0.50	0.24	0			0.500	
3	3123	maximum			0.25	0.85	1.24	0		1	0.032	
						0.55	0.28	0			0.506	
3	3124	maximum			0.25	0.95	1.21	0		1	0.030	
						0.59	0.50	0			0.499	
3	3125	maximum			0.25	1.03	1.20	0		1	0.025	
						0.60	0.60	0			0.505	
3	3126	maximum			0.25	0.98	1.09	0		1	0.035	
						0.85	0.88	0			0.506	
3	3127	maximum			0.25	0.96	1.74	0		1	0.129	
						0.91	0.67	0			0.505	
3	3128	maximum			0.25	0.99	1.78	0		1	0.139	
						0.74	0.40	0			0.502	
3	3129	maximum			0.25	1.08	1.51	0		1	0.147	
						0.71	0.42	0			0.505	
3	3130	maximum			0.25	1.18	1.51	0		1	0.150	
						0.78	0.67	0			0.508	
3	3131	maximum			0.25	1.24	1.27	0		1	0.146	
						1.03	1.46	0			0.513	
3	3132	maximum			0.25	1.02	1.32	0		1	0.118	
						0.97	1.82	0			0.514	
3	3133	maximum			0.25	1.09	2.70	0		1	0.173	
						0.95	0.92	0			0.498	
3	3134	maximum			0.25	1.15	2.57	0		1	0.178	
						0.92	0.89	0			0.498	
3	3135	maximum			0.25	1.24	2.58	0		1	0.177	
						0.80	0.70	0			0.498	
3	3136	maximum			0.25	1.26	2.48	0		1	0.169	
						0.88	0.76	0			0.498	
3	3137	maximum			0.25	1.24	2.18	0		1	0.162	
						1.17	1.25	0			0.497	
3	3138	maximum			0.25	1.25	1.85	0		1	0.132	
						0.85	1.58	0			0.501	
3	3139	maximum			0.25	1.78	3.86	0		1	0.303	
						0.71	0.62	0			0.518	
3	3140	maximum			0.25	1.83	4.03	0		1	0.305	
						0.64	0.57	0			0.513	
3	3141	maximum			0.25	1.89	4.14	0		1	0.311	
						0.89	0.88	0			0.509	
3	3142	maximum			0.25	1.82	4.11	0		1	0.320	
						0.77	0.67	0			0.511	
3	3143	maximum			0.25	1.97	3.73	0		1	0.333	
						1.04	1.02	0			0.525	
3	3144	maximum			0.25	2.04	2.58	0		1	0.365	
						1.58	2.94	0			0.496	
3	3145	maximum			0.25	1.93	6.27	0		1	0.316	
						0.63	0.46	0			0.503	
3	3146	maximum			0.25	2.09	5.96	0		1	0.319	
						0.63	0.44	0			0.507	
3	3147	maximum			0.25	1.93	5.96	0		1	0.325	
						0.84	0.56	0			0.511	
3	3148	maximum			0.25	1.94	6.11	0		1	0.335	
						0.40	0.27	0			0.517	
3	3149	maximum			0.25	2.32	6.48	0		1	0.344	
						0.61	0.29	0			0.508	
3	3150	maximum			0.25	2.32	6.37	0		1	0.368	
						0.99	0.24	0			0.496	
3	3151	maximum			0.25	2.31	9.17	0		1	0.359	
						0.55	0.11	0			0.516	
3	3152	maximum			0.25	2.27	8.81	0		1	0.362	
						0.46	0.10	0			0.522	
3	3153	maximum			0.25	2.41	9.11	0		1	0.369	
						0.38	0.14	0			0.512	
3	3154	maximum			0.25	2.57	9.23	0		1	0.379	
						0.35	0.17	0			0.507	
3	3155	maximum			0.25	2.62	9.38	0		1	0.386	
						0.32	0.23	0			0.512	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN CRASH

REINFORCEMENT ACC. TO DIN1045-1 2008 in [cm2/m] upper/lower
General load safety factor - as defined in BEMESS: Gamma-f = 1.00
Shear: stresses VEd/d and VRd,ct/d with d=effective depth = h-hm
Shear index 2m = minimum shear reinforcement

Grp	ELEM No	LC No	MAT No	GEO No	h [m]	Reinforcement main	cross	dir	dphi deg	Shr zon	VEd/d [MPa] VRd,ct/d	Ass [cm2/m2]
3	3156	maximum			0.25	3.20	9.70	0		1	0.372	
						0.20	0.26	0			0.499	
3	3157	maximum			0.25	3.15	14.02	0		1	0.138	
						1.09	1.11	0			0.527	
3	3158	maximum			0.25	2.69	13.47	0		1	0.137	
						0.92	0.91	0			0.503	
3	3159	maximum			0.25	3.37	14.47	0		1	0.141	
						0.85	1.05	0			0.524	
3	3160	maximum			0.25	4.05	15.39	0		1	0.151	
						0.62	1.34	0			0.538	
3	3161	maximum			0.25	4.40	15.51	0		1	0.163	
						0.70	1.56	0			0.507	
3	3162	maximum			0.25	4.81	15.05	0		1	0.204	
						0.55	1.87	0			0.518	
3	3163	maximum			0.25	1.73	1.96	0		1	0.371	
						2.98	1.20	0			0.505	
3	3164	maximum			0.25	1.42	1.67	0		1	0.310	
						2.58	0.97	0			0.505	
3	3165	maximum			0.25	1.98	2.14	0		1	0.409	
						3.13	1.56	0			0.520	
3	3166	maximum			0.25	2.53	2.61	0		2	0.513	8.57
						3.48	2.32	0			0.521	
3	3167	maximum			0.25	3.00	2.99	0		2	0.610	10.19
						2.68	2.03	0			0.496	
3	3168	maximum			0.25	2.77	3.48	0		2	0.671	11.21
						2.08	2.16	0			0.529	
8	8001	maximum			1.20	0.36	1.81	0		1	0.045	
						0.56	2.81	0			0.316	
8	8002	maximum			1.20	0.51	2.55	0		1	0.090	
						1.28	6.40	0			0.323	
8	8003	maximum			1.20	0.88	4.42	0		1	0.108	
						5.84	14.54	0			0.364	
8	8004	maximum			1.20	1.81	2.51	0		1	0.115	
						1.10	5.52	0			0.329	
8	8005	maximum			1.20	3.26	5.50	0		1	0.117	
						7.13	15.59	0			0.368	
8	8006	maximum			1.20	0.97	2.63	0		1	0.138	
						1.04	5.20	0			0.327	
8	8007	maximum			1.20	1.06	1.84	0		1	0.022	
						0.78	3.39	0			0.311	
8	8008	maximum			1.20	1.17	2.32	0		1	0.142	
						1.88	5.71	0			0.327	
8	8009	maximum			1.20	1.25	1.39	0		1	0.072	
						5.79	12.23	0			0.334	
8	8010	maximum			1.20	1.44	1.81	0		1	0.120	
						1.26	5.64	0			0.318	
8	8011	maximum			1.20	2.48	2.12	0		1	0.061	
						4.73	9.73	0			0.332	
8	8012	maximum			1.20	2.49	1.94	0		1	0.100	
						1.59	5.74	0			0.325	
8	8013	maximum			1.20	3.81	2.93	0		1	0.036	
						3.76	8.07	0			0.326	
8	8014	maximum			1.20	2.36	2.57	0		1	0.087	
						2.35	6.12	0			0.326	
8	8015	maximum			1.20	4.01	2.47	0		1	0.047	
						4.75	7.38	0			0.324	
8	8016	maximum			1.20	3.27	3.38	0		1	0.087	
						1.99	5.80	0			0.327	
8	8017	maximum			1.20	7.85	6.71	0		1	0.053	
						4.56	10.27	0			0.336	
8	8018	maximum			1.20	1.54	3.25	0		1	0.102	
						2.24	5.77	0			0.328	
8	8019	maximum			1.20	3.81	4.19	0		1	0.079	
						4.55	11.31	0			0.346	
8	8020	maximum			1.20	0.50	2.43	0		1	0.126	
						1.09	5.43	0			0.324	
8	8021	maximum			1.20	2.01	4.50	0		1	0.030	
						2.92	3.79	0			0.316	
8	8022	maximum			1.20	0.86	1.66	0		1	0.149	
						1.79	5.79	0			0.328	
8	8023	maximum			1.20	0.77	3.86	0		1	0.115	
						7.66	16.45	0			0.367	
8	8024	maximum			1.20	0.73	1.61	0		1	0.142	
						1.16	5.82	0			0.326	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN CRASH

REINFORCEMENT ACC. TO DIN1045-1 2008 in [cm2/m] upper/lower
General load safety factor - as defined in BEMESS: Gamma-f = 1.00
Shear: stresses VEd/d and VRd,ct/d with d=effective depth = h-hm
Shear index 2m = minimum shear reinforcement

Grp	ELEM No	LC No	MAT No	GEO No	h [m]	Reinforcement main	Reinforcement cross	dir	dphi deg	Shr zon	VEd/d [MPa] VRd,ct/d	Ass [cm2/m2]
8	8025	maximum			1.20	2.79	3.41	0		1	0.112	
						6.33	16.09	0			0.350	
8	8026	maximum			1.20	1.71	0.34	0		1	0.127	
						1.16	5.78	0			0.351	
8	8027	maximum			1.20	2.43	0.49	0		1	0.044	
						2.86	2.68	0			0.410	
8	8028	maximum			1.20	0.25	0.05	0		1	0.114	
						1.17	5.84	0			0.362	
8	8029	maximum			1.20	0.38	1.88	0		1	0.043	
						0.56	2.79	0			0.314	
8	8030	maximum			1.20	0.93	4.66	0		1	0.108	
						5.77	14.34	0			0.362	
8	8031	maximum			1.20	0.52	2.60	0		1	0.091	
						1.27	6.36	0			0.324	
8	8032	maximum			1.20	1.79	2.62	0		1	0.115	
						1.09	5.43	0			0.325	
8	8033	maximum			1.20	3.49	5.83	0		1	0.118	
						7.00	15.40	0			0.366	
8	8034	maximum			1.20	1.10	1.92	0		1	0.022	
						0.67	3.19	0			0.303	
8	8035	maximum			1.20	1.28	1.48	0		1	0.071	
						5.73	12.20	0			0.334	
8	8036	maximum			1.20	0.95	2.74	0		1	0.139	
						1.02	5.10	0			0.322	
8	8037	maximum			1.20	1.10	2.43	0		1	0.142	
						1.84	5.64	0			0.326	
8	8038	maximum			1.20	1.32	1.88	0		1	0.120	
						1.20	5.62	0			0.318	
8	8039	maximum			1.20	2.38	2.25	0		1	0.062	
						4.46	9.60	0			0.333	
8	8040	maximum			1.20	3.71	2.86	0		1	0.037	
						3.77	8.09	0			0.323	
8	8041	maximum			1.20	2.36	1.93	0		1	0.101	
						1.46	5.75	0			0.324	
8	8042	maximum			1.20	2.14	2.37	0		1	0.078	
						2.15	6.12	0			0.327	
8	8043	maximum			1.20	3.79	2.55	0		1	0.047	
						4.23	7.38	0			0.325	
8	8044	maximum			1.20	7.51	6.64	0		1	0.054	
						4.61	10.37	0			0.334	
8	8045	maximum			1.20	3.06	3.32	0		1	0.088	
						2.52	6.19	0			0.326	
8	8046	maximum			1.20	0.99	2.97	0		1	0.104	
						1.67	5.80	0			0.327	
8	8047	maximum			1.20	2.46	4.01	0		1	0.080	
						4.45	11.41	0			0.343	
8	8048	maximum			1.20	0.57	2.86	0		1	0.030	
						1.18	3.83	0			0.313	
8	8049	maximum			1.20	0.71	1.56	0		1	0.117	
						7.77	16.66	0			0.375	
8	8050	maximum			1.20	0.38	1.92	0		1	0.127	
						1.10	5.48	0			0.323	
8	8051	maximum			1.20	0.83	1.33	0		1	0.151	
						1.82	5.86	0			0.329	
8	8052	maximum			1.20	0.72	0.49	0		1	0.144	
						1.18	5.89	0			0.327	
8	8053	maximum			1.20	6.67	1.95	0		1	0.113	
						6.40	16.28	0			0.350	
8	8054	maximum			1.20	1.17	0.23	0		1	0.045	
						1.35	2.70	0			0.379	
8	8055	maximum			1.20	1.71	0.34	0		1	0.126	
						1.17	5.85	0			0.337	
8	8056	maximum			1.20	0.25	0.05	0		1	0.113	
						1.18	5.91	0			0.354	

Explanations shear state Shr zon:
1 = check without necessary shear reinforcement
2 = shear reinforcement required
m = minimum shear reinforcement
Acc. DIN 1045-1 10.3.4(2) the leverarm z was limited to d-2*nomc.

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN CRASH

Maximal values of the shear design
Only elements with shear reinforcement are printed.
At punching points punching reinforcement is printed.

element	ass [cm2/m2]	tau [MPa]	acc.VED/VRDmax	acc.cot_theta	min_z [m]
3067	6.65	0.40	0.110	1.75	0.141
3068	6.46	0.39	0.107	1.75	0.142
3080	9.16	0.55	0.152	1.75	0.145
3081	10.50	0.63	0.174	1.75	0.163
3082	10.54	0.63	0.175	1.75	0.163
3083	9.56	0.57	0.158	1.75	0.145
3166	8.57	0.51	0.142	1.75	0.145
3167	10.19	0.61	0.169	1.75	0.123
3168	11.21	0.67	0.183	1.75	0.123

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_CRASH_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Selected Beam Elements

FROM	TO	INC	X-VALUE	NC	MEMBER	CS0	CS1	CS2	CS3	CS4	CS5
10001											
10005											
10006											
10009											
10010											
10014											
10016											
10020											
10021											
10024											
10025											
10029											
12000	12150	1									

Default design code is DIN Fachbericht 102 Massivbrȯcken (2003) (Germany)
Klasse(Tab.4.118): D
Wind zone : Binnenland

Materials

No. 1 C 25/30 (DIN 1045-1)
No. 3 C 25/30 (DIN 1045-1)
No. 4 C 25/30 (DIN 1045-1)
No. 5 C 25/30 (DIN 1045-1)
No. 6 C 25/30 (DIN 1045-1)
No. 7 C 25/30 (DIN 1045-1)
No. 8 C 25/30 (DIN 1045-1)
No. 9 C 25/30 (DIN 1045-1)
No. 10 C 25/30 (DIN 1045-1)
No. 12 BSt 500 SA (DIN 1045-1)

Reinforcement will be accounted for sectional values as defined in AQUA
Reinforcements saved as design case LCR 515

Considered Load Cases

No.	refer	act on	Title/type of load case	gam-u	gam-f	psi-0	psi-1	psi-2	psi-1'	
1	part.	CS 1	I.B. ΚΑΤΑΚ.ΣΤΟΙΧΕΙΩΝ	1.35	1.00	1.00	1.00	1.00	1.00	G perm
			G (total dead load)							
2	part.	CS 1	I.B. ΔΟΚΩΝ	1.35	1.00	1.00	1.00	1.00	1.00	G perm
			G (total dead load)							
3	part.	CS 1	I.B. ΧΥΤΗΣ ΠΛΑΚΑΣ	1.35	1.00	1.00	1.00	1.00	1.00	G perm
			G (total dead load)							
11	part.	CS 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+0.5	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
12	part.	CS 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+0.5	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
13	part.	CS 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+1.0	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
14	part.	CS 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+1.0	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
100	part.	CS 1	CRASH_-Y_Pos.1 _DECK	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
			L_C (Traffic load UIC of EC/DIN-FB)							
101	part.	CS 1	CRASH_-Y_Pos.2 _DECK	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
			L_C (Traffic load UIC of EC/DIN-FB)							
102	part.	CS 1	CRASH_-Y_Pos.3 _DECK	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
			L_C (Traffic load UIC of EC/DIN-FB)							
103	part.	CS 1	CRASH_-Y_Pos.4 _DECK	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
			L_C (Traffic load UIC of EC/DIN-FB)							
104	part.	CS 1	CRASH_-Y_Pos.5 _DECK	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
			L_C (Traffic load UIC of EC/DIN-FB)							
105	part.	CS 1	CRASH_-Y_Pos.6 _DECK	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
			L_C (Traffic load UIC of EC/DIN-FB)							
106	part.	CS 1	CRASH_-Y_Pos.7 _DECK	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
			L_C (Traffic load UIC of EC/DIN-FB)							
107	part.	CS 1	CRASH_-Y_Pos.8 _DECK	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
			L_C (Traffic load UIC of EC/DIN-FB)							
108	part.	CS 1	CRASH_-Y_Pos.9 _DECK	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
			L_C (Traffic load UIC of EC/DIN-FB)							
109	part.	CS 1	CRASH_-Y_Pos.10 _DECK	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
			L_C (Traffic load UIC of EC/DIN-FB)							
110	part.	CS 1	CRASH_-Y_Pos.11 _DECK	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
			L_C (Traffic load UIC of EC/DIN-FB)							
111	part.	CS 1	CRASH_-Y_Pos.12 _DECK	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
			L_C (Traffic load UIC of EC/DIN-FB)							
1501	part.	CS 1	MAX-MY BEAM	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
1502	part.	CS 1	MIN-MY BEAM	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
1503	part.	CS 1	MAX-VZ BEAM	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
1504	part.	CS 1	MIN-VZ BEAM	1.50	0.00	0.75	0.75	0.20	0.80	Q excl

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_CRASH_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Considered Load Cases

No.	refer	act on	Title/type of load case	gam-u	gam-f	psi-0	psi-1	psi-2	psi-1'
			L (live loading)						
1505	part.	CS 1	MAX-MZ BEAM	1.50	0.00	0.75	0.75	0.20	0.80 Q excl
			L (live loading)						
1506	part.	CS 1	MIN-MZ BEAM	1.50	0.00	0.75	0.75	0.20	0.80 Q excl
			L (live loading)						
1507	part.	CS 1	MAX-VY BEAM	1.50	0.00	0.75	0.75	0.20	0.80 Q excl
			L (live loading)						
1508	part.	CS 1	MIN-VY BEAM	1.50	0.00	0.75	0.75	0.20	0.80 Q excl
			L (live loading)						
1509	part.	CS 1	MAX-N BEAM	1.50	0.00	0.75	0.75	0.20	0.80 Q excl
			L (live loading)						
1510	part.	CS 1	MIN-N BEAM	1.50	0.00	0.75	0.75	0.20	0.80 Q excl
			L (live loading)						
1511	part.	CS 1	MAX-MT BEAM	1.50	0.00	0.75	0.75	0.20	0.80 Q excl
			L (live loading)						
1512	part.	CS 1	MIN-MT BEAM	1.50	0.00	0.75	0.75	0.20	0.80 Q excl
			L (live loading)						

Ultimate Load Design

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Design for ultimate loads DIN Fachbericht 102 Massivbrȯcken (2003)
Biaxial bending, uniaxial stress calculated in y-z axis

Safety factors	SC-1	SC-2	SC-S	SS-1	SS-2	PIIa
	1.30	1.30	1.50	1.00	1.00	7

Strain limits	C1	C2	S1	S2	Z1	Z2
max	-3.50	-2.00	3.00	25.00	-3.50	25.00

parameters for reinforcements

Minimum reinforcements		compression	min. reinforcem.	maximum-
Bending.	Compress.	e/d	N/Npl	requ. section	reinforc.
0.00 [cm2]	0.30 [o/o]	3.50	0.0010	0.00	0.15	9.00

Tensile forces in the longitudinal reinforcements due to shear are NOT accounted for.
Material of sections uses Ultimate Limit strain-stress law with global safety factors
Material of reinforcements uses Ultimate Limit strain-stress law without safety factors

MNo.	temp lev.	Material-safety [-]	max.compr stress [MPa]	at strain [o/oo]	max.tens stress [MPa]	at strain [o/oo]	tension-stiffening [MPa]
1	0	1.300	-16.35	-2.00	0.00	0.00	
3	0	1.300	-16.35	-2.00	0.00	0.00	
4	0	1.300	-16.35	-2.00	0.00	0.00	
5	0	1.300	-16.35	-2.00	0.00	0.00	
6	0	1.300	-16.35	-2.00	0.00	0.00	
7	0	1.300	-16.35	-2.00	0.00	0.00	
8	0	1.300	-16.35	-2.00	0.00	0.00	
9	0	1.300	-16.35	-2.00	0.00	0.00	
10	0	1.300	-16.35	-2.00	0.00	0.00	
12	0	1.000	-525.00	-25.00	525.00	25.00	

Combinations For Ultimate Design

1513 (gross) max_my-1513
MAX + MY :
1.00 * G + 1.00 * L_A + 1.00 * L_B + 1.00 * L_C
1514 (gross) min_my-1514
MIN + MY :
1.00 * G + 1.00 * L_A + 1.00 * L_B + 1.00 * L_C

Shear Design

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Design for shear DIN 1045-1 (2003)
Minimum shear factor or tan of inclination of compressive struts 0.57 / 1.72

MNo	f-cd [MPa]	tau-rd [MPa]	sigIIQ [MPa]	sigIIT [MPa]	sigIIQ+ [MPa]	fyd [MPa]
1	14.17	0.10	10.62	7.44	10.62	
3	14.17	0.10	10.62	7.44	10.62	
4	14.17	0.10	10.62	7.44	10.62	
5	14.17	0.10	10.62	7.44	10.62	
6	14.17	0.10	10.62	7.44	10.62	
7	14.17	0.10	10.62	7.44	10.62	
8	14.17	0.10	10.62	7.44	10.62	
9	14.17	0.10	10.62	7.44	10.62	
10	14.17	0.10	10.62	7.44	10.62	
12						500.00

Tolerance for exceeding maximum shear or principal compression stress 0.0200

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_CRASH_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Longitudinal Reinforcements Accumulated minimum
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
10001	0.000	8	0.03	4.25		3.98T	0.14		0.14	
10001	0.200	8	0.02	3.29		3.29T				
10005	0.000	8	0.15	19.83		13.82T	4.08		1.93	
10005	0.200	8	0.14	18.71		15.67T	2.64		0.40	
10006	0.000	8	0.01	0.83		0.69T	0.14			
10006	0.400	8	0.02	2.47		1.24T	0.19		1.04	
10009	0.000	8	0.02	2.00		1.60T	0.40			
10009	0.400	8	0.02	2.01		1.67T	0.34			
10010	0.000	8	0.13	17.21		15.37T	0.78		1.06	
10010	0.200	8	0.14	18.85		15.31T			3.55	
10014	0.000	8	0.02	3.28		3.28T				
10014	0.200	8	0.03	3.60		3.39T	0.11		0.11	
10016	0.000	8	0.03	4.22		3.94T	0.14		0.14	
10016	0.200	8	0.03	3.56		3.56T				
10020	0.000	8	0.15	19.60		13.74T	4.01		1.85	
10020	0.200	8	0.14	18.60		15.56T	2.63		0.40	
10021	0.000	8	0.01	1.92		0.74T	1.18			
10021	0.400	8	0.02	2.44		1.25T	0.19		1.01	
10024	0.000	8	0.01	1.87		1.50T	0.37			
10024	0.400	8	0.01	1.87		1.53T	0.34			
10025	0.000	8	0.13	16.87		15.05T	0.75		1.07	
10025	0.200	8	0.15	20.06		16.54T			3.52	
10029	0.000	8	0.03	3.43		3.43T				
10029	0.200	8	0.03	3.55		3.35T	0.10		0.10	
12001	0.000	9	0.69	34.65			34.65T			
12001	1.000	9	0.41	20.40			20.40T			
12002	0.000	9	0.41	20.40			20.40T			
12002	1.000	9	0.48	24.24			24.24T			
12003	0.000	9	0.48	24.24			24.24T			
12003	1.000	9	0.98	49.36			49.36T			
12004	0.000	9	0.30	15.08			15.08T			
12004	1.000	9	0.30	15.08			15.08T			
12005	0.000	9	0.30	15.08			15.08T			
12005	1.000	9	0.49	24.68			24.68T			
12006	0.000	9	0.49	24.68			24.68T			
12006	1.000	9	0.97	48.95			48.95T			
12007	0.000	9	0.40	20.22			20.22T			
12007	1.000	9	0.30	15.08			15.08T			
12008	0.000	9	0.30	15.08			15.08T			
12008	1.000	9	0.49	24.60			24.60T			
12009	0.000	9	0.49	24.60			24.60T			
12009	1.000	9	0.97	48.90			48.90T			
12010	0.000	9	0.69	34.65			34.65T			
12010	1.000	9	0.41	20.40			20.40T			
12011	0.000	9	0.41	20.40			20.40T			
12011	1.000	9	0.48	24.24			24.24T			
12012	0.000	9	0.48	24.24			24.24T			
12012	1.000	9	0.97	48.90			48.90T			
12013	0.000	9	0.31	15.65			15.65T			
12013	1.000	9	0.30	15.08			15.08T			
12014	0.000	9	0.30	15.08			15.08T			
12014	1.000	9	0.48	24.31			24.31T			
12015	0.000	9	0.48	24.31			24.31T			
12015	1.000	9	0.97	48.95			48.95T			
12016	0.000	9	0.46	22.97			22.97T			
12016	1.000	9	0.31	15.70			15.70T			
12017	0.000	9	0.31	15.70			15.70T			
12017	1.000	9	0.49	24.60			24.60T			
12018	0.000	9	0.49	24.60			24.60T			
12018	1.000	9	0.98	49.36			49.36T			
12019	0.000	9	0.98	49.36			49.36T			
12019	1.000	9	1.40	70.29			70.29T			
12020	0.000	9	1.40	70.23			70.23T			
12020	1.000	9	1.47	74.07			74.07T			
12021	0.000	9	1.47	74.03			74.03T			
12021	1.000	9	1.31	65.97			65.97T			
12022	0.000	9	1.31	65.94			65.94T			
12022	1.000	9	1.04	52.06			52.06T			
12023	0.000	9	1.04	52.04			52.04T			
12023	1.000	9	0.72	36.21			36.21T			
12024	0.000	9	0.72	36.20			36.20T			
12024	1.000	9	0.42	21.35			21.35T			
12025	0.000	9	0.42	21.35			21.35T			
12025	1.000	9	0.31	15.68			15.68T			
12026	0.000	9	0.31	15.68			15.68T			
12026	1.000	9	0.30	15.08			15.08T			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_CRASH_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Longitudinal Reinforcements Accumulated minimum
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
12027	0.000	9	0.30	15.08			15.08T			
12027	1.000	9	0.30	15.08			15.08T			
12028	0.000	9	0.30	15.08			15.08T			
12028	1.000	9	0.30	15.08			15.08T			
12029	0.000	9	0.30	15.08			15.08T			
12029	1.000	9	0.30	15.08			15.08T			
12030	0.000	9	0.30	15.08			15.08T			
12030	1.000	9	0.30	15.08			15.08T			
12031	0.000	9	0.30	15.08			15.08T			
12031	1.000	9	0.30	15.08			15.08T			
12032	0.000	9	0.30	15.08			15.08T			
12032	1.000	9	0.30	15.08			15.08T			
12033	0.000	9	0.30	15.08			15.08T			
12033	1.000	9	0.30	15.08			15.08T			
12034	0.000	9	0.30	15.08			15.08T			
12034	1.000	9	0.30	15.08			15.08T			
12035	0.000	9	0.30	15.08			15.08T			
12035	1.000	9	0.30	15.08			15.08T			
12036	0.000	9	0.30	15.08			15.08T			
12036	1.000	9	0.30	15.08			15.08T			
12037	0.000	9	0.30	15.08			15.08T			
12037	1.000	9	0.30	15.08			15.08T			
12038	0.000	9	0.30	15.08			15.08T			
12038	1.000	9	0.30	15.08			15.08T			
12039	0.000	9	0.30	15.08			15.08T			
12039	1.000	9	0.30	15.08			15.08T			
12040	0.000	9	0.30	15.08			15.08			
12040	1.000	9	0.30	15.08			15.08			
12041	0.000	9	0.97	48.95			48.95T			
12041	1.000	9	1.39	69.85			69.85T			
12042	0.000	9	1.39	69.79			69.79T			
12042	1.000	9	1.47	73.69			73.69T			
12043	0.000	9	1.47	73.65			73.65T			
12043	1.000	9	1.31	65.97			65.97T			
12044	0.000	9	1.31	65.94			65.94T			
12044	1.000	9	1.04	52.05			52.05T			
12045	0.000	9	1.04	52.03			52.03T			
12045	1.000	9	0.72	36.19			36.19T			
12046	0.000	9	0.72	36.19			36.19T			
12046	1.000	9	0.42	21.33			21.33T			
12047	0.000	9	0.42	21.33			21.33T			
12047	1.000	9	0.30	15.08			15.08T			
12048	0.000	9	0.30	15.08			15.08T			
12048	1.000	9	0.30	15.08			15.08T			
12049	0.000	9	0.30	15.08			15.08T			
12049	1.000	9	0.30	15.08			15.08T			
12050	0.000	9	0.30	15.08			15.08T			
12050	1.000	9	0.30	15.08			15.08T			
12051	0.000	9	0.30	15.08			15.08T			
12051	1.000	9	0.30	15.08			15.08T			
12052	0.000	9	0.30	15.08			15.08T			
12052	1.000	9	0.30	15.08			15.08T			
12053	0.000	9	0.30	15.08			15.08T			
12053	1.000	9	0.30	15.08			15.08T			
12054	0.000	9	0.30	15.08			15.08T			
12054	1.000	9	0.30	15.08			15.08T			
12055	0.000	9	0.30	15.08			15.08T			
12055	1.000	9	0.30	15.08			15.08T			
12056	0.000	9	0.30	15.08			15.08T			
12056	1.000	9	0.30	15.08			15.08T			
12057	0.000	9	0.30	15.08			15.08T			
12057	1.000	9	0.30	15.08			15.08T			
12058	0.000	9	0.30	15.08			15.08T			
12058	1.000	9	0.30	15.08			15.08T			
12059	0.000	9	0.30	15.08			15.08T			
12059	1.000	9	0.30	15.08			15.08T			
12060	0.000	9	0.30	15.08			15.08T			
12060	1.000	9	0.30	15.08			15.08T			
12061	0.000	9	0.30	15.08			15.08T			
12061	1.000	9	0.30	15.08			15.08T			
12062	0.000	9	0.30	15.08			15.08'			
12062	1.000	9	0.30	15.08			15.08'			
12063	0.000	9	0.97	48.90			48.90T			
12063	1.000	9	1.39	69.82			69.82T			
12064	0.000	9	1.39	69.76			69.76T			
12064	1.000	9	1.47	73.68			73.68T			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_CRASH_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Longitudinal Reinforcements Accumulated minimum
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
12065	0.000	9	1.46	73.63			73.63T			
12065	1.000	9	1.32	66.24			66.24T			
12066	0.000	9	1.32	66.21			66.21T			
12066	1.000	9	1.04	52.19			52.19T			
12067	0.000	9	1.04	52.17			52.17T			
12067	1.000	9	0.72	36.21			36.21T			
12068	0.000	9	0.72	36.20			36.20T			
12068	1.000	9	0.42	21.25			21.25T			
12069	0.000	9	0.42	21.24			21.24T			
12069	1.000	9	0.30	15.08			15.08T			
12070	0.000	9	0.30	15.08			15.08T			
12070	1.000	9	0.30	15.08			15.08T			
12071	0.000	9	0.30	15.08			15.08T			
12071	1.000	9	0.30	15.08			15.08T			
12072	0.000	9	0.30	15.08			15.08T			
12072	1.000	9	0.30	15.08			15.08T			
12073	0.000	9	0.30	15.08			15.08T			
12073	1.000	9	0.30	15.08			15.08T			
12074	0.000	9	0.30	15.08			15.08T			
12074	1.000	9	0.30	15.08			15.08T			
12075	0.000	9	0.30	15.08			15.08T			
12075	1.000	9	0.30	15.08			15.08T			
12076	0.000	9	0.30	15.08			15.08T			
12076	1.000	9	0.30	15.08			15.08T			
12077	0.000	9	0.30	15.08			15.08T			
12077	1.000	9	0.30	15.08			15.08T			
12078	0.000	9	0.30	15.08			15.08T			
12078	1.000	9	0.30	15.08			15.08T			
12079	0.000	9	0.30	15.08			15.08T			
12079	1.000	9	0.30	15.08			15.08T			
12080	0.000	9	0.30	15.08			15.08T			
12080	1.000	9	0.30	15.08			15.08T			
12081	0.000	9	0.30	15.08			15.08T			
12081	1.000	9	0.30	15.08			15.08T			
12082	0.000	9	0.30	15.08			15.08T			
12082	1.000	9	0.30	15.08			15.08T			
12083	0.000	9	0.30	15.08			15.08T			
12083	1.000	9	0.30	15.08			15.08T			
12084	0.000	9	0.30	15.08			15.08'			
12084	1.000	9	0.30	15.08			15.08'			
12085	0.000	9	0.97	48.90			48.90T			
12085	1.000	9	1.39	69.82			69.82T			
12086	0.000	9	1.39	69.76			69.76T			
12086	1.000	9	1.47	73.68			73.68T			
12087	0.000	9	1.46	73.63			73.63T			
12087	1.000	9	1.32	66.24			66.24T			
12088	0.000	9	1.32	66.21			66.21T			
12088	1.000	9	1.04	52.06			52.06T			
12089	0.000	9	1.04	52.04			52.04T			
12089	1.000	9	0.72	36.21			36.21T			
12090	0.000	9	0.72	36.20			36.20T			
12090	1.000	9	0.42	21.35			21.35T			
12091	0.000	9	0.42	21.35			21.35T			
12091	1.000	9	0.31	15.36			15.36T			
12092	0.000	9	0.31	15.36			15.36T			
12092	1.000	9	0.30	15.08			15.08T			
12093	0.000	9	0.30	15.08			15.08T			
12093	1.000	9	0.30	15.08			15.08T			
12094	0.000	9	0.30	15.08			15.08T			
12094	1.000	9	0.30	15.08			15.08T			
12095	0.000	9	0.30	15.08			15.08T			
12095	1.000	9	0.30	15.08			15.08T			
12096	0.000	9	0.30	15.08			15.08T			
12096	1.000	9	0.30	15.08			15.08T			
12097	0.000	9	0.30	15.08			15.08T			
12097	1.000	9	0.30	15.08			15.08T			
12098	0.000	9	0.30	15.08			15.08T			
12098	1.000	9	0.30	15.08			15.08T			
12099	0.000	9	0.30	15.08			15.08T			
12099	1.000	9	0.30	15.08			15.08T			
12100	0.000	9	0.30	15.08			15.08T			
12100	1.000	9	0.30	15.08			15.08T			
12101	0.000	9	0.30	15.08			15.08T			
12101	1.000	9	0.30	15.08			15.08T			
12102	0.000	9	0.30	15.08			15.08T			
12102	1.000	9	0.30	15.08			15.08T			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_CRASH_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Longitudinal Reinforcements Accumulated minimum
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
12103	0.000	9	0.30	15.08			15.08T			
12103	1.000	9	0.30	15.08			15.08T			
12104	0.000	9	0.30	15.08			15.08T			
12104	1.000	9	0.30	15.08			15.08T			
12105	0.000	9	0.30	15.08			15.08T			
12105	1.000	9	0.30	15.08			15.08T			
12106	0.000	9	0.30	15.08			15.08			
12106	1.000	9	0.30	15.08			15.08			
12107	0.000	9	0.97	48.95			48.95T			
12107	1.000	9	1.39	69.85			69.85T			
12108	0.000	9	1.39	69.79			69.79T			
12108	1.000	9	1.47	73.69			73.69T			
12109	0.000	9	1.47	73.65			73.65T			
12109	1.000	9	1.32	66.23			66.23T			
12110	0.000	9	1.32	66.21			66.21T			
12110	1.000	9	1.04	52.17			52.17T			
12111	0.000	9	1.04	52.16			52.16T			
12111	1.000	9	0.72	36.19			36.19T			
12112	0.000	9	0.72	36.19			36.19T			
12112	1.000	9	0.42	21.23			21.23T			
12113	0.000	9	0.42	21.23			21.23T			
12113	1.000	9	0.30	15.08			15.08T			
12114	0.000	9	0.30	15.08			15.08T			
12114	1.000	9	0.30	15.08			15.08T			
12115	0.000	9	0.30	15.08			15.08T			
12115	1.000	9	0.30	15.08			15.08T			
12116	0.000	9	0.30	15.08			15.08T			
12116	1.000	9	0.30	15.08			15.08T			
12117	0.000	9	0.30	15.08			15.08T			
12117	1.000	9	0.30	15.08			15.08T			
12118	0.000	9	0.30	15.08			15.08T			
12118	1.000	9	0.30	15.08			15.08T			
12119	0.000	9	0.30	15.08			15.08T			
12119	1.000	9	0.30	15.08			15.08T			
12120	0.000	9	0.30	15.08			15.08T			
12120	1.000	9	0.30	15.08			15.08T			
12121	0.000	9	0.30	15.08			15.08T			
12121	1.000	9	0.30	15.08			15.08T			
12122	0.000	9	0.30	15.08			15.08T			
12122	1.000	9	0.30	15.08			15.08T			
12123	0.000	9	0.30	15.08			15.08T			
12123	1.000	9	0.30	15.08			15.08T			
12124	0.000	9	0.30	15.08			15.08T			
12124	1.000	9	0.30	15.08			15.08T			
12125	0.000	9	0.30	15.08			15.08T			
12125	1.000	9	0.30	15.08			15.08T			
12126	0.000	9	0.30	15.08			15.08T			
12126	1.000	9	0.30	15.08			15.08T			
12127	0.000	9	0.30	15.08			15.08T			
12127	1.000	9	0.30	15.08			15.08T			
12128	0.000	9	0.30	15.08			15.08'			
12128	1.000	9	0.30	15.08			15.08'			
12129	0.000	9	0.98	49.36			49.36T			
12129	1.000	9	1.40	70.29			70.29T			
12130	0.000	9	1.40	70.23			70.23T			
12130	1.000	9	1.47	74.07			74.07T			
12131	0.000	9	1.47	74.03			74.03T			
12131	1.000	9	1.31	65.97			65.97T			
12132	0.000	9	1.31	65.94			65.94T			
12132	1.000	9	1.04	52.06			52.06T			
12133	0.000	9	1.04	52.04			52.04T			
12133	1.000	9	0.72	36.21			36.21T			
12134	0.000	9	0.72	36.20			36.20T			
12134	1.000	9	0.42	21.25			21.25T			
12135	0.000	9	0.42	21.24			21.24T			
12135	1.000	9	0.30	15.08			15.08T			
12136	0.000	9	0.30	15.08			15.08T			
12136	1.000	9	0.30	15.08			15.08T			
12137	0.000	9	0.30	15.08			15.08T			
12137	1.000	9	0.30	15.08			15.08T			
12138	0.000	9	0.30	15.08			15.08T			
12138	1.000	9	0.30	15.08			15.08T			
12139	0.000	9	0.30	15.08			15.08T			
12139	1.000	9	0.30	15.08			15.08T			
12140	0.000	9	0.30	15.08			15.08T			
12140	1.000	9	0.30	15.08			15.08T			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_CRASH_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Longitudinal Reinforcements Accumulated minimum
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
12141	0.000	9	0.30	15.08			15.08T			
12141	1.000	9	0.30	15.08			15.08T			
12142	0.000	9	0.30	15.08			15.08T			
12142	1.000	9	0.30	15.08			15.08T			
12143	0.000	9	0.30	15.08			15.08T			
12143	1.000	9	0.30	15.08			15.08T			
12144	0.000	9	0.30	15.08			15.08T			
12144	1.000	9	0.30	15.08			15.08T			
12145	0.000	9	0.30	15.08			15.08T			
12145	1.000	9	0.30	15.08			15.08T			
12146	0.000	9	0.30	15.08			15.08T			
12146	1.000	9	0.30	15.08			15.08T			
12147	0.000	9	0.30	15.08			15.08T			
12147	1.000	9	0.30	15.08			15.08T			
12148	0.000	9	0.30	15.08			15.08T			
12148	1.000	9	0.30	15.08			15.08T			
12149	0.000	9	0.30	15.08			15.08T			
12149	1.000	9	0.30	15.08			15.08T			
12150	0.000	9	0.30	15.08			15.08'			
12150	1.000	9	0.30	15.08			15.08'			

Shear Reinforcements per Cutted Part of Section Accumulated minimum

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
10001	0.000	8	0.93	1.76				
10001	0.200	8	0.80	1.89				
10005	0.000	8	3.35	5.83				
10005	0.200	8	3.65	6.87				
10006	0.000	8	0.24	5.43				
10006	0.400	8	0.30	5.57				
10009	0.000	8	0.44	4.48				
10009	0.400	8	0.39	4.55				
10010	0.000	8	3.58	7.12				
10010	0.200	8	3.71	8.02				
10014	0.000	8	0.80	1.45				
10014	0.200	8	0.80	1.33				
10016	0.000	8	0.92	1.76				
10016	0.200	8	0.86	1.89				
10020	0.000	8	3.34	5.62				
10020	0.200	8	3.63	6.65				
10021	0.000	8	0.25	5.24				
10021	0.400	8	0.30	5.38				
10024	0.000	8	0.36	3.98				
10024	0.400	8	0.36	4.44				
10025	0.000	8	3.51	7.07				
10025	0.200	8	4.02	8.06				
10029	0.000	8	0.83	1.46				
10029	0.200	8	0.79	1.31				
12001	0.000	9	0.01	5.20				
12001	1.000	9	0.01	5.40				
12002	0.000	9	0.01	5.40				
12002	1.000	9	0.01	6.60				
12003	0.000	9	0.01	6.60				
12003	1.000	9	0.00	9.13				
12004	0.000	9	0.00	5.11				
12004	1.000	9	0.00	5.12				
12005	0.000	9	0.00	5.12				
12005	1.000	9	0.00	6.57				
12006	0.000	9	0.00	6.57				
12006	1.000	9	0.01	10.21				
12007	0.000	9	0.00	5.13				
12007	1.000	9	0.00	5.12				
12008	0.000	9	0.00	5.12				
12008	1.000	9	0.00	6.58				
12009	0.000	9	0.00	6.58				
12009	1.000	9	0.01	9.68				
12010	0.000	9	0.01	5.21				
12010	1.000	9	0.01	5.41				
12011	0.000	9	0.00	5.41				
12011	1.000	9	0.01	6.60				
12012	0.000	9	0.00	6.60				
12012	1.000	9	0.01	9.14				
12013	0.000	9	0.00	5.11				
12013	1.000	9	0.00	5.13				
12014	0.000	9	0.00	5.13				
12014	1.000	9	0.00	6.59				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_CRASH_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Shear Reinforcements per Cutted Part of Section Accumulated minimum

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
12015	0.000	9	0.00	6.59				
12015	1.000	9	0.00	9.13				
12016	0.000	9	0.00	5.13				
12016	1.000	9	0.00	5.12				
12017	0.000	9	0.00	5.12				
12017	1.000	9	0.00	6.58				
12018	0.000	9	0.00	6.58				
12018	1.000	9	0.00	9.13				
12019	0.000	9	0.00	9.13				
12019	1.000	9	0.00	5.17				
12020	0.000	9	0.00	5.17				
12020	1.000	9	0.00	5.16				
12021	0.000	9	0.00	5.16				
12021	1.000	9	0.00	5.16				
12022	0.000	9	0.00	5.16				
12022	1.000	9	0.00	5.19				
12023	0.000	9	0.00	5.19				
12023	1.000	9	0.00	5.25				
12024	0.000	9	0.00	5.25				
12024	1.000	9	0.00	5.34				
12025	0.000	9	0.00	5.34				
12025	1.000	9	0.00	5.49				
12026	0.000	9	0.00	5.49				
12026	1.000	9	0.00	5.61				
12027	0.000	9	0.00	5.61				
12027	1.000	9	0.00	5.71				
12028	0.000	9	0.00	5.71				
12028	1.000	9	0.00	5.74				
12029	0.000	9	0.00	5.74				
12029	1.000	9	0.00	4.58				
12030	0.000	9	0.00	4.58				
12030	1.000	9	0.00	5.74				
12031	0.000	9	0.00	5.74				
12031	1.000	9	0.00	5.74				
12032	0.000	9	0.00	5.74				
12032	1.000	9	0.00	5.74				
12033	0.000	9	0.00	5.74				
12033	1.000	9	0.00	5.74				
12034	0.000	9	0.00	5.74				
12034	1.000	9	0.00	5.74				
12035	0.000	9	0.00	5.74				
12035	1.000	9	0.00	5.74				
12036	0.000	9	0.00	5.74				
12036	1.000	9	0.00	5.74				
12037	0.000	9	0.00	5.74				
12037	1.000	9	0.00	5.74				
12038	0.000	9	0.00	5.74				
12038	1.000	9	0.00	5.74				
12039	0.000	9	0.00	5.74				
12039	1.000	9	0.00	4.58				
12040	0.000	9	0.00	11.73				
12040	1.000	9	0.00	4.58				
12041	0.000	9	0.01	10.21				
12041	1.000	9	0.00	5.09				
12042	0.000	9	0.00	5.09				
12042	1.000	9	0.00	5.05				
12043	0.000	9	0.00	5.07				
12043	1.000	9	0.00	5.07				
12044	0.000	9	0.00	5.07				
12044	1.000	9	0.00	5.10				
12045	0.000	9	0.00	5.10				
12045	1.000	9	0.00	5.15				
12046	0.000	9	0.00	5.15				
12046	1.000	9	0.00	5.09				
12047	0.000	9	0.00	5.09				
12047	1.000	9	0.00	5.05				
12048	0.000	9	0.00	5.05				
12048	1.000	9	0.00	4.84				
12049	0.000	9	0.00	4.84				
12049	1.000	9	0.00	4.64				
12050	0.000	9	0.00	4.64				
12050	1.000	9	0.00	4.58				
12051	0.000	9	0.00	4.58				
12051	1.000	9	0.00	4.58				
12052	0.000	9	0.00	4.58				
12052	1.000	9	0.00	4.58				
12053	0.000	9	0.00	4.58				
12053	1.000	9	0.00	4.58				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_CRASH_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Shear Reinforcements per Cutted Part of Section Accumulated minimum

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
12054	0.000	9	0.00	4.58				
12054	1.000	9	0.00	4.58				
12055	0.000	9	0.00	4.58				
12055	1.000	9	0.00	4.58				
12056	0.000	9	0.00	4.58				
12056	1.000	9	0.00	4.58				
12057	0.000	9	0.00	4.58				
12057	1.000	9	0.00	4.58				
12058	0.000	9	0.00	4.58				
12058	1.000	9	0.00	4.58				
12059	0.000	9	0.00	4.58				
12059	1.000	9	0.00	4.58				
12060	0.000	9	0.00	4.58				
12060	1.000	9	0.00	4.58				
12061	0.000	9	0.00	4.58				
12061	1.000	9	0.00	4.58				
12062	0.000	9	0.00	4.58				
12062	1.000	9	0.00	4.58				
12063	0.000	9	0.01	9.68				
12063	1.000	9	0.00	5.05				
12064	0.000	9	0.00	5.05				
12064	1.000	9	0.00	5.03				
12065	0.000	9	0.00	5.03				
12065	1.000	9	0.00	5.05				
12066	0.000	9	0.00	5.05				
12066	1.000	9	0.00	5.02				
12067	0.000	9	0.00	5.02				
12067	1.000	9	0.00	5.03				
12068	0.000	9	0.00	5.03				
12068	1.000	9	0.00	5.07				
12069	0.000	9	0.00	5.07				
12069	1.000	9	0.00	5.05				
12070	0.000	9	0.00	5.05				
12070	1.000	9	0.00	4.83				
12071	0.000	9	0.00	4.83				
12071	1.000	9	0.00	4.64				
12072	0.000	9	0.00	4.64				
12072	1.000	9	0.00	4.58				
12073	0.000	9	0.00	4.58				
12073	1.000	9	0.00	4.58				
12074	0.000	9	0.00	4.58				
12074	1.000	9	0.00	4.58				
12075	0.000	9	0.00	4.58				
12075	1.000	9	0.00	4.58				
12076	0.000	9	0.00	4.58				
12076	1.000	9	0.00	4.58				
12077	0.000	9	0.00	4.58				
12077	1.000	9	0.00	4.58				
12078	0.000	9	0.00	4.58				
12078	1.000	9	0.00	4.58				
12079	0.000	9	0.00	4.58				
12079	1.000	9	0.00	4.58				
12080	0.000	9	0.00	4.58				
12080	1.000	9	0.00	4.58				
12081	0.000	9	0.00	4.58				
12081	1.000	9	0.00	4.58				
12082	0.000	9	0.00	4.58				
12082	1.000	9	0.00	4.58				
12083	0.000	9	0.00	4.58				
12083	1.000	9	0.00	4.58				
12084	0.000	9	0.00	4.58				
12084	1.000	9	0.00	4.58				
12085	0.000	9	0.00	9.14				
12085	1.000	9	0.00	5.29				
12086	0.000	9	0.00	5.29				
12086	1.000	9	0.00	5.25				
12087	0.000	9	0.00	5.25				
12087	1.000	9	0.00	5.26				
12088	0.000	9	0.00	5.26				
12088	1.000	9	0.00	5.19				
12089	0.000	9	0.00	5.19				
12089	1.000	9	0.00	5.25				
12090	0.000	9	0.00	5.25				
12090	1.000	9	0.00	5.34				
12091	0.000	9	0.00	5.34				
12091	1.000	9	0.00	5.49				
12092	0.000	9	0.00	5.49				
12092	1.000	9	0.00	5.62				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_CRASH_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Shear Reinforcements per Cutted Part of Section Accumulated minimum

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
12093	0.000	9	0.00	5.62				
12093	1.000	9	0.00	5.71				
12094	0.000	9	0.00	5.71				
12094	1.000	9	0.00	5.74				
12095	0.000	9	0.00	5.74				
12095	1.000	9	0.00	4.58				
12096	0.000	9	0.00	4.58				
12096	1.000	9	0.00	5.74				
12097	0.000	9	0.00	5.74				
12097	1.000	9	0.00	5.74				
12098	0.000	9	0.00	5.74				
12098	1.000	9	0.00	5.74				
12099	0.000	9	0.00	5.74				
12099	1.000	9	0.00	5.74				
12100	0.000	9	0.00	5.74				
12100	1.000	9	0.00	5.74				
12101	0.000	9	0.00	5.74				
12101	1.000	9	0.00	5.74				
12102	0.000	9	0.00	5.74				
12102	1.000	9	0.00	5.74				
12103	0.000	9	0.00	5.74				
12103	1.000	9	0.00	5.74				
12104	0.000	9	0.00	5.74				
12104	1.000	9	0.00	5.74				
12105	0.000	9	0.00	5.74				
12105	1.000	9	0.00	12.81				
12106	0.000	9	0.00	11.73				
12106	1.000	9	0.00	4.58				
12107	0.000	9	0.00	9.13				
12107	1.000	9	0.00	5.12				
12108	0.000	9	0.00	5.12				
12108	1.000	9	0.00	5.10				
12109	0.000	9	0.00	5.07				
12109	1.000	9	0.00	5.14				
12110	0.000	9	0.00	5.14				
12110	1.000	9	0.00	5.12				
12111	0.000	9	0.00	5.12				
12111	1.000	9	0.00	5.07				
12112	0.000	9	0.00	5.07				
12112	1.000	9	0.00	5.07				
12113	0.000	9	0.00	5.07				
12113	1.000	9	0.00	5.05				
12114	0.000	9	0.00	5.05				
12114	1.000	9	0.00	4.83				
12115	0.000	9	0.00	4.83				
12115	1.000	9	0.00	4.64				
12116	0.000	9	0.00	4.64				
12116	1.000	9	0.00	4.58				
12117	0.000	9	0.00	4.58				
12117	1.000	9	0.00	4.58				
12118	0.000	9	0.00	4.58				
12118	1.000	9	0.00	4.58				
12119	0.000	9	0.00	4.58				
12119	1.000	9	0.00	4.58				
12120	0.000	9	0.00	4.58				
12120	1.000	9	0.00	4.58				
12121	0.000	9	0.00	4.58				
12121	1.000	9	0.00	4.58				
12122	0.000	9	0.00	4.58				
12122	1.000	9	0.00	4.58				
12123	0.000	9	0.00	4.58				
12123	1.000	9	0.00	4.58				
12124	0.000	9	0.00	4.58				
12124	1.000	9	0.00	4.58				
12125	0.000	9	0.00	4.58				
12125	1.000	9	0.00	4.58				
12126	0.000	9	0.00	4.58				
12126	1.000	9	0.00	4.58				
12127	0.000	9	0.00	4.58				
12127	1.000	9	0.00	4.58				
12128	0.000	9	0.00	4.58				
12128	1.000	9	0.00	4.58				
12129	0.000	9	0.00	9.13				
12129	1.000	9	0.00	5.10				
12130	0.000	9	0.00	5.10				
12130	1.000	9	0.00	5.09				
12131	0.000	9	0.00	5.09				
12131	1.000	9	0.00	5.07				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_CRASH_ΠΑΣΣΑΛΟΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Shear Reinforcements per Cutted Part of Section Accumulated minimum

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
12132	0.000	9	0.00	5.07				
12132	1.000	9	0.00	5.04				
12133	0.000	9	0.00	5.04				
12133	1.000	9	0.00	5.03				
12134	0.000	9	0.00	5.03				
12134	1.000	9	0.00	5.07				
12135	0.000	9	0.00	5.07				
12135	1.000	9	0.00	5.05				
12136	0.000	9	0.00	5.05				
12136	1.000	9	0.00	4.83				
12137	0.000	9	0.00	4.83				
12137	1.000	9	0.00	4.64				
12138	0.000	9	0.00	4.64				
12138	1.000	9	0.00	4.58				
12139	0.000	9	0.00	4.58				
12139	1.000	9	0.00	4.58				
12140	0.000	9	0.00	4.58				
12140	1.000	9	0.00	4.58				
12141	0.000	9	0.00	4.58				
12141	1.000	9	0.00	4.58				
12142	0.000	9	0.00	4.58				
12142	1.000	9	0.00	4.58				
12143	0.000	9	0.00	4.58				
12143	1.000	9	0.00	4.58				
12144	0.000	9	0.00	4.58				
12144	1.000	9	0.00	4.58				
12145	0.000	9	0.00	4.58				
12145	1.000	9	0.00	4.58				
12146	0.000	9	0.00	4.58				
12146	1.000	9	0.00	4.58				
12147	0.000	9	0.00	4.58				
12147	1.000	9	0.00	4.58				
12148	0.000	9	0.00	4.58				
12148	1.000	9	0.00	4.58				
12149	0.000	9	0.00	4.58				
12149	1.000	9	0.00	4.58				
12150	0.000	9	0.00	4.58				
12150	1.000	9	0.00	4.58				

Maximum Degree of Utilization

		N sig-c	Vy sig-t	Vz tau	Mt sig-*	My tend.	Mz As-l	Mb As-v	Mt2 crack	Total sigdyn	lamda tau-*
Cross sect.	8	0.000	0.000	0.118	0.144	0.000	0.000	0.000	0.000	1.001	0.000
DOKOS-4		0.000	0.000	0.000	0.000	0.000	1.001	20.266	0.000	0.000	0.000
Cross sect.	9	0.000	0.000	0.212	0.000	0.000	0.000	0.000	0.000	1.000	0.000
section pile		0.000	0.000	0.000	0.000	0.000	1.000	0.869	0.000	0.000	0.000
Total System		0.000	0.000	0.212	0.144	0.000	0.000	0.000	0.000	1.001	0.000
		0.000	0.000	0.000	0.000	0.000	1.001	20.266	0.000	0.000	0.000

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_CRASH_DOKOI

Selected Beam Elements

FROM	TO	INC	X-VALUE	NC	MEMBER	CS0	CS1	CS2	CS3	CS4	CS5
1000	1060	1		1	bending	10	40				
2000	2020	1									

Default design code is DIN Fachbericht 102 Massivbrȯcken (2003) (Germany)
Klasse(Tab.4.118): D
Wind zone : Binnenland

Materials

No. 1 C 25/30 (DIN 1045-1)
No. 3 C 25/30 (DIN 1045-1)
No. 4 C 25/30 (DIN 1045-1)
No. 5 C 25/30 (DIN 1045-1)
No. 6 C 25/30 (DIN 1045-1)
No. 7 C 25/30 (DIN 1045-1)
No. 8 C 25/30 (DIN 1045-1)
No. 9 C 25/30 (DIN 1045-1)
No. 10 C 25/30 (DIN 1045-1)
No. 12 BSt 500 SA (DIN 1045-1)

Reinforcement will be accounted for sectional values as defined in AQUA
Reinforcements saved as design case LCR 516

Considered Load Cases

No.	refer	act on	Title/type of load case	gam-u	gam-f	psi-0	psi-1	psi-2	psi-1'	
1	part.	CS 0	I.B. ΚΑΤΑΚ.ΣΤΟΙΧΕΙΩΝ	1.35	1.00	1.00	1.00	1.00	1.00	G perm
			G (total dead load)							
2	part.	CS 0	I.B. ΔΟΚΩΝ	1.35	1.00	1.00	1.00	1.00	1.00	G perm
			G (total dead load)							
3	part.	CS 0	I.B. ΧΥΤΗΣ ΠΛΑΚΑΣ	1.35	1.00	1.00	1.00	1.00	1.00	G perm
			G (total dead load)							
11	part.	CS 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+0.5	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
12	part.	CS 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+0.5	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
13	part.	CS 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+1.0	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
14	part.	CS 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+1.0	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
100	part.	CS 1	CRASH_-Y_Pos.1 _DECK	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
			L_C (Traffic load UIC of EC/DIN-FB)							
101	part.	CS 1	CRASH_-Y_Pos.2 _DECK	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
			L_C (Traffic load UIC of EC/DIN-FB)							
102	part.	CS 1	CRASH_-Y_Pos.3 _DECK	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
			L_C (Traffic load UIC of EC/DIN-FB)							
103	part.	CS 1	CRASH_-Y_Pos.4 _DECK	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
			L_C (Traffic load UIC of EC/DIN-FB)							
104	part.	CS 1	CRASH_-Y_Pos.5 _DECK	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
			L_C (Traffic load UIC of EC/DIN-FB)							
105	part.	CS 1	CRASH_-Y_Pos.6 _DECK	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
			L_C (Traffic load UIC of EC/DIN-FB)							
106	part.	CS 1	CRASH_-Y_Pos.7 _DECK	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
			L_C (Traffic load UIC of EC/DIN-FB)							
107	part.	CS 1	CRASH_-Y_Pos.8 _DECK	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
			L_C (Traffic load UIC of EC/DIN-FB)							
108	part.	CS 1	CRASH_-Y_Pos.9 _DECK	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
			L_C (Traffic load UIC of EC/DIN-FB)							
109	part.	CS 1	CRASH_-Y_Pos.10 _DECK	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
			L_C (Traffic load UIC of EC/DIN-FB)							
110	part.	CS 1	CRASH_-Y_Pos.11 _DECK	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
			L_C (Traffic load UIC of EC/DIN-FB)							
111	part.	CS 1	CRASH_-Y_Pos.12 _DECK	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
			L_C (Traffic load UIC of EC/DIN-FB)							
1501	part.	CS 1	MAX-MY BEAM	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
1502	part.	CS 1	MIN-MY BEAM	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
1503	part.	CS 1	MAX-VZ BEAM	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
1504	part.	CS 1	MIN-VZ BEAM	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
1505	part.	CS 1	MAX-MZ BEAM	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
1506	part.	CS 1	MIN-MZ BEAM	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
1507	part.	CS 1	MAX-VY BEAM	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
1508	part.	CS 1	MIN-VY BEAM	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							
1509	part.	CS 1	MAX-N BEAM	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
			L (live loading)							

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_CRASH_DOKOI

Considered Load Cases

No.	refer	act on	Title/type of load case	gam-u	gam-f	psi-0	psi-1	psi-2	psi-1'	
1510	part.	CS 1	MIN-N BEAM L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
1511	part.	CS 1	MAX-MT BEAM L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
1512	part.	CS 1	MIN-MT BEAM L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
5015	part.	CS 0	K creep step C (creep + shrinkage)	1.00	1.00	1.00	1.00	1.00	1.00	P perm
5025	part.	CS 0	K creep step C (creep + shrinkage)	1.00	1.00	1.00	1.00	1.00	1.00	P perm
5055	part.	CS 1	K creep step C (creep + shrinkage)	1.00	1.00	1.00	1.00	1.00	1.00	P perm
5060	part.	CS 1	K creep step C (creep + shrinkage)	1.00	1.00	1.00	1.00	1.00	1.00	P perm
5061	part.	CS 1	K creep step C (creep + shrinkage)	1.00	1.00	1.00	1.00	1.00	1.00	P perm
5062	part.	CS 1	K creep step C (creep + shrinkage)	1.00	1.00	1.00	1.00	1.00	1.00	P perm
5063	part.	CS 1	K creep step C (creep + shrinkage)	1.00	1.00	1.00	1.00	1.00	1.00	P perm
5064	part.	CS 1	K creep step C (creep + shrinkage)	1.00	1.00	1.00	1.00	1.00	1.00	P perm
6015	part.	CS 0	15 K creep step C (creep + shrinkage)							P perm
6025	part.	CS 0	25 K creep step C (creep + shrinkage)							P perm
6055	part.	CS 1	55 K creep step C (creep + shrinkage)							P perm
6060	part.	CS 1	60 K creep step C (creep + shrinkage)							P perm
6061	part.	CS 1	61 K creep step C (creep + shrinkage)							P perm
6062	part.	CS 1	62 K creep step C (creep + shrinkage)							P perm
6063	part.	CS 1	63 K creep step C (creep + shrinkage)							P perm
6064	part.	CS 1	64 K creep step C (creep + shrinkage)							P perm

Ultimate Load Design

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Design for ultimate loads DIN Fachbericht 102 Massivbrücken (2003)
Uniaxial bending due to symmetry

Safety factors	SC-1	SC-2	SC-S	SS-1	SS-2	PIIa
	1.30	1.30	1.50	1.00	1.00	7

Strain limits	C1	C2	S1	S2	Z1	Z2
max	-3.50	-2.00	3.00	25.00	-3.50	25.00

parameters for reinforcements

Minimum reinforcements Bending.	Compress.	compression e/d	N/Npl	min. reinforcem. requ. section	maximum- reinforc.
0.00 [cm2]	0.30 [o/o]	3.50	0.0010	0.00	0.15	9.00

Tensile forces in the longitudinal reinforcements due to shear are NOT accounted for.
Material of sections uses Ultimate Limit strain-stress law with global safety factors
Material of reinforcements uses Ultimate Limit strain-stress law without safety factors

MNo.	temp lev.	Material-safety [-]	max.compr stress [MPa]	at strain [o/oo]	max.tens stress [MPa]	at strain [o/oo]	tension-stiffening [MPa]
1	0	1.300	-16.35	-2.00	0.00	0.00	
3	0	1.300	-16.35	-2.00	0.00	0.00	
4	0	1.300	-16.35	-2.00	0.00	0.00	
5	0	1.300	-16.35	-2.00	0.00	0.00	
6	0	1.300	-16.35	-2.00	0.00	0.00	
7	0	1.300	-16.35	-2.00	0.00	0.00	
8	0	1.300	-16.35	-2.00	0.00	0.00	
9	0	1.300	-16.35	-2.00	0.00	0.00	
10	0	1.300	-16.35	-2.00	0.00	0.00	
12	0	1.000	-525.00	-25.00	525.00	25.00	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_CRASH_DOKOI

Combinations For Ultimate Design

1515 (CS-1: 1) max_my-1515
MAX + MY :
1.00 * G + 1.00 * L_A + 1.00 * L_B + 1.00 * C + 1.00 * L_C

1516 (CS-1: 1) min_my-1516
MIN + MY :
1.00 * G + 1.00 * L_A + 1.00 * L_B + 1.00 * C + 1.00 * L_C

Shear Design

==========

Design for shear DIN 1045-1 (2003)
Minimum shear factor or tan of inclination of compressive struts 0.57 / 1.72

MNo	f-cd [MPa]	tau-rd [MPa]	sigIIQ [MPa]	sigIIT [MPa]	sigIIQ+ [MPa]	fyd [MPa]
1	14.17	0.10	10.62	7.44	10.62	
3	14.17	0.10	10.62	7.44	10.62	
4	14.17	0.10	10.62	7.44	10.62	
5	14.17	0.10	10.62	7.44	10.62	
6	14.17	0.10	10.62	7.44	10.62	
7	14.17	0.10	10.62	7.44	10.62	
8	14.17	0.10	10.62	7.44	10.62	
9	14.17	0.10	10.62	7.44	10.62	
10	14.17	0.10	10.62	7.44	10.62	
12						500.00

Tolerance for exceeding maximum shear or principal compression stress 0.0200

Longitudinal Reinforcements Accumulated minimum
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
1001	0.000	1	0.34	17.16		3.90T	3.94		9.32	
1001	0.883	1	0.30	14.81		3.85T	5.72		5.23	
1002	0.000	1	0.33	16.17		5.29T	5.65		5.23	
1002	0.883	1	0.36	17.76		5.36T	10.15		2.24	
1003	0.000	1	0.37	18.31		5.92T	10.14		2.25	
1003	0.883	1	0.42	20.78		6.98T	13.80			
1004	0.000	1	0.40	20.04		6.22T	13.79		0.04	
1004	0.883	1	0.44	21.71		5.17T	16.54			
1005	0.000	1	0.44	21.81		5.29T	16.53			
1005	0.883	1	0.49	24.57		6.27T	18.30			
1006	0.000	1	0.49	24.53		6.24T	18.29			
1006	0.883	1	0.51	25.21		6.46T	18.74			
1007	0.000	1	0.50	24.77		6.03T	18.74			
1007	0.883	1	0.49	24.42		6.15T	18.27			
1008	0.000	1	0.49	24.25		5.97T	18.28			
1008	0.883	1	0.43	21.55		5.11T	16.44			
1009	0.000	1	0.43	21.41		4.95T	16.46			
1009	0.883	1	0.38	19.02		5.06T	13.96			
1010	0.000	1	0.42	20.67		6.71T	13.97			
1010	0.883	1	0.37	18.19		5.69T	10.24		2.25	
1011	0.000	1	0.36	17.66		5.16T	10.26		2.24	
1011	0.883	1	0.33	16.20		5.10T	5.87		5.23	
1012	0.000	1	0.31	15.31		4.13T	5.95		5.23	
1012	0.883	1	0.35	17.53		4.02T	4.19		9.32	
1013	0.000	1	0.29	14.23		3.28T	1.82		9.12	
1013	0.883	1	0.25	12.31		3.23T	3.81		5.26	
1014	0.000	1	0.28	13.86		4.79T	3.81		5.26	
1014	0.883	1	0.30	15.12		4.84T	8.03		2.25	
1015	0.000	1	0.32	15.75		5.45T	8.05		2.25	
1015	0.883	1	0.36	17.86		6.38T	11.47			
1016	0.000	1	0.37	18.28		6.79T	11.49			
1016	0.883	1	0.37	18.62		4.70T	13.92			
1017	0.000	1	0.39	19.52		5.61T	13.92			
1017	0.883	1	0.42	20.72		5.12T	15.61			
1018	0.000	1	0.43	21.48		5.87T	15.61			
1018	0.883	1	0.37	18.39		2.37T	16.02			
1019	0.000	1	0.37	18.40		2.37T	16.02			
1019	0.883	1	0.44	21.90		6.33T	15.56			
1020	0.000	1	0.42	21.11		5.54T	15.57			
1020	0.883	1	0.39	19.51		5.55T	13.96			
1021	0.000	1	0.37	18.53		4.59T	13.94			
1021	0.883	1	0.36	18.03		6.47T	11.55			
1022	0.000	1	0.36	17.67		6.13T	11.54			
1022	0.883	1	0.31	15.54		5.23T	8.06		2.25	
1023	0.000	1	0.30	14.96		4.67T	8.04		2.25	
1023	0.883	1	0.28	13.69		4.62T	3.81		5.26	
1024	0.000	1	0.24	12.18		3.10T	3.81		5.26	
1024	0.883	1	0.29	14.21		3.16T	1.93		9.12	
1025	0.000	1	0.25	12.36		2.37T	0.75		9.23	
1025	0.883	1	0.23	11.63		2.37T	3.91		5.34	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
FASH-2_CRASH_DOKOI

Longitudinal Reinforcements Accumulated minimum
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
1026	0.000	1	0.27	13.26		4.01T	3.91		5.34	
1026	0.883	1	0.26	12.90		2.66T	7.99		2.24	
1027	0.000	1	0.26	13.18		2.94T	7.99		2.24	
1027	0.883	1	0.32	15.71		4.60T	11.10			
1028	0.000	1	0.30	14.80		3.69T	11.10			
1028	0.883	1	0.38	19.03		5.68T	13.36			
1029	0.000	1	0.36	18.04		4.68T	13.36			
1029	0.883	1	0.35	17.27		2.37T	14.90			
1030	0.000	1	0.42	20.75		5.86T	14.90			
1030	0.883	1	0.35	17.64		2.37T	15.27			
1031	0.000	1	0.35	17.64		2.37T	15.27			
1031	0.883	1	0.42	20.99		6.12T	14.87			
1032	0.000	1	0.35	17.25		2.37T	14.87			
1032	0.883	1	0.35	17.30		3.94T	13.36			
1033	0.000	1	0.39	19.47		6.11T	13.36			
1033	0.883	1	0.28	14.10		2.90T	11.20			
1034	0.000	1	0.31	15.22		4.02T	11.20			
1034	0.883	1	0.28	13.81		3.55T	8.02		2.24	
1035	0.000	1	0.26	13.16		2.90T	8.02		2.24	
1035	0.883	1	0.26	13.09		3.87T	3.88		5.34	
1036	0.000	1	0.23	11.59		2.37T	3.87		5.34	
1036	0.883	1	0.25	12.37		2.37T	0.76		9.23	
1037	0.000	1	0.30	14.89		3.92T	1.92		9.05	
1037	0.883	1	0.27	13.65		4.11T	4.22		5.32	
1038	0.000	1	0.31	15.43		5.91T	4.20		5.32	
1038	0.883	1	0.33	16.29		5.98T	8.11		2.20	
1039	0.000	1	0.34	17.06		6.76T	8.10		2.20	
1039	0.883	1	0.30	14.87		3.89T	10.98			
1040	0.000	1	0.32	15.69		4.73T	10.97			
1040	0.883	1	0.36	17.80		4.70T	13.09			
1041	0.000	1	0.37	18.47		5.38T	13.09			
1041	0.883	1	0.38	19.01		4.50T	14.50			
1042	0.000	1	0.40	19.70		5.20T	14.50			
1042	0.883	1	0.39	19.19		4.34T	14.84			
1043	0.000	1	0.37	18.58		3.75T	14.84			
1043	0.883	1	0.41	20.24		5.74T	14.50			
1044	0.000	1	0.37	18.42		3.92T	14.49			
1044	0.883	1	0.37	18.52		5.45T	13.07			
1045	0.000	1	0.36	17.81		4.73T	13.08			
1045	0.883	1	0.31	15.30		4.21T	11.10			
1046	0.000	1	0.29	14.51		3.41T	11.11			
1046	0.883	1	0.35	17.28		6.93T	8.15		2.20	
1047	0.000	1	0.33	16.47		6.12T	8.16		2.20	
1047	0.883	1	0.31	15.52		6.05T	4.16		5.32	
1048	0.000	1	0.28	13.69		4.20T	4.17		5.32	
1048	0.883	1	0.30	15.09		4.00T	2.03		9.05	
1049	0.000	1	0.48	24.08		10.65T	4.07		9.36	
1049	0.883	1	0.44	21.66		9.56T	6.80		5.30	
1050	0.000	1	0.38	18.90		7.45T	6.15		5.30	
1050	0.883	1	0.49	24.27		12.81T	9.36		2.10	
1051	0.000	1	0.43	21.20		10.50T	8.59		2.11	
1051	0.883	1	0.51	25.50		14.26T	11.24			
1052	0.000	1	0.45	22.63		11.97T	10.57		0.08	
1052	0.883	1	0.55	27.24		14.84T	12.40			
1053	0.000	1	0.51	25.26		13.37T	11.89			
1053	0.883	1	0.57	28.21		15.15T	13.05			
1054	0.000	1	0.54	26.66		13.78T	12.88			
1054	0.883	1	0.56	27.81		14.63T	13.18			
1055	0.000	1	0.34	16.75		3.41T	13.34			
1055	0.883	1	0.54	27.01		14.09T	12.91			
1056	0.000	1	0.34	17.03		3.87T	13.16			
1056	0.883	1	0.52	26.05		13.85T	12.20			
1057	0.000	1	0.43	21.25		8.77T	12.48			
1057	0.883	1	0.48	23.70		12.71T	10.94		0.04	
1058	0.000	1	0.40	19.80		8.54T	11.26			
1058	0.883	1	0.46	22.65		11.50T	9.04		2.11	
1059	0.000	1	0.41	20.28		8.82T	9.36		2.10	
1059	0.883	1	0.41	20.56		8.76T	6.50		5.30	
1060	0.000	1	0.44	21.82		9.74T	6.78		5.29	
1060	0.883	1	0.49	24.56		11.44T	3.77		9.36	
2001	0.000	2	0.22	17.44		0.35T	3.69		13.41	
2001	0.300	2	0.19	15.24		0.24T	3.90		11.09	
2002	0.000	2	0.20	16.11		1.11T	3.88		11.12	
2002	0.300	2	0.18	14.61		1.11T	4.04		9.46	
2003	0.000	2	0.18	14.79		1.04T	4.29		9.45	
2003	0.300	2	0.20	16.30		1.04T	4.15		11.12	

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Longitudinal Reinforcements Accumulated minimum
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
2004	0.000	2	0.20	15.66		0.41T	4.16		11.09	
2004	0.300	2	0.22	17.77		0.41T	3.96		13.40	
2005	0.000	2	0.17	13.69		0.22T	1.30		12.17	
2005	0.300	2	0.16	12.67		0.22T	1.61		10.84	
2006	0.000	2	0.17	13.26		0.78T	1.61		10.87	
2006	0.300	2	0.15	11.92		0.78T	1.88		9.26	
2007	0.000	2	0.15	12.07		0.82T	1.99		9.26	
2007	0.300	2	0.17	13.41		0.82T	1.72		10.87	
2008	0.000	2	0.16	12.79		0.23T	1.72		10.84	
2008	0.300	2	0.17	13.80		0.23T	1.40		12.17	
2009	0.000	2	0.17	13.62		0.52T	0.68		12.42	
2009	0.300	2	0.15	12.29		0.52T	0.78		11.00	
2010	0.000	2	0.15	12.08		0.29T	0.76		11.03	
2010	0.300	2	0.13	10.51		0.30T	0.84		9.37	
2011	0.000	2	0.13	10.52		0.30T	0.85		9.37	
2011	0.300	2	0.15	12.08		0.29T	0.75		11.04	
2012	0.000	2	0.15	12.26		0.49T	0.77		11.00	
2012	0.300	2	0.17	13.57		0.49T	0.65		12.42	
2013	0.000	2	0.22	17.27		2.27T	2.95		12.04	
2013	0.300	2	0.18	14.57		2.16T	1.67		10.74	
2014	0.000	2	0.17	13.98		1.08T	2.13		10.77	
2014	0.300	2	0.14	11.54		1.08T	1.27		9.19	
2015	0.000	2	0.14	11.47		1.10T	1.18		9.19	
2015	0.300	2	0.18	14.20		1.10T	2.34		10.77	
2016	0.000	2	0.19	14.99		2.51T	1.74		10.74	
2016	0.300	2	0.21	17.17		1.99T	3.14		12.04	
2017	0.000	2	0.23	18.70		3.91T	2.42		12.36	
2017	0.300	2	0.22	17.48		3.91T	2.52		11.05	
2018	0.000	2	0.31	25.03		11.20T	2.75		11.08	
2018	0.300	2	0.31	24.95		11.20T	4.25		9.50	
2019	0.000	2	0.19	15.48		2.30T	3.69		9.49	
2019	0.300	2	0.20	16.22		2.31T	2.84		11.07	
2020	0.000	2	0.17	13.93		0.41T	2.48		11.05	
2020	0.300	2	0.18	14.80		0.41T	2.03		12.36	

Shear Reinforcements per Cutted Part of Section Accumulated minimum

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
1001	0.000	1	2.25	7.22				
1001	0.883	1	2.19	6.93				
1002	0.000	1	3.01	6.46				
1002	0.883	1	3.01	6.23				
1003	0.000	1	3.32	5.68				
1003	0.883	1	3.32	5.44				
1004	0.000	1	3.45	4.81				
1004	0.883	1	3.45	4.57				
1005	0.000	1	3.48	3.87				
1005	0.883	1	3.48	3.64				
1006	0.000	1	3.45	3.16				
1006	0.883	1	3.70	3.37				
1007	0.000	1	3.45	3.05				
1007	0.883	1	3.52	3.02				
1008	0.000	1	3.31	3.28				
1008	0.883	1	3.32	3.51				
1009	0.000	1	3.30	4.18				
1009	0.883	1	3.30	4.41				
1010	0.000	1	3.19	4.75				
1010	0.883	1	3.19	4.97				
1011	0.000	1	2.89	5.66				
1011	0.883	1	2.89	5.88				
1012	0.000	1	2.24	6.51				
1012	0.883	1	2.24	6.75				
1013	0.000	1	1.87	5.60				
1013	0.883	1	1.82	5.35				
1014	0.000	1	2.70	5.17				
1014	0.883	1	2.70	4.95				
1015	0.000	1	3.04	4.52				
1015	0.883	1	3.04	4.29				
1016	0.000	1	3.24	3.74				
1016	0.883	1	2.66	3.51				
1017	0.000	1	3.18	2.87				
1017	0.883	1	2.91	2.66				
1018	0.000	1	3.34	2.39				
1018	0.883	1	0.84	1.79				
1019	0.000	1	0.89	1.28				
1019	0.883	1	3.60	2.63				

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FASH-2_CRASH_DOKOI

Shear Reinforcements per Cutted Part of Section Accumulated minimum

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
1020	0.000	1	3.15	2.23				
1020	0.883	1	3.15	2.66				
1021	0.000	1	2.60	2.93				
1021	0.883	1	3.08	3.24				
1022	0.000	1	2.92	3.59				
1022	0.883	1	2.92	3.82				
1023	0.000	1	2.60	4.33				
1023	0.883	1	2.60	4.56				
1024	0.000	1	1.75	5.10				
1024	0.883	1	1.80	5.36				
1025	0.000	1	0.64	4.69				
1025	0.883	1	0.95	4.43				
1026	0.000	1	2.24	4.18				
1026	0.883	1	1.49	3.95				
1027	0.000	1	1.63	3.48				
1027	0.883	1	2.59	3.25				
1028	0.000	1	2.08	2.84				
1028	0.883	1	3.21	2.83				
1029	0.000	1	2.65	2.21				
1029	0.883	1	0.58	1.98				
1030	0.000	1	3.33	2.20				
1030	0.883	1	0.56	1.37				
1031	0.000	1	0.54	1.03				
1031	0.883	1	3.47	2.37				
1032	0.000	1	0.59	1.41				
1032	0.883	1	2.23	1.85				
1033	0.000	1	3.46	3.29				
1033	0.883	1	1.63	2.29				
1034	0.000	1	2.27	2.75				
1034	0.883	1	1.97	2.74				
1035	0.000	1	1.62	3.28				
1035	0.883	1	2.17	3.78				
1036	0.000	1	0.93	3.93				
1036	0.883	1	0.72	3.68				
1037	0.000	1	2.24	4.81				
1037	0.883	1	2.32	4.54				
1038	0.000	1	3.33	4.21				
1038	0.883	1	3.33	3.98				
1039	0.000	1	3.77	4.05				
1039	0.883	1	2.19	3.51				
1040	0.000	1	2.66	3.25				
1040	0.883	1	2.66	3.02				
1041	0.000	1	3.05	2.69				
1041	0.883	1	2.56	2.45				
1042	0.000	1	2.95	2.08				
1042	0.883	1	2.47	2.10				
1043	0.000	1	2.13	1.90				
1043	0.883	1	3.26	2.47				
1044	0.000	1	2.23	2.10				
1044	0.883	1	3.09	2.41				
1045	0.000	1	2.68	2.84				
1045	0.883	1	2.37	2.85				
1046	0.000	1	1.92	3.14				
1046	0.883	1	3.86	4.11				
1047	0.000	1	3.41	3.91				
1047	0.883	1	3.41	4.05				
1048	0.000	1	2.37	3.79				
1048	0.883	1	2.29	3.81				
1049	0.000	1	5.86	5.27				
1049	0.883	1	5.33	6.03				
1050	0.000	1	4.15	4.59				
1050	0.883	1	7.17	7.50				
1051	0.000	1	5.88	4.48				
1051	0.883	1	8.00	7.71				
1052	0.000	1	6.73	4.71				
1052	0.883	1	8.35	7.60				
1053	0.000	1	7.53	5.99				
1053	0.883	1	8.53	7.37				
1054	0.000	1	7.77	6.75				
1054	0.883	1	8.24	7.02				
1055	0.000	1	1.88	1.49				
1055	0.883	1	7.94	6.43				
1056	0.000	1	2.13	1.77				
1056	0.883	1	7.80	5.87				
1057	0.000	1	4.18	4.15				
1057	0.883	1	7.14	4.84				
1058	0.000	1	4.07	4.39				
1058	0.883	1	6.43	4.54				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
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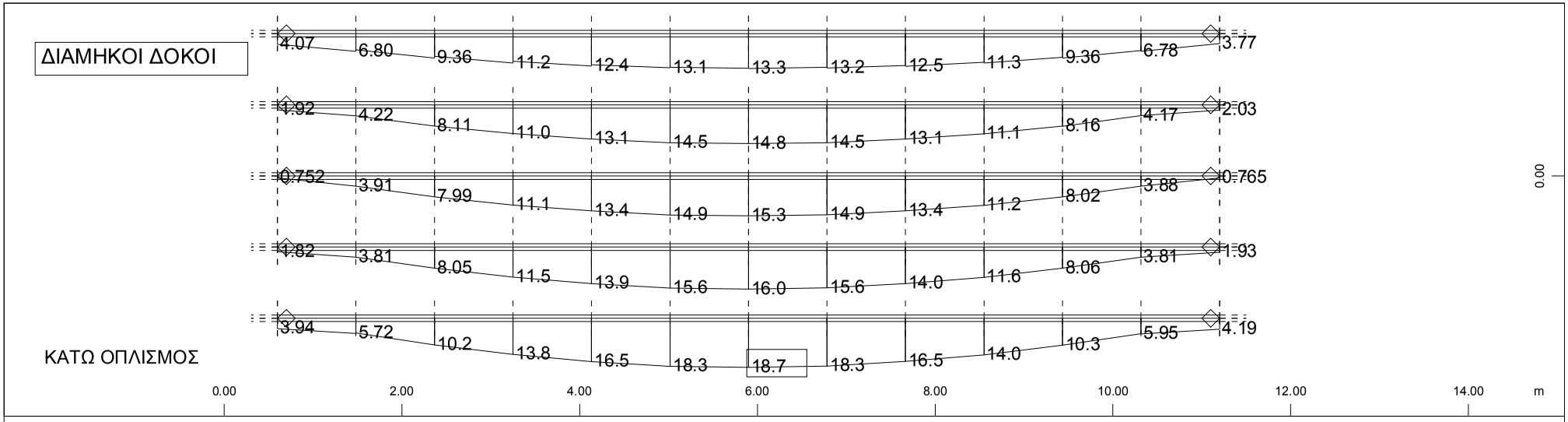
Shear Reinforcements per Cutted Part of Section Accumulated minimum

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
1059	0.000	1	4.94	5.00				
1059	0.883	1	4.87	4.65				
1060	0.000	1	5.43	5.58				
1060	0.883	1	5.45	5.59				
2001	0.000	2	0.15	6.31				
2001	0.300	2	0.11	6.05				
2002	0.000	2	0.50	6.76				
2002	0.300	2	0.50	6.55				
2003	0.000	2	0.46	5.34				
2003	0.300	2	0.46	5.47				
2004	0.000	2	0.18	5.38				
2004	0.300	2	0.18	5.52				
2005	0.000	2	0.10	4.61				
2005	0.300	2	0.10	4.47				
2006	0.000	2	0.33	5.03				
2006	0.300	2	0.33	4.85				
2007	0.000	2	0.35	4.66				
2007	0.300	2	0.35	4.83				
2008	0.000	2	0.10	4.26				
2008	0.300	2	0.10	4.41				
2009	0.000	2	0.22	4.86				
2009	0.300	2	0.22	4.73				
2010	0.000	2	0.13	5.06				
2010	0.300	2	0.13	4.89				
2011	0.000	2	0.13	3.48				
2011	0.300	2	0.13	3.67				
2012	0.000	2	0.21	3.33				
2012	0.300	2	0.21	3.40				
2013	0.000	2	0.98	5.15				
2013	0.300	2	0.93	4.94				
2014	0.000	2	0.48	5.11				
2014	0.300	2	0.48	4.95				
2015	0.000	2	0.49	3.38				
2015	0.300	2	0.49	3.56				
2016	0.000	2	1.08	4.00				
2016	0.300	2	0.85	4.04				
2017	0.000	2	1.68	3.68				
2017	0.300	2	1.68	3.57				
2018	0.000	2	3.59	5.53				
2018	0.300	2	3.59	5.52				
2019	0.000	2	0.99	3.45				
2019	0.300	2	0.99	3.64				
2020	0.000	2	0.18	2.60				
2020	0.300	2	0.18	3.26				

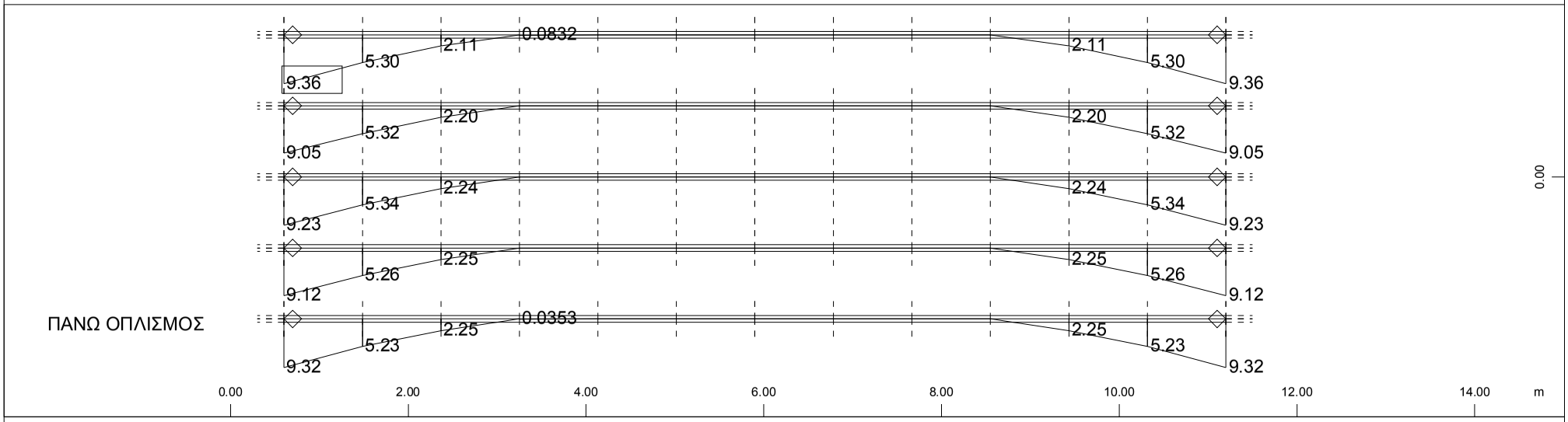
Maximum Degree of Utilization

		N sig-c	Vy sig-t	Vz tau	Mt sig-*	My tend.	Mz As-l	Mb As-v	Mt2 crack	Total sigdyn	lamda tau-*
Cross sect.	1	0.000	0.000	0.208	0.765	0.000	0.000	0.000	0.000	1.001	0.000
		0.000	0.000	0.000	0.000	0.000	1.000	3.751	0.000	0.000	0.000
Cross sect.	2	0.000	0.000	0.076	0.240	0.000	0.000	0.000	0.000	1.001	0.000
DOKOS-2		0.000	0.000	0.000	0.000	0.000	1.001	0.722	0.000	0.000	0.000
Total System		0.000	0.000	0.208	0.765	0.000	0.000	0.000	0.000	1.001	0.000
		0.000	0.000	0.000	0.000	0.000	1.001	3.751	0.000	0.000	0.000



Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case 516 , 1 cm 3D = 22.4 cm2 (Max=18.7)

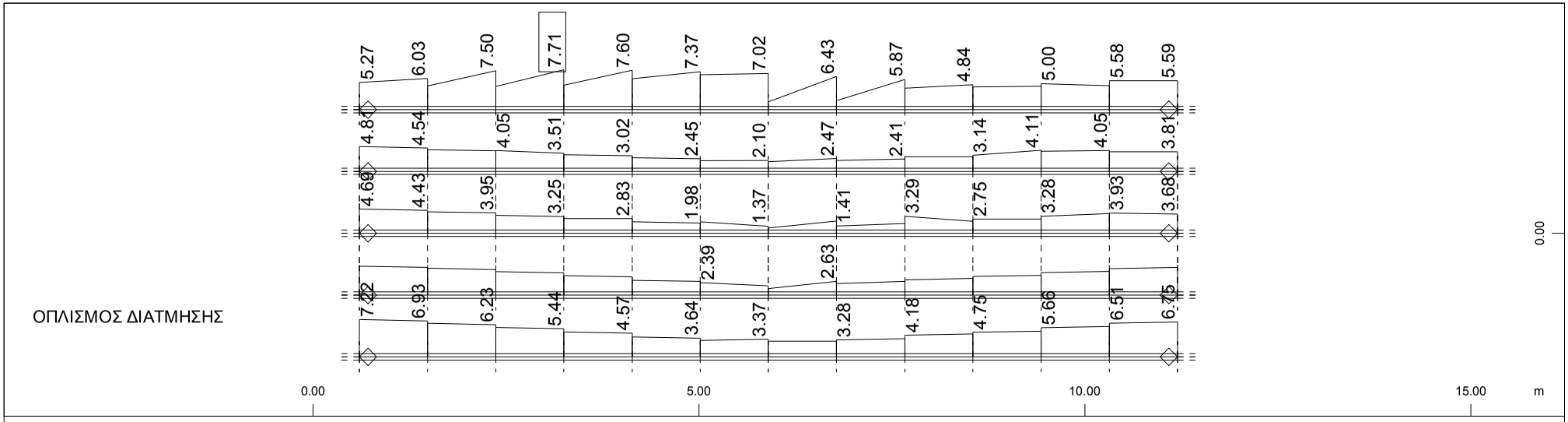
M 1 : 66



Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case 516 , 1 cm 3D = 11.2 cm2 (Max=9.36)

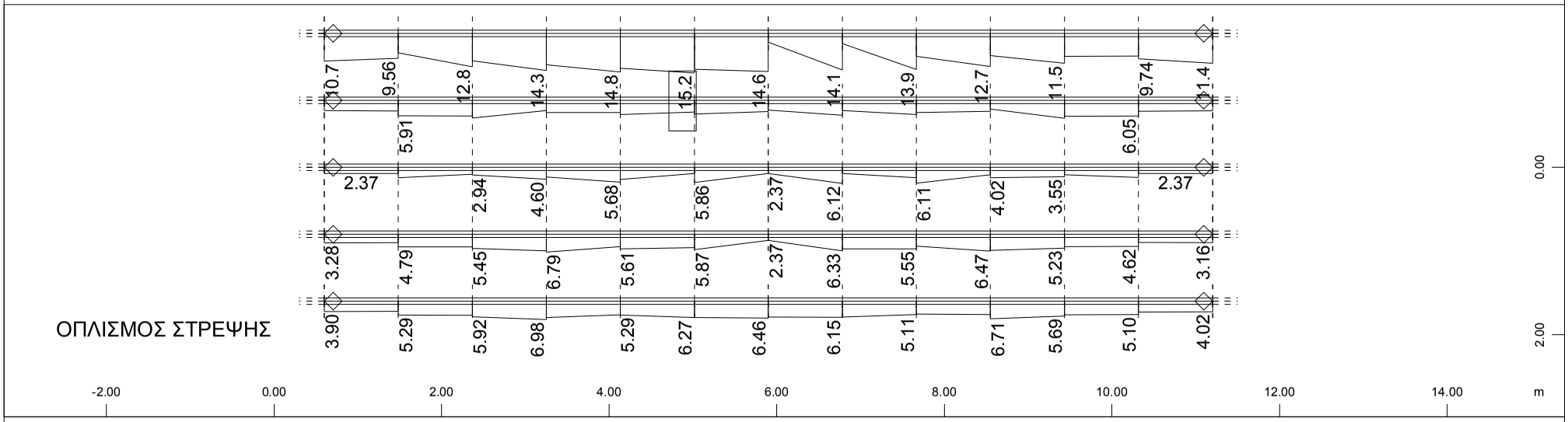
M 1 : 66

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΦΑΣΗ-2_CRASH



Sector of system Beam Elements Group 1
Beam Elements , Shear reinforcements (maximum), Design Case 516 , 1 cm 3D = 11.2 cm2/m (Max=7.71)

M 1 : 76

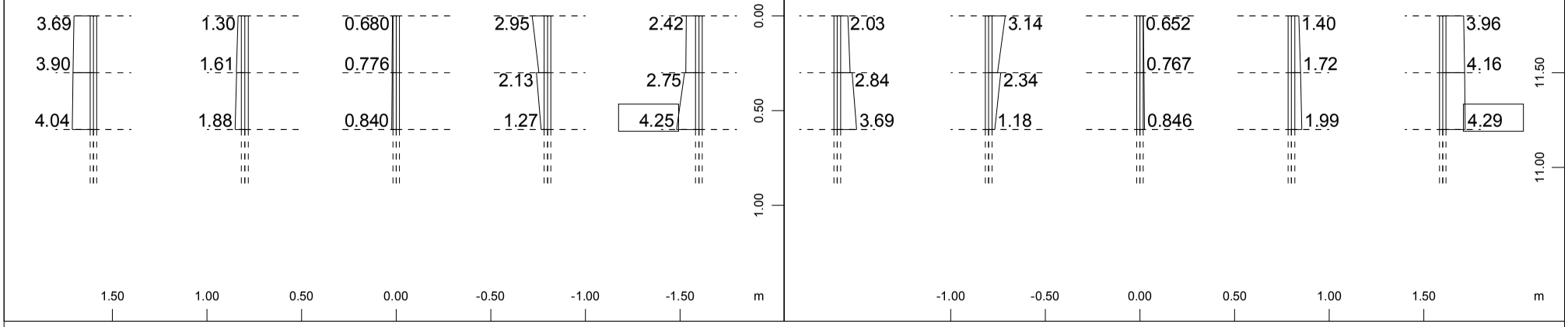


Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case 516 , 1 cm 3D = 22.4 cm2 (Max=15.2)

M 1 : 70

ΔΙΑΜΗΚΗ ΔΟΚΟΙ-ΣΤΗΡΙΞΗΣ

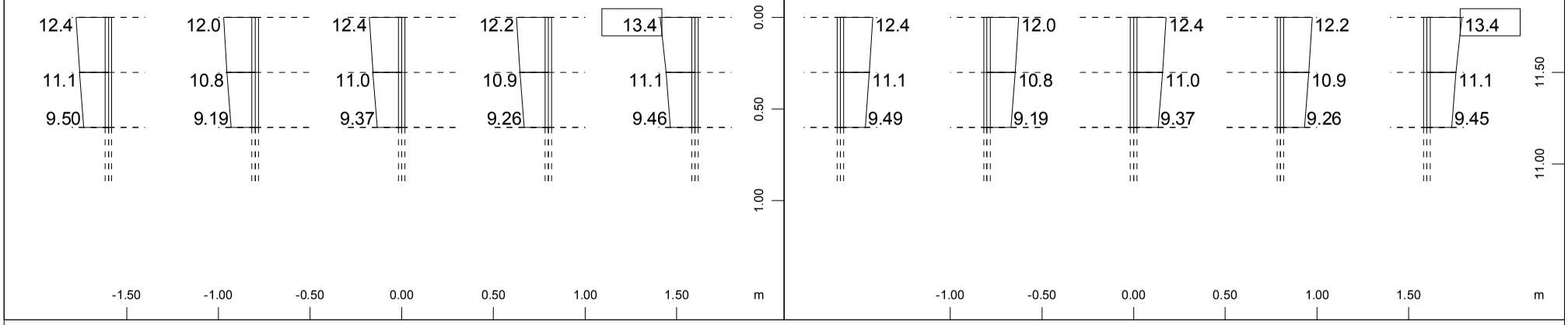
ΚΑΤΩ ΟΠΛΙΣΜΟΣ



Y Z X
Sector of system Beam Elements Group 2 M 1 : 31
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
516 , 1 cm 3D = 11.2 cm2 (Max=4.25)

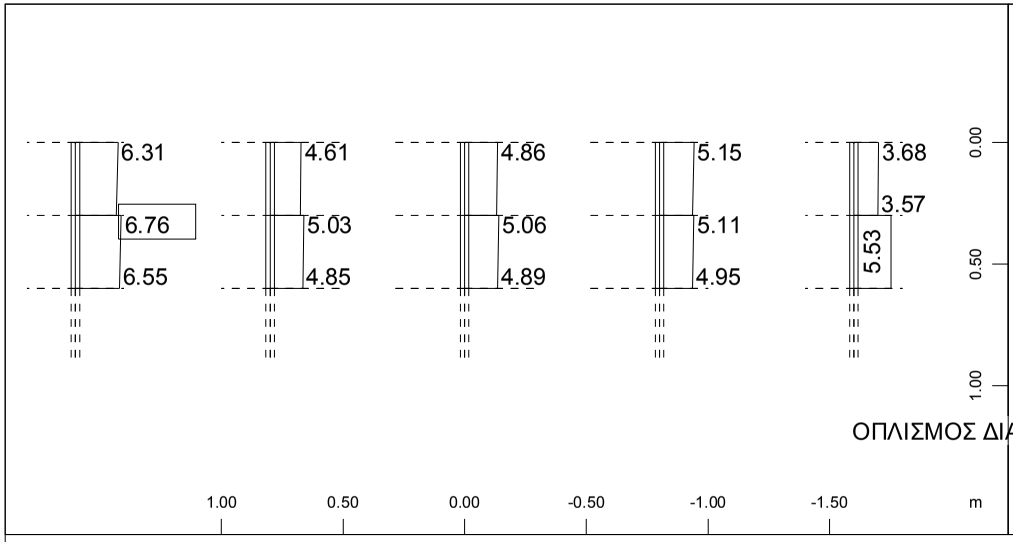
X Z Y
Sector of system Beam Elements Group 2 M 1 : 31
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
516 , 1 cm 3D = 11.2 cm2 (Max=4.29)

ΠΑΝΩ ΟΠΛΙΣΜΟΣ



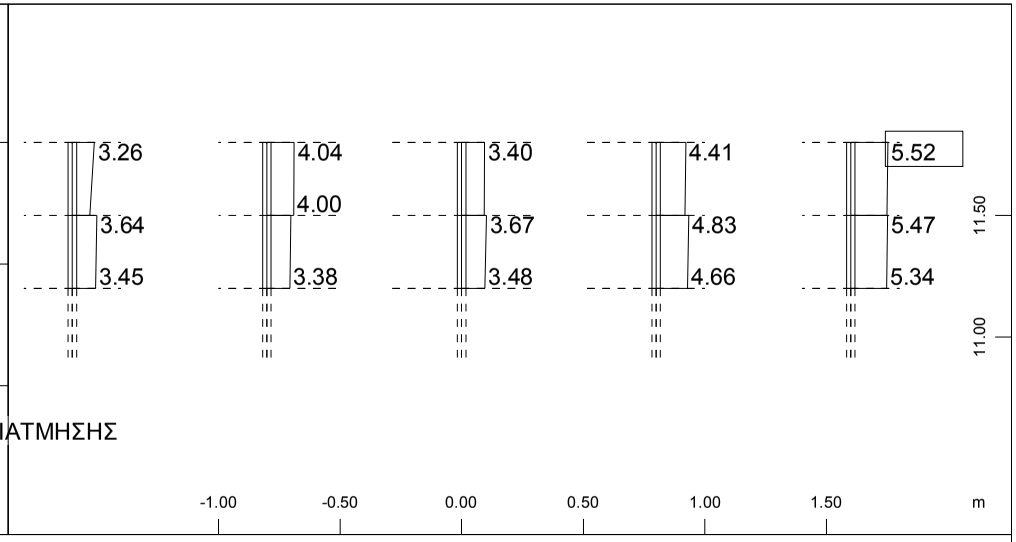
Z Y X
Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
516 , 1 cm 3D = 22.4 cm2 (Max=13.4)

X Z Y
Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
516 , 1 cm 3D = 22.4 cm2 (Max=13.4)

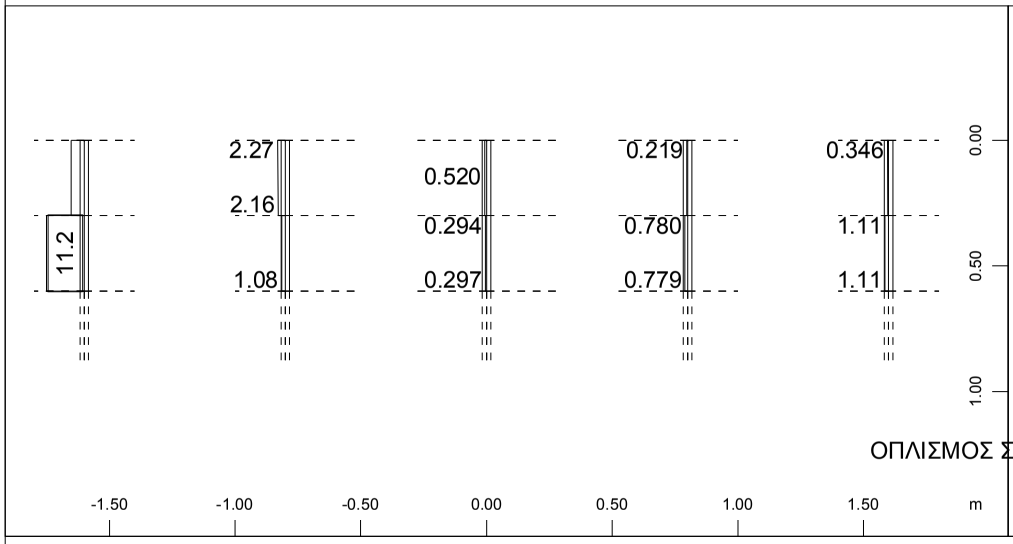


ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ

Y Z X
Sector of system Beam Elements Group 2
Beam Elements , Shear reinforcements (maximum), Design Case 516
, 1 cm 3D = 11.2 cm2/m (Max=6.76)
M 1 : 31

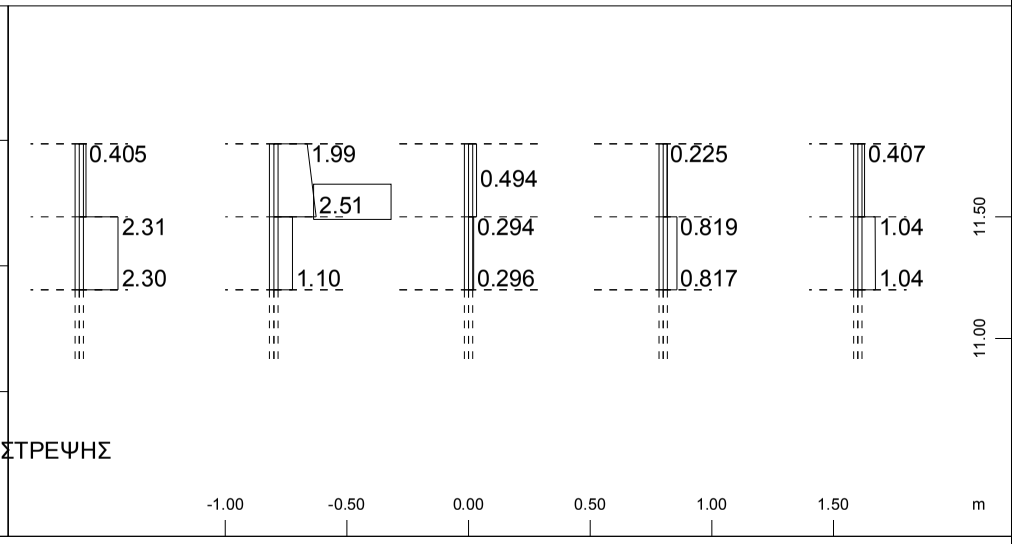


X Z Y
Sector of system Beam Elements Group 2
Beam Elements , Shear reinforcements (maximum), Design Case 516
, 1 cm 3D = 11.2 cm2/m (Max=5.52)
M 1 : 31



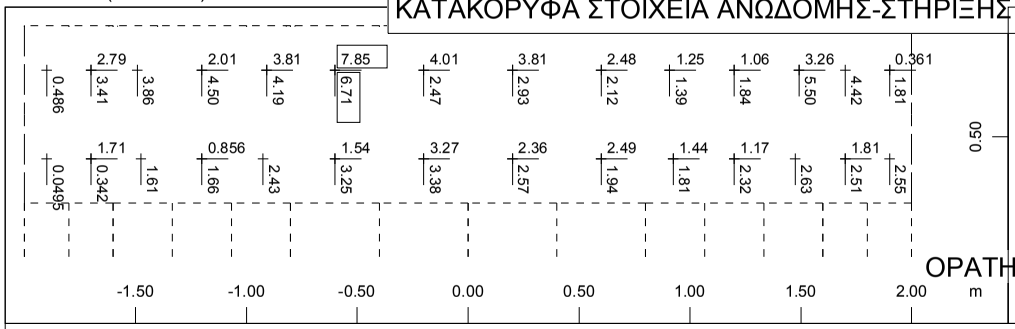
ΟΠΛΙΣΜΟΣ ΣΤΡΕΨΗΣ

Z Y X
Sector of system Beam Elements Group 2
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
516 , 1 cm 3D = 22.4 cm2 (Max=11.2)
M 1 : 30



X Z Y
Sector of system Beam Elements Group 2
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
516 , 1 cm 3D = 4.48 cm2 (Max=2.51)
M 1 : 31

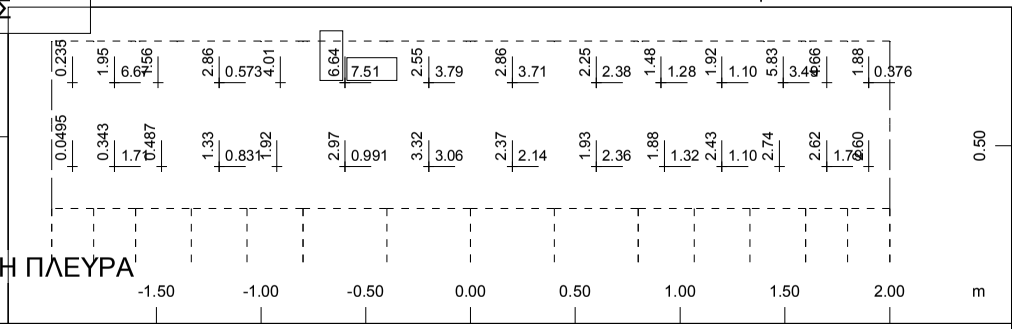
ΚΑΤΑΚΟΡΥΦΑ ΣΤΟΙΧΕΙΑ ΑΝΩΔΟΜΗΣ-ΣΤΗΡΙΞΗΣ



Sector of system Quadrilateral Elements Group 8
upper Reinforcements in Elements in cm2/m, Design Case 524 ULS
design (Max=7.85)

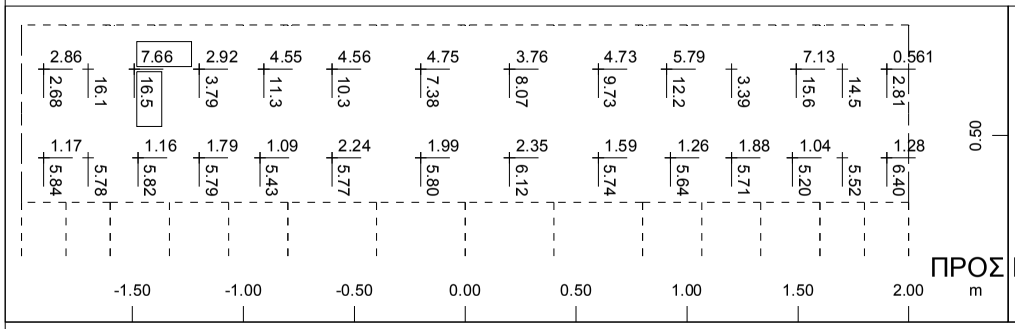
M 1 : 34

ΟΡΑΤΗ ΠΛΕΥΡΑ



Sector of system Quadrilateral Elements Group 8
upper Reinforcements in Elements in cm2/m, Design Case 524 ULS
design (Max=7.51)

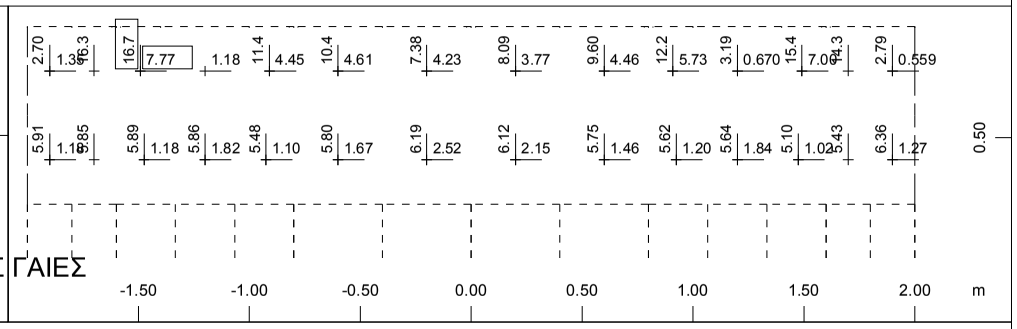
M 1 : 36



Sector of system Quadrilateral Elements Group 8
lower Reinforcements in Elements in cm2/m, Design Case 524 ULS
design (Max=16.5)

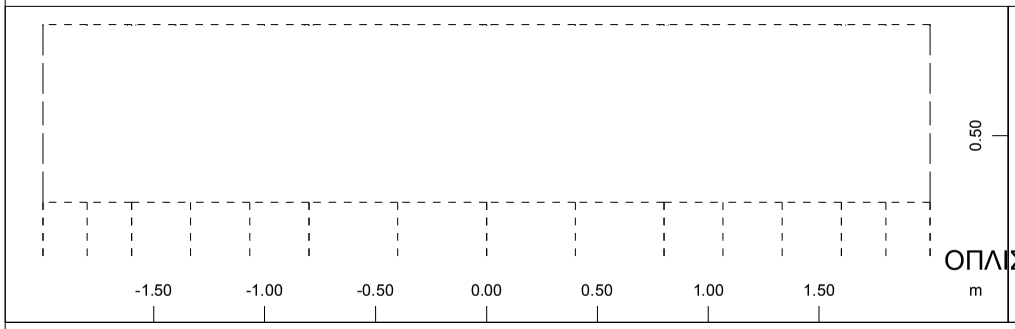
M 1 : 34

ΠΡΟΣ ΓΑΙΕΣ



Sector of system Quadrilateral Elements Group 8
lower Reinforcements in Elements in cm2/m, Design Case 524 ULS
design (Max=16.7)

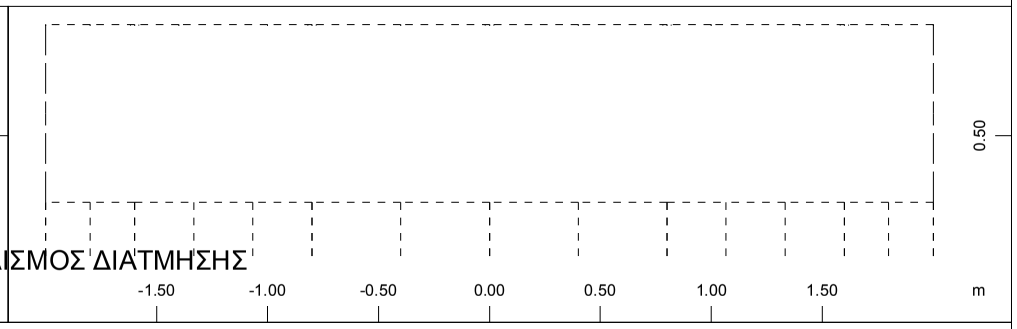
M 1 : 34



Sector of system Quadrilateral Elements Group 8
Shear reinforcement from middle of element in cm2/m2, Design
Case 524 ULS design (Max=0)

M 1 : 34

ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ

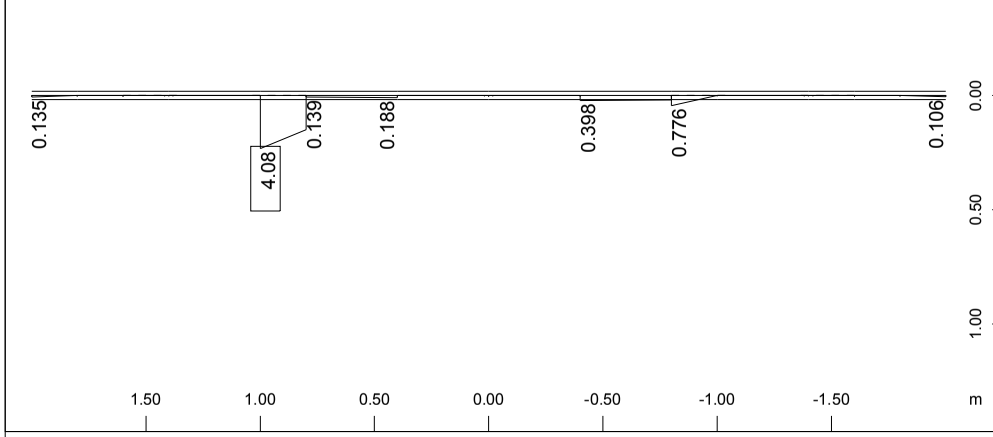


Sector of system Quadrilateral Elements Group 8
Shear reinforcement from middle of element in cm2/m2, Design
Case 524 ULS design (Max=0)

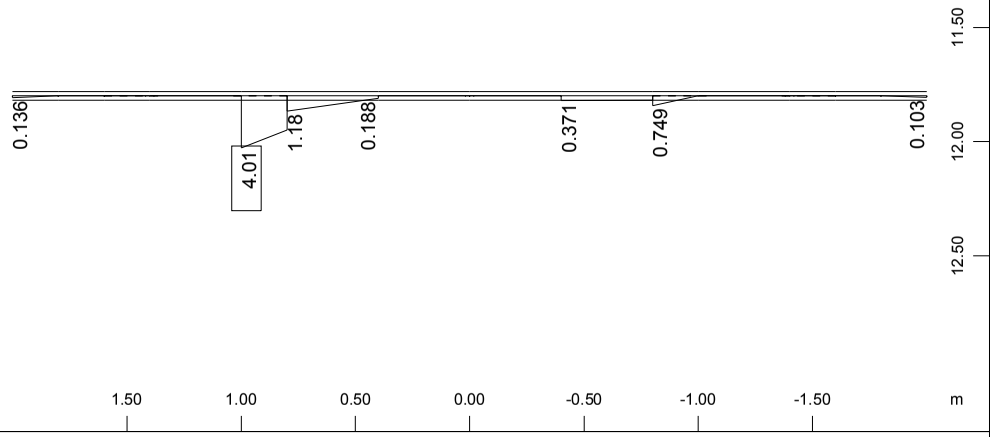
M 1 : 34

ΠΑΣΣΑΛΟΔΕΣΜΟΣ

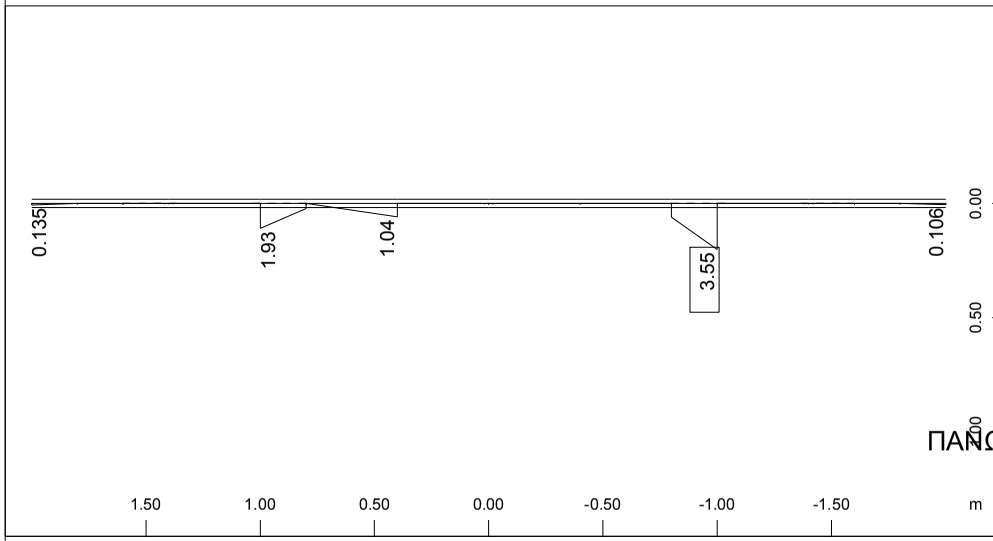
ΚΑΤΩ ΟΠΛΙΣΜΟΣ



Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
515 , 1 cm 3D = 5.81 cm2 (Max=4.08)

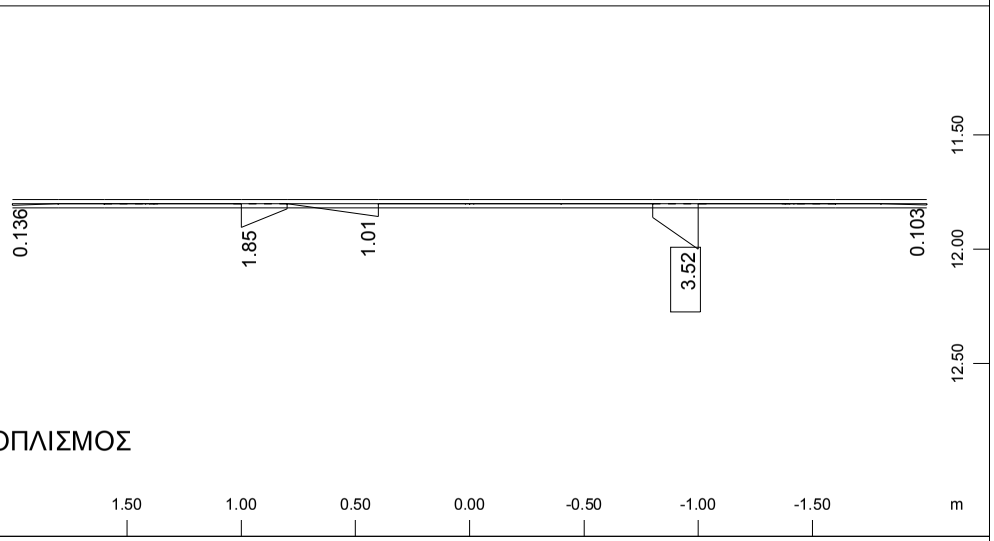


Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
515 , 1 cm 3D = 5.81 cm2 (Max=4.01)

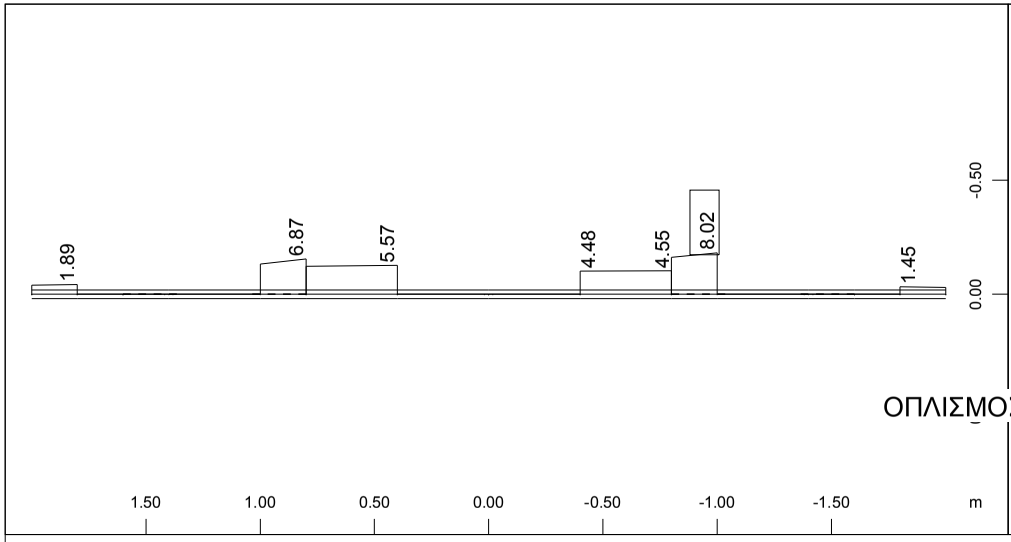


Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
515 , 1 cm 3D = 5.81 cm2 (Max=3.55)

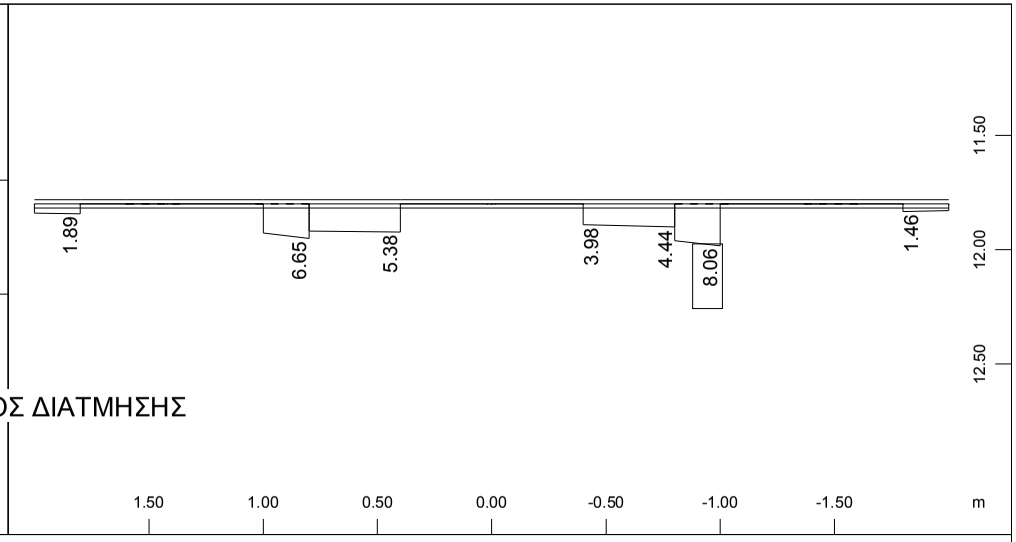
ΠΑΝΩ ΟΠΛΙΣΜΟΣ



Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
515 , 1 cm 3D = 5.81 cm2 (Max=3.52)

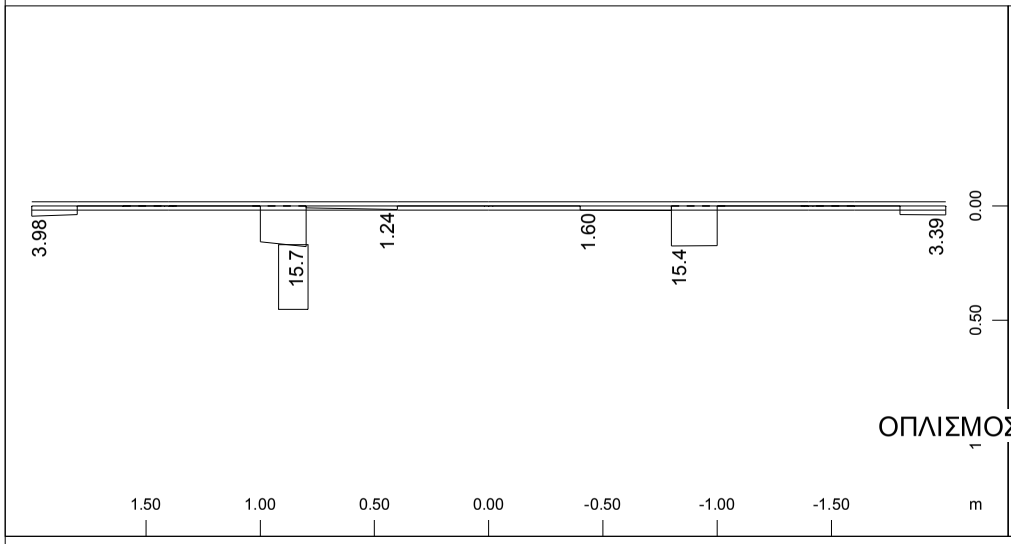


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Shear reinforcements (maximum), Design Case 515
, 1 cm 3D = 14.5 cm2/m (Max=8.02)

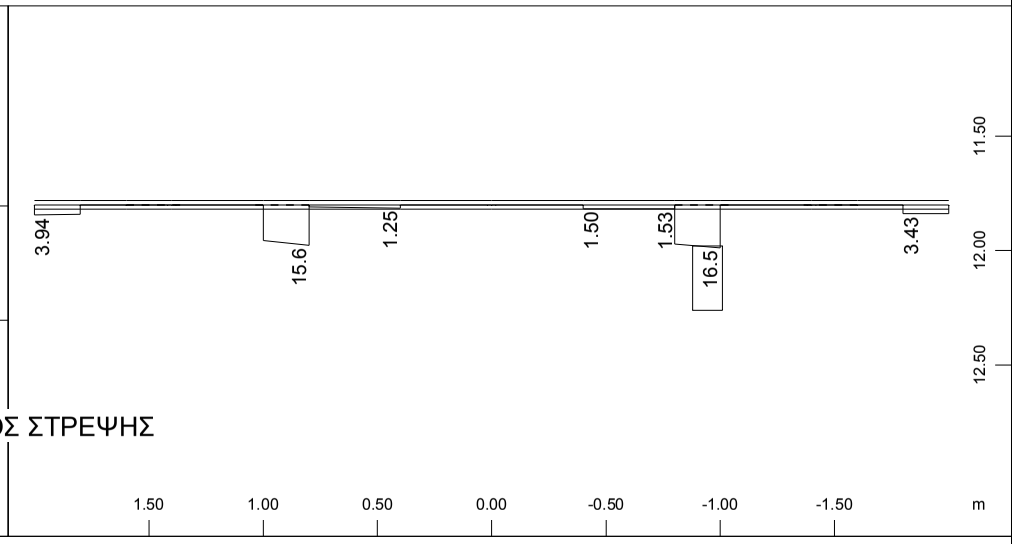


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Shear reinforcements (maximum), Design Case 515
, 1 cm 3D = 14.5 cm2/m (Max=8.06)

ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ

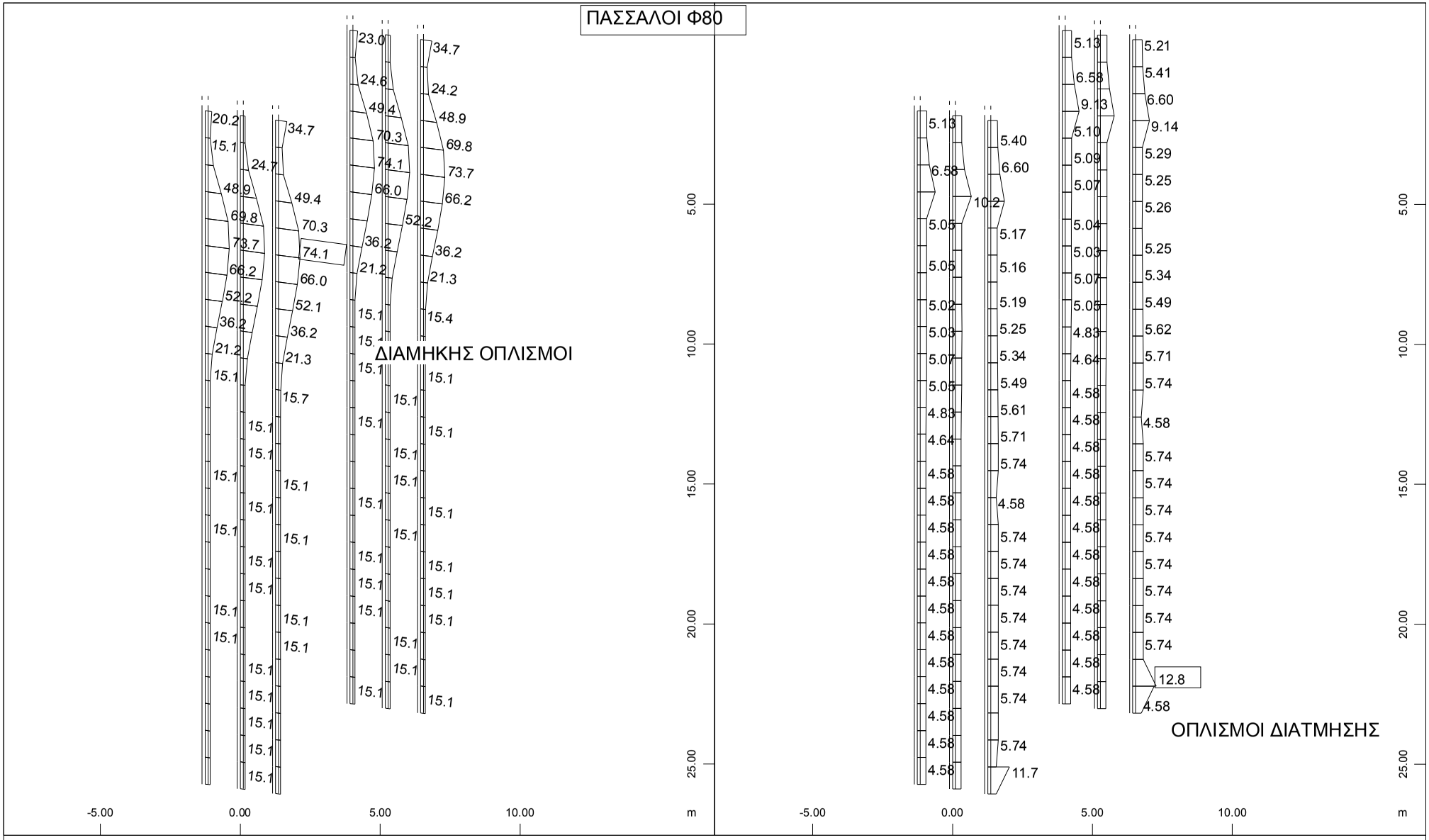


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
515 , 1 cm 3D = 29.1 cm2 (Max=15.7)



Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
515 , 1 cm 3D = 29.1 cm2 (Max=16.5)

ΟΠΛΙΣΜΟΣ ΣΤΡΕΨΗΣ



Sector of system Beam Elements Group 12
Beam Elements , Longitudinal Reinforcements Lay. 1, Design
Case 515 , 1 cm 3D = 145.3 cm2 (Max=74.1)

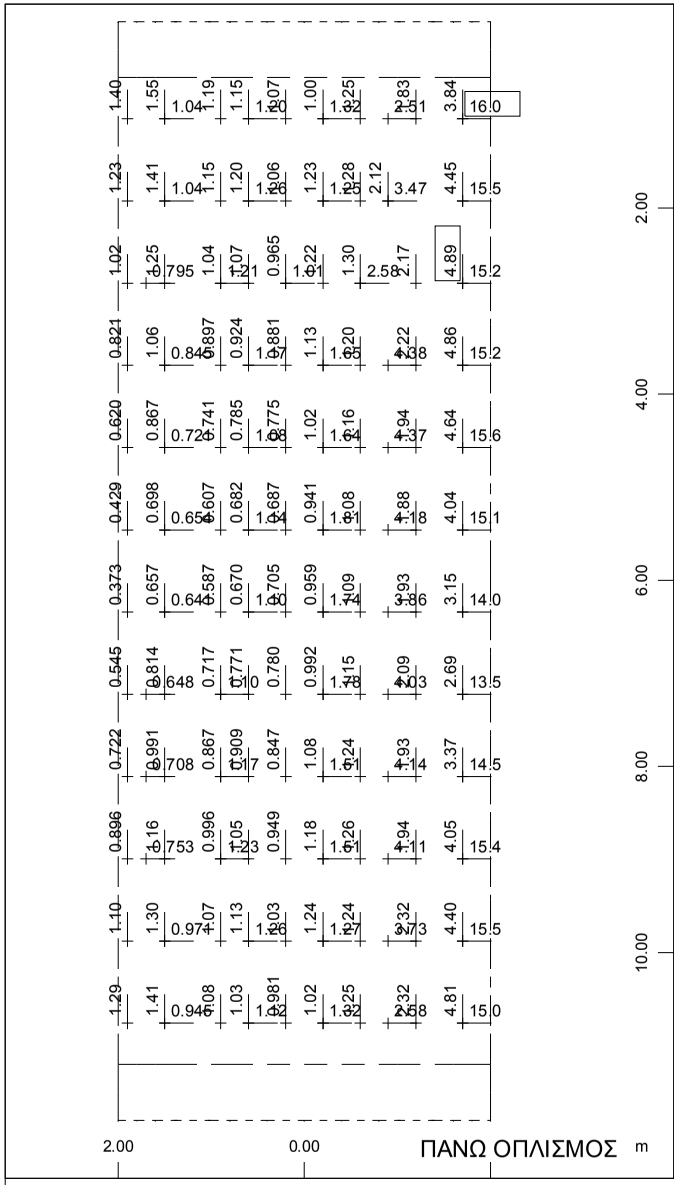
M 1 : 191
X * 0.502
Y * 0.906
Z * 0.962

Sector of system Beam Elements Group 12
Beam Elements , Shear reinforcements (maximum), Design Case
515 , 1 cm 3D = 29.1 cm2/m (Max=12.8)

M 1 : 191
X * 0.502
Y * 0.906
Z * 0.962

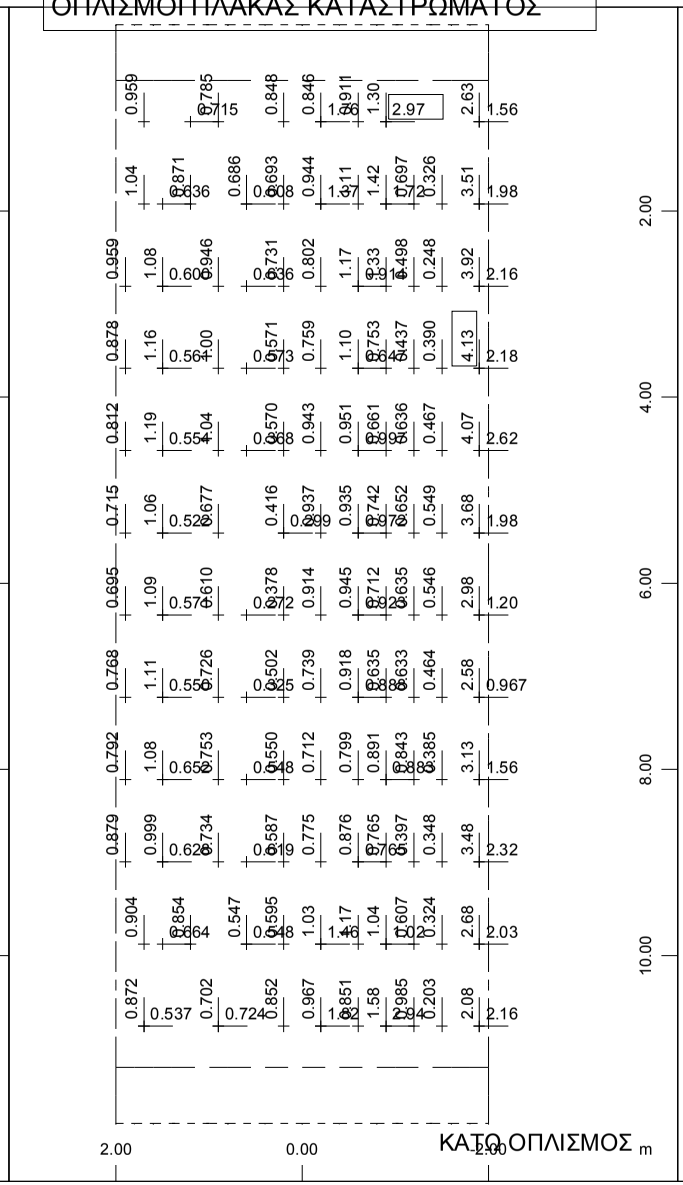
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΦΑΣΗ-2_CRASH

ΟΠΛΙΣΜΟΙ ΠΛΑΚΑΣ ΚΑΤΑΣΤΡΩΜΑΤΟΣ



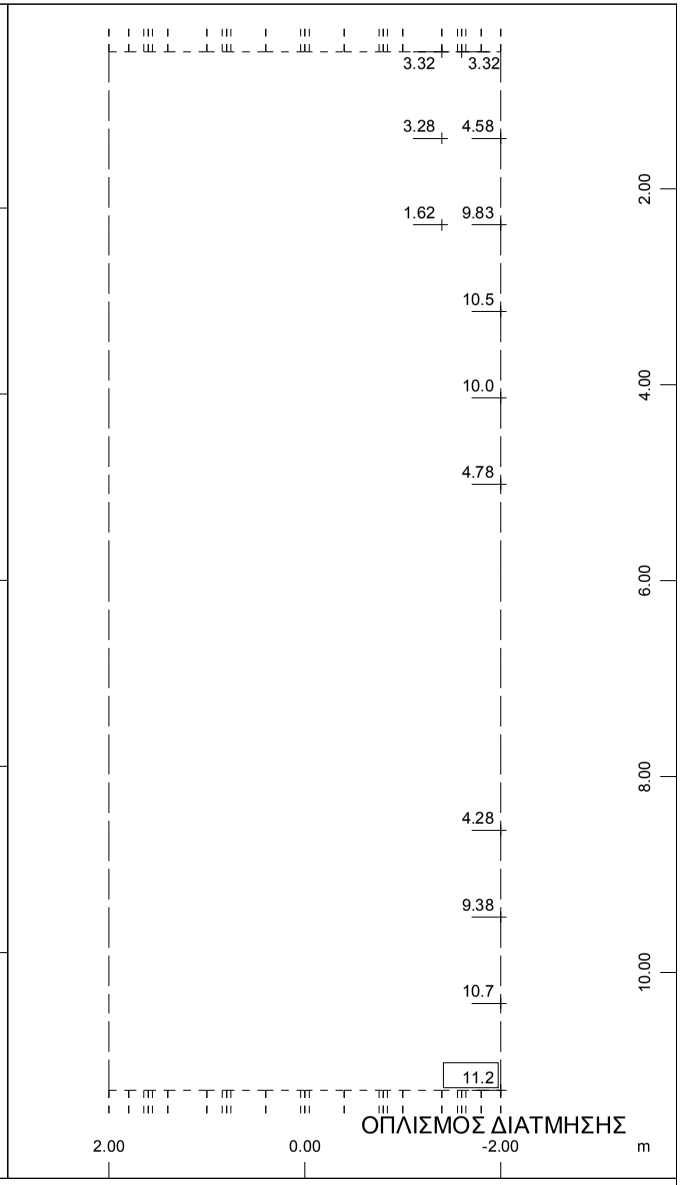
Sector of system Quadrilateral Elements Group 3 4
upper Reinforcements in Elements in cm2/m, Design Case
524 ULS design (Max=16.0)

M 1 : 81



Sector of system Quadrilateral Elements Group 3 4
lower Reinforcements in Elements in cm2/m, Design Case
524 ULS design (Max=4.13)

M 1 : 81



Sector of system Quadrilateral Elements Group 3
Shear reinforcement from middle of element in cm2/m2,
Design Case 524 ULS design (Max=11.2)

M 1 : 77

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00

9) ΦΑΣΗ-2 ΕΛΕΓΧΟΣ ΦΟΡΕΑ ΣΕ SLS - ΡΗΓΜΑΤΩΣΗ

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
SLS(QUASI-PERMANENT)-CRACK w=0.30

Selected Beam Elements

FROM	TO	INC	X-VALUE	NC	MEMBER	CS0	CS1	CS2	CS3	CS4	CS5
1000	1060	1		1		10	40				
2000	2020	1									
10001											
10005											
10006											
10009											
10010											
10014											
10016											
10020											
10021											
10024											
10025											
10029											

Default design code is DIN Fachbericht 102 Massivbrȯcken (2003) (Germany)
Klasse(Tab.4.118): D
Wind zone : Binnenland

Materials

No. 1 C 25/30 (DIN 1045-1)
No. 3 C 25/30 (DIN 1045-1)
No. 4 C 25/30 (DIN 1045-1)
No. 5 C 25/30 (DIN 1045-1)
No. 6 C 25/30 (DIN 1045-1)
No. 7 C 25/30 (DIN 1045-1)
No. 8 C 25/30 (DIN 1045-1)
No. 9 C 25/30 (DIN 1045-1)
No. 10 C 25/30 (DIN 1045-1)
No. 12 BSt 500 SA (DIN 1045-1)

Reinforcement will be accounted for sectional values as defined in AQUA
Reinforcements saved as design case LCR 508

Considered Load Cases

No.	refer	act on	Title/type of load case	gam-u	gam-f	psi-0	psi-1	psi-2	psi-1'	
1	part.	CS 0	I.B. ΚΑΤΑΚ.ΣΤΟΙΧΕΙΩΝ G (total dead load)	1.35	1.00	1.00	1.00	1.00	1.00	G perm
2	part.	CS 0	I.B. ΔΟΚΩΝ G (total dead load)	1.35	1.00	1.00	1.00	1.00	1.00	G perm
3	part.	CS 0	I.B. ΧΥΤΗΣ ΠΛΑΚΑΣ G (total dead load)	1.35	1.00	1.00	1.00	1.00	1.00	G perm
11	part.	CS 0	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+0.5 L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
12	part.	CS 0	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+0.5 L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
13	part.	CS 0	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+1.0 L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
14	part.	CS 0	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+1.0 L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
31	gross	CS 1	ΡΥΣΕΙΣ-ΟΔΟΣΤΡΩΣΙΑ G (total dead load)	1.35	1.00	1.00	1.00	1.00	1.00	G perm
32	gross	CS 1	PEZODROMIO G (total dead load)	1.35	1.00	1.00	1.00	1.00	1.00	G perm
33	gross	CS 1	ΠΡΟΣΘΕΤΗ ΟΔΟΣΤΡΩΣΙΑ 0.50 G (total dead load)	1.35	1.00	1.00	1.00	1.00	1.00	G perm
36	gross	CS 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+0.5 L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
37	gross	CS 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+0.5 L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
38	gross	CS 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+1.0 L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
39	gross	CS 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+1.0 L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
80	gross	CS 1	ΠΙΘΑΝΕΣ ΚΑΘΙΖΗΣΕΙΣ A1 Q (variable load)	1.50	0.00	0.70	0.50	0.30	1.00	Q cond
81	gross	CS 1	ΠΙΘΑΝΕΣ ΚΑΘΙΖΗΣΕΙΣ A2 Q (variable load)	1.50	0.00	0.70	0.50	0.30	1.00	Q cond
90	gross	CS 1	(+ΔTN)+0.75*(+ΔTM) L_C (Traffic load UIC of EC/DIN-FB)	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
91	gross	CS 1	(+ΔTN)+0.75*(-ΔTM) L_C (Traffic load UIC of EC/DIN-FB)	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
92	gross	CS 1	(-ΔTN)+0.75*(+ΔTM) L_C (Traffic load UIC of EC/DIN-FB)	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
93	gross	CS 1	(-ΔTN)+0.75*(-ΔTM) L_C (Traffic load UIC of EC/DIN-FB)	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
94	gross	CS 1	0.35*(+ΔTN)+(+ΔTM) L_C (Traffic load UIC of EC/DIN-FB)	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
95	gross	CS 1	0.35*(+ΔTN)+(-ΔTM) L_C (Traffic load UIC of EC/DIN-FB)	1.45	0.00	0.80	0.80	0.00	1.00	Q excl

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
SLS(QUASI-PERMANENT)-CRACK w=0.30

Considered Load Cases

No.	refer	act on	Title/type of load case	gam-u	gam-f	psi-0	psi-1	psi-2	psi-1'	
96	gross	CS 1	0.35*(-ΔTN)+(+ΔTM)	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
			L_C (Traffic load UIC of EC/DIN-FB)							
97	gross	CS 1	0.35*(-ΔTN)+(-ΔTM)	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
			L_C (Traffic load UIC of EC/DIN-FB)							
5015	part.	CS 0	K creep step	1.00	1.00	1.00	1.00	1.00	1.00	P perm
			C (creep + shrinkage)							
5025	part.	CS 0	K creep step	1.00	1.00	1.00	1.00	1.00	1.00	P perm
			C (creep + shrinkage)							
5055	part.	CS 1	K creep step	1.00	1.00	1.00	1.00	1.00	1.00	P perm
			C (creep + shrinkage)							
5060	part.	CS 1	K creep step	1.00	1.00	1.00	1.00	1.00	1.00	P perm
			C (creep + shrinkage)							
5061	part.	CS 1	K creep step	1.00	1.00	1.00	1.00	1.00	1.00	P perm
			C (creep + shrinkage)							
5062	part.	CS 1	K creep step	1.00	1.00	1.00	1.00	1.00	1.00	P perm
			C (creep + shrinkage)							
5063	part.	CS 1	K creep step	1.00	1.00	1.00	1.00	1.00	1.00	P perm
			C (creep + shrinkage)							
5064	part.	CS 1	K creep step	1.00	1.00	1.00	1.00	1.00	1.00	P perm
			C (creep + shrinkage)							
6015	part.	CS 0	15 K creep step							P perm
			C (creep + shrinkage)							
6025	part.	CS 0	25 K creep step							P perm
			C (creep + shrinkage)							
6055	part.	CS 1	55 K creep step							P perm
			C (creep + shrinkage)							
6060	part.	CS 1	60 K creep step							P perm
			C (creep + shrinkage)							
6061	part.	CS 1	61 K creep step							P perm
			C (creep + shrinkage)							
6062	part.	CS 1	62 K creep step							P perm
			C (creep + shrinkage)							
6063	part.	CS 1	63 K creep step							P perm
			C (creep + shrinkage)							
6064	part.	CS 1	64 K creep step							P perm
			C (creep + shrinkage)							

Combinations For Serviceability

1217 (CS-1: 1) max_my-CRACK
MAX + MY :
1.00 * G + 1.00 * L_A + 1.00 * L_B + 1.00 * Q + 1.00 * C + 0.60 * L_C

1218 (CS-1: 1) max_my-CRACK
MIN + MY :
1.00 * G + 1.00 * L_A + 1.00 * L_B + 1.00 * Q + 1.00 * C + 0.60 * L_C

Parameters for nonlinear stresses

Iteration for all forces and moments

Material of sections uses Serviceability strain-stress law without safety factors
Material of reinforcements uses Serviceability strain-stress law without safety factors

MNo.	temp lev.	Material-safety [-]	max.compr stress [MPa]	at strain [o/oo]	max.tens stress [MPa]	at strain [o/oo]	tension-stiffening [MPa]
1	0	1.000	-33.00	-2.20	0.00	0.00	
3	0	1.000	-33.00	-2.20	0.00	0.00	
4	0	1.000	-33.00	-2.20	0.00	0.00	
5	0	1.000	-33.00	-2.20	0.00	0.00	
6	0	1.000	-33.00	-2.20	0.00	0.00	
7	0	1.000	-33.00	-2.20	0.00	0.00	
8	0	1.000	-33.00	-2.20	0.00	0.00	
9	0	1.000	-33.00	-2.20	0.00	0.00	
10	0	1.000	-33.00	-2.20	0.00	0.00	
12	0	1.000	-550.00	-25.00	550.00	25.00	

Interaction thin walled normal- and shearstress via Prandtl flow rule

Parameters for nonlinear stress / Crackwidth DIN 1045-1 (neu)

MNo	design width [mm]	[mm]	bond [-]	load [-]	h-max [m]
12	0.300	0.300	0.80	0.25	0.800

Check for crack width passed with additional reinforcements

Stiffness is not saved in database

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
SLS(QUASI-PERMANENT)-CRACK w=0.30

Longitudinal Reinforcements Accumulated minimum
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
1001	0.000	1	0.34	17.16		3.90T	3.94		9.32	
1001	0.883	1	0.30	14.81		3.85T	5.72		5.23	
1002	0.000	1	0.33	16.17		5.29T	5.65		5.23	
1002	0.883	1	0.36	17.76		5.36T	10.15		2.24	
1003	0.000	1	0.37	18.31		5.92T	10.14		2.25	
1003	0.883	1	0.42	20.78		6.98T	13.80			
1004	0.000	1	0.40	20.04		6.22T	13.79		0.04	
1004	0.883	1	0.44	21.71		5.17T	16.54			
1005	0.000	1	0.44	21.81		5.29T	16.53			
1005	0.883	1	0.49	24.57		6.27T	18.30			
1006	0.000	1	0.49	24.53		6.24T	18.29			
1006	0.883	1	0.51	25.21		6.46T	18.74			
1007	0.000	1	0.50	24.77		6.03T	18.74			
1007	0.883	1	0.49	24.42		6.15T	18.27			
1008	0.000	1	0.49	24.25		5.97T	18.28			
1008	0.883	1	0.43	21.55		5.11T	16.44			
1009	0.000	1	0.43	21.41		4.95T	16.46			
1009	0.883	1	0.38	19.02		5.06T	13.96			
1010	0.000	1	0.42	20.67		6.71T	13.97			
1010	0.883	1	0.37	18.19		5.69T	10.24		2.25	
1011	0.000	1	0.36	17.66		5.16T	10.26		2.24	
1011	0.883	1	0.33	16.20		5.10T	5.87		5.23	
1012	0.000	1	0.31	15.31		4.13T	5.95		5.23	
1012	0.883	1	0.35	17.53		4.02T	4.19		9.32	
1013	0.000	1	0.29	14.23		3.28T	1.82		9.12	
1013	0.883	1	0.25	12.31		3.23T	3.81		5.26	
1014	0.000	1	0.28	13.86		4.79T	3.81		5.26	
1014	0.883	1	0.30	15.12		4.84T	8.03		2.25	
1015	0.000	1	0.32	15.75		5.45T	8.05		2.25	
1015	0.883	1	0.36	17.86		6.38T	11.47			
1016	0.000	1	0.37	18.28		6.79T	11.49			
1016	0.883	1	0.37	18.62		4.70T	13.92			
1017	0.000	1	0.39	19.52		5.61T	13.92			
1017	0.883	1	0.42	20.72		5.12T	15.61			
1018	0.000	1	0.43	21.48		5.87T	15.61			
1018	0.883	1	0.37	18.39		2.37T	16.02			
1019	0.000	1	0.37	18.40		2.37T	16.02			
1019	0.883	1	0.44	21.90		6.33T	15.56			
1020	0.000	1	0.42	21.11		5.54T	15.57			
1020	0.883	1	0.39	19.51		5.55T	13.96			
1021	0.000	1	0.37	18.53		4.59T	13.94			
1021	0.883	1	0.36	18.03		6.47T	11.55			
1022	0.000	1	0.36	17.67		6.13T	11.54			
1022	0.883	1	0.31	15.54		5.23T	8.06		2.25	
1023	0.000	1	0.30	14.96		4.67T	8.04		2.25	
1023	0.883	1	0.28	13.69		4.62T	3.81		5.26	
1024	0.000	1	0.24	12.18		3.10T	3.81		5.26	
1024	0.883	1	0.29	14.21		3.16T	1.93		9.12	
1025	0.000	1	0.25	12.36		2.37T	0.75		9.23	
1025	0.883	1	0.23	11.63		2.37T	3.91		5.34	
1026	0.000	1	0.27	13.26		4.01T	3.91		5.34	
1026	0.883	1	0.26	12.90		2.66T	7.99		2.24	
1027	0.000	1	0.26	13.18		2.94T	7.99		2.24	
1027	0.883	1	0.32	15.71		4.60T	11.10			
1028	0.000	1	0.30	14.80		3.69T	11.10			
1028	0.883	1	0.38	19.03		5.68T	13.36			
1029	0.000	1	0.36	18.04		4.68T	13.36			
1029	0.883	1	0.35	17.27		2.37T	14.90			
1030	0.000	1	0.42	20.75		5.86T	14.90			
1030	0.883	1	0.35	17.64		2.37T	15.27			
1031	0.000	1	0.35	17.64		2.37T	15.27			
1031	0.883	1	0.42	20.99		6.12T	14.87			
1032	0.000	1	0.35	17.25		2.37T	14.87			
1032	0.883	1	0.35	17.30		3.94T	13.36			
1033	0.000	1	0.39	19.47		6.11T	13.36			
1033	0.883	1	0.28	14.10		2.90T	11.20			
1034	0.000	1	0.31	15.22		4.02T	11.20			
1034	0.883	1	0.28	13.81		3.55T	8.02		2.24	
1035	0.000	1	0.26	13.16		2.90T	8.02		2.24	
1035	0.883	1	0.26	13.09		3.87T	3.88		5.34	
1036	0.000	1	0.23	11.59		2.37T	3.87		5.34	
1036	0.883	1	0.25	12.37		2.37T	0.76		9.23	
1037	0.000	1	0.30	14.89		3.92T	1.92		9.05	
1037	0.883	1	0.27	13.65		4.11T	4.22		5.32	
1038	0.000	1	0.31	15.43		5.91T	4.20		5.32	
1038	0.883	1	0.33	16.29		5.98T	8.11		2.20	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
SLS(QUASI-PERMANENT)-CRACK w=0.30

Longitudinal Reinforcements Accumulated minimum
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
1039	0.000	1	0.34	17.06		6.76T	8.10		2.20	
1039	0.883	1	0.30	14.87		3.89T	10.98			
1040	0.000	1	0.32	15.69		4.73T	10.97			
1040	0.883	1	0.36	17.80		4.70T	13.09			
1041	0.000	1	0.37	18.47		5.38T	13.09			
1041	0.883	1	0.38	19.01		4.50T	14.50			
1042	0.000	1	0.40	19.70		5.20T	14.50			
1042	0.883	1	0.39	19.19		4.34T	14.84			
1043	0.000	1	0.37	18.58		3.75T	14.84			
1043	0.883	1	0.41	20.24		5.74T	14.50			
1044	0.000	1	0.37	18.42		3.92T	14.49			
1044	0.883	1	0.37	18.52		5.45T	13.07			
1045	0.000	1	0.36	17.81		4.73T	13.08			
1045	0.883	1	0.31	15.30		4.21T	11.10			
1046	0.000	1	0.29	14.51		3.41T	11.11			
1046	0.883	1	0.35	17.28		6.93T	8.15		2.20	
1047	0.000	1	0.33	16.47		6.12T	8.16		2.20	
1047	0.883	1	0.31	15.52		6.05T	4.16		5.32	
1048	0.000	1	0.28	13.69		4.20T	4.17		5.32	
1048	0.883	1	0.30	15.09		4.00T	2.03		9.05	
1049	0.000	1	0.48	24.08		10.65T	4.07		9.36	
1049	0.883	1	0.44	21.66		9.56T	6.80		5.30	
1050	0.000	1	0.38	18.90		7.45T	6.15		5.30	
1050	0.883	1	0.49	24.27		12.81T	9.36		2.10	
1051	0.000	1	0.43	21.20		10.50T	8.59		2.11	
1051	0.883	1	0.51	25.50		14.26T	11.24			
1052	0.000	1	0.45	22.63		11.97T	10.57		0.08	
1052	0.883	1	0.55	27.24		14.84T	12.40			
1053	0.000	1	0.51	25.26		13.37T	11.89			
1053	0.883	1	0.57	28.21		15.15T	13.05			
1054	0.000	1	0.54	26.66		13.78T	12.88			
1054	0.883	1	0.56	27.81		14.63T	13.18			
1055	0.000	1	0.34	16.75		3.41T	13.34			
1055	0.883	1	0.54	27.01		14.09T	12.91			
1056	0.000	1	0.34	17.03		3.87T	13.16			
1056	0.883	1	0.52	26.05		13.85T	12.20			
1057	0.000	1	0.43	21.25		8.77T	12.48			
1057	0.883	1	0.48	23.70		12.71T	10.94		0.04	
1058	0.000	1	0.40	19.80		8.54T	11.26			
1058	0.883	1	0.46	22.65		11.50T	9.04		2.11	
1059	0.000	1	0.41	20.28		8.82T	9.36		2.10	
1059	0.883	1	0.41	20.56		8.76T	6.50		5.30	
1060	0.000	1	0.44	21.82		9.74T	6.78		5.29	
1060	0.883	1	0.49	24.56		11.44T	3.77		9.36	
2001	0.000	2	0.26	20.90		0.43T	3.69		16.78	
2001	0.300	2	0.25	19.76		0.34T	3.90		15.51	
2002	0.000	2	0.26	20.50		1.47T	3.88		15.14	
2002	0.300	2	0.24	19.50		1.58T	4.04		13.88	
2003	0.000	2	0.25	19.69		1.49T	4.29		13.91	
2003	0.300	2	0.26	20.70		1.38T	4.15		15.17	
2004	0.000	2	0.25	20.16		0.57T	4.16		15.43	
2004	0.300	2	0.27	21.21		0.50T	3.96		16.74	
2005	0.000	2	0.22	17.89		0.29T	1.30		16.30	
2005	0.300	2	0.21	17.01		0.31T	1.61		15.09	
2006	0.000	2	0.22	17.52		1.05T	1.61		14.86	
2006	0.300	2	0.21	16.81		1.14T	1.88		13.80	
2007	0.000	2	0.21	16.96		1.19T	1.99		13.78	
2007	0.300	2	0.22	17.66		1.10T	1.72		14.85	
2008	0.000	2	0.21	17.13		0.31T	1.72		15.10	
2008	0.300	2	0.23	18.01		0.30T	1.40		16.31	
2009	0.000	2	0.22	17.96		0.69T	0.68		16.59	
2009	0.300	2	0.21	16.76		0.71T	0.78		15.27	
2010	0.000	2	0.21	16.58		0.41T	0.76		15.42	
2010	0.300	2	0.19	15.49		0.45T	0.84		14.20	
2011	0.000	2	0.19	15.50		0.44T	0.85		14.20	
2011	0.300	2	0.21	16.58		0.41T	0.75		15.43	
2012	0.000	2	0.21	16.74		0.68T	0.77		15.29	
2012	0.300	2	0.22	17.91		0.65T	0.65		16.61	
2013	0.000	2	0.27	21.70		2.86T	2.95		15.89	
2013	0.300	2	0.24	19.15		2.81T	1.67		14.67	
2014	0.000	2	0.23	18.78		1.48T	2.13		15.17	
2014	0.300	2	0.21	16.79		1.59T	1.27		13.93	
2015	0.000	2	0.21	16.73		1.62T	1.18		13.93	
2015	0.300	2	0.24	19.00		1.50T	2.34		15.16	
2016	0.000	2	0.24	19.54		3.21T	1.74		14.59	
2016	0.300	2	0.27	21.67		2.53T	3.14		16.00	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
SLS(QUASI-PERMANENT)-CRACK w=0.30

Longitudinal Reinforcements Accumulated minimum
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
2017	0.000	2	0.30	24.11		4.90T	2.42		16.78	
2017	0.300	2	0.28	22.75		4.94T	2.52		15.29	
2018	0.000	2	0.36	28.88		12.79T	2.75		13.34	
2018	0.300	2	0.34	27.51		12.26T	4.25		11.00	
2019	0.000	2	0.26	20.99		3.21T	3.69		14.10	
2019	0.300	2	0.27	21.53		3.09T	2.84		15.61	
2020	0.000	2	0.24	19.53		0.60T	2.48		16.45	
2020	0.300	2	0.26	20.54		0.58T	2.03		17.92	
10001	0.000	8	0.04	5.17		4.84T	0.16		0.16	
10001	0.200	8	0.51	67.49		67.49T				
10005	0.000	8	0.15	19.83		13.82T	4.08		1.93	
10005	0.200	8	0.14	18.71		15.67T	2.64		0.40	
10006	0.000	8	0.01	0.83		0.69T	0.14			
10006	0.400	8	0.30	39.45		23.26T	15.15		1.04	
10009	0.000	8	0.28	37.13		21.23T	15.90			
10009	0.400	8	0.04	4.72		2.96T	1.76			
10010	0.000	8	0.13	17.21		15.37T	0.78		1.06	
10010	0.200	8	0.14	18.85		15.31T			3.55	
10014	0.000	8	0.51	67.02		67.02T				
10014	0.200	8	0.04	5.23		4.92T	0.15		0.15	
10016	0.000	8	0.04	5.27		4.93T	0.17		0.17	
10016	0.200	8	0.51	67.37		67.37T				
10020	0.000	8	0.15	19.60		13.74T	4.01		1.85	
10020	0.200	8	0.14	18.60		15.56T	2.63		0.40	
10021	0.000	8	0.01	1.92		0.74T	1.18			
10021	0.400	8	0.28	37.18		21.91T	14.26		1.01	
10024	0.000	8	0.28	37.17		21.26T	15.91			
10024	0.400	8	0.04	4.77		2.97T	1.80			
10025	0.000	8	0.13	16.87		15.05T	0.75		1.07	
10025	0.200	8	0.15	20.06		16.54T			3.52	
10029	0.000	8	0.03	3.43		3.43T				
10029	0.200	8	0.05	7.23		6.81T	0.21		0.21	

Shear Reinforcements per Cutted Part of Section Accumulated minimum

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
1001	0.000	1	2.25	7.22				
1001	0.883	1	2.19	6.93				
1002	0.000	1	3.01	6.46				
1002	0.883	1	3.01	6.23				
1003	0.000	1	3.32	5.68				
1003	0.883	1	3.32	5.44				
1004	0.000	1	3.45	4.81				
1004	0.883	1	3.45	4.57				
1005	0.000	1	3.48	3.87				
1005	0.883	1	3.48	3.64				
1006	0.000	1	3.45	3.16				
1006	0.883	1	3.70	3.37				
1007	0.000	1	3.45	3.05				
1007	0.883	1	3.52	3.02				
1008	0.000	1	3.31	3.28				
1008	0.883	1	3.32	3.51				
1009	0.000	1	3.30	4.18				
1009	0.883	1	3.30	4.41				
1010	0.000	1	3.19	4.75				
1010	0.883	1	3.19	4.97				
1011	0.000	1	2.89	5.66				
1011	0.883	1	2.89	5.88				
1012	0.000	1	2.24	6.51				
1012	0.883	1	2.24	6.75				
1013	0.000	1	1.87	5.60				
1013	0.883	1	1.82	5.35				
1014	0.000	1	2.70	5.17				
1014	0.883	1	2.70	4.95				
1015	0.000	1	3.04	4.52				
1015	0.883	1	3.04	4.29				
1016	0.000	1	3.24	3.74				
1016	0.883	1	2.66	3.51				
1017	0.000	1	3.18	2.87				
1017	0.883	1	2.91	2.66				
1018	0.000	1	3.34	2.39				
1018	0.883	1	0.84	1.79				
1019	0.000	1	0.89	1.28				
1019	0.883	1	3.60	2.63				
1020	0.000	1	3.15	2.23				
1020	0.883	1	3.15	2.66				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
SLS(QUASI-PERMANENT)-CRACK w=0.30

Shear Reinforcements per Cutted Part of Section Accumulated minimum

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
1021	0.000	1	2.60	2.93				
1021	0.883	1	3.08	3.24				
1022	0.000	1	2.92	3.59				
1022	0.883	1	2.92	3.82				
1023	0.000	1	2.60	4.33				
1023	0.883	1	2.60	4.56				
1024	0.000	1	1.75	5.10				
1024	0.883	1	1.80	5.36				
1025	0.000	1	0.64	4.69				
1025	0.883	1	0.95	4.43				
1026	0.000	1	2.24	4.18				
1026	0.883	1	1.49	3.95				
1027	0.000	1	1.63	3.48				
1027	0.883	1	2.59	3.25				
1028	0.000	1	2.08	2.84				
1028	0.883	1	3.21	2.83				
1029	0.000	1	2.65	2.21				
1029	0.883	1	0.58	1.98				
1030	0.000	1	3.33	2.20				
1030	0.883	1	0.56	1.37				
1031	0.000	1	0.54	1.03				
1031	0.883	1	3.47	2.37				
1032	0.000	1	0.59	1.41				
1032	0.883	1	2.23	1.85				
1033	0.000	1	3.46	3.29				
1033	0.883	1	1.63	2.29				
1034	0.000	1	2.27	2.75				
1034	0.883	1	1.97	2.74				
1035	0.000	1	1.62	3.28				
1035	0.883	1	2.17	3.78				
1036	0.000	1	0.93	3.93				
1036	0.883	1	0.72	3.68				
1037	0.000	1	2.24	4.81				
1037	0.883	1	2.32	4.54				
1038	0.000	1	3.33	4.21				
1038	0.883	1	3.33	3.98				
1039	0.000	1	3.77	4.05				
1039	0.883	1	2.19	3.51				
1040	0.000	1	2.66	3.25				
1040	0.883	1	2.66	3.02				
1041	0.000	1	3.05	2.69				
1041	0.883	1	2.56	2.45				
1042	0.000	1	2.95	2.08				
1042	0.883	1	2.47	2.10				
1043	0.000	1	2.13	1.90				
1043	0.883	1	3.26	2.47				
1044	0.000	1	2.23	2.10				
1044	0.883	1	3.09	2.41				
1045	0.000	1	2.68	2.84				
1045	0.883	1	2.37	2.85				
1046	0.000	1	1.92	3.14				
1046	0.883	1	3.86	4.11				
1047	0.000	1	3.41	3.91				
1047	0.883	1	3.41	4.05				
1048	0.000	1	2.37	3.79				
1048	0.883	1	2.29	3.81				
1049	0.000	1	5.86	5.27				
1049	0.883	1	5.33	6.03				
1050	0.000	1	4.15	4.59				
1050	0.883	1	7.17	7.50				
1051	0.000	1	5.88	4.48				
1051	0.883	1	8.00	7.71				
1052	0.000	1	6.73	4.71				
1052	0.883	1	8.35	7.60				
1053	0.000	1	7.53	5.99				
1053	0.883	1	8.53	7.37				
1054	0.000	1	7.77	6.75				
1054	0.883	1	8.24	7.02				
1055	0.000	1	1.88	1.49				
1055	0.883	1	7.94	6.43				
1056	0.000	1	2.13	1.77				
1056	0.883	1	7.80	5.87				
1057	0.000	1	4.18	4.15				
1057	0.883	1	7.14	4.84				
1058	0.000	1	4.07	4.39				
1058	0.883	1	6.43	4.54				
1059	0.000	1	4.94	5.00				
1059	0.883	1	4.87	4.65				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
SLS(QUASI-PERMANENT)-CRACK w=0.30

Shear Reinforcements per Cutted Part of Section Accumulated minimum

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
1060	0.000	1	5.43	5.58				
1060	0.883	1	5.45	5.59				
2001	0.000	2	0.15	6.31				
2001	0.300	2	0.11	6.05				
2002	0.000	2	0.50	6.76				
2002	0.300	2	0.50	6.55				
2003	0.000	2	0.46	5.34				
2003	0.300	2	0.46	5.47				
2004	0.000	2	0.18	5.38				
2004	0.300	2	0.18	5.52				
2005	0.000	2	0.10	4.61				
2005	0.300	2	0.10	4.47				
2006	0.000	2	0.33	5.03				
2006	0.300	2	0.33	4.85				
2007	0.000	2	0.35	4.66				
2007	0.300	2	0.35	4.83				
2008	0.000	2	0.10	4.26				
2008	0.300	2	0.10	4.41				
2009	0.000	2	0.22	4.86				
2009	0.300	2	0.22	4.73				
2010	0.000	2	0.13	5.06				
2010	0.300	2	0.13	4.89				
2011	0.000	2	0.13	3.48				
2011	0.300	2	0.13	3.67				
2012	0.000	2	0.21	3.33				
2012	0.300	2	0.21	3.40				
2013	0.000	2	0.98	5.15				
2013	0.300	2	0.93	4.94				
2014	0.000	2	0.48	5.11				
2014	0.300	2	0.48	4.95				
2015	0.000	2	0.49	3.38				
2015	0.300	2	0.49	3.56				
2016	0.000	2	1.08	4.00				
2016	0.300	2	0.85	4.04				
2017	0.000	2	1.68	3.68				
2017	0.300	2	1.68	3.57				
2018	0.000	2	3.59	5.53				
2018	0.300	2	3.59	5.52				
2019	0.000	2	0.99	3.45				
2019	0.300	2	0.99	3.64				
2020	0.000	2	0.18	2.60				
2020	0.300	2	0.18	3.26				
10001	0.000	8	0.93	1.76				
10001	0.200	8	0.80	1.89				
10005	0.000	8	3.35	5.83				
10005	0.200	8	3.65	6.87				
10006	0.000	8	0.24	5.43				
10006	0.400	8	0.30	5.57				
10009	0.000	8	0.44	4.48				
10009	0.400	8	0.39	4.55				
10010	0.000	8	3.58	7.12				
10010	0.200	8	3.71	8.02				
10014	0.000	8	0.80	1.45				
10014	0.200	8	0.80	1.33				
10016	0.000	8	0.92	1.76				
10016	0.200	8	0.86	1.89				
10020	0.000	8	3.34	5.62				
10020	0.200	8	3.63	6.65				
10021	0.000	8	0.25	5.24				
10021	0.400	8	0.30	5.38				
10024	0.000	8	0.36	3.98				
10024	0.400	8	0.36	4.44				
10025	0.000	8	3.51	7.07				
10025	0.200	8	4.02	8.06				
10029	0.000	8	0.83	1.46				
10029	0.200	8	0.79	1.31				

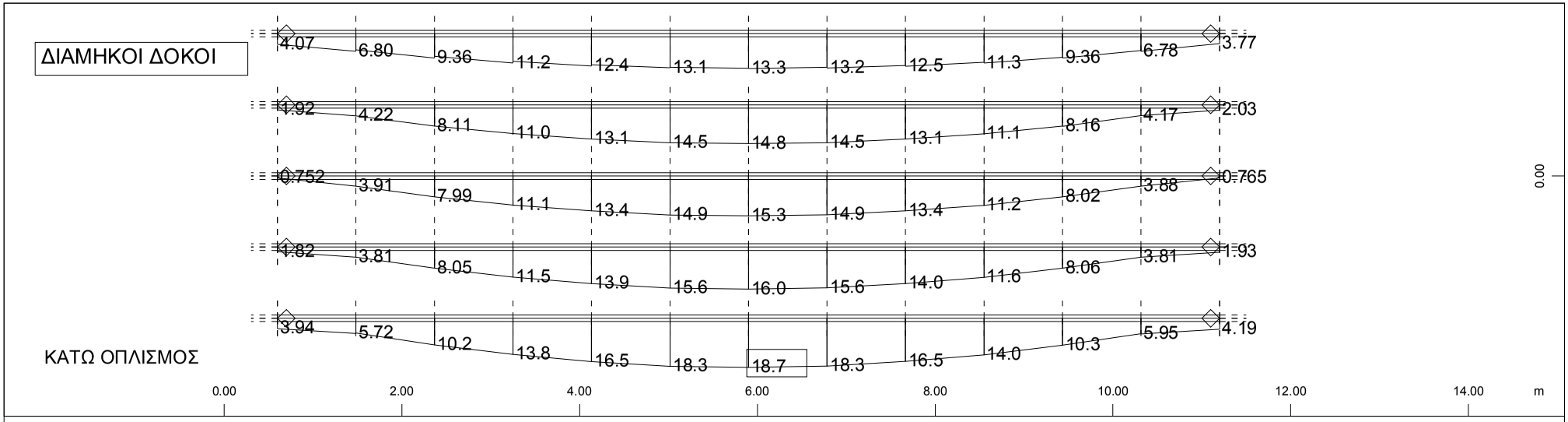
Maximum Degree of Utilization

		N sig-c	Vy sig-t	Vz tau	Mt sig-*	My tend.	Mz As-l	Mb As-v	Mt2 crack	Total sigdyn	lamda tau-*
Cross sect.	1	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
		0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.802	0.000	0.000
Cross sect.	2	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
DOKOS-2		0.000	0.000	0.000	0.000	0.000	0.000	0.000	2.260	0.000	0.000
Cross sect.	8	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
DOKOS-4		0.000	0.000	0.000	0.000	0.000	0.000	0.000	9.173	0.000	0.000

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
SLS(QUASI-PERMANENT)-CRACK w=0.30

Maximum Degree of Utilization

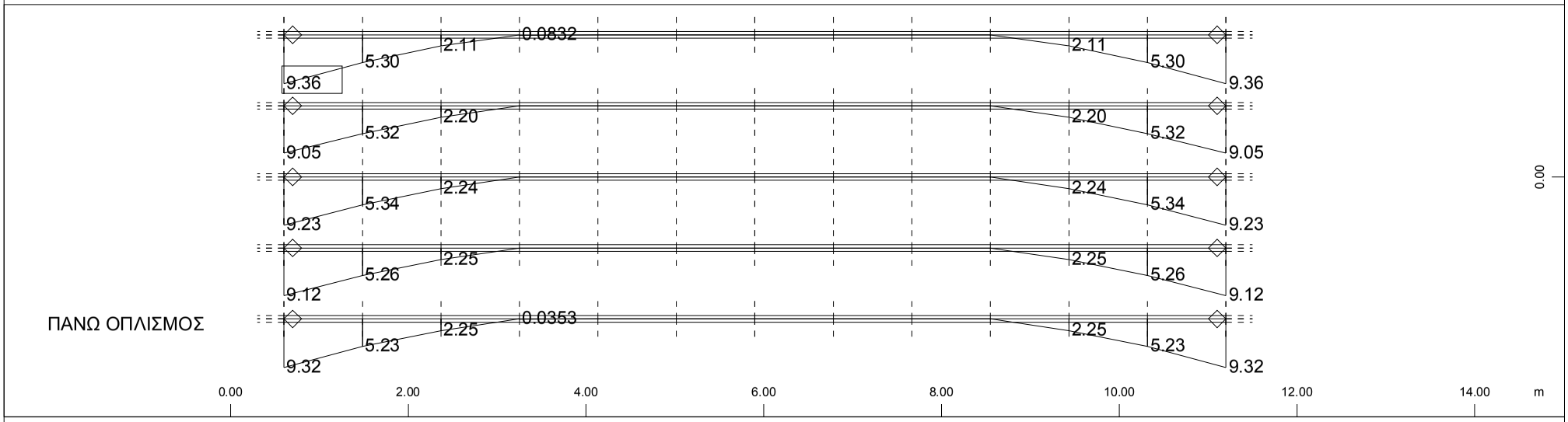
	N sig-c	Vy sig-t	Vz tau	Mt sig-*	My tend.	Mz As-l	Mb As-v	Mt2 crack	Total sigdyn	lamda tau-*
Total System	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
	0.000	0.000	0.000	0.000	0.000	0.000	0.000	9.173	0.000	0.000



Z X
Y

Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case 508 , 1 cm 3D = 22.4 cm2 (Max=18.7)

M 1 : 66

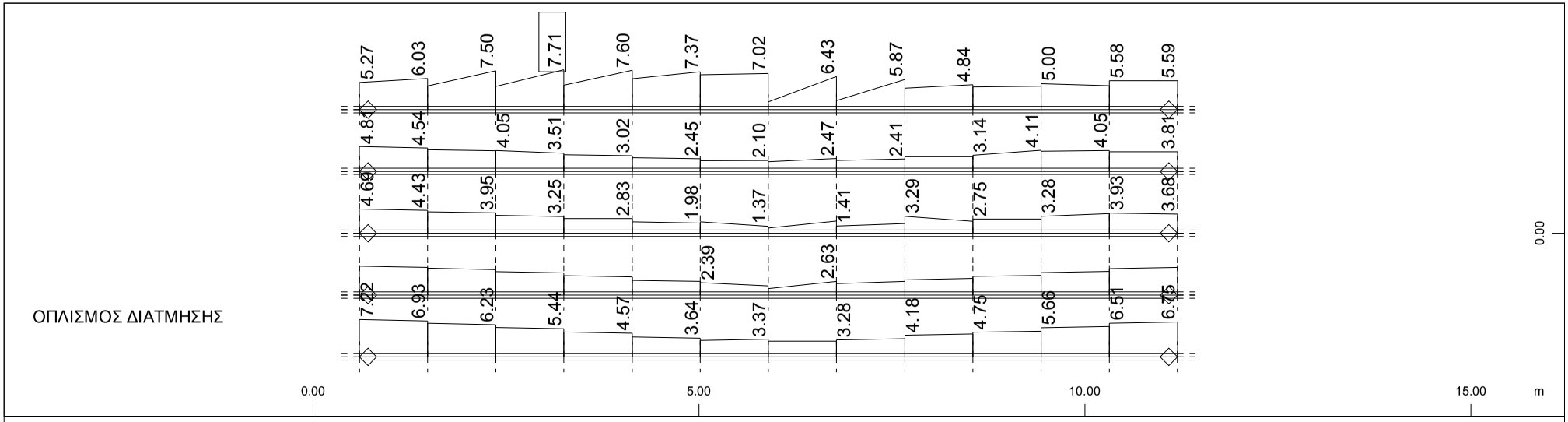


Z X
Y

Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case 508 , 1 cm 3D = 11.2 cm2 (Max=9.36)

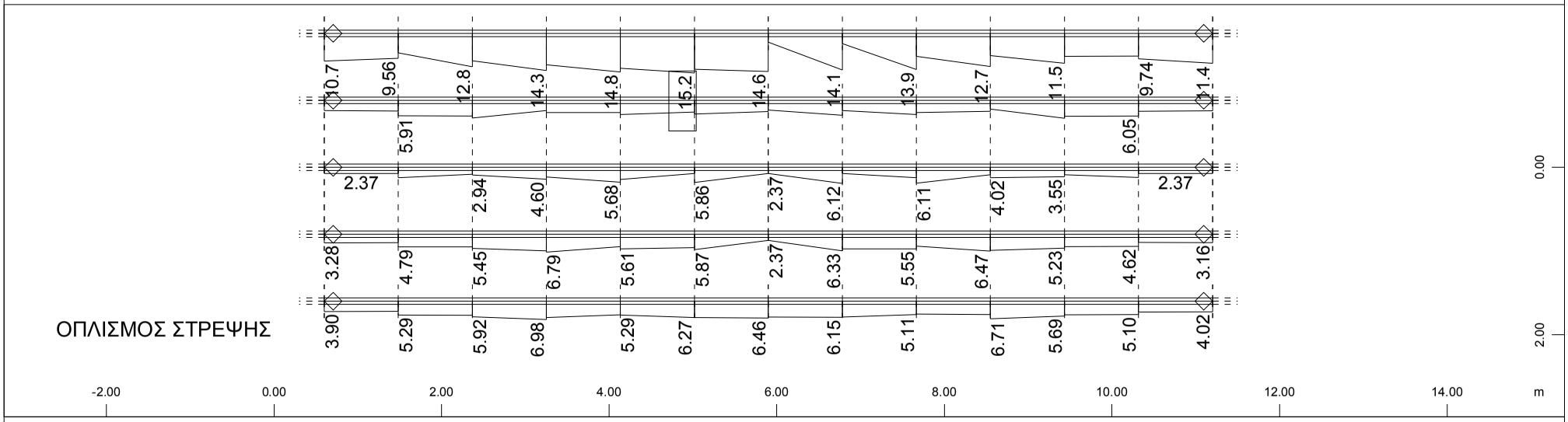
M 1 : 66

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΦΑΣΗ-2_SLS



Sector of system Beam Elements Group 1
Beam Elements , Shear reinforcements (maximum), Design Case 508 , 1 cm 3D = 11.2 cm2/m (Max=7.71)

M 1 : 76

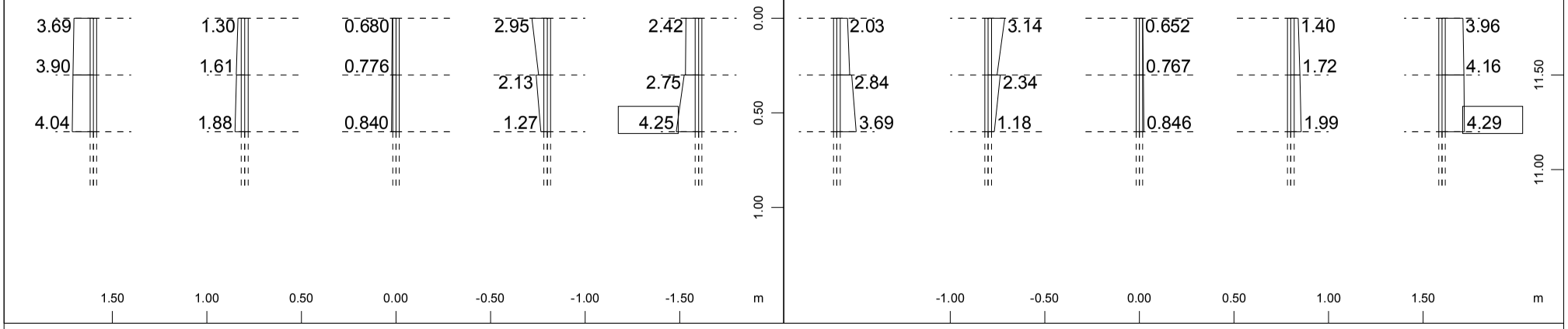


Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case 508 , 1 cm 3D = 22.4 cm2 (Max=15.2)

M 1 : 70

ΔΙΑΜΗΚΗ ΔΟΚΟΙ-ΣΤΗΡΙΞΗΣ

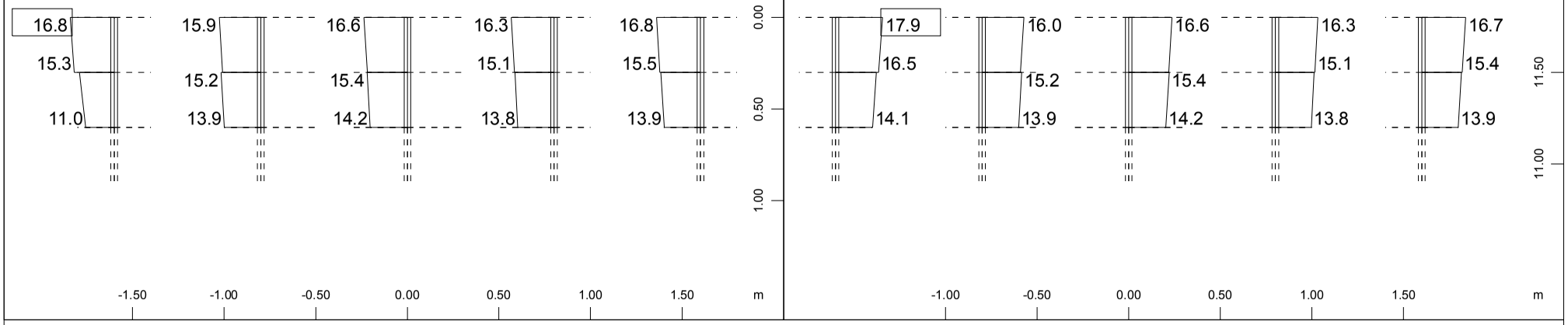
ΚΑΤΩ ΟΠΛΙΣΜΟΣ



Y Z X
Sector of system Beam Elements Group 2 M 1 : 31
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
508 , 1 cm 3D = 11.2 cm2 (Max=4.25)

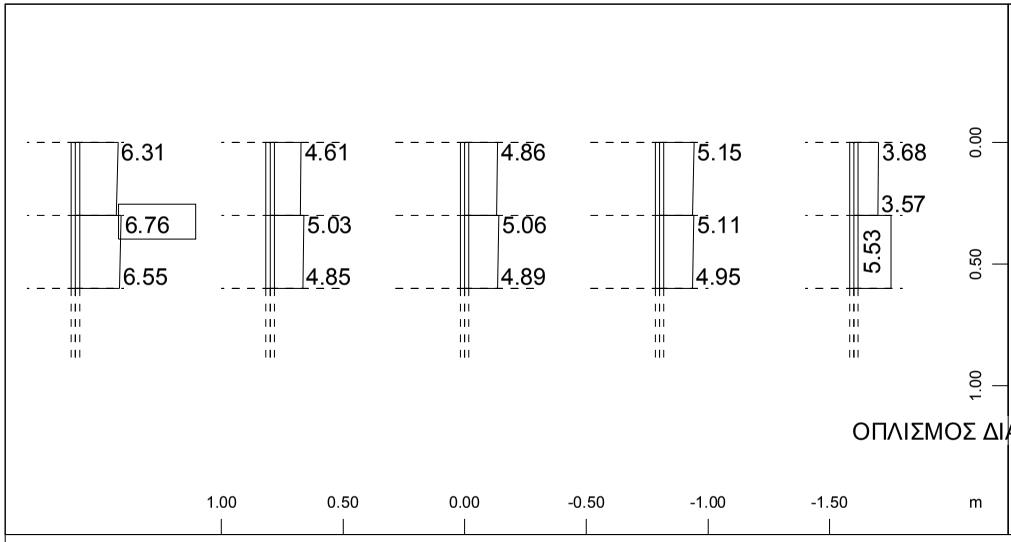
X Z Y
Sector of system Beam Elements Group 2 M 1 : 31
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
508 , 1 cm 3D = 11.2 cm2 (Max=4.29)

ΠΑΝΩ ΟΠΛΙΣΜΟΣ



Z Y X
Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
508 , 1 cm 3D = 22.4 cm2 (Max=16.8)

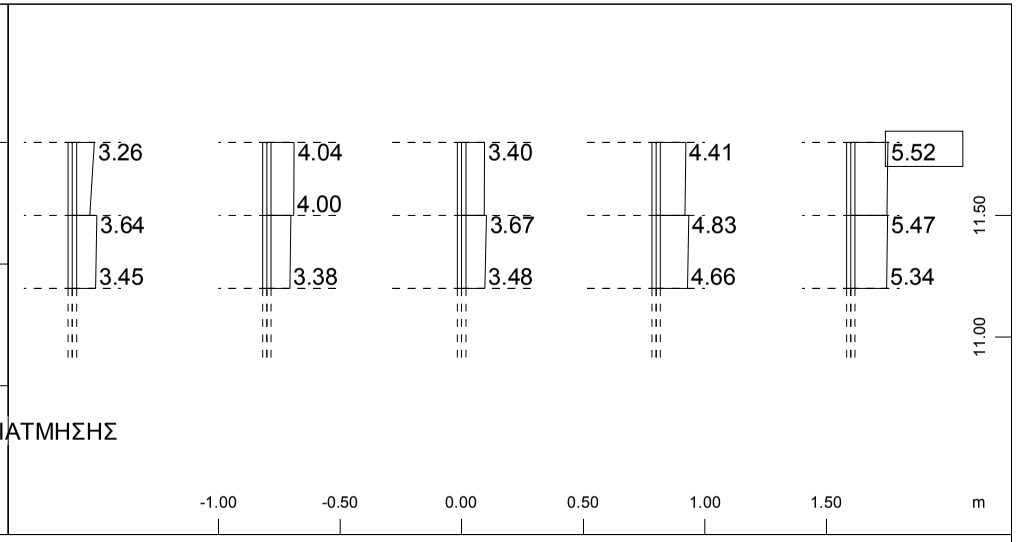
X Z Y
Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
508 , 1 cm 3D = 22.4 cm2 (Max=17.9)



ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ

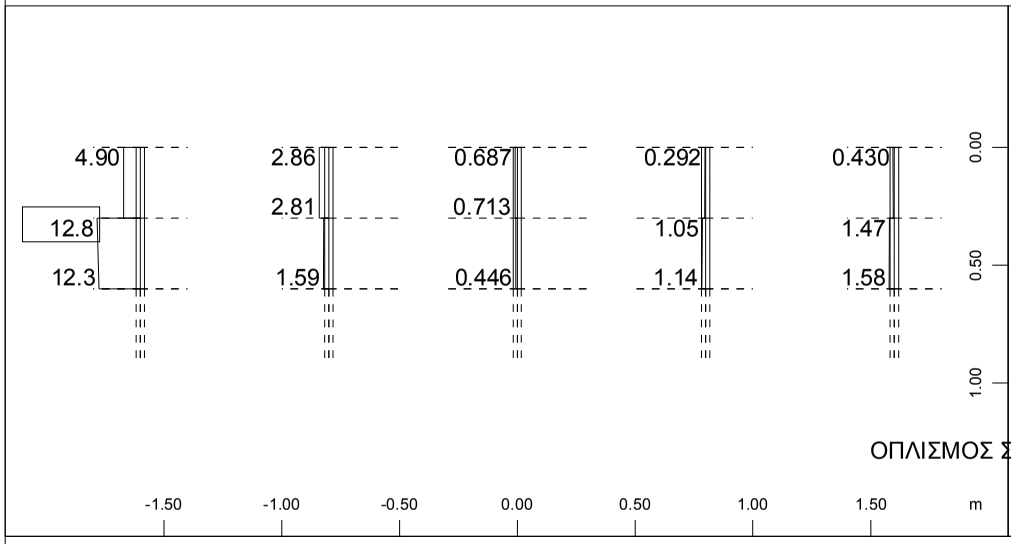
Y Z X

Sector of system Beam Elements Group 2 M 1 : 31
Beam Elements , Shear reinforcements (maximum), Design Case 508
, 1 cm 3D = 11.2 cm2/m (Max=6.76)



X Z Y

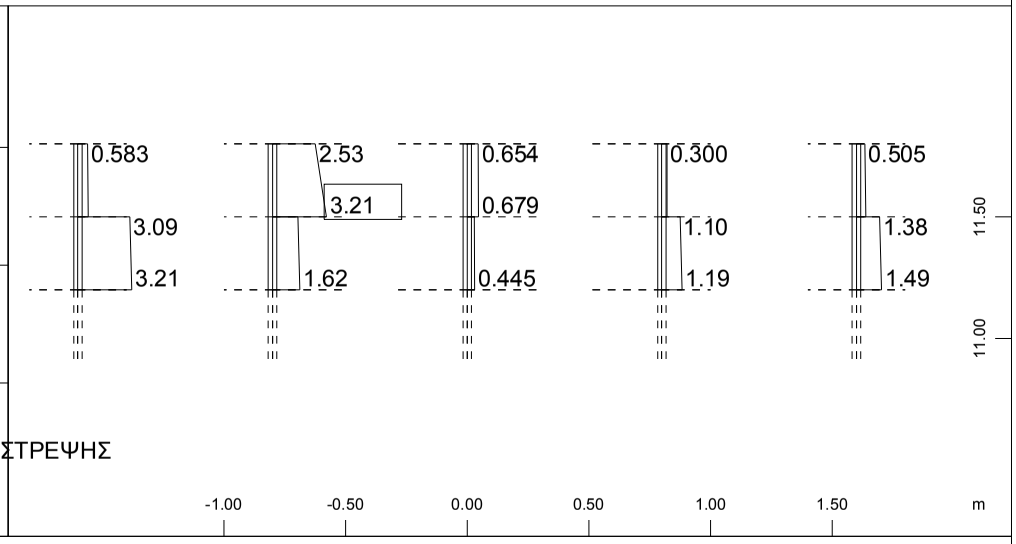
Sector of system Beam Elements Group 2 M 1 : 31
Beam Elements , Shear reinforcements (maximum), Design Case 508
, 1 cm 3D = 11.2 cm2/m (Max=5.52)



ΟΠΛΙΣΜΟΣ ΣΤΡΕΨΗΣ

Z Y X

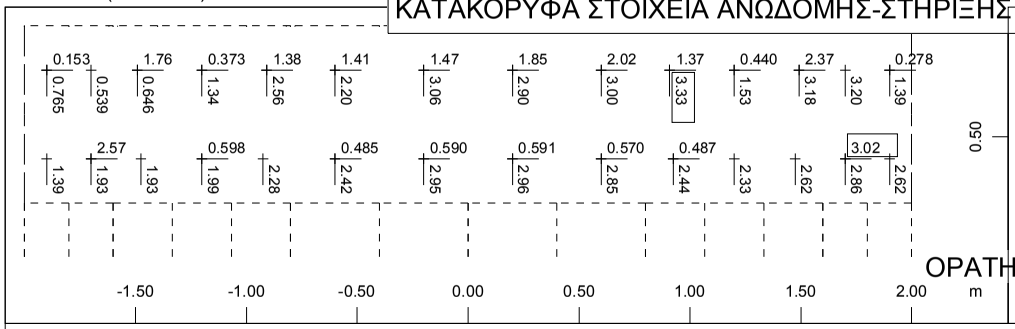
Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
508 , 1 cm 3D = 22.4 cm2 (Max=12.8)



X Z Y

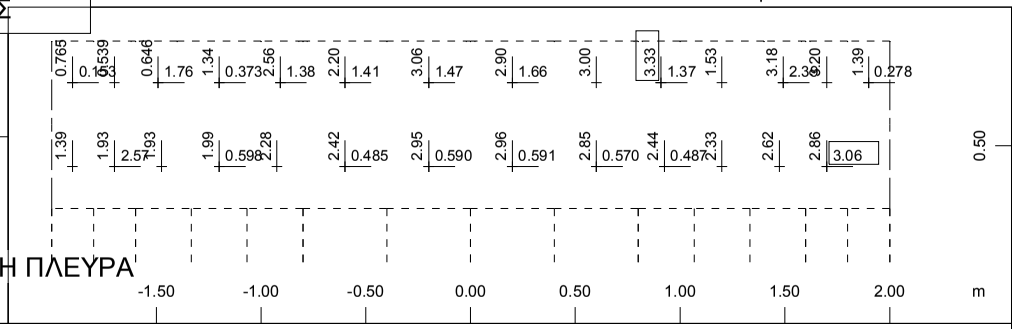
Sector of system Beam Elements Group 2 M 1 : 31
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
508 , 1 cm 3D = 4.48 cm2 (Max=3.21)

ΚΑΤΑΚΟΡΥΦΑ ΣΤΟΙΧΕΙΑ ΑΝΩΔΟΜΗΣ-ΣΤΗΡΙΞΗΣ

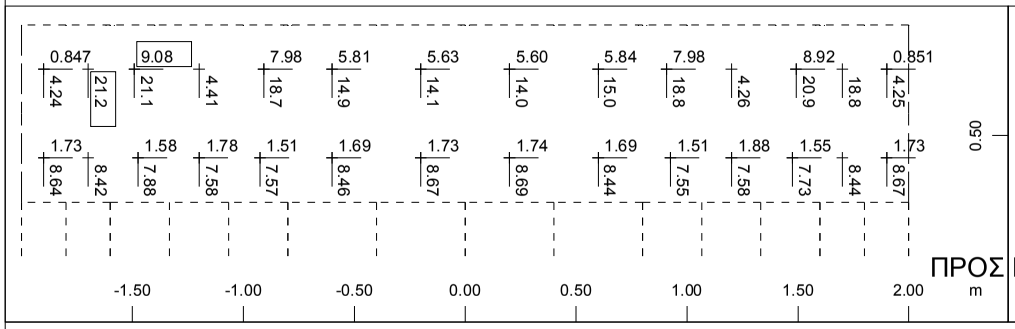


Sector of system Quadrilateral Elements Group 8 M 1 : 34
upper Reinforcements in Elements in cm2/m, Design Case 521 ULS
design (Max=3.33)

ΟΡΑΤΗ ΠΛΕΥΡΑ

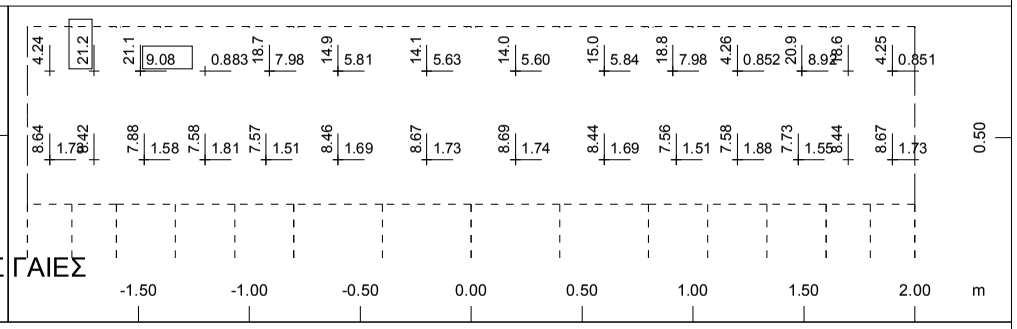


Sector of system Quadrilateral Elements Group 8 M 1 : 36
upper Reinforcements in Elements in cm2/m, Design Case 521 ULS
design (Max=3.33)

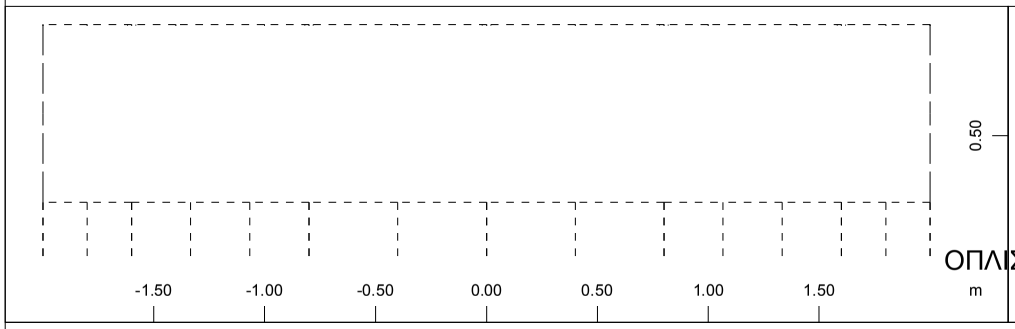


Sector of system Quadrilateral Elements Group 8 M 1 : 34
lower Reinforcements in Elements in cm2/m, Design Case 521 ULS
design (Max=21.2)

ΠΡΟΣ ΓΑΙΕΣ

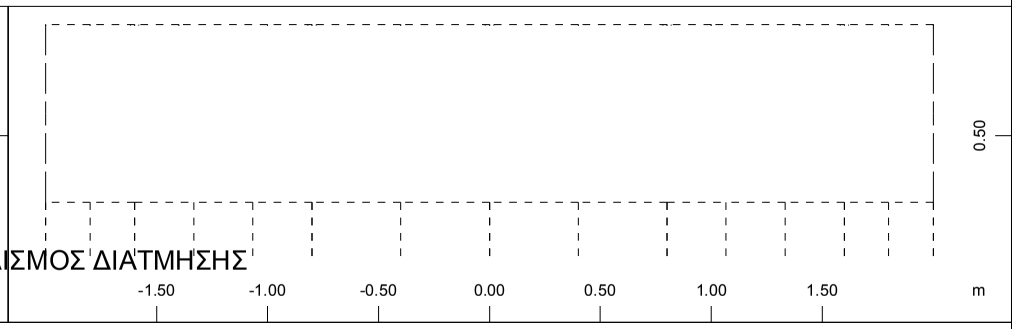


Sector of system Quadrilateral Elements Group 8 M 1 : 34
lower Reinforcements in Elements in cm2/m, Design Case 521 ULS
design (Max=21.2)



Sector of system Quadrilateral Elements Group 8 M 1 : 34
Shear reinforcement from middle of element in cm2/m2, Design
Case 521 ULS design (Max=0)

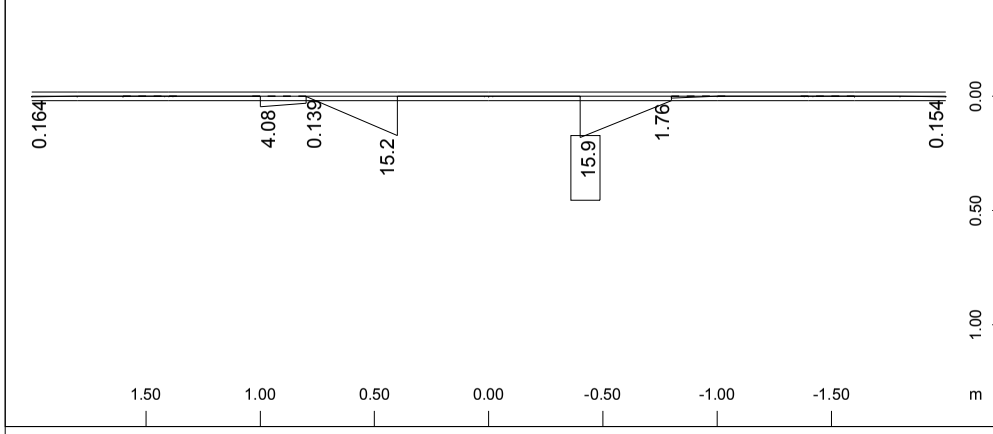
ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ



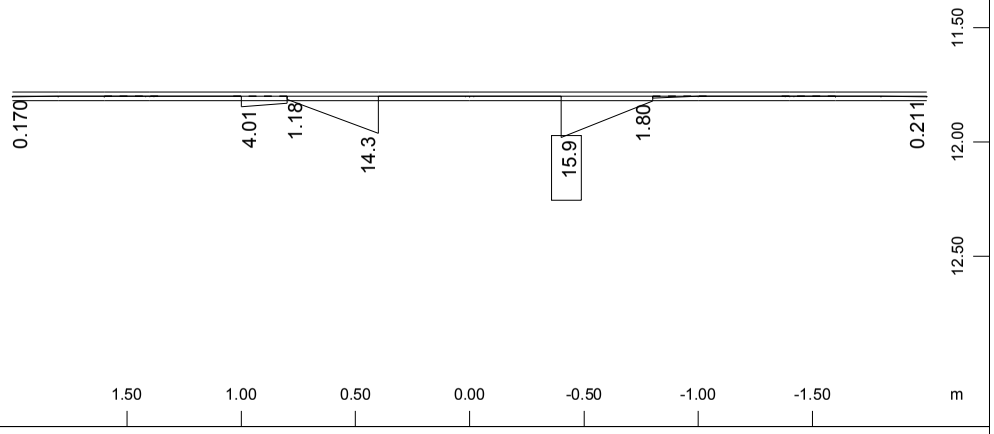
Sector of system Quadrilateral Elements Group 8 M 1 : 34
Shear reinforcement from middle of element in cm2/m2, Design
Case 521 ULS design (Max=0)

ΠΑΣΣΑΛΟΔΕΣΜΟΣ

ΚΑΤΩ ΟΠΛΙΣΜΟΣ

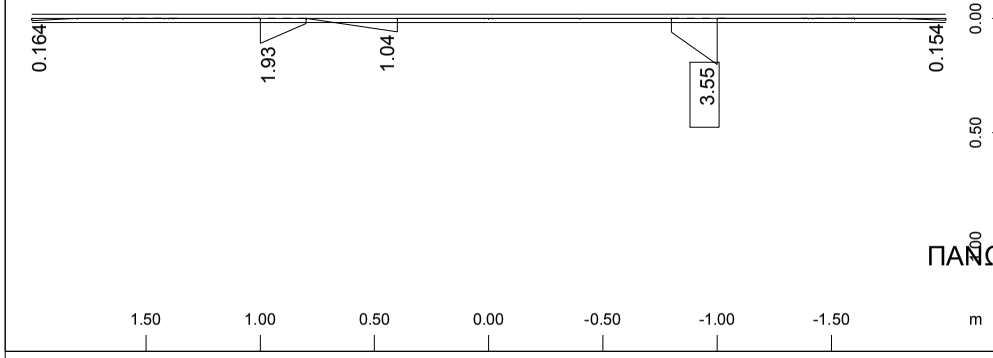


Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
508 , 1 cm 3D = 29.1 cm2 (Max=15.9)

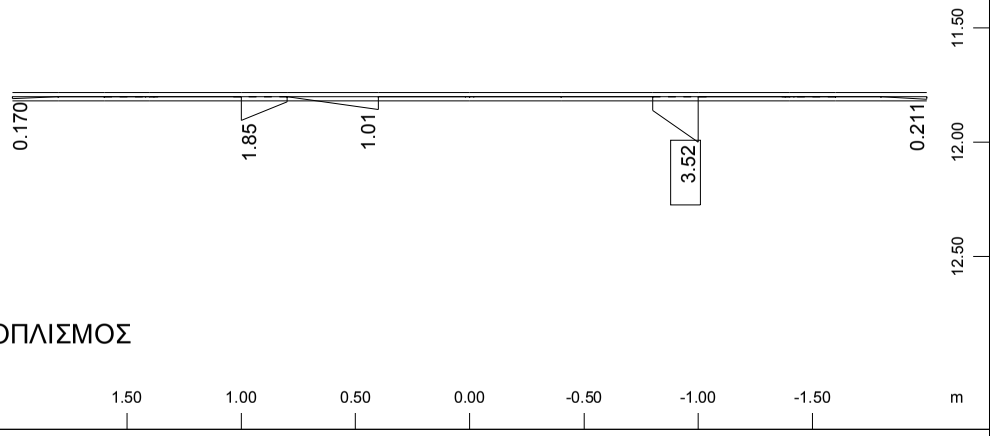


Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
508 , 1 cm 3D = 29.1 cm2 (Max=15.9)

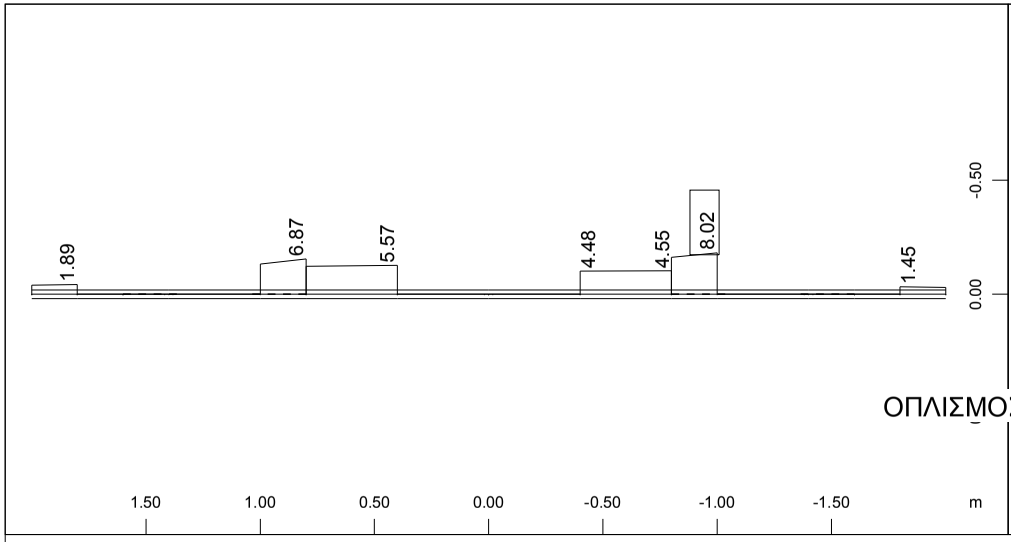
ΠΑΝΩ ΟΠΛΙΣΜΟΣ



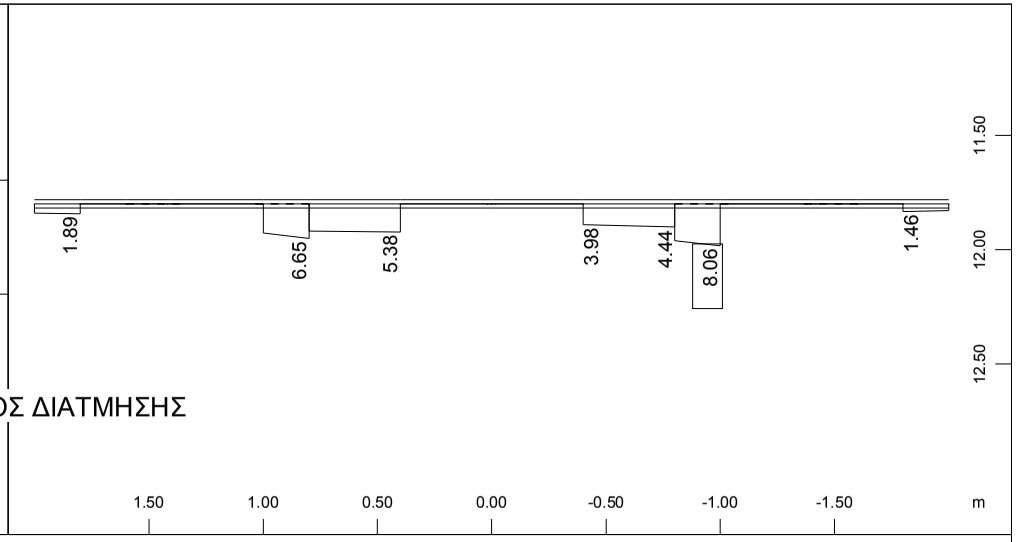
Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
508 , 1 cm 3D = 5.81 cm2 (Max=3.55)



Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
508 , 1 cm 3D = 5.81 cm2 (Max=3.52)

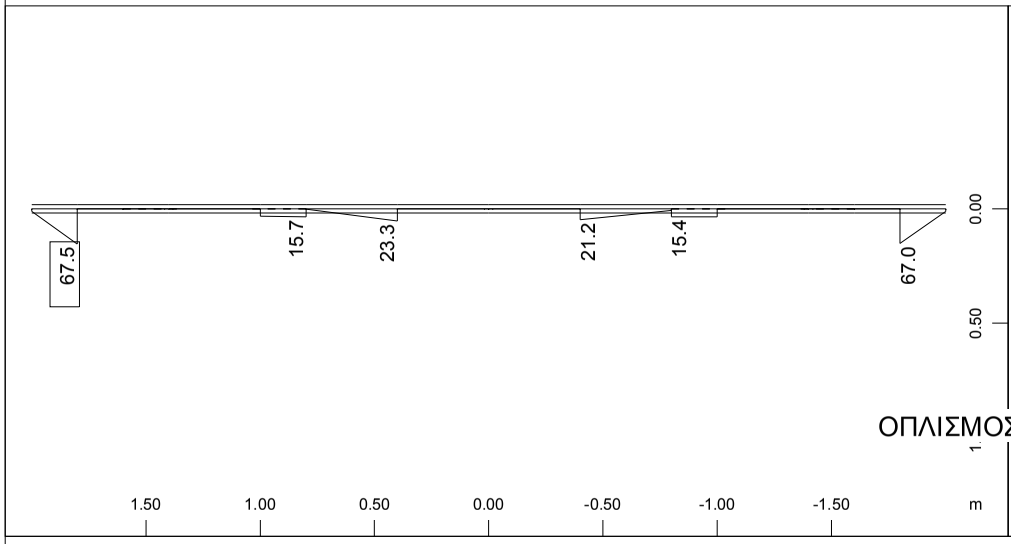


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Shear reinforcements (maximum), Design Case 508
, 1 cm 3D = 14.5 cm2/m (Max=8.02)

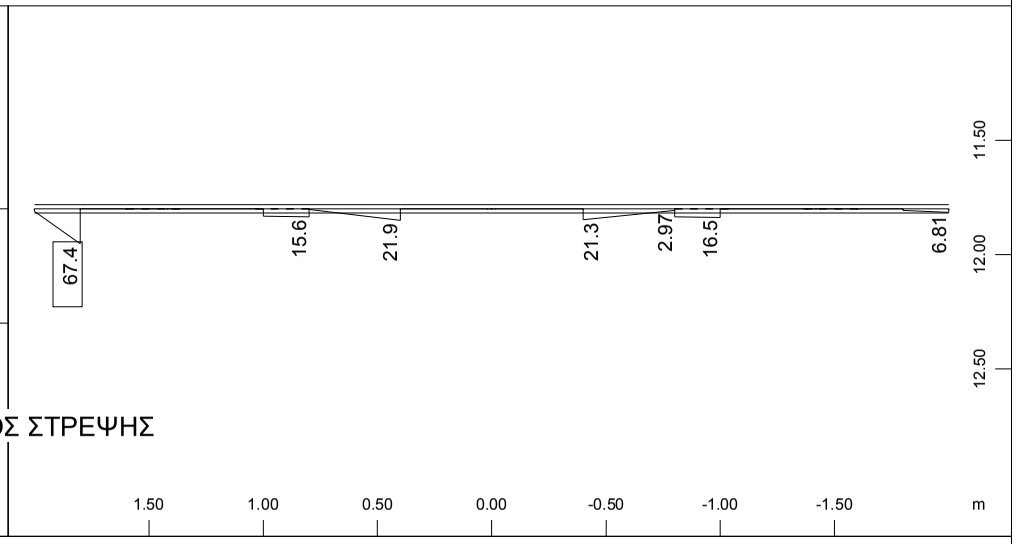


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Shear reinforcements (maximum), Design Case 508
, 1 cm 3D = 14.5 cm2/m (Max=8.06)

ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ

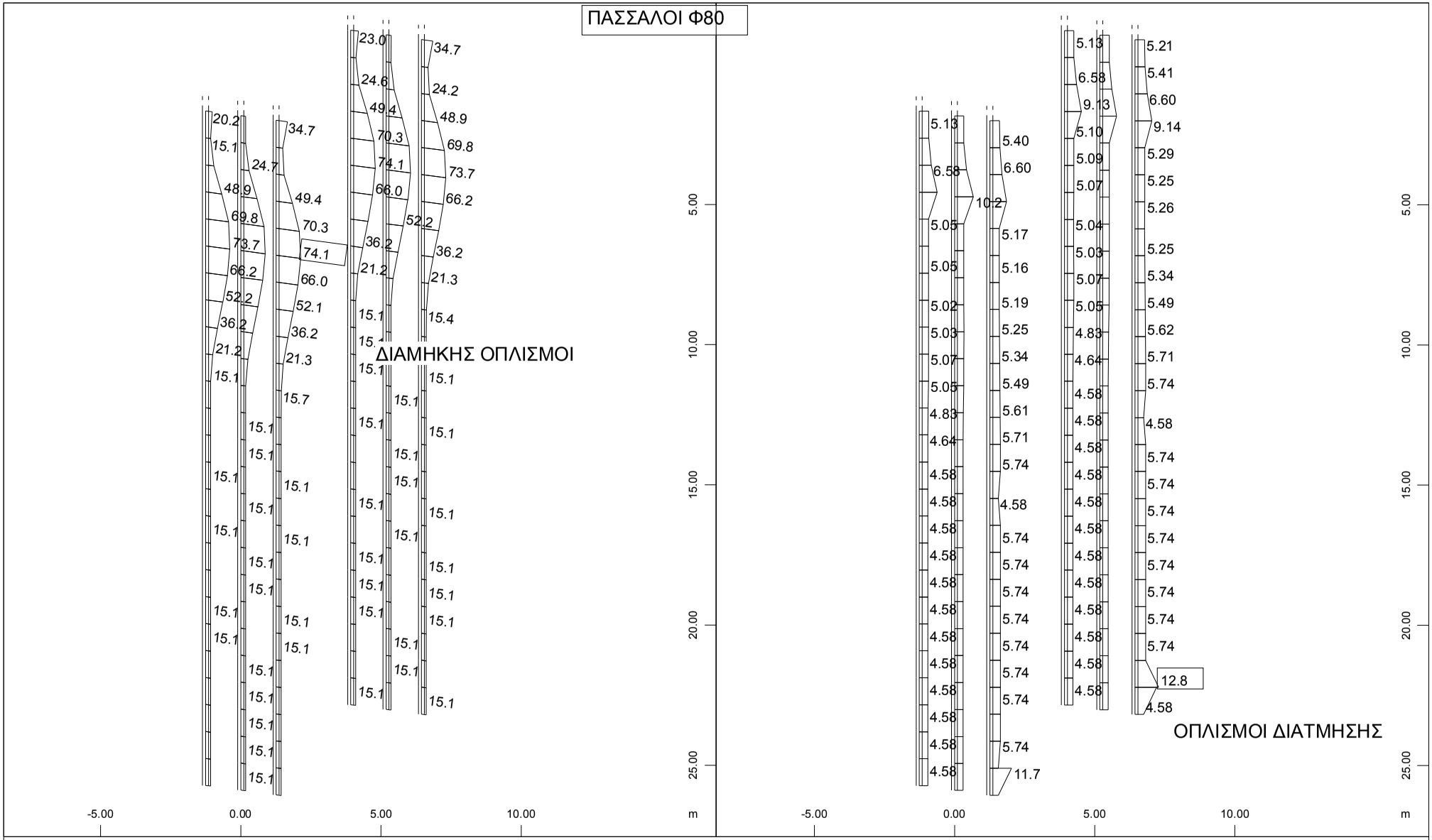


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
508 , 1 cm 3D = 145.3 cm2 (Max=67.5)



Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
508 , 1 cm 3D = 145.3 cm2 (Max=67.4)

ΟΠΛΙΣΜΟΣ ΣΤΡΕΨΗΣ



Sector of system Beam Elements Group 12
Beam Elements , Longitudinal Reinforcements Lay. 1, Design
Case 508 , 1 cm 3D = 145.3 cm2 (Max=74.1)

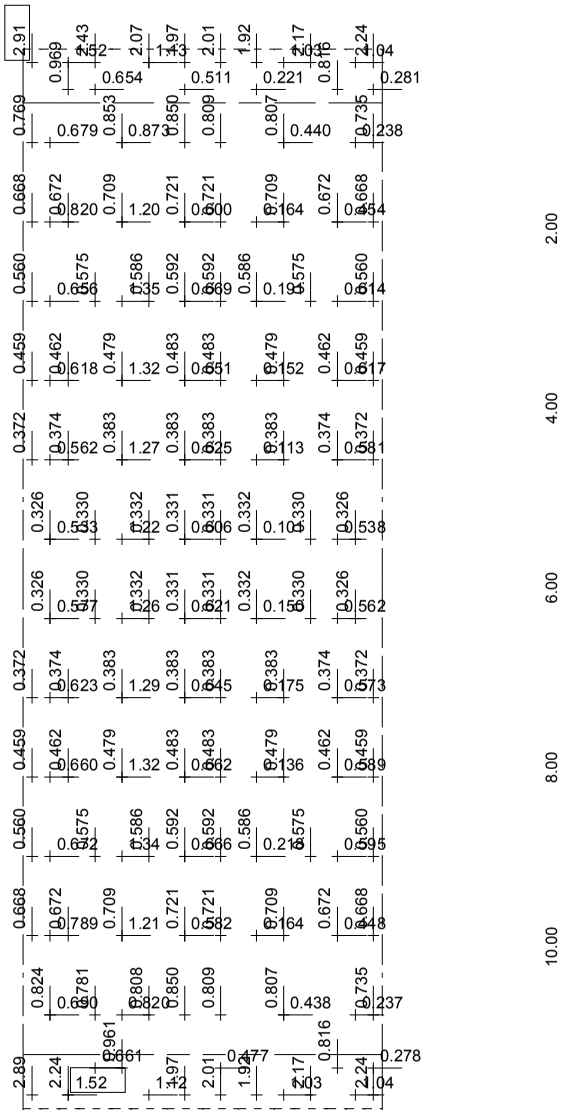
M 1 : 191
X * 0.502
Y * 0.906
Z * 0.962

Sector of system Beam Elements Group 12
Beam Elements , Shear reinforcements (maximum), Design Case
508 , 1 cm 3D = 29.1 cm2/m (Max=12.8)

M 1 : 191
X * 0.502
Y * 0.906
Z * 0.962

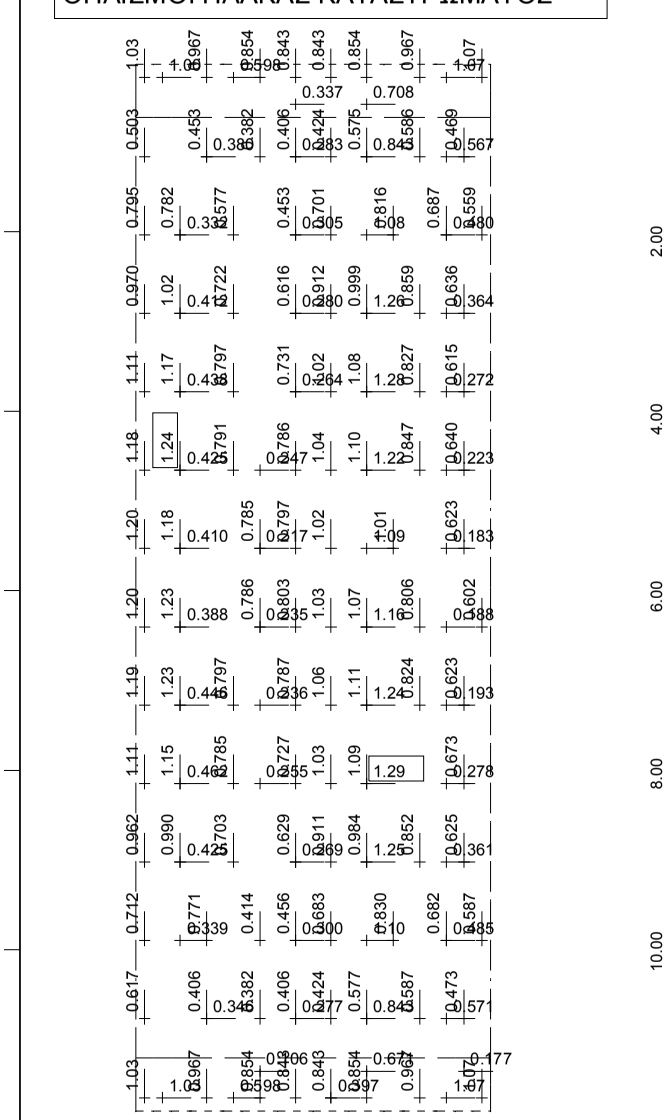
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΦΑΣΗ-2_SLS

ΟΠΛΙΣΜΟΙ ΠΛΑΚΑΣ ΚΑΤΑΣΤΡΩΜΑΤΟΣ



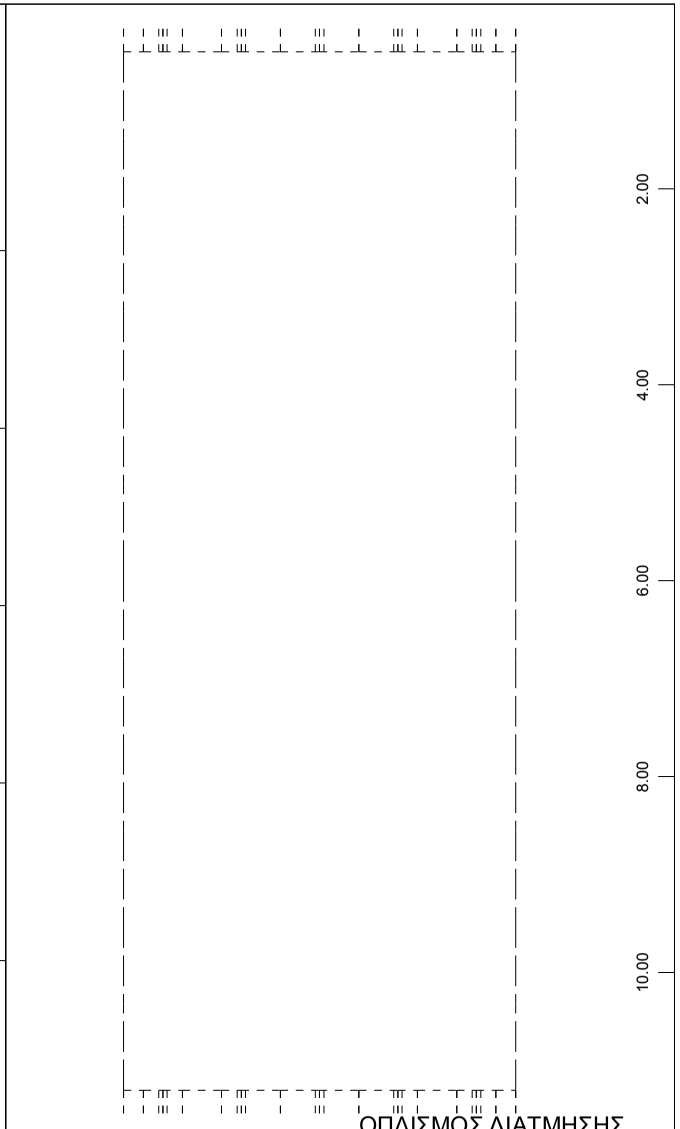
Sector of system Quadrilateral Elements Group 3 4
upper Reinforcements in Elements in cm2/m, Design Case
521 ULS design (Max=2.91)

M 1 : 84



Sector of system Quadrilateral Elements Group 3 4
lower Reinforcements in Elements in cm2/m, Design Case
521 ULS design (Max=1.29)

M 1 : 85



Sector of system Quadrilateral Elements Group 3
Shear reinforcement from middle of element in cm2/m2,
Design Case 521 ULS design (Max=0)

M 1 : 77

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΦΑΣΗ-2_SLS

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
COMBINATION SLS

Combination rule Number 1
COMB.ULS-1-BEAM
Resulting loadcases type Design Combination
Loadcase selection

Number	factor	type		Title
31	1.00	Exclusive LC	AG	ΡΥΣΕΙΣ-ΟΔΟΣΤΡΩΣΙΑ
32	1.00	Combined with LC		ΡΕΖΟDROMIO
33	1.00	Combined with LC		ΠΡΟΣΘΕΤΗ ΟΔΟΣΤΡΩΣΙΑ 0.50
31	1.00	Exclusive LC	AG	ΡΥΣΕΙΣ-ΟΔΟΣΤΡΩΣΙΑ
32	1.00	Combined with LC		ΡΕΖΟDROMIO
33	1.00	Combined with LC		ΠΡΟΣΘΕΤΗ ΟΔΟΣΤΡΩΣΙΑ 0.50
36	1.00	Exclusive LC	A 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+0.5
37	1.00	Exclusive LC	A 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+0.5
38	1.00	Exclusive LC	A 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+1.0
39	1.00	Exclusive LC	A 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+1.0
1101	1.00	Exclusive LC	A 2	MAX-MY BEAM
1102	1.00	Exclusive LC	A 2	MIN-MY BEAM
1103	1.00	Exclusive LC	A 2	MAX-VZ BEAM
1104	1.00	Exclusive LC	A 2	MIN-VZ BEAM
1105	1.00	Exclusive LC	A 2	MAX-MZ BEAM
1106	1.00	Exclusive LC	A 2	MIN-MZ BEAM
1107	1.00	Exclusive LC	A 2	MAX-VY BEAM
1108	1.00	Exclusive LC	A 2	MIN-VY BEAM
1109	1.00	Exclusive LC	A 2	MAX-N BEAM
1110	1.00	Exclusive LC	A 2	MIN-N BEAM
1111	1.00	Exclusive LC	A 2	MAX-MT BEAM
1112	1.00	Exclusive LC	A 2	MIN-MT BEAM
1121	1.00	Exclusive LC	A 2	MAX-MY BEAM
1122	1.00	Exclusive LC	A 2	MIN-MY BEAM
1123	1.00	Exclusive LC	A 2	MAX-VZ BEAM
1124	1.00	Exclusive LC	A 2	MIN-VZ BEAM
1125	1.00	Exclusive LC	A 2	MAX-MZ BEAM
1126	1.00	Exclusive LC	A 2	MIN-MZ BEAM
1127	1.00	Exclusive LC	A 2	MAX-VY BEAM
1128	1.00	Exclusive LC	A 2	MIN-VY BEAM
1129	1.00	Exclusive LC	A 2	MAX-N BEAM
1130	1.00	Exclusive LC	A 2	MIN-N BEAM
1131	1.00	Exclusive LC	A 2	MAX-MT BEAM
1132	1.00	Exclusive LC	A 2	MIN-MT BEAM
80	1.00	Conditional LC		ΠΙΘΑΝΕΣ ΚΑΘΙΖΗΣΕΙΣ A1
81	1.00	Conditional LC		ΠΙΘΑΝΕΣ ΚΑΘΙΖΗΣΕΙΣ A2

Generated Loadcases

Number	Comb	Title
1301	1	MAX-MY BEAM
1302	1	MIN-MY BEAM
1303	1	MAX-VZ BEAM
1304	1	MIN-VZ BEAM
1305	1	MAX-MZ BEAM
1306	1	MIN-MZ BEAM
1307	1	MAX-VY BEAM
1308	1	MIN-VY BEAM
1309	1	MAX-N BEAM
1310	1	MIN-N BEAM
1311	1	MAX-MT BEAM
1312	1	MIN-MT BEAM

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CHARACTERISTIC COMBINATION (0.60*fck),(0.80*fyk)

Selected Beam Elements

FROM	TO	INC	X-VALUE	NC	MEMBER	CS0	CS1	CS2	CS3	CS4	CS5
1000	1060	1		1		10	40				
2000	2020	1									

Default design code is DIN Fachbericht 102 Massivbrȯcken (2003) (Germany)
Klasse(Tab.4.118): D
Wind zone : Binnenland

Materials

No. 1 C 25/30 (DIN 1045-1)
No. 3 C 25/30 (DIN 1045-1)
No. 4 C 25/30 (DIN 1045-1)
No. 5 C 25/30 (DIN 1045-1)
No. 6 C 25/30 (DIN 1045-1)
No. 7 C 25/30 (DIN 1045-1)
No. 8 C 25/30 (DIN 1045-1)
No. 9 C 25/30 (DIN 1045-1)
No. 10 C 25/30 (DIN 1045-1)
No. 12 BSt 500 SA (DIN 1045-1)

Reinforcement will be accounted for sectional values as defined in AQUA
Reinforcements saved as design case LCR 1

Considered Load Cases

No.	refer	act on	Title/type of load case	gam-u	gam-f	psi-0	psi-1	psi-2	psi-1'	
1	part.	CS 0	I.B. ΚΑΤΑΚ.ΣΤΟΙΧΕΙΩΝ G (total dead load)	1.35	1.00	1.00	1.00	1.00	1.00	G perm
2	part.	CS 0	I.B. ΔΟΚΩΝ G (total dead load)	1.35	1.00	1.00	1.00	1.00	1.00	G perm
3	part.	CS 0	I.B. ΧΥΤΗΣ ΠΛΑΚΑΣ G (total dead load)	1.35	1.00	1.00	1.00	1.00	1.00	G perm
11	part.	CS 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+0.5 L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
12	part.	CS 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+0.5 L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
13	part.	CS 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:0.5*A1+1.0 L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
14	part.	CS 1	ΩΘΗΣΕΙΣ ΓΑΙΩΝ:1.0*A1+1.0 L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
90	gross	CS 1	(+ΔΤΝ)+0.75*(+ΔΤΜ) L_C (Traffic load UIC of EC/DIN-FB)	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
91	gross	CS 1	(+ΔΤΝ)+0.75*(-ΔΤΜ) L_C (Traffic load UIC of EC/DIN-FB)	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
92	gross	CS 1	(-ΔΤΝ)+0.75*(+ΔΤΜ) L_C (Traffic load UIC of EC/DIN-FB)	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
93	gross	CS 1	(-ΔΤΝ)+0.75*(-ΔΤΜ) L_C (Traffic load UIC of EC/DIN-FB)	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
94	gross	CS 1	0.35*(+ΔΤΝ)+(+ΔΤΜ) L_C (Traffic load UIC of EC/DIN-FB)	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
95	gross	CS 1	0.35*(+ΔΤΝ)+(-ΔΤΜ) L_C (Traffic load UIC of EC/DIN-FB)	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
96	gross	CS 1	0.35*(-ΔΤΝ)+(+ΔΤΜ) L_C (Traffic load UIC of EC/DIN-FB)	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
97	gross	CS 1	0.35*(-ΔΤΝ)+(-ΔΤΜ) L_C (Traffic load UIC of EC/DIN-FB)	1.45	0.00	0.80	0.80	0.00	1.00	Q excl
1301	part.	CS 1	MAX-MY BEAM L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
1302	part.	CS 1	MIN-MY BEAM L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
1303	part.	CS 1	MAX-VZ BEAM L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
1304	part.	CS 1	MIN-VZ BEAM L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
1305	part.	CS 1	MAX-MZ BEAM L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
1306	part.	CS 1	MIN-MZ BEAM L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
1307	part.	CS 1	MAX-VY BEAM L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
1308	part.	CS 1	MIN-VY BEAM L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
1309	part.	CS 1	MAX-N BEAM L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
1310	part.	CS 1	MIN-N BEAM L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
1311	part.	CS 1	MAX-MT BEAM L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
1312	part.	CS 1	MIN-MT BEAM L (live loading)	1.50	0.00	0.75	0.75	0.20	0.80	Q excl
5015	part.	CS 0	K creep step C (creep + shrinkage)	1.00	1.00	1.00	1.00	1.00	1.00	P perm

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CHARACTERISTIC COMBINATION (0.60*fck),(0.80*fyk)

Considered Load Cases

No.	refer	act on	Title/type of load case	gam-u	gam-f	psi-0	psi-1	psi-2	psi-1'	
5025	part.	CS 0	K creep step	1.00	1.00	1.00	1.00	1.00	1.00	P perm
			C (creep + shrinkage)							
5055	part.	CS 1	K creep step	1.00	1.00	1.00	1.00	1.00	1.00	P perm
			C (creep + shrinkage)							
5060	part.	CS 1	K creep step	1.00	1.00	1.00	1.00	1.00	1.00	P perm
			C (creep + shrinkage)							
5061	part.	CS 1	K creep step	1.00	1.00	1.00	1.00	1.00	1.00	P perm
			C (creep + shrinkage)							
5062	part.	CS 1	K creep step	1.00	1.00	1.00	1.00	1.00	1.00	P perm
			C (creep + shrinkage)							
5063	part.	CS 1	K creep step	1.00	1.00	1.00	1.00	1.00	1.00	P perm
			C (creep + shrinkage)							
5064	part.	CS 1	K creep step	1.00	1.00	1.00	1.00	1.00	1.00	P perm
			C (creep + shrinkage)							
6015	part.	CS 0	15 K creep step							P perm
			C (creep + shrinkage)							
6025	part.	CS 0	25 K creep step							P perm
			C (creep + shrinkage)							
6055	part.	CS 1	55 K creep step							P perm
			C (creep + shrinkage)							
6060	part.	CS 1	60 K creep step							P perm
			C (creep + shrinkage)							
6061	part.	CS 1	61 K creep step							P perm
			C (creep + shrinkage)							
6062	part.	CS 1	62 K creep step							P perm
			C (creep + shrinkage)							
6063	part.	CS 1	63 K creep step							P perm
			C (creep + shrinkage)							
6064	part.	CS 1	64 K creep step							P perm
			C (creep + shrinkage)							

Combinations For Stress Design

1219 max_my-1219
MAX + MY :
1.00 * G + 1.00 * L_A + 1.00 * L_B + 1.00 * C + 0.60 * L_C

1220 min_my-1220
MIN + MY :
1.00 * G + 1.00 * L_A + 1.00 * L_B + 1.00 * C + 0.60 * L_C

Stresses [MPa]

Beam	x[m]	NoS	LC	M	A	sig-	sig+	tau	sig-I	sig-II	sig-s	N/Npl*
1001	0.000	1	1219	1		-0.95	0.70	0.50	0.41	-1.14	3.05	
				4		-0.64	-0.13	0.00		-0.64	-4.85	
			1220	1		0.98	-3.63	1.13	1.48	-3.67	-26.32	
				4		3.37	1.79	0.00			-26.32	
	0.883	1	1219	1		-1.86	2.10	1.76	1.48	-2.81	-15.73	
				4		-1.44	-0.48	0.00		-1.44	-27.35	
			1220	1		-0.06	-2.26	0.82	0.73	-2.29	-15.12	
				4		2.41	1.32	0.00			-15.12	
1002	0.000	1	1219	1		-1.86	2.10	1.17	1.36	-2.36	-15.72	
				4		-1.44	-0.48	0.00		-1.44	-27.32	
			1220	1		-0.06	-2.26	0.77	0.69	-2.29	-15.12	
				4		2.41	1.32	0.00			-15.12	
	0.883	1	1219	1		-2.87	3.55	1.57	2.33	-3.48	-28.10	
				4		-2.55	-1.03	0.00		-2.55	-48.87	
			1220	1		-0.86	-1.24	0.71	0.37	-1.35	-16.46	
				4		1.72	0.97	0.00			-20.24	
1003	0.000	1	1219	1		-2.86	3.55	0.97	2.29	-3.13	-28.09	
				4		-2.55	-1.03	0.00		-2.55	-48.84	
			1220	1		-0.86	-1.24	0.65	0.32	-1.30	-16.46	
				4		1.72	0.98	0.00			-20.23	
	0.883	1	1219	1		-3.62	4.63	0.90	3.00	-3.81	-37.57	
				4		-3.37	-1.43	0.00		-3.37	-65.25	
			1220	1		-1.51	-0.39	0.59	0.25	-1.69	-25.31	
				4		1.12	0.68	0.00			-35.16	
1004	0.000	1	1219	1		-3.62	4.62	0.30	2.98	-3.64	-37.56	
				4		-3.36	-1.43	0.00		-3.36	-65.23	
			1220	1		-1.51	-0.39	0.53	0.20	-1.66	-25.30	
				4		1.12	0.68	0.00			-35.16	
	0.883	1	1219	1		-4.18	5.42	0.70	3.51	-4.28	-44.42	
				4		-3.97	-1.73	0.00		-3.97	-77.14	
			1220	1		-2.02	0.29	0.47	0.16	-2.10	-31.84	
				4		0.62	0.43	0.00			-46.32	
1005	0.000	1	1219	1		-4.18	5.42	0.11	3.50	-4.18	-44.41	
				4		-3.97	-1.73	0.00		-3.97	-77.12	
			1220	1		-2.01	0.29	0.41	0.12	-2.08	-31.83	
				4		0.62	0.43	0.00			-46.32	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CHARACTERISTIC COMBINATION (0.60*fck),(0.80*fyk)

Stresses [MPa]

Beam	x[m]	NoS	LC	M	A	sig-	sig+	tau	sig-I	sig-II	sig-s	N/Npl*
1005	0.883	1	1219	1		-4.50	5.91	0.52	3.83	-4.55	-48.42	
				4		-4.32	-1.90	0.00		-4.32	-84.17	
			1220	1		-2.36	0.70	0.29	0.11	-2.39	-36.09	
				4		0.26	0.24	0.00			-53.46	
1006	0.000	1	1219	1		-4.50	5.91	0.06	3.83	-4.50	-48.42	
				4		-4.32	-1.90	0.00		-4.32	-84.16	
			1220	1		-2.36	0.70	0.23	0.10	-2.38	-36.09	
				4		0.26	0.24	0.00			-53.46	
	0.883	1	1219	1		-4.59	6.04	0.38	3.91	-4.62	-49.70	
				4		-4.41	-1.94	0.00		-4.41	-86.33	
			1220	1		-2.52	0.78	0.03	0.12	-2.52	-37.79	
				4		0.11	0.14	0.00			-56.08	
1007	0.000	1	1219	1		-4.59	6.04	0.27	3.91	-4.61	-49.70	
				4		-4.41	-1.94	0.00		-4.41	-86.34	
			1220	1		-2.52	0.78	0.03	0.12	-2.52	-37.79	
				4		0.11	0.14	0.00			-56.08	
	0.883	1	1219	1		-4.49	5.91	0.19	3.82	-4.50	-48.40	
				4		-4.32	-1.89	0.00		-4.32	-84.12	
			1220	1		-2.36	0.70	0.22	0.10	-2.38	-36.09	
				4		0.26	0.24	0.00			-53.46	
1008	0.000	1	1219	1		-4.49	5.91	0.42	3.83	-4.53	-48.41	
				4		-4.32	-1.89	0.00		-4.32	-84.13	
			1220	1		-2.36	0.70	0.29	0.11	-2.39	-36.09	
				4		0.26	0.24	0.00			-53.46	
	0.883	1	1219	1		-4.17	5.40	0.47	3.49	-4.21	-44.36	
				4		-3.95	-1.72	0.00		-3.95	-77.00	
			1220	1		-2.01	0.29	0.40	0.12	-2.08	-31.83	
				4		0.62	0.43	0.00			-46.32	
1009	0.000	1	1219	1		-4.17	5.40	0.61	3.50	-4.25	-44.36	
				4		-3.95	-1.72	0.00		-3.95	-77.01	
			1220	1		-2.01	0.29	0.46	0.16	-2.10	-31.84	
				4		0.62	0.43	0.00			-46.32	
	0.883	1	1219	1		-3.65	4.67	0.68	3.02	-3.75	-37.70	
				4		-3.41	-1.45	0.00		-3.41	-65.55	
			1220	1		-1.51	-0.39	0.52	0.20	-1.66	-25.30	
				4		1.12	0.68	0.00			-35.16	
1010	0.000	1	1219	1		-3.65	4.67	1.30	3.05	-4.02	-37.71	
				4		-3.41	-1.45	0.00		-3.41	-65.56	
			1220	1		-1.51	-0.39	0.59	0.24	-1.69	-25.31	
				4		1.12	0.68	0.00			-35.16	
	0.883	1	1219	1		-2.88	3.57	0.89	2.31	-3.10	-28.16	
				4		-2.58	-1.04	0.00		-2.58	-49.01	
			1220	1		-0.86	-1.24	0.65	0.32	-1.30	-16.46	
				4		1.72	0.98	0.00			-20.22	
1011	0.000	1	1219	1		-2.88	3.58	1.50	2.34	-3.45	-28.17	
				4		-2.58	-1.04	0.00		-2.58	-49.04	
			1220	1		-0.86	-1.24	0.71	0.37	-1.35	-16.46	
				4		1.72	0.97	0.00			-20.24	
	0.883	1	1219	1		-1.86	2.10	1.09	1.35	-2.31	-15.72	
				4		-1.44	-0.48	0.00		-1.44	-27.33	
			1220	1		-0.06	-2.26	0.77	0.68	-2.29	-15.12	
				4		2.41	1.32	0.00			-15.12	
1012	0.000	1	1219	1		-1.86	2.10	1.68	1.39	-2.74	-15.73	
				4		-1.45	-0.48	0.00		-1.45	-27.36	
			1220	1		-0.06	-2.26	0.82	0.73	-2.29	-15.12	
				4		2.41	1.32	0.00			-15.12	
	0.883	1	1219	1		-0.95	0.70	0.50	0.41	-1.14	3.05	
				4		-0.64	-0.13	0.00		-0.64	-4.86	
			1220	1		0.98	-3.63	1.13	1.48	-3.67	-26.32	
				4		3.37	1.79	0.00			-26.32	
1013	0.000	1	1219	1		-0.99	0.74	0.36	0.40	-1.09	3.27	
				4		-0.71	-0.17	0.00		-0.71	-5.31	
			1220	1		0.94	-3.66	0.99	1.42	-3.68	-26.58	
				4		3.33	1.75	0.00			-26.58	
	0.883	1	1219	1		-1.71	1.68	1.18	1.04	-2.25	-14.91	
				4		-1.11	-0.34	0.00		-1.11	-25.13	
			1220	1		-0.06	-2.35	0.70	0.62	-2.37	-15.76	
				4		2.44	1.32	0.00			-15.76	
1014	0.000	1	1219	1		-1.72	1.69	0.91	1.03	-2.06	-14.93	
				4		-1.12	-0.34	0.00		-1.12	-25.17	
			1220	1		-0.06	-2.35	0.67	0.58	-2.36	-15.75	
				4		2.44	1.32	0.00			-15.75	
	0.883	1	1219	1		-2.67	3.01	1.13	1.91	-3.03	-26.98	
				4		-2.11	-0.83	0.00		-2.11	-45.89	
			1220	1		-0.86	-1.33	0.61	0.29	-1.35	-16.54	
				4		1.74	0.97	0.00			-20.09	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CHARACTERISTIC COMBINATION (0.60*fck),(0.80*fyk)

Stresses [MPa]

Beam	x[m]	NoS	LC	M	A	sig-	sig+	tau	sig-I	sig-II	sig-s	N/Npl*
1015	0.000	1	1219	1		-2.67	3.01	0.79	1.89	-2.86	-26.99	
				4		-2.11	-0.83	0.00		-2.11	-45.92	
			1220	1		-0.86	-1.33	0.57	0.25	-1.34	-16.54	
				4		1.74	0.97	0.00			-20.09	
	0.883	1	1219	1		-3.41	4.04	0.99	2.58	-3.64	-36.36	
				4		-2.88	-1.22	0.00		-2.88	-62.03	
			1220	1		-1.52	-0.47	0.51	0.19	-1.65	-25.41	
				4		1.14	0.67	0.00			-35.09	
1016	0.000	1	1219	1		-3.41	4.04	0.62	2.56	-3.51	-36.37	
				4		-2.89	-1.22	0.00		-2.89	-62.06	
			1220	1		-1.52	-0.47	0.46	0.16	-1.63	-25.41	
				4		1.14	0.67	0.00			-35.09	
	0.883	1	1219	1		-3.93	4.76	0.56	3.03	-4.00	-42.99	
				4		-3.42	-1.48	0.00		-3.42	-73.43	
			1220	1		-2.03	0.22	0.40	0.11	-2.09	-31.97	
				4		0.63	0.42	0.00			-46.31	
1017	0.000	1	1219	1		-3.93	4.76	0.18	3.02	-3.93	-42.99	
				4		-3.42	-1.48	0.00		-3.42	-73.42	
			1220	1		-2.03	0.22	0.36	0.09	-2.08	-31.97	
				4		0.63	0.42	0.00			-46.31	
	0.883	1	1219	1		-4.24	5.24	0.39	3.34	-4.27	-46.96	
				4		-3.75	-1.64	0.00		-3.75	-80.36	
			1220	1		-2.38	0.63	0.25	0.07	-2.40	-36.25	
				4		0.26	0.22	0.00			-53.50	
1018	0.000	1	1219	1		-4.24	5.24	0.04	3.34	-4.24	-46.96	
				4		-3.75	-1.64	0.00		-3.75	-80.36	
			1220	1		-2.38	0.63	0.21	0.06	-2.40	-36.25	
				4		0.26	0.22	0.00			-53.50	
	0.883	1	1219	1		-4.33	5.35	0.26	3.42	-4.35	-48.22	
				4		-3.84	-1.68	0.00		-3.84	-82.49	
			1220	1		-2.30	0.95	0.01	0.30	-2.30	-36.17	
				4		0.35	0.36	0.00			-54.33	
1019	0.000	1	1219	1		-4.33	5.35	0.20	3.41	-4.34	-48.22	
				4		-3.84	-1.68	0.00		-3.84	-82.49	
			1220	1		-2.30	0.95	0.00	0.30	-2.30	-36.17	
				4		0.35	0.36	0.00			-54.33	
	0.883	1	1219	1		-4.23	5.22	0.06	3.33	-4.23	-46.92	
				4		-3.74	-1.63	0.00		-3.74	-80.28	
			1220	1		-2.38	0.63	0.21	0.06	-2.40	-36.25	
				4		0.26	0.22	0.00			-53.50	
1020	0.000	1	1219	1		-4.23	5.22	0.34	3.33	-4.26	-46.92	
				4		-3.74	-1.63	0.00		-3.74	-80.29	
			1220	1		-2.38	0.63	0.25	0.07	-2.40	-36.25	
				4		0.26	0.22	0.00			-53.50	
	0.883	1	1219	1		-3.94	4.77	0.38	3.03	-3.97	-43.04	
				4		-3.43	-1.49	0.00		-3.43	-73.53	
			1220	1		-2.03	0.22	0.36	0.09	-2.08	-31.97	
				4		0.63	0.42	0.00			-46.31	
1021	0.000	1	1219	1		-3.93	4.77	0.77	3.04	-4.06	-43.03	
				4		-3.43	-1.49	0.00		-3.43	-73.50	
			1220	1		-2.03	0.22	0.40	0.11	-2.09	-31.97	
				4		0.63	0.42	0.00			-46.31	
	0.883	1	1219	1		-3.42	4.06	0.57	2.57	-3.50	-36.41	
				4		-2.90	-1.22	0.00		-2.90	-62.15	
			1220	1		-1.52	-0.47	0.46	0.16	-1.63	-25.41	
				4		1.13	0.67	0.00			-35.09	
1022	0.000	1	1219	1		-3.41	4.06	0.95	2.58	-3.63	-36.40	
				4		-2.90	-1.22	0.00		-2.90	-62.13	
			1220	1		-1.52	-0.47	0.51	0.19	-1.65	-25.41	
				4		1.13	0.67	0.00			-35.09	
	0.883	1	1219	1		-2.67	3.01	0.75	1.88	-2.84	-26.98	
				4		-2.11	-0.83	0.00		-2.11	-45.90	
			1220	1		-0.87	-1.33	0.57	0.25	-1.34	-16.54	
				4		1.74	0.97	0.00			-20.10	
1023	0.000	1	1219	1		-2.66	3.01	1.09	1.90	-3.01	-26.97	
				4		-2.11	-0.83	0.00		-2.11	-45.88	
			1220	1		-0.86	-1.33	0.61	0.29	-1.35	-16.54	
				4		1.74	0.97	0.00			-20.10	
	0.883	1	1219	1		-1.71	1.68	0.86	1.02	-2.02	-14.88	
				4		-1.10	-0.33	0.00		-1.10	-25.08	
			1220	1		-0.06	-2.35	0.67	0.58	-2.36	-15.75	
				4		2.44	1.32	0.00			-15.75	
1024	0.000	1	1219	1		-1.71	1.67	1.12	1.03	-2.20	-14.87	
				4		-1.10	-0.33	0.00		-1.10	-25.04	
			1220	1		-0.06	-2.35	0.70	0.62	-2.37	-15.76	
				4		2.44	1.32	0.00			-15.76	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CHARACTERISTIC COMBINATION (0.60*fck),(0.80*fyk)

Stresses [MPa]

Beam	x[m]	NoS	LC	M	A	sig-	sig+	tau	sig-I	sig-II	sig-s	N/Npl*
1024	0.883	1	1219	1		-0.99	0.74	0.36	0.40	-1.09	3.27	
				4		-0.71	-0.17	0.00		-0.71	-5.30	
			1220	1		0.94	-3.66	0.99	1.42	-3.68	-26.59	
				4		3.33	1.75	0.00			-26.59	
1025	0.000	1	1219	1		-0.99	0.73	0.40	0.40	-1.12	3.23	
				4		-0.71	-0.18	0.00		-0.71	-5.35	
			1220	1		0.95	-3.73	0.96	1.45	-3.74	-27.02	
				4		3.36	1.77	0.00			-27.02	
	0.883	1	1219	1		-1.74	1.75	1.08	1.09	-2.19	-15.09	
				4		-1.17	-0.36	0.00		-1.17	-25.55	
			1220	1		-0.05	-2.40	0.76	0.68	-2.41	-16.13	
				4		2.47	1.33	0.00			-16.13	
1026	0.000	1	1219	1		-1.74	1.75	0.76	1.07	-1.99	-15.09	
				4		-1.17	-0.36	0.00		-1.17	-25.55	
			1220	1		-0.05	-2.40	0.75	0.67	-2.41	-16.12	
				4		2.46	1.33	0.00			-16.12	
	0.883	1	1219	1		-2.66	3.00	0.91	1.88	-2.90	-26.94	
				4		-2.10	-0.82	0.00		-2.10	-45.81	
			1220	1		-0.87	-1.36	0.69	0.35	-1.37	-16.58	
				4		1.74	0.97	0.00			-20.08	
1027	0.000	1	1219	1		-2.66	3.00	0.59	1.87	-2.77	-26.94	
				4		-2.10	-0.82	0.00		-2.10	-45.81	
			1220	1		-0.87	-1.36	0.66	0.32	-1.37	-16.58	
				4		1.74	0.97	0.00			-20.08	
	0.883	1	1219	1		-3.36	3.94	0.73	2.48	-3.49	-36.08	
				4		-2.79	-1.17	0.00		-2.79	-61.38	
			1220	1		-1.54	-0.48	0.60	0.25	-1.71	-25.49	
				4		1.12	0.66	0.00			-35.17	
1028	0.000	1	1219	1		-3.36	3.94	0.46	2.48	-3.42	-36.08	
				4		-2.79	-1.17	0.00		-2.79	-61.39	
			1220	1		-1.54	-0.48	0.55	0.21	-1.69	-25.49	
				4		1.12	0.66	0.00			-35.17	
	0.883	1	1219	1		-3.86	4.61	0.40	2.91	-3.89	-42.57	
				4		-3.27	-1.41	0.00		-3.27	-72.45	
			1220	1		-2.05	0.22	0.49	0.16	-2.14	-32.07	
				4		0.61	0.41	0.00			-46.44	
1029	0.000	1	1219	1		-3.86	4.61	0.16	2.91	-3.87	-42.57	
				4		-3.27	-1.41	0.00		-3.27	-72.46	
			1220	1		-2.05	0.22	0.42	0.12	-2.12	-32.07	
				4		0.61	0.41	0.00			-46.44	
	0.883	1	1219	1		-4.16	5.05	0.27	3.20	-4.17	-46.43	
				4		-3.57	-1.55	0.00		-3.57	-79.14	
			1220	1		-2.40	0.64	0.30	0.07	-2.44	-36.37	
				4		0.24	0.20	0.00			-53.67	
1030	0.000	1	1219	1		-4.16	5.05	0.03	3.20	-4.16	-46.43	
				4		-3.57	-1.55	0.00		-3.57	-79.15	
			1220	1		-2.40	0.64	0.23	0.06	-2.42	-36.37	
				4		0.24	0.20	0.00			-53.67	
	0.883	1	1219	1		-4.25	5.16	0.19	3.27	-4.25	-47.67	
				4		-3.64	-1.59	0.00		-3.64	-81.21	
			1220	1		-2.30	0.92	0.01	0.28	-2.30	-36.14	
				4		0.36	0.36	0.00			-54.22	
1031	0.000	1	1219	1		-4.25	5.16	0.14	3.27	-4.25	-47.67	
				4		-3.64	-1.59	0.00		-3.64	-81.21	
			1220	1		-2.30	0.92	0.01	0.28	-2.30	-36.14	
				4		0.36	0.36	0.00			-54.22	
	0.883	1	1219	1		-4.15	5.04	0.08	3.20	-4.15	-46.41	
				4		-3.56	-1.55	0.00		-3.56	-79.10	
			1220	1		-2.40	0.64	0.23	0.06	-2.42	-36.37	
				4		0.24	0.20	0.00			-53.67	
1032	0.000	1	1219	1		-4.15	5.04	0.24	3.20	-4.16	-46.41	
				4		-3.56	-1.55	0.00		-3.56	-79.10	
			1220	1		-2.40	0.64	0.30	0.07	-2.44	-36.37	
				4		0.24	0.20	0.00			-53.67	
	0.883	1	1219	1		-3.87	4.61	0.28	2.92	-3.88	-42.60	
				4		-3.28	-1.41	0.00		-3.28	-72.51	
			1220	1		-2.05	0.22	0.42	0.12	-2.12	-32.07	
				4		0.61	0.41	0.00			-46.44	
1033	0.000	1	1219	1		-3.87	4.61	0.55	2.92	-3.93	-42.60	
				4		-3.28	-1.41	0.00		-3.28	-72.50	
			1220	1		-2.05	0.22	0.48	0.16	-2.14	-32.07	
				4		0.61	0.41	0.00			-46.44	
	0.883	1	1219	1		-3.37	3.96	0.43	2.50	-3.42	-36.14	
				4		-2.81	-1.18	0.00		-2.81	-61.54	
			1220	1		-1.54	-0.48	0.55	0.21	-1.69	-25.49	
				4		1.12	0.66	0.00			-35.17	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CHARACTERISTIC COMBINATION (0.60*fck),(0.80*fyk)

Stresses [MPa]

Beam	x[m]	NoS	LC	M	A	sig-	sig+	tau	sig-I	sig-II	sig-s	N/Npl*
1034	0.000	1	1219	1		-3.37	3.96	0.71	2.50	-3.50	-36.14	
				4		-2.81	-1.18	0.00		-2.81	-61.53	
			1220	1		-1.54	-0.48	0.60	0.25	-1.71	-25.49	
				4		1.12	0.66	0.00			-35.17	
	0.883	1	1219	1		-2.66	3.00	0.56	1.88	-2.76	-26.95	
				4		-2.10	-0.83	0.00		-2.10	-45.84	
			1220	1		-0.87	-1.36	0.66	0.32	-1.37	-16.58	
				4		1.74	0.97	0.00			-20.08	
1035	0.000	1	1219	1		-2.66	3.00	0.88	1.88	-2.89	-26.95	
				4		-2.10	-0.83	0.00		-2.10	-45.84	
			1220	1		-0.87	-1.36	0.69	0.35	-1.37	-16.58	
				4		1.74	0.97	0.00			-20.08	
	0.883	1	1219	1		-1.73	1.74	0.73	1.06	-1.96	-15.04	
				4		-1.16	-0.36	0.00		-1.16	-25.45	
			1220	1		-0.05	-2.40	0.75	0.67	-2.41	-16.12	
				4		2.46	1.33	0.00			-16.12	
1036	0.000	1	1219	1		-1.73	1.74	1.04	1.08	-2.16	-15.04	
				4		-1.16	-0.36	0.00		-1.16	-25.45	
			1220	1		-0.05	-2.41	0.76	0.68	-2.41	-16.13	
				4		2.47	1.33	0.00			-16.13	
	0.883	1	1219	1		-0.99	0.73	0.40	0.40	-1.12	3.22	
				4		-0.71	-0.18	0.00		-0.71	-5.35	
			1220	1		0.95	-3.73	0.96	1.45	-3.74	-27.02	
				4		3.37	1.77	0.00			-27.02	
1037	0.000	1	1219	1		-0.94	0.66	0.56	0.43	-1.18	-2.75	
				4		-0.64	-0.14	0.00		-0.64	-4.85	
			1220	1		0.94	-3.64	1.05	1.41	-3.65	-26.41	
				4		3.30	1.74	0.00			-26.41	
	0.883	1	1219	1		-1.75	1.82	1.04	1.14	-2.17	-15.17	
				4		-1.22	-0.38	0.00		-1.22	-25.84	
			1220	1		-0.04	-2.38	0.90	0.82	-2.40	-15.98	
				4		2.46	1.33	0.00			-15.98	
1038	0.000	1	1219	1		-1.75	1.81	0.82	1.12	-2.04	-15.15	
				4		-1.22	-0.38	0.00		-1.22	-25.81	
			1220	1		-0.05	-2.38	0.91	0.83	-2.41	-15.97	
				4		2.46	1.33	0.00			-15.97	
	0.883	1	1219	1		-2.65	3.02	0.87	1.90	-2.87	-26.87	
				4		-2.09	-0.82	0.00		-2.09	-45.76	
			1220	1		-0.88	-1.32	0.85	0.48	-1.50	-16.60	
				4		1.73	0.96	0.00			-20.20	
1039	0.000	1	1219	1		-2.65	3.01	0.64	1.88	-2.78	-26.86	
				4		-2.09	-0.81	0.00		-2.09	-45.74	
			1220	1		-0.88	-1.32	0.80	0.44	-1.45	-16.60	
				4		1.73	0.96	0.00			-20.20	
	0.883	1	1219	1		-3.33	3.89	0.75	2.46	-3.47	-35.82	
				4		-2.72	-1.13	0.00		-2.72	-60.91	
			1220	1		-1.55	-0.43	0.73	0.35	-1.82	-25.55	
				4		1.09	0.65	0.00			-35.39	
1040	0.000	1	1219	1		-3.33	3.89	0.46	2.45	-3.38	-35.82	
				4		-2.71	-1.13	0.00		-2.71	-60.89	
			1220	1		-1.55	-0.43	0.65	0.28	-1.76	-25.55	
				4		1.09	0.65	0.00			-35.39	
	0.883	1	1219	1		-3.81	4.53	0.39	2.86	-3.85	-42.22	
				4		-3.16	-1.35	0.00		-3.16	-71.75	
			1220	1		-2.07	0.29	0.59	0.22	-2.21	-32.16	
				4		0.57	0.39	0.00			-46.74	
1041	0.000	1	1219	1		-3.81	4.53	0.19	2.86	-3.82	-42.22	
				4		-3.16	-1.35	0.00		-3.16	-71.75	
			1220	1		-2.07	0.29	0.48	0.15	-2.17	-32.16	
				4		0.57	0.39	0.00			-46.74	
	0.883	1	1219	1		-4.10	4.93	0.25	3.13	-4.11	-45.99	
				4		-3.43	-1.48	0.00		-3.43	-78.22	
			1220	1		-2.24	0.87	0.13	0.27	-2.24	-35.00	
				4		0.40	0.38	0.00			-52.42	
1042	0.000	1	1219	1		-4.10	4.93	0.07	3.13	-4.10	-45.99	
				4		-3.43	-1.48	0.00		-3.43	-78.22	
			1220	1		-2.24	0.87	0.08	0.25	-2.24	-35.00	
				4		0.40	0.38	0.00			-52.42	
	0.883	1	1219	1		-4.18	5.03	0.24	3.19	-4.19	-47.20	
				4		-3.49	-1.51	0.00		-3.49	-80.22	
			1220	1		-2.30	0.93	0.03	0.28	-2.30	-36.09	
				4		0.38	0.37	0.00			-54.16	
1043	0.000	1	1219	1		-4.18	5.03	0.16	3.19	-4.19	-47.19	
				4		-3.49	-1.51	0.00		-3.49	-80.22	
			1220	1		-2.30	0.93	0.03	0.28	-2.30	-36.09	
				4		0.38	0.37	0.00			-54.16	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CHARACTERISTIC COMBINATION (0.60*fck),(0.80*fyk)

Stresses [MPa]

Beam	x[m]	NoS	LC	M	A	sig-	sig+	tau	sig-I	sig-II	sig-s	N/Npl*
1043	0.883	1	1219	1		-4.09	4.93	0.17	3.13	-4.10	-45.98	
				4		-3.43	-1.48	0.00		-3.43	-78.21	
			1220	1		-2.24	0.87	0.08	0.25	-2.24	-35.00	
				4		0.40	0.38	0.00			-52.42	
1044	0.000	1	1219	1		-4.09	4.93	0.22	3.12	-4.10	-45.98	
				4		-3.43	-1.48	0.00		-3.43	-78.20	
			1220	1		-2.24	0.87	0.13	0.27	-2.24	-35.00	
				4		0.40	0.38	0.00			-52.42	
	0.883	1	1219	1		-3.81	4.53	0.27	2.86	-3.83	-42.22	
				4		-3.16	-1.35	0.00		-3.16	-71.75	
			1220	1		-2.07	0.29	0.48	0.15	-2.17	-32.16	
				4		0.57	0.39	0.00			-46.74	
1045	0.000	1	1219	1		-3.81	4.53	0.62	2.86	-3.90	-42.23	
				4		-3.17	-1.35	0.00		-3.17	-71.76	
			1220	1		-2.07	0.29	0.58	0.22	-2.21	-32.16	
				4		0.57	0.39	0.00			-46.74	
	0.883	1	1219	1		-3.34	3.93	0.44	2.47	-3.39	-35.90	
				4		-2.74	-1.14	0.00		-2.74	-61.10	
			1220	1		-1.55	-0.43	0.64	0.28	-1.76	-25.55	
				4		1.09	0.65	0.00			-35.39	
1046	0.000	1	1219	1		-3.34	3.93	0.72	2.48	-3.47	-35.91	
				4		-2.75	-1.14	0.00		-2.75	-61.12	
			1220	1		-1.55	-0.43	0.73	0.35	-1.82	-25.55	
				4		1.09	0.65	0.00			-35.39	
	0.883	1	1219	1		-2.66	3.02	0.63	1.89	-2.78	-26.89	
				4		-2.10	-0.82	0.00		-2.10	-45.81	
			1220	1		-0.88	-1.32	0.79	0.43	-1.45	-16.60	
				4		1.73	0.96	0.00			-20.20	
1047	0.000	1	1219	1		-2.66	3.03	0.85	1.90	-2.87	-26.89	
				4		-2.10	-0.82	0.00		-2.10	-45.83	
			1220	1		-0.88	-1.32	0.85	0.48	-1.50	-16.60	
				4		1.73	0.96	0.00			-20.20	
	0.883	1	1219	1		-1.74	1.80	0.81	1.11	-2.02	-15.11	
				4		-1.20	-0.37	0.00		-1.20	-25.71	
			1220	1		-0.05	-2.38	0.91	0.83	-2.41	-15.97	
				4		2.46	1.33	0.00			-15.97	
1048	0.000	1	1219	1		-1.75	1.80	1.03	1.13	-2.15	-15.12	
				4		-1.20	-0.37	0.00		-1.20	-25.73	
			1220	1		-0.04	-2.38	0.90	0.82	-2.40	-15.98	
				4		2.46	1.33	0.00			-15.98	
	0.883	1	1219	1		-0.94	0.66	0.56	0.42	-1.18	-2.75	
				4		-0.64	-0.14	0.00		-0.64	-4.85	
			1220	1		0.94	-3.64	1.05	1.41	-3.65	-26.41	
				4		3.30	1.74	0.00			-26.41	
1049	0.000	1	1219	1		-0.88	0.59	0.73	0.55	-1.25	-2.44	
				4		-0.57	-0.09	0.00		-0.57	-4.29	
			1220	1		1.01	-3.63	1.20	1.46	-3.65	-26.22	
				4		3.35	1.79	0.00			-26.22	
	0.883	1	1219	1		-1.70	1.62	0.77	1.01	-1.96	-14.90	
				4		-1.08	-0.33	0.00		-1.08	-24.94	
			1220	1		-0.04	-2.28	1.07	0.99	-2.32	-15.27	
				4		2.43	1.33	0.00			-15.27	
1050	0.000	1	1219	1		-1.70	1.61	0.69	1.01	-1.92	-14.89	
				4		-1.08	-0.33	0.00		-1.08	-24.92	
			1220	1		-0.04	-2.29	1.01	0.93	-2.33	-15.28	
				4		2.43	1.33	0.00			-15.28	
	0.883	1	1219	1		-2.48	2.69	0.71	1.66	-2.64	-25.80	
				4		-1.74	-0.64	0.00		-1.74	-43.43	
			1220	1		-0.89	-1.18	0.95	0.58	-1.56	-16.61	
				4		1.67	0.95	0.00			-20.58	
1051	0.000	1	1219	1		-2.49	2.69	0.59	1.66	-2.60	-25.83	
				4		-1.75	-0.64	0.00		-1.75	-43.46	
			1220	1		-0.89	-1.19	0.86	0.50	-1.48	-16.60	
				4		1.67	0.95	0.00			-20.55	
	0.883	1	1219	1		-3.15	3.55	0.52	2.21	-3.23	-34.70	
				4		-2.35	-0.94	0.00		-2.35	-58.45	
			1220	1		-1.58	-0.29	0.80	0.40	-1.88	-25.57	
				4		1.04	0.63	0.00			-35.79	
1052	0.000	1	1219	1		-3.15	3.55	0.44	2.21	-3.21	-34.70	
				4		-2.35	-0.94	0.00		-2.35	-58.46	
			1220	1		-1.58	-0.29	0.69	0.32	-1.81	-25.57	
				4		1.04	0.64	0.00			-35.78	
	0.883	1	1219	1		-3.61	4.12	0.35	2.58	-3.64	-40.93	
				4		-2.74	-1.14	0.00		-2.74	-68.91	
			1220	1		-2.10	0.43	0.63	0.25	-2.25	-32.18	
				4		0.51	0.38	0.00			-47.12	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CHARACTERISTIC COMBINATION (0.60*fck),(0.80*fyk)

Stresses [MPa]

Beam	x[m]	NoS	LC	M	A	sig-	sig+	tau	sig-I	sig-II	sig-s	N/Npl*
1053	0.000	1	1219	1		-3.61	4.13	0.24	2.58	-3.63	-40.94	
				4		-2.74	-1.14	0.00		-2.74	-68.92	
			1220	1		-2.10	0.43	0.50	0.17	-2.20	-32.17	
				4		0.52	0.38	0.00			-47.11	
	0.883	1	1219	1		-3.88	4.49	0.20	2.81	-3.89	-44.59	
				4		-2.97	-1.25	0.00		-2.97	-75.13	
			1220	1		-2.23	0.90	0.14	0.30	-2.24	-34.86	
				4		0.43	0.40	0.00			-52.31	
1054	0.000	1	1219	1		-3.88	4.49	0.12	2.82	-3.88	-44.59	
				4		-2.97	-1.25	0.00		-2.97	-75.13	
			1220	1		-2.23	0.90	0.09	0.28	-2.23	-34.86	
				4		0.43	0.40	0.00			-52.31	
	0.883	1	1219	1		-3.96	4.58	0.14	2.87	-3.96	-45.77	
				4		-3.02	-1.28	0.00		-3.02	-77.06	
			1220	1		-2.29	0.96	0.03	0.31	-2.30	-35.94	
				4		0.40	0.39	0.00			-54.02	
1055	0.000	1	1219	1		-3.96	4.58	0.11	2.87	-3.96	-45.77	
				4		-3.02	-1.28	0.00		-3.02	-77.06	
			1220	1		-2.29	0.96	0.03	0.31	-2.30	-35.94	
				4		0.40	0.39	0.00			-54.02	
	0.883	1	1219	1		-3.87	4.48	0.13	2.81	-3.88	-44.57	
				4		-2.96	-1.24	0.00		-2.96	-75.07	
			1220	1		-2.23	0.90	0.09	0.28	-2.23	-34.86	
				4		0.43	0.40	0.00			-52.31	
1056	0.000	1	1219	1		-3.87	4.48	0.21	2.81	-3.88	-44.57	
				4		-2.96	-1.24	0.00		-2.96	-75.08	
			1220	1		-2.23	0.90	0.14	0.30	-2.24	-34.86	
				4		0.43	0.40	0.00			-52.31	
	0.883	1	1219	1		-3.62	4.13	0.25	2.58	-3.63	-40.96	
				4		-2.75	-1.14	0.00		-2.75	-68.96	
			1220	1		-2.10	0.43	0.50	0.17	-2.20	-32.17	
				4		0.52	0.38	0.00			-47.11	
1057	0.000	1	1219	1		-3.62	4.13	0.34	2.58	-3.64	-40.95	
				4		-2.74	-1.14	0.00		-2.74	-68.95	
			1220	1		-2.10	0.43	0.62	0.25	-2.25	-32.18	
				4		0.51	0.38	0.00			-47.12	
	0.883	1	1219	1		-3.15	3.55	0.45	2.21	-3.21	-34.69	
				4		-2.34	-0.94	0.00		-2.34	-58.44	
			1220	1		-1.58	-0.29	0.69	0.32	-1.81	-25.57	
				4		1.04	0.64	0.00			-35.78	
1058	0.000	1	1219	1		-3.15	3.55	0.53	2.21	-3.23	-34.69	
				4		-2.34	-0.94	0.00		-2.34	-58.43	
			1220	1		-1.58	-0.29	0.80	0.40	-1.88	-25.57	
				4		1.04	0.63	0.00			-35.79	
	0.883	1	1219	1		-2.49	2.70	0.59	1.67	-2.60	-25.85	
				4		-1.75	-0.65	0.00		-1.75	-43.51	
			1220	1		-0.89	-1.19	0.86	0.50	-1.48	-16.60	
				4		1.67	0.95	0.00			-20.55	
1059	0.000	1	1219	1		-2.49	2.69	0.69	1.67	-2.64	-25.84	
				4		-1.75	-0.65	0.00		-1.75	-43.50	
			1220	1		-0.89	-1.18	0.95	0.58	-1.56	-16.61	
				4		1.67	0.95	0.00			-20.58	
	0.883	1	1219	1		-1.70	1.61	0.69	1.01	-1.92	-14.89	
				4		-1.08	-0.33	0.00		-1.08	-24.93	
			1220	1		-0.04	-2.29	1.01	0.93	-2.33	-15.28	
				4		2.43	1.33	0.00			-15.28	
1060	0.000	1	1219	1		-1.70	1.62	0.77	1.01	-1.96	-14.90	
				4		-1.08	-0.33	0.00		-1.08	-24.94	
			1220	1		-0.04	-2.28	1.07	0.98	-2.32	-15.27	
				4		2.43	1.33	0.00			-15.27	
	0.883	1	1219	1		-0.88	0.59	0.73	0.55	-1.25	-2.44	
				4		-0.57	-0.09	0.00		-0.57	-4.29	
			1220	1		1.01	-3.62	1.20	1.46	-3.65	-26.21	
				4		3.35	1.79	0.00			-26.21	
2001	0.000	2	1219	3		-0.21	0.15	0.15	0.13	-0.21	0.26	
			1220	3		3.97	-4.03	0.48	0.46	-4.03	4.03	
	0.300	2	1219	3		-0.40	0.33	0.15	0.13	-0.40	0.40	
			1220	3		3.48	-3.60	0.30	0.27	-3.60	3.60	
2002	0.000	2	1219	3		-0.40	0.33	0.16	0.14	-0.40	0.40	
			1220	3		3.49	-3.60	0.37	0.34	-3.60	3.60	
	0.300	2	1219	3		-0.57	0.51	0.16	0.15	-0.57	0.57	
			1220	3		3.12	-3.23	0.35	0.32	-3.23	3.23	
2003	0.000	2	1219	3		-0.57	0.51	0.16	0.15	-0.57	0.57	
			1220	3		3.12	-3.23	0.35	0.32	-3.23	3.23	
	0.300	2	1219	3		-0.40	0.33	0.16	0.14	-0.40	0.40	
			1220	3		3.49	-3.60	0.37	0.34	-3.60	3.60	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CHARACTERISTIC COMBINATION (0.60*fck),(0.80*fyk)

Stresses [MPa]

Beam	x[m]	NoS	LC	M	A	sig-	sig+	tau	sig-I	sig-II	sig-s	N/Npl*
2004	0.000	2	1219	3		-0.40	0.33	0.15	0.13	-0.40	0.40	
			1220	3		3.48	-3.60	0.30	0.27	-3.60	3.60	
	0.300	2	1219	3		-0.22	0.15	0.15	0.13	-0.22	0.26	
			1220	3		3.96	-4.03	0.49	0.47	-4.03	4.03	
2005	0.000	2	1219	3		-0.34	0.25	0.11	0.09	-0.34	0.34	
			1220	3		3.73	-3.89	0.30	0.26	-3.89	3.89	
	0.300	2	1219	3		-0.49	0.40	0.11	0.09	-0.49	0.49	
			1220	3		3.43	-3.59	0.28	0.25	-3.59	3.59	
2006	0.000	2	1219	3		-0.49	0.40	0.11	0.09	-0.49	0.49	
			1220	3		3.44	-3.60	0.34	0.30	-3.60	3.60	
	0.300	2	1219	3		-0.63	0.54	0.12	0.10	-0.63	0.63	
			1220	3		3.09	-3.25	0.32	0.28	-3.25	3.25	
2007	0.000	2	1219	3		-0.63	0.54	0.12	0.10	-0.63	0.63	
			1220	3		3.09	-3.25	0.32	0.28	-3.25	3.25	
	0.300	2	1219	3		-0.49	0.40	0.12	0.09	-0.49	0.49	
			1220	3		3.44	-3.60	0.34	0.30	-3.60	3.60	
2008	0.000	2	1219	3		-0.49	0.40	0.11	0.09	-0.49	0.49	
			1220	3		3.44	-3.60	0.29	0.25	-3.60	3.60	
	0.300	2	1219	3		-0.34	0.25	0.11	0.09	-0.34	0.34	
			1220	3		3.73	-3.89	0.30	0.26	-3.89	3.89	
2009	0.000	2	1219	3		-0.34	0.24	0.11	0.09	-0.34	0.34	
			1220	3		3.81	-3.98	0.32	0.28	-3.98	3.98	
	0.300	2	1219	3		-0.49	0.39	0.12	0.09	-0.49	0.49	
			1220	3		3.49	-3.66	0.31	0.27	-3.66	3.66	
2010	0.000	2	1219	3		-0.49	0.39	0.12	0.10	-0.49	0.49	
			1220	3		3.50	-3.67	0.35	0.30	-3.67	3.67	
	0.300	2	1219	3		-0.64	0.54	0.13	0.10	-0.64	0.64	
			1220	3		3.13	-3.30	0.32	0.28	-3.30	3.30	
2011	0.000	2	1219	3		-0.64	0.54	0.13	0.10	-0.64	0.64	
			1220	3		3.13	-3.30	0.32	0.28	-3.30	3.30	
	0.300	2	1219	3		-0.49	0.39	0.12	0.10	-0.49	0.49	
			1220	3		3.50	-3.67	0.35	0.30	-3.67	3.67	
2012	0.000	2	1219	3		-0.49	0.39	0.12	0.09	-0.49	0.49	
			1220	3		3.49	-3.66	0.31	0.27	-3.66	3.66	
	0.300	2	1219	3		-0.34	0.24	0.11	0.09	-0.34	0.34	
			1220	3		3.81	-3.98	0.32	0.28	-3.98	3.98	
2013	0.000	2	1219	3		-0.26	0.16	0.13	0.10	-0.26	0.26	
			1220	3		3.69	-3.85	0.29	0.26	-3.85	3.85	
	0.300	2	1219	3		-0.41	0.32	0.13	0.11	-0.41	0.41	
			1220	3		3.41	-3.57	0.28	0.24	-3.57	3.57	
2014	0.000	2	1219	3		-0.41	0.32	0.14	0.12	-0.41	0.41	
			1220	3		3.42	-3.57	0.34	0.30	-3.57	3.57	
	0.300	2	1219	3		-0.57	0.47	0.15	0.13	-0.57	0.57	
			1220	3		3.07	-3.23	0.32	0.28	-3.23	3.23	
2015	0.000	2	1219	3		-0.57	0.47	0.15	0.13	-0.57	0.57	
			1220	3		3.07	-3.23	0.32	0.28	-3.23	3.23	
	0.300	2	1219	3		-0.41	0.32	0.14	0.12	-0.41	0.41	
			1220	3		3.42	-3.57	0.34	0.30	-3.57	3.57	
2016	0.000	2	1219	3		-0.41	0.32	0.13	0.11	-0.41	0.41	
			1220	3		3.41	-3.57	0.28	0.24	-3.57	3.57	
	0.300	2	1219	3		-0.26	0.16	0.13	0.10	-0.26	0.26	
			1220	3		3.69	-3.85	0.29	0.26	-3.85	3.85	
2017	0.000	2	1219	3		-0.02	-0.06	0.20	0.19	-0.22	0.36	
			1220	3		3.75	-3.86	0.31	0.28	-3.86	3.86	
	0.300	2	1219	3		-0.26	0.18	0.21	0.19	-0.26	0.36	
			1220	3		3.46	-3.57	0.29	0.27	-3.57	3.57	
2018	0.000	2	1219	3		-0.26	0.18	0.22	0.20	-0.26	0.39	
			1220	3		3.47	-3.57	0.37	0.34	-3.57	3.57	
	0.300	2	1219	3		-0.49	0.42	0.22	0.21	-0.49	0.49	
			1220	3		3.11	-3.22	0.35	0.32	-3.22	3.22	
2019	0.000	2	1219	3		-0.50	0.42	0.22	0.21	-0.50	0.50	
			1220	3		3.11	-3.22	0.35	0.32	-3.22	3.22	
	0.300	2	1219	3		-0.26	0.18	0.22	0.20	-0.26	0.39	
			1220	3		3.46	-3.57	0.37	0.34	-3.57	3.57	
2020	0.000	2	1219	3		-0.26	0.18	0.21	0.19	-0.26	0.36	
			1220	3		3.46	-3.56	0.29	0.27	-3.56	3.56	
	0.300	2	1219	3		-0.02	-0.06	0.20	0.19	-0.22	0.36	
			1220	3		3.75	-3.86	0.31	0.28	-3.86	3.86	

Stresses [MPa]

Beam	x[m]	NoS	LC	M	A	sig-	sig+	tau	sig-I	sig-II	sig-s	N/Npl*
Total System			MIN	1		-4.59	-3.73	0.00	0.06	-1.09	-2.44	
Total System			MAX	1		1.01	6.04	1.76	3.91	-4.62	-49.70	
Total System			MIN	3		-0.64	-4.03	0.11	0.09	-0.21	0.00	
Total System			MAX	3		3.97	0.54	0.49	0.47	-4.03	0.00	
Total System			MIN	4		-4.41	-1.94	0.00			-4.29	
Total System			MAX	4		3.37	1.79	0.00		-4.41	-86.34	
Total System			MIN	12		-4.29	-7.47					
Total System			MAX	12		-86.34	45.54					

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CHARACTERISTIC COMBINATION (0.60*fck),(0.80*fyk)

Maximum Degree of Utilization

		N sig-c	Vy sig-t	Vz tau	Mt sig-*	My tend.	Mz As-l	Mb As-v	Mt2 crack	Total sigdyn	lamda tau-*
Cross sect.	1	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
		0.306	0.000	0.000	0.000	0.216	0.000	0.000	0.000	0.000	0.000
Cross sect.	2	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
DOKOS-2		0.269	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Total System		0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
		0.306	0.000	0.000	0.000	0.216	0.000	0.000	0.000	0.000	0.000

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00

10) ΦΑΣΗ-2 ΕΛΕΓΧΟΣ ΦΟΡΕΑ ULS-ΣΕΙΣΜΙΚΑ (Q=1.50)

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΣΕΙΣΜΙΚΕΣ ΜΕΜΟΝΩΜΕΝΕΣ ΦΟΡΤΙΣΕΙΣ

Load Case 201 ΔΥΝΑΜΕΙΣ ΑΔΡΑΝΕΙΑΣ-[+Ex]

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Factor forces and moments          1.000
Factor dead weight      DL-XX      0.360
Factor dead weight      DL-YY      0.000
Factor dead weight      DL-ZZ      0.000
effective construction stage          40 to     40
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Meshfree Loading

Kind	Referenceto	Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
Area			0.000	2.000	0.000	PXX	0.004	[MN/m2]
			11.800	2.000	0.000		0.004	[MN/m2]
			11.800	-1.000	0.000		0.002	[MN/m2]
			0.000	-1.000	0.000		0.002	[MN/m2]
	QGRP	3	3.000	(--)	activated		89.83	percent
Area			0.000	2.000	0.000	PXX	0.004	[MN/m2]
			11.800	2.000	0.000		0.004	[MN/m2]
			11.800	-1.000	0.000		0.002	[MN/m2]
			0.000	-1.000	0.000		0.002	[MN/m2]
	QGRP	4	3.000	(--)	activated		10.17	percent
Area			11.800	-2.000	0.000	PXX	0.006	[MN/m2]
			0.000	-2.000	0.000		0.006	[MN/m2]
			0.000	-1.000	0.000		0.006	[MN/m2]
			11.800	-1.000	0.000		0.006	[MN/m2]
	QGRP	3	3.000	(--)	activated		89.83	percent
Area			11.800	-2.000	0.000	PXX	0.006	[MN/m2]
			0.000	-2.000	0.000		0.006	[MN/m2]
			0.000	-1.000	0.000		0.006	[MN/m2]
			11.800	-1.000	0.000		0.006	[MN/m2]
	QGRP	4	3.000	(--)	activated		10.17	percent
Area			0.000	2.000	0.000	PXX	0.000	[MN/m2]
			11.800	2.000	0.000		0.000	[MN/m2]
			11.800	-1.000	0.000		0.000	[MN/m2]
			0.000	-1.000	0.000		0.000	[MN/m2]
	QGRP	3	3.000	(--)	activated		89.83	percent
Area			0.000	2.000	0.000	PXX	0.000	[MN/m2]
			11.800	2.000	0.000		0.000	[MN/m2]
			11.800	-1.000	0.000		0.000	[MN/m2]
			0.000	-1.000	0.000		0.000	[MN/m2]
	QGRP	4	3.000	(--)	activated		10.17	percent

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
1001	PXX	0.000	0.883	0.003	[MN/m]				
1001	PXX	0.000	0.883	0.002	[MN/m]				
1002	PXX	0.000	0.883	0.003	[MN/m]				
1002	PXX	0.000	0.883	0.002	[MN/m]				
1003	PXX	0.000	0.883	0.003	[MN/m]				
1003	PXX	0.000	0.883	0.002	[MN/m]				
1004	PXX	0.000	0.883	0.003	[MN/m]				
1004	PXX	0.000	0.883	0.002	[MN/m]				
1005	PXX	0.000	0.883	0.003	[MN/m]				
1005	PXX	0.000	0.883	0.002	[MN/m]				
1006	PXX	0.000	0.883	0.003	[MN/m]				
1006	PXX	0.000	0.883	0.002	[MN/m]				
1007	PXX	0.000	0.883	0.003	[MN/m]				
1007	PXX	0.000	0.883	0.002	[MN/m]				
1008	PXX	0.000	0.883	0.003	[MN/m]				
1008	PXX	0.000	0.883	0.002	[MN/m]				
1009	PXX	0.000	0.883	0.003	[MN/m]				
1009	PXX	0.000	0.883	0.002	[MN/m]				
1010	PXX	0.000	0.883	0.003	[MN/m]				
1010	PXX	0.000	0.883	0.002	[MN/m]				
1011	PXX	0.000	0.883	0.003	[MN/m]				
1011	PXX	0.000	0.883	0.002	[MN/m]				
1012	PXX	0.000	0.883	0.003	[MN/m]				
1012	PXX	0.000	0.883	0.002	[MN/m]				
1013	PXX	0.000	0.883	0.003	[MN/m]				
1013	PXX	0.000	0.883	0.002	[MN/m]				
1014	PXX	0.000	0.883	0.003	[MN/m]				
1014	PXX	0.000	0.883	0.002	[MN/m]				
1015	PXX	0.000	0.883	0.003	[MN/m]				
1015	PXX	0.000	0.883	0.002	[MN/m]				
1016	PXX	0.000	0.883	0.003	[MN/m]				
1016	PXX	0.000	0.883	0.002	[MN/m]				
1017	PXX	0.000	0.883	0.003	[MN/m]				
1017	PXX	0.000	0.883	0.002	[MN/m]				
1018	PXX	0.000	0.883	0.003	[MN/m]				
1018	PXX	0.000	0.883	0.002	[MN/m]				
1019	PXX	0.000	0.883	0.003	[MN/m]				
1019	PXX	0.000	0.883	0.002	[MN/m]				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΣΕΙΣΜΙΚΕΣ ΜΕΜΟΝΩΜΕΝΕΣ ΦΟΡΤΙΣΕΙΣ

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
1020	PXX	0.000	0.883	0.003	[MN/m]				
1020	PXX	0.000	0.883	0.002	[MN/m]				
1021	PXX	0.000	0.883	0.003	[MN/m]				
1021	PXX	0.000	0.883	0.002	[MN/m]				
1022	PXX	0.000	0.883	0.003	[MN/m]				
1022	PXX	0.000	0.883	0.002	[MN/m]				
1023	PXX	0.000	0.883	0.003	[MN/m]				
1023	PXX	0.000	0.883	0.002	[MN/m]				
1024	PXX	0.000	0.883	0.003	[MN/m]				
1024	PXX	0.000	0.883	0.002	[MN/m]				
1025	PXX	0.000	0.883	0.003	[MN/m]				
1025	PXX	0.000	0.883	0.002	[MN/m]				
1026	PXX	0.000	0.883	0.003	[MN/m]				
1026	PXX	0.000	0.883	0.002	[MN/m]				
1027	PXX	0.000	0.883	0.003	[MN/m]				
1027	PXX	0.000	0.883	0.002	[MN/m]				
1028	PXX	0.000	0.883	0.003	[MN/m]				
1028	PXX	0.000	0.883	0.002	[MN/m]				
1029	PXX	0.000	0.883	0.003	[MN/m]				
1029	PXX	0.000	0.883	0.002	[MN/m]				
1030	PXX	0.000	0.883	0.003	[MN/m]				
1030	PXX	0.000	0.883	0.002	[MN/m]				
1031	PXX	0.000	0.883	0.003	[MN/m]				
1031	PXX	0.000	0.883	0.002	[MN/m]				
1032	PXX	0.000	0.883	0.003	[MN/m]				
1032	PXX	0.000	0.883	0.002	[MN/m]				
1033	PXX	0.000	0.883	0.003	[MN/m]				
1033	PXX	0.000	0.883	0.002	[MN/m]				
1034	PXX	0.000	0.883	0.003	[MN/m]				
1034	PXX	0.000	0.883	0.002	[MN/m]				
1035	PXX	0.000	0.883	0.003	[MN/m]				
1035	PXX	0.000	0.883	0.002	[MN/m]				
1036	PXX	0.000	0.883	0.003	[MN/m]				
1036	PXX	0.000	0.883	0.002	[MN/m]				
1037	PXX	0.000	0.883	0.003	[MN/m]				
1037	PXX	0.000	0.883	0.002	[MN/m]				
1038	PXX	0.000	0.883	0.003	[MN/m]				
1038	PXX	0.000	0.883	0.002	[MN/m]				
1039	PXX	0.000	0.883	0.003	[MN/m]				
1039	PXX	0.000	0.883	0.002	[MN/m]				
1040	PXX	0.000	0.883	0.003	[MN/m]				
1040	PXX	0.000	0.883	0.002	[MN/m]				
1041	PXX	0.000	0.883	0.003	[MN/m]				
1041	PXX	0.000	0.883	0.002	[MN/m]				
1042	PXX	0.000	0.883	0.003	[MN/m]				
1042	PXX	0.000	0.883	0.002	[MN/m]				
1043	PXX	0.000	0.883	0.003	[MN/m]				
1043	PXX	0.000	0.883	0.002	[MN/m]				
1044	PXX	0.000	0.883	0.003	[MN/m]				
1044	PXX	0.000	0.883	0.002	[MN/m]				
1045	PXX	0.000	0.883	0.003	[MN/m]				
1045	PXX	0.000	0.883	0.002	[MN/m]				
1046	PXX	0.000	0.883	0.003	[MN/m]				
1046	PXX	0.000	0.883	0.002	[MN/m]				
1047	PXX	0.000	0.883	0.003	[MN/m]				
1047	PXX	0.000	0.883	0.002	[MN/m]				
1048	PXX	0.000	0.883	0.003	[MN/m]				
1048	PXX	0.000	0.883	0.002	[MN/m]				
1049	PXX	0.000	0.883	0.003	[MN/m]				
1049	PXX	0.000	0.883	0.002	[MN/m]				
1050	PXX	0.000	0.883	0.003	[MN/m]				
1050	PXX	0.000	0.883	0.002	[MN/m]				
1051	PXX	0.000	0.883	0.003	[MN/m]				
1051	PXX	0.000	0.883	0.002	[MN/m]				
1052	PXX	0.000	0.883	0.003	[MN/m]				
1052	PXX	0.000	0.883	0.002	[MN/m]				
1053	PXX	0.000	0.883	0.003	[MN/m]				
1053	PXX	0.000	0.883	0.002	[MN/m]				
1054	PXX	0.000	0.883	0.003	[MN/m]				
1054	PXX	0.000	0.883	0.002	[MN/m]				
1055	PXX	0.000	0.883	0.003	[MN/m]				
1055	PXX	0.000	0.883	0.002	[MN/m]				
1056	PXX	0.000	0.883	0.003	[MN/m]				
1056	PXX	0.000	0.883	0.002	[MN/m]				
1057	PXX	0.000	0.883	0.003	[MN/m]				
1057	PXX	0.000	0.883	0.002	[MN/m]				
1058	PXX	0.000	0.883	0.003	[MN/m]				
1058	PXX	0.000	0.883	0.002	[MN/m]				
1059	PXX	0.000	0.883	0.003	[MN/m]				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΣΕΙΣΜΙΚΕΣ ΜΕΜΟΝΩΜΕΝΕΣ ΦΟΡΤΙΣΕΙΣ

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
1059	PXX	0.000	0.883	0.002	[MN/m]				
1060	PXX	0.000	0.883	0.003	[MN/m]				
1060	PXX	0.000	0.883	0.002	[MN/m]				

Loads acting on QUAD-elements

Elements from	to	inc	Load Prim Type LC/CC	Load val.	Dimension	Variation dP/dX	dP/dY	dP/dZ
3000	3999	1	PXX	0.001	[MN/m2]			
4000	4999	1	PXX	0.001	[MN/m2]			

Load Case 202 ΔΥΝΑΜΕΙΣ ΑΔΡΑΝΕΙΑΣ-[+Ey]

Factor forces and moments 1.000
Factor dead weight DL-XX 0.000
Factor dead weight DL-YY 0.360
Factor dead weight DL-ZZ 0.000
effective construction stage 40 to 40

Meshfree Loading

Kind	Referenceto	Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
Area			0.000	2.000	0.000	PYY	0.004	[MN/m2]
			11.800	2.000	0.000		0.004	[MN/m2]
			11.800	-1.000	0.000		0.002	[MN/m2]
			0.000	-1.000	0.000		0.002	[MN/m2]
	QGRP	3	3.000	(--)	activated		89.83	percent
Area			0.000	2.000	0.000	PYY	0.004	[MN/m2]
			11.800	2.000	0.000		0.004	[MN/m2]
			11.800	-1.000	0.000		0.002	[MN/m2]
			0.000	-1.000	0.000		0.002	[MN/m2]
	QGRP	4	3.000	(--)	activated		10.17	percent
Area			11.800	-2.000	0.000	PYY	0.006	[MN/m2]
			0.000	-2.000	0.000		0.006	[MN/m2]
			0.000	-1.000	0.000		0.006	[MN/m2]
			11.800	-1.000	0.000		0.006	[MN/m2]
	QGRP	3	3.000	(--)	activated		89.83	percent
Area			11.800	-2.000	0.000	PYY	0.006	[MN/m2]
			0.000	-2.000	0.000		0.006	[MN/m2]
			0.000	-1.000	0.000		0.006	[MN/m2]
			11.800	-1.000	0.000		0.006	[MN/m2]
	QGRP	4	3.000	(--)	activated		10.17	percent
Area			0.000	2.000	0.000	PYY	0.000	[MN/m2]
			11.800	2.000	0.000		0.000	[MN/m2]
			11.800	-1.000	0.000		0.000	[MN/m2]
			0.000	-1.000	0.000		0.000	[MN/m2]
	QGRP	3	3.000	(--)	activated		89.83	percent
Area			0.000	2.000	0.000	PYY	0.000	[MN/m2]
			11.800	2.000	0.000		0.000	[MN/m2]
			11.800	-1.000	0.000		0.000	[MN/m2]
			0.000	-1.000	0.000		0.000	[MN/m2]
	QGRP	4	3.000	(--)	activated		10.17	percent

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
1001	PYY	0.000	0.883	0.003	[MN/m]				
1001	PYY	0.000	0.883	0.002	[MN/m]				
1002	PYY	0.000	0.883	0.003	[MN/m]				
1002	PYY	0.000	0.883	0.002	[MN/m]				
1003	PYY	0.000	0.883	0.003	[MN/m]				
1003	PYY	0.000	0.883	0.002	[MN/m]				
1004	PYY	0.000	0.883	0.003	[MN/m]				
1004	PYY	0.000	0.883	0.002	[MN/m]				
1005	PYY	0.000	0.883	0.003	[MN/m]				
1005	PYY	0.000	0.883	0.002	[MN/m]				
1006	PYY	0.000	0.883	0.003	[MN/m]				
1006	PYY	0.000	0.883	0.002	[MN/m]				
1007	PYY	0.000	0.883	0.003	[MN/m]				
1007	PYY	0.000	0.883	0.002	[MN/m]				
1008	PYY	0.000	0.883	0.003	[MN/m]				
1008	PYY	0.000	0.883	0.002	[MN/m]				
1009	PYY	0.000	0.883	0.003	[MN/m]				
1009	PYY	0.000	0.883	0.002	[MN/m]				
1010	PYY	0.000	0.883	0.003	[MN/m]				
1010	PYY	0.000	0.883	0.002	[MN/m]				
1011	PYY	0.000	0.883	0.003	[MN/m]				
1011	PYY	0.000	0.883	0.002	[MN/m]				
1012	PYY	0.000	0.883	0.003	[MN/m]				
1012	PYY	0.000	0.883	0.002	[MN/m]				
1013	PYY	0.000	0.883	0.003	[MN/m]				
1013	PYY	0.000	0.883	0.002	[MN/m]				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΣΕΙΣΜΙΚΕΣ ΜΕΜΟΝΩΜΕΝΕΣ ΦΟΡΤΙΣΕΙΣ

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
1014	PYY	0.000	0.883	0.003	[MN/m]				
1014	PYY	0.000	0.883	0.002	[MN/m]				
1015	PYY	0.000	0.883	0.003	[MN/m]				
1015	PYY	0.000	0.883	0.002	[MN/m]				
1016	PYY	0.000	0.883	0.003	[MN/m]				
1016	PYY	0.000	0.883	0.002	[MN/m]				
1017	PYY	0.000	0.883	0.003	[MN/m]				
1017	PYY	0.000	0.883	0.002	[MN/m]				
1018	PYY	0.000	0.883	0.003	[MN/m]				
1018	PYY	0.000	0.883	0.002	[MN/m]				
1019	PYY	0.000	0.883	0.003	[MN/m]				
1019	PYY	0.000	0.883	0.002	[MN/m]				
1020	PYY	0.000	0.883	0.003	[MN/m]				
1020	PYY	0.000	0.883	0.002	[MN/m]				
1021	PYY	0.000	0.883	0.003	[MN/m]				
1021	PYY	0.000	0.883	0.002	[MN/m]				
1022	PYY	0.000	0.883	0.003	[MN/m]				
1022	PYY	0.000	0.883	0.002	[MN/m]				
1023	PYY	0.000	0.883	0.003	[MN/m]				
1023	PYY	0.000	0.883	0.002	[MN/m]				
1024	PYY	0.000	0.883	0.003	[MN/m]				
1024	PYY	0.000	0.883	0.002	[MN/m]				
1025	PYY	0.000	0.883	0.003	[MN/m]				
1025	PYY	0.000	0.883	0.002	[MN/m]				
1026	PYY	0.000	0.883	0.003	[MN/m]				
1026	PYY	0.000	0.883	0.002	[MN/m]				
1027	PYY	0.000	0.883	0.003	[MN/m]				
1027	PYY	0.000	0.883	0.002	[MN/m]				
1028	PYY	0.000	0.883	0.003	[MN/m]				
1028	PYY	0.000	0.883	0.002	[MN/m]				
1029	PYY	0.000	0.883	0.003	[MN/m]				
1029	PYY	0.000	0.883	0.002	[MN/m]				
1030	PYY	0.000	0.883	0.003	[MN/m]				
1030	PYY	0.000	0.883	0.002	[MN/m]				
1031	PYY	0.000	0.883	0.003	[MN/m]				
1031	PYY	0.000	0.883	0.002	[MN/m]				
1032	PYY	0.000	0.883	0.003	[MN/m]				
1032	PYY	0.000	0.883	0.002	[MN/m]				
1033	PYY	0.000	0.883	0.003	[MN/m]				
1033	PYY	0.000	0.883	0.002	[MN/m]				
1034	PYY	0.000	0.883	0.003	[MN/m]				
1034	PYY	0.000	0.883	0.002	[MN/m]				
1035	PYY	0.000	0.883	0.003	[MN/m]				
1035	PYY	0.000	0.883	0.002	[MN/m]				
1036	PYY	0.000	0.883	0.003	[MN/m]				
1036	PYY	0.000	0.883	0.002	[MN/m]				
1037	PYY	0.000	0.883	0.003	[MN/m]				
1037	PYY	0.000	0.883	0.002	[MN/m]				
1038	PYY	0.000	0.883	0.003	[MN/m]				
1038	PYY	0.000	0.883	0.002	[MN/m]				
1039	PYY	0.000	0.883	0.003	[MN/m]				
1039	PYY	0.000	0.883	0.002	[MN/m]				
1040	PYY	0.000	0.883	0.003	[MN/m]				
1040	PYY	0.000	0.883	0.002	[MN/m]				
1041	PYY	0.000	0.883	0.003	[MN/m]				
1041	PYY	0.000	0.883	0.002	[MN/m]				
1042	PYY	0.000	0.883	0.003	[MN/m]				
1042	PYY	0.000	0.883	0.002	[MN/m]				
1043	PYY	0.000	0.883	0.003	[MN/m]				
1043	PYY	0.000	0.883	0.002	[MN/m]				
1044	PYY	0.000	0.883	0.003	[MN/m]				
1044	PYY	0.000	0.883	0.002	[MN/m]				
1045	PYY	0.000	0.883	0.003	[MN/m]				
1045	PYY	0.000	0.883	0.002	[MN/m]				
1046	PYY	0.000	0.883	0.003	[MN/m]				
1046	PYY	0.000	0.883	0.002	[MN/m]				
1047	PYY	0.000	0.883	0.003	[MN/m]				
1047	PYY	0.000	0.883	0.002	[MN/m]				
1048	PYY	0.000	0.883	0.003	[MN/m]				
1048	PYY	0.000	0.883	0.002	[MN/m]				
1049	PYY	0.000	0.883	0.003	[MN/m]				
1049	PYY	0.000	0.883	0.002	[MN/m]				
1050	PYY	0.000	0.883	0.003	[MN/m]				
1050	PYY	0.000	0.883	0.002	[MN/m]				
1051	PYY	0.000	0.883	0.003	[MN/m]				
1051	PYY	0.000	0.883	0.002	[MN/m]				
1052	PYY	0.000	0.883	0.003	[MN/m]				
1052	PYY	0.000	0.883	0.002	[MN/m]				
1053	PYY	0.000	0.883	0.003	[MN/m]				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΣΕΙΣΜΙΚΕΣ ΜΕΜΟΝΩΜΕΝΕΣ ΦΟΡΤΙΣΕΙΣ

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
1053	PYY	0.000	0.883	0.002	[MN/m]				
1054	PYY	0.000	0.883	0.003	[MN/m]				
1054	PYY	0.000	0.883	0.002	[MN/m]				
1055	PYY	0.000	0.883	0.003	[MN/m]				
1055	PYY	0.000	0.883	0.002	[MN/m]				
1056	PYY	0.000	0.883	0.003	[MN/m]				
1056	PYY	0.000	0.883	0.002	[MN/m]				
1057	PYY	0.000	0.883	0.003	[MN/m]				
1057	PYY	0.000	0.883	0.002	[MN/m]				
1058	PYY	0.000	0.883	0.003	[MN/m]				
1058	PYY	0.000	0.883	0.002	[MN/m]				
1059	PYY	0.000	0.883	0.003	[MN/m]				
1059	PYY	0.000	0.883	0.002	[MN/m]				
1060	PYY	0.000	0.883	0.003	[MN/m]				
1060	PYY	0.000	0.883	0.002	[MN/m]				

Loads acting on QUAD-elements

Elements from	to	inc	Load Prim Type LC/CC	Load val.	Dimension	Variation dP/dX	dP/dY	dP/dZ
3000	3999	1	PYY	0.001	[MN/m2]			
4000	4999	1	PYY	0.001	[MN/m2]			

Load Case 203 ΔΥΝΑΜΕΙΣ ΑΔΡΑΝΕΙΑΣ-[+Ez]

Factor forces and moments 1.000
Factor dead weight DL-XX 0.000
Factor dead weight DL-YY 0.000
Factor dead weight DL-ZZ 0.432
effective construction stage 40 to 40

Meshfree Loading

Kind	Referenceto	Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
Area			0.000	2.000	0.000	PG	0.004	[MN/m2]
			11.800	2.000	0.000		0.004	[MN/m2]
			11.800	-1.000	0.000		0.002	[MN/m2]
			0.000	-1.000	0.000		0.002	[MN/m2]
	QGRP	3	3.000	(--)	activated		89.83	percent
Area			0.000	2.000	0.000	PG	0.004	[MN/m2]
			11.800	2.000	0.000		0.004	[MN/m2]
			11.800	-1.000	0.000		0.002	[MN/m2]
			0.000	-1.000	0.000		0.002	[MN/m2]
	QGRP	4	3.000	(--)	activated		10.17	percent
Area			11.800	-2.000	0.000	PG	0.007	[MN/m2]
			0.000	-2.000	0.000		0.007	[MN/m2]
			0.000	-1.000	0.000		0.007	[MN/m2]
			11.800	-1.000	0.000		0.007	[MN/m2]
	QGRP	3	3.000	(--)	activated		89.83	percent
Area			11.800	-2.000	0.000	PG	0.007	[MN/m2]
			0.000	-2.000	0.000		0.007	[MN/m2]
			0.000	-1.000	0.000		0.007	[MN/m2]
			11.800	-1.000	0.000		0.007	[MN/m2]
	QGRP	4	3.000	(--)	activated		10.17	percent
Area			0.000	2.000	0.000	PG	0.000	[MN/m2]
			11.800	2.000	0.000		0.000	[MN/m2]
			11.800	-1.000	0.000		0.000	[MN/m2]
			0.000	-1.000	0.000		0.000	[MN/m2]
	QGRP	3	3.000	(--)	activated		89.83	percent
Area			0.000	2.000	0.000	PG	0.000	[MN/m2]
			11.800	2.000	0.000		0.000	[MN/m2]
			11.800	-1.000	0.000		0.000	[MN/m2]
			0.000	-1.000	0.000		0.000	[MN/m2]
	QGRP	4	3.000	(--)	activated		10.17	percent

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
1001	PG	0.000	0.883	0.003	[MN/m]				
1001	PG	0.000	0.883	0.002	[MN/m]				
1002	PG	0.000	0.883	0.003	[MN/m]				
1002	PG	0.000	0.883	0.002	[MN/m]				
1003	PG	0.000	0.883	0.003	[MN/m]				
1003	PG	0.000	0.883	0.002	[MN/m]				
1004	PG	0.000	0.883	0.003	[MN/m]				
1004	PG	0.000	0.883	0.002	[MN/m]				
1005	PG	0.000	0.883	0.003	[MN/m]				
1005	PG	0.000	0.883	0.002	[MN/m]				
1006	PG	0.000	0.883	0.003	[MN/m]				
1006	PG	0.000	0.883	0.002	[MN/m]				
1007	PG	0.000	0.883	0.003	[MN/m]				
1007	PG	0.000	0.883	0.002	[MN/m]				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΣΕΙΣΜΙΚΕΣ ΜΕΜΟΝΩΜΕΝΕΣ ΦΟΡΤΙΣΕΙΣ

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
1008	PG	0.000	0.883	0.003	[MN/m]				
1008	PG	0.000	0.883	0.002	[MN/m]				
1009	PG	0.000	0.883	0.003	[MN/m]				
1009	PG	0.000	0.883	0.002	[MN/m]				
1010	PG	0.000	0.883	0.003	[MN/m]				
1010	PG	0.000	0.883	0.002	[MN/m]				
1011	PG	0.000	0.883	0.003	[MN/m]				
1011	PG	0.000	0.883	0.002	[MN/m]				
1012	PG	0.000	0.883	0.003	[MN/m]				
1012	PG	0.000	0.883	0.002	[MN/m]				
1013	PG	0.000	0.883	0.003	[MN/m]				
1013	PG	0.000	0.883	0.002	[MN/m]				
1014	PG	0.000	0.883	0.003	[MN/m]				
1014	PG	0.000	0.883	0.002	[MN/m]				
1015	PG	0.000	0.883	0.003	[MN/m]				
1015	PG	0.000	0.883	0.002	[MN/m]				
1016	PG	0.000	0.883	0.003	[MN/m]				
1016	PG	0.000	0.883	0.002	[MN/m]				
1017	PG	0.000	0.883	0.003	[MN/m]				
1017	PG	0.000	0.883	0.002	[MN/m]				
1018	PG	0.000	0.883	0.003	[MN/m]				
1018	PG	0.000	0.883	0.002	[MN/m]				
1019	PG	0.000	0.883	0.003	[MN/m]				
1019	PG	0.000	0.883	0.002	[MN/m]				
1020	PG	0.000	0.883	0.003	[MN/m]				
1020	PG	0.000	0.883	0.002	[MN/m]				
1021	PG	0.000	0.883	0.003	[MN/m]				
1021	PG	0.000	0.883	0.002	[MN/m]				
1022	PG	0.000	0.883	0.003	[MN/m]				
1022	PG	0.000	0.883	0.002	[MN/m]				
1023	PG	0.000	0.883	0.003	[MN/m]				
1023	PG	0.000	0.883	0.002	[MN/m]				
1024	PG	0.000	0.883	0.003	[MN/m]				
1024	PG	0.000	0.883	0.002	[MN/m]				
1025	PG	0.000	0.883	0.003	[MN/m]				
1025	PG	0.000	0.883	0.002	[MN/m]				
1026	PG	0.000	0.883	0.003	[MN/m]				
1026	PG	0.000	0.883	0.002	[MN/m]				
1027	PG	0.000	0.883	0.003	[MN/m]				
1027	PG	0.000	0.883	0.002	[MN/m]				
1028	PG	0.000	0.883	0.003	[MN/m]				
1028	PG	0.000	0.883	0.002	[MN/m]				
1029	PG	0.000	0.883	0.003	[MN/m]				
1029	PG	0.000	0.883	0.002	[MN/m]				
1030	PG	0.000	0.883	0.003	[MN/m]				
1030	PG	0.000	0.883	0.002	[MN/m]				
1031	PG	0.000	0.883	0.003	[MN/m]				
1031	PG	0.000	0.883	0.002	[MN/m]				
1032	PG	0.000	0.883	0.003	[MN/m]				
1032	PG	0.000	0.883	0.002	[MN/m]				
1033	PG	0.000	0.883	0.003	[MN/m]				
1033	PG	0.000	0.883	0.002	[MN/m]				
1034	PG	0.000	0.883	0.003	[MN/m]				
1034	PG	0.000	0.883	0.002	[MN/m]				
1035	PG	0.000	0.883	0.003	[MN/m]				
1035	PG	0.000	0.883	0.002	[MN/m]				
1036	PG	0.000	0.883	0.003	[MN/m]				
1036	PG	0.000	0.883	0.002	[MN/m]				
1037	PG	0.000	0.883	0.003	[MN/m]				
1037	PG	0.000	0.883	0.002	[MN/m]				
1038	PG	0.000	0.883	0.003	[MN/m]				
1038	PG	0.000	0.883	0.002	[MN/m]				
1039	PG	0.000	0.883	0.003	[MN/m]				
1039	PG	0.000	0.883	0.002	[MN/m]				
1040	PG	0.000	0.883	0.003	[MN/m]				
1040	PG	0.000	0.883	0.002	[MN/m]				
1041	PG	0.000	0.883	0.003	[MN/m]				
1041	PG	0.000	0.883	0.002	[MN/m]				
1042	PG	0.000	0.883	0.003	[MN/m]				
1042	PG	0.000	0.883	0.002	[MN/m]				
1043	PG	0.000	0.883	0.003	[MN/m]				
1043	PG	0.000	0.883	0.002	[MN/m]				
1044	PG	0.000	0.883	0.003	[MN/m]				
1044	PG	0.000	0.883	0.002	[MN/m]				
1045	PG	0.000	0.883	0.003	[MN/m]				
1045	PG	0.000	0.883	0.002	[MN/m]				
1046	PG	0.000	0.883	0.003	[MN/m]				
1046	PG	0.000	0.883	0.002	[MN/m]				
1047	PG	0.000	0.883	0.003	[MN/m]				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΣΕΙΣΜΙΚΕΣ ΜΕΜΟΝΩΜΕΝΕΣ ΦΟΡΤΙΣΕΙΣ

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
1047	PG	0.000	0.883	0.002	[MN/m]				
1048	PG	0.000	0.883	0.003	[MN/m]				
1048	PG	0.000	0.883	0.002	[MN/m]				
1049	PG	0.000	0.883	0.003	[MN/m]				
1049	PG	0.000	0.883	0.002	[MN/m]				
1050	PG	0.000	0.883	0.003	[MN/m]				
1050	PG	0.000	0.883	0.002	[MN/m]				
1051	PG	0.000	0.883	0.003	[MN/m]				
1051	PG	0.000	0.883	0.002	[MN/m]				
1052	PG	0.000	0.883	0.003	[MN/m]				
1052	PG	0.000	0.883	0.002	[MN/m]				
1053	PG	0.000	0.883	0.003	[MN/m]				
1053	PG	0.000	0.883	0.002	[MN/m]				
1054	PG	0.000	0.883	0.003	[MN/m]				
1054	PG	0.000	0.883	0.002	[MN/m]				
1055	PG	0.000	0.883	0.003	[MN/m]				
1055	PG	0.000	0.883	0.002	[MN/m]				
1056	PG	0.000	0.883	0.003	[MN/m]				
1056	PG	0.000	0.883	0.002	[MN/m]				
1057	PG	0.000	0.883	0.003	[MN/m]				
1057	PG	0.000	0.883	0.002	[MN/m]				
1058	PG	0.000	0.883	0.003	[MN/m]				
1058	PG	0.000	0.883	0.002	[MN/m]				
1059	PG	0.000	0.883	0.003	[MN/m]				
1059	PG	0.000	0.883	0.002	[MN/m]				
1060	PG	0.000	0.883	0.003	[MN/m]				
1060	PG	0.000	0.883	0.002	[MN/m]				

Loads acting on QUAD-elements

Elements from	to	inc	Load Prim Type LC/CC	Load val.	Dimension	Variation dP/dX	dP/dY	dP/dZ
3000	3999	1	PG	0.002	[MN/m2]			
4000	4999	1	PG	0.002	[MN/m2]			

Load Case 204 ΟΜΟΙΟΜΟΡΦΟ ΚΙΝΗΤΩΝ ΣΕΙΣΜ.ΣΥΝΔ.

Factor forces and moments		1.000
Factor dead weight	DL-XX	0.000
Factor dead weight	DL-YY	0.000
Factor dead weight	DL-ZZ	0.000

Loads acting on QUAD-elements

Elements from	to	inc	Load Prim Type LC/CC	Load val.	Dimension	Variation dP/dX	dP/dY	dP/dZ
3000	3999	1	PG	0.020	[MN/m2]			
4000	4999	1	PG	0.020	[MN/m2]			

Load Case 205 ΟΜΟΦΟΡΕΣ ΣΕΙΣΜΙΚΕΣ ΩΘΗΣΕΙΣ ΓΑΙΩΝ-[

Factor forces and moments		1.000
Factor dead weight	DL-XX	0.000
Factor dead weight	DL-YY	0.000
Factor dead weight	DL-ZZ	0.000

Meshfree Loading

Kind	Referenceto	Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
Area			0.000	2.000	0.000	PXX	0.023	[MN/m2]
			0.000	2.000	0.800		0.023	[MN/m2]
			0.000	-2.000	0.800		0.023	[MN/m2]
			0.000	-2.000	0.000		0.023	[MN/m2]
	QGRP	8	3.000		activated		100.00	percent
Area			0.000	2.000	0.800	PXX	0.023	[MN/m2]
			0.000	-2.000	0.800		0.023	[MN/m2]
			0.000	-2.000	1.350		0.023	[MN/m2]
			0.000	2.000	1.350		0.023	[MN/m2]
	QGRP	9	3.000		activated		100.00	percent
Area			11.800	2.000	0.000	PXX	0.023	[MN/m2]
			11.800	2.000	0.800		0.023	[MN/m2]
			11.800	-2.000	0.800		0.023	[MN/m2]
			11.800	-2.000	0.000		0.023	[MN/m2]
	QGRP	8	3.000		activated		100.00	percent
Area			11.800	2.000	0.800	PXX	0.023	[MN/m2]
			11.800	-2.000	0.800		0.023	[MN/m2]
			11.800	-2.000	1.350		0.023	[MN/m2]
			11.800	2.000	1.350		0.023	[MN/m2]
	QGRP	9	3.000		activated		100.00	percent

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΣΕΙΣΜΙΚΕΣ ΜΕΜΟΝΩΜΕΝΕΣ ΦΟΡΤΙΣΕΙΣ

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
11001	PXX	0.000	0.550	0.032	[MN/m]				
11002	PXX	0.000	0.550	0.032	[MN/m]				
11003	PXX	0.000	0.550	0.032	[MN/m]				
11004	PXX	0.000	0.550	0.032	[MN/m]				
11005	PXX	0.000	0.550	0.032	[MN/m]				
11006	PXX	0.000	0.550	0.032	[MN/m]				
12001	PXX	0.000	1.000	0.032	[MN/m]				
12002	PXX	0.000	1.000	0.032	[MN/m]				
12003	PXX	0.000	1.000	0.032	[MN/m]				
12004	PXX	0.000	1.000	0.032	[MN/m]				
12005	PXX	0.000	1.000	0.032	[MN/m]				
12006	PXX	0.000	1.000	0.032	[MN/m]				
12007	PXX	0.000	1.000	0.032	[MN/m]				
12008	PXX	0.000	1.000	0.032	[MN/m]				
12009	PXX	0.000	1.000	0.032	[MN/m]				
12010	PXX	0.000	1.000	0.032	[MN/m]				
12011	PXX	0.000	1.000	0.032	[MN/m]				
12012	PXX	0.000	1.000	0.032	[MN/m]				
12013	PXX	0.000	1.000	0.032	[MN/m]				
12014	PXX	0.000	1.000	0.032	[MN/m]				
12015	PXX	0.000	1.000	0.032	[MN/m]				
12016	PXX	0.000	1.000	0.032	[MN/m]				
12017	PXX	0.000	1.000	0.032	[MN/m]				
12018	PXX	0.000	1.000	0.032	[MN/m]				

Load Case 711 Unidirectional +EX

```
Factor forces and moments          1.000
Factor dead weight      DL-XX      0.360
Factor dead weight      DL-YY      0.000
Factor dead weight      DL-ZZ      0.000
Loads partially  copied from load case   201 with factor    1.000
Loads partially  copied from load case   205 with factor    1.000
```

Meshfree Loading

Kind	Referenceto	Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
Area			0.000	2.000	0.000	PXX	0.004	[MN/m2]
			11.800	2.000	0.000		0.004	[MN/m2]
			11.800	-1.000	0.000		0.002	[MN/m2]
			0.000	-1.000	0.000		0.002	[MN/m2]
	QGRP	3	3.000	(--)	activated		89.83	percent
Area			0.000	2.000	0.000	PXX	0.004	[MN/m2]
			11.800	2.000	0.000		0.004	[MN/m2]
			11.800	-1.000	0.000		0.002	[MN/m2]
			0.000	-1.000	0.000		0.002	[MN/m2]
	QGRP	4	3.000	(--)	activated		10.17	percent
Area			11.800	-2.000	0.000	PXX	0.006	[MN/m2]
			0.000	-2.000	0.000		0.006	[MN/m2]
			0.000	-1.000	0.000		0.006	[MN/m2]
			11.800	-1.000	0.000		0.006	[MN/m2]
	QGRP	3	3.000	(--)	activated		89.83	percent
Area			11.800	-2.000	0.000	PXX	0.006	[MN/m2]
			0.000	-2.000	0.000		0.006	[MN/m2]
			0.000	-1.000	0.000		0.006	[MN/m2]
			11.800	-1.000	0.000		0.006	[MN/m2]
	QGRP	4	3.000	(--)	activated		10.17	percent
Area			0.000	2.000	0.000	PXX	0.000	[MN/m2]
			11.800	2.000	0.000		0.000	[MN/m2]
			11.800	-1.000	0.000		0.000	[MN/m2]
			0.000	-1.000	0.000		0.000	[MN/m2]
	QGRP	3	3.000	(--)	activated		89.83	percent
Area			0.000	2.000	0.000	PXX	0.000	[MN/m2]
			11.800	2.000	0.000		0.000	[MN/m2]
			11.800	-1.000	0.000		0.000	[MN/m2]
			0.000	-1.000	0.000		0.000	[MN/m2]
	QGRP	4	3.000	(--)	activated		10.17	percent
Area			0.000	2.000	0.000	PXX	0.023	[MN/m2]
			0.000	2.000	0.800		0.023	[MN/m2]
			0.000	-2.000	0.800		0.023	[MN/m2]
			0.000	-2.000	0.000		0.023	[MN/m2]
	QGRP	8	3.000		activated		100.00	percent
Area			0.000	2.000	0.800	PXX	0.023	[MN/m2]
			0.000	-2.000	0.800		0.023	[MN/m2]
			0.000	-2.000	1.350		0.023	[MN/m2]
			0.000	2.000	1.350		0.023	[MN/m2]
	QGRP	9	3.000		activated		100.00	percent
Area			11.800	2.000	0.000	PXX	0.023	[MN/m2]
			11.800	2.000	0.800		0.023	[MN/m2]
			11.800	-2.000	0.800		0.023	[MN/m2]

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΣΕΙΣΜΙΚΕΣ ΜΕΜΟΝΩΜΕΝΕΣ ΦΟΡΤΙΣΕΙΣ

Meshfree Loading

Kind	Referenceto		Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
				11.800	-2.000	0.000		0.023	[MN/m2]
	QGRP	8	3.000			activated		100.00	percent
Area				11.800	2.000	0.800	PXX	0.023	[MN/m2]
				11.800	-2.000	0.800		0.023	[MN/m2]
				11.800	-2.000	1.350		0.023	[MN/m2]
				11.800	2.000	1.350		0.023	[MN/m2]
	QGRP	9	3.000			activated		100.00	percent

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
1001	PXX	0.000	0.883	0.002	[MN/m]				
1001	PXX	0.000	0.883	0.003	[MN/m]				
1002	PXX	0.000	0.883	0.002	[MN/m]				
1002	PXX	0.000	0.883	0.003	[MN/m]				
1003	PXX	0.000	0.883	0.002	[MN/m]				
1003	PXX	0.000	0.883	0.003	[MN/m]				
1004	PXX	0.000	0.883	0.002	[MN/m]				
1004	PXX	0.000	0.883	0.003	[MN/m]				
1005	PXX	0.000	0.883	0.002	[MN/m]				
1005	PXX	0.000	0.883	0.003	[MN/m]				
1006	PXX	0.000	0.883	0.002	[MN/m]				
1006	PXX	0.000	0.883	0.003	[MN/m]				
1007	PXX	0.000	0.883	0.002	[MN/m]				
1007	PXX	0.000	0.883	0.003	[MN/m]				
1008	PXX	0.000	0.883	0.002	[MN/m]				
1008	PXX	0.000	0.883	0.003	[MN/m]				
1009	PXX	0.000	0.883	0.002	[MN/m]				
1009	PXX	0.000	0.883	0.003	[MN/m]				
1010	PXX	0.000	0.883	0.002	[MN/m]				
1010	PXX	0.000	0.883	0.003	[MN/m]				
1011	PXX	0.000	0.883	0.002	[MN/m]				
1011	PXX	0.000	0.883	0.003	[MN/m]				
1012	PXX	0.000	0.883	0.002	[MN/m]				
1012	PXX	0.000	0.883	0.003	[MN/m]				
1013	PXX	0.000	0.883	0.002	[MN/m]				
1013	PXX	0.000	0.883	0.003	[MN/m]				
1014	PXX	0.000	0.883	0.002	[MN/m]				
1014	PXX	0.000	0.883	0.003	[MN/m]				
1015	PXX	0.000	0.883	0.002	[MN/m]				
1015	PXX	0.000	0.883	0.003	[MN/m]				
1016	PXX	0.000	0.883	0.002	[MN/m]				
1016	PXX	0.000	0.883	0.003	[MN/m]				
1017	PXX	0.000	0.883	0.002	[MN/m]				
1017	PXX	0.000	0.883	0.003	[MN/m]				
1018	PXX	0.000	0.883	0.002	[MN/m]				
1018	PXX	0.000	0.883	0.003	[MN/m]				
1019	PXX	0.000	0.883	0.002	[MN/m]				
1019	PXX	0.000	0.883	0.003	[MN/m]				
1020	PXX	0.000	0.883	0.002	[MN/m]				
1020	PXX	0.000	0.883	0.003	[MN/m]				
1021	PXX	0.000	0.883	0.002	[MN/m]				
1021	PXX	0.000	0.883	0.003	[MN/m]				
1022	PXX	0.000	0.883	0.002	[MN/m]				
1022	PXX	0.000	0.883	0.003	[MN/m]				
1023	PXX	0.000	0.883	0.002	[MN/m]				
1023	PXX	0.000	0.883	0.003	[MN/m]				
1024	PXX	0.000	0.883	0.002	[MN/m]				
1024	PXX	0.000	0.883	0.003	[MN/m]				
1025	PXX	0.000	0.883	0.002	[MN/m]				
1025	PXX	0.000	0.883	0.003	[MN/m]				
1026	PXX	0.000	0.883	0.002	[MN/m]				
1026	PXX	0.000	0.883	0.003	[MN/m]				
1027	PXX	0.000	0.883	0.002	[MN/m]				
1027	PXX	0.000	0.883	0.003	[MN/m]				
1028	PXX	0.000	0.883	0.002	[MN/m]				
1028	PXX	0.000	0.883	0.003	[MN/m]				
1029	PXX	0.000	0.883	0.002	[MN/m]				
1029	PXX	0.000	0.883	0.003	[MN/m]				
1030	PXX	0.000	0.883	0.002	[MN/m]				
1030	PXX	0.000	0.883	0.003	[MN/m]				
1031	PXX	0.000	0.883	0.002	[MN/m]				
1031	PXX	0.000	0.883	0.003	[MN/m]				
1032	PXX	0.000	0.883	0.002	[MN/m]				
1032	PXX	0.000	0.883	0.003	[MN/m]				
1033	PXX	0.000	0.883	0.002	[MN/m]				
1033	PXX	0.000	0.883	0.003	[MN/m]				
1034	PXX	0.000	0.883	0.002	[MN/m]				
1034	PXX	0.000	0.883	0.003	[MN/m]				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΣΕΙΣΜΙΚΕΣ ΜΕΜΟΝΩΜΕΝΕΣ ΦΟΡΤΙΣΕΙΣ

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
1035	PXX	0.000	0.883	0.002	[MN/m]				
1035	PXX	0.000	0.883	0.003	[MN/m]				
1036	PXX	0.000	0.883	0.002	[MN/m]				
1036	PXX	0.000	0.883	0.003	[MN/m]				
1037	PXX	0.000	0.883	0.002	[MN/m]				
1037	PXX	0.000	0.883	0.003	[MN/m]				
1038	PXX	0.000	0.883	0.002	[MN/m]				
1038	PXX	0.000	0.883	0.003	[MN/m]				
1039	PXX	0.000	0.883	0.002	[MN/m]				
1039	PXX	0.000	0.883	0.003	[MN/m]				
1040	PXX	0.000	0.883	0.002	[MN/m]				
1040	PXX	0.000	0.883	0.003	[MN/m]				
1041	PXX	0.000	0.883	0.002	[MN/m]				
1041	PXX	0.000	0.883	0.003	[MN/m]				
1042	PXX	0.000	0.883	0.002	[MN/m]				
1042	PXX	0.000	0.883	0.003	[MN/m]				
1043	PXX	0.000	0.883	0.002	[MN/m]				
1043	PXX	0.000	0.883	0.003	[MN/m]				
1044	PXX	0.000	0.883	0.002	[MN/m]				
1044	PXX	0.000	0.883	0.003	[MN/m]				
1045	PXX	0.000	0.883	0.002	[MN/m]				
1045	PXX	0.000	0.883	0.003	[MN/m]				
1046	PXX	0.000	0.883	0.002	[MN/m]				
1046	PXX	0.000	0.883	0.003	[MN/m]				
1047	PXX	0.000	0.883	0.002	[MN/m]				
1047	PXX	0.000	0.883	0.003	[MN/m]				
1048	PXX	0.000	0.883	0.002	[MN/m]				
1048	PXX	0.000	0.883	0.003	[MN/m]				
1049	PXX	0.000	0.883	0.002	[MN/m]				
1049	PXX	0.000	0.883	0.003	[MN/m]				
1050	PXX	0.000	0.883	0.002	[MN/m]				
1050	PXX	0.000	0.883	0.003	[MN/m]				
1051	PXX	0.000	0.883	0.002	[MN/m]				
1051	PXX	0.000	0.883	0.003	[MN/m]				
1052	PXX	0.000	0.883	0.002	[MN/m]				
1052	PXX	0.000	0.883	0.003	[MN/m]				
1053	PXX	0.000	0.883	0.002	[MN/m]				
1053	PXX	0.000	0.883	0.003	[MN/m]				
1054	PXX	0.000	0.883	0.002	[MN/m]				
1054	PXX	0.000	0.883	0.003	[MN/m]				
1055	PXX	0.000	0.883	0.002	[MN/m]				
1055	PXX	0.000	0.883	0.003	[MN/m]				
1056	PXX	0.000	0.883	0.002	[MN/m]				
1056	PXX	0.000	0.883	0.003	[MN/m]				
1057	PXX	0.000	0.883	0.002	[MN/m]				
1057	PXX	0.000	0.883	0.003	[MN/m]				
1058	PXX	0.000	0.883	0.002	[MN/m]				
1058	PXX	0.000	0.883	0.003	[MN/m]				
1059	PXX	0.000	0.883	0.002	[MN/m]				
1059	PXX	0.000	0.883	0.003	[MN/m]				
1060	PXX	0.000	0.883	0.002	[MN/m]				
1060	PXX	0.000	0.883	0.003	[MN/m]				
11001	PXX	0.000	0.550	0.032	[MN/m]				
11002	PXX	0.000	0.550	0.032	[MN/m]				
11003	PXX	0.000	0.550	0.032	[MN/m]				
11004	PXX	0.000	0.550	0.032	[MN/m]				
11005	PXX	0.000	0.550	0.032	[MN/m]				
11006	PXX	0.000	0.550	0.032	[MN/m]				
12001	PXX	0.000	1.000	0.032	[MN/m]				
12002	PXX	0.000	1.000	0.032	[MN/m]				
12003	PXX	0.000	1.000	0.032	[MN/m]				
12004	PXX	0.000	1.000	0.032	[MN/m]				
12005	PXX	0.000	1.000	0.032	[MN/m]				
12006	PXX	0.000	1.000	0.032	[MN/m]				
12007	PXX	0.000	1.000	0.032	[MN/m]				
12008	PXX	0.000	1.000	0.032	[MN/m]				
12009	PXX	0.000	1.000	0.032	[MN/m]				
12010	PXX	0.000	1.000	0.032	[MN/m]				
12011	PXX	0.000	1.000	0.032	[MN/m]				
12012	PXX	0.000	1.000	0.032	[MN/m]				
12013	PXX	0.000	1.000	0.032	[MN/m]				
12014	PXX	0.000	1.000	0.032	[MN/m]				
12015	PXX	0.000	1.000	0.032	[MN/m]				
12016	PXX	0.000	1.000	0.032	[MN/m]				
12017	PXX	0.000	1.000	0.032	[MN/m]				
12018	PXX	0.000	1.000	0.032	[MN/m]				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΣΕΙΣΜΙΚΕΣ ΜΕΜΟΝΩΜΕΝΕΣ ΦΟΡΤΙΣΕΙΣ

Loads acting on QUAD-elements

Elements from	to	inc	Load Prim Type LC/CC	Load val.	Dimension	Variation dP/dX	dP/dY	dP/dZ
3000	3999	1	PXX	0.001	[MN/m2]			
4000	4999	1	PXX	0.001	[MN/m2]			

Load Case 712 Unidirectional -EX

Factor forces and moments 1.000
Factor dead weight DL-XX -0.360
Factor dead weight DL-YY 0.000
Factor dead weight DL-ZZ 0.000
Loads partially copied from load case 201 with factor -1.000
Loads partially copied from load case 205 with factor -1.000

Meshfree Loading

Kind	Referenceto		Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
Area				0.000	2.000	0.000	PXX	-0.004	[MN/m2]
				11.800	2.000	0.000		-0.004	[MN/m2]
				11.800	-1.000	0.000		-0.002	[MN/m2]
				0.000	-1.000	0.000		-0.002	[MN/m2]
	QGRP	3	3.000	(--)		activated		89.83	percent
Area				0.000	2.000	0.000	PXX	-0.004	[MN/m2]
				11.800	2.000	0.000		-0.004	[MN/m2]
				11.800	-1.000	0.000		-0.002	[MN/m2]
				0.000	-1.000	0.000		-0.002	[MN/m2]
	QGRP	4	3.000	(--)		activated		10.17	percent
Area				11.800	-2.000	0.000	PXX	-0.006	[MN/m2]
				0.000	-2.000	0.000		-0.006	[MN/m2]
				0.000	-1.000	0.000		-0.006	[MN/m2]
				11.800	-1.000	0.000		-0.006	[MN/m2]
	QGRP	3	3.000	(--)		activated		89.83	percent
Area				11.800	-2.000	0.000	PXX	-0.006	[MN/m2]
				0.000	-2.000	0.000		-0.006	[MN/m2]
				0.000	-1.000	0.000		-0.006	[MN/m2]
				11.800	-1.000	0.000		-0.006	[MN/m2]
	QGRP	4	3.000	(--)		activated		10.17	percent
Area				0.000	2.000	0.000	PXX	0.000	[MN/m2]
				11.800	2.000	0.000		0.000	[MN/m2]
				11.800	-1.000	0.000		0.000	[MN/m2]
				0.000	-1.000	0.000		0.000	[MN/m2]
	QGRP	3	3.000	(--)		activated		89.83	percent
Area				0.000	2.000	0.000	PXX	0.000	[MN/m2]
				11.800	2.000	0.000		0.000	[MN/m2]
				11.800	-1.000	0.000		0.000	[MN/m2]
				0.000	-1.000	0.000		0.000	[MN/m2]
	QGRP	4	3.000	(--)		activated		10.17	percent
Area				0.000	2.000	0.000	PXX	-0.023	[MN/m2]
				0.000	2.000	0.800		-0.023	[MN/m2]
				0.000	-2.000	0.800		-0.023	[MN/m2]
				0.000	-2.000	0.000		-0.023	[MN/m2]
	QGRP	8	3.000			activated		100.00	percent
Area				0.000	2.000	0.800	PXX	-0.023	[MN/m2]
				0.000	-2.000	0.800		-0.023	[MN/m2]
				0.000	-2.000	1.350		-0.023	[MN/m2]
				0.000	2.000	1.350		-0.023	[MN/m2]
	QGRP	9	3.000			activated		100.00	percent
Area				11.800	2.000	0.000	PXX	-0.023	[MN/m2]
				11.800	2.000	0.800		-0.023	[MN/m2]
				11.800	-2.000	0.800		-0.023	[MN/m2]
				11.800	-2.000	0.000		-0.023	[MN/m2]
	QGRP	8	3.000			activated		100.00	percent
Area				11.800	2.000	0.800	PXX	-0.023	[MN/m2]
				11.800	-2.000	0.800		-0.023	[MN/m2]
				11.800	-2.000	1.350		-0.023	[MN/m2]
				11.800	2.000	1.350		-0.023	[MN/m2]
	QGRP	9	3.000			activated		100.00	percent

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
1001	PXX	0.000	0.883	-0.002	[MN/m]				
1001	PXX	0.000	0.883	-0.003	[MN/m]				
1002	PXX	0.000	0.883	-0.002	[MN/m]				
1002	PXX	0.000	0.883	-0.003	[MN/m]				
1003	PXX	0.000	0.883	-0.002	[MN/m]				
1003	PXX	0.000	0.883	-0.003	[MN/m]				
1004	PXX	0.000	0.883	-0.002	[MN/m]				
1004	PXX	0.000	0.883	-0.003	[MN/m]				
1005	PXX	0.000	0.883	-0.002	[MN/m]				
1005	PXX	0.000	0.883	-0.003	[MN/m]				
1006	PXX	0.000	0.883	-0.002	[MN/m]				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΣΕΙΣΜΙΚΕΣ ΜΕΜΟΝΩΜΕΝΕΣ ΦΟΡΤΙΣΕΙΣ

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
1006	PXX	0.000	0.883	-0.003	[MN/m]				
1007	PXX	0.000	0.883	-0.002	[MN/m]				
1007	PXX	0.000	0.883	-0.003	[MN/m]				
1008	PXX	0.000	0.883	-0.002	[MN/m]				
1008	PXX	0.000	0.883	-0.003	[MN/m]				
1009	PXX	0.000	0.883	-0.002	[MN/m]				
1009	PXX	0.000	0.883	-0.003	[MN/m]				
1010	PXX	0.000	0.883	-0.002	[MN/m]				
1010	PXX	0.000	0.883	-0.003	[MN/m]				
1011	PXX	0.000	0.883	-0.002	[MN/m]				
1011	PXX	0.000	0.883	-0.003	[MN/m]				
1012	PXX	0.000	0.883	-0.002	[MN/m]				
1012	PXX	0.000	0.883	-0.003	[MN/m]				
1013	PXX	0.000	0.883	-0.002	[MN/m]				
1013	PXX	0.000	0.883	-0.003	[MN/m]				
1014	PXX	0.000	0.883	-0.002	[MN/m]				
1014	PXX	0.000	0.883	-0.003	[MN/m]				
1015	PXX	0.000	0.883	-0.002	[MN/m]				
1015	PXX	0.000	0.883	-0.003	[MN/m]				
1016	PXX	0.000	0.883	-0.002	[MN/m]				
1016	PXX	0.000	0.883	-0.003	[MN/m]				
1017	PXX	0.000	0.883	-0.002	[MN/m]				
1017	PXX	0.000	0.883	-0.003	[MN/m]				
1018	PXX	0.000	0.883	-0.002	[MN/m]				
1018	PXX	0.000	0.883	-0.003	[MN/m]				
1019	PXX	0.000	0.883	-0.002	[MN/m]				
1019	PXX	0.000	0.883	-0.003	[MN/m]				
1020	PXX	0.000	0.883	-0.002	[MN/m]				
1020	PXX	0.000	0.883	-0.003	[MN/m]				
1021	PXX	0.000	0.883	-0.002	[MN/m]				
1021	PXX	0.000	0.883	-0.003	[MN/m]				
1022	PXX	0.000	0.883	-0.002	[MN/m]				
1022	PXX	0.000	0.883	-0.003	[MN/m]				
1023	PXX	0.000	0.883	-0.002	[MN/m]				
1023	PXX	0.000	0.883	-0.003	[MN/m]				
1024	PXX	0.000	0.883	-0.002	[MN/m]				
1024	PXX	0.000	0.883	-0.003	[MN/m]				
1025	PXX	0.000	0.883	-0.002	[MN/m]				
1025	PXX	0.000	0.883	-0.003	[MN/m]				
1026	PXX	0.000	0.883	-0.002	[MN/m]				
1026	PXX	0.000	0.883	-0.003	[MN/m]				
1027	PXX	0.000	0.883	-0.002	[MN/m]				
1027	PXX	0.000	0.883	-0.003	[MN/m]				
1028	PXX	0.000	0.883	-0.002	[MN/m]				
1028	PXX	0.000	0.883	-0.003	[MN/m]				
1029	PXX	0.000	0.883	-0.002	[MN/m]				
1029	PXX	0.000	0.883	-0.003	[MN/m]				
1030	PXX	0.000	0.883	-0.002	[MN/m]				
1030	PXX	0.000	0.883	-0.003	[MN/m]				
1031	PXX	0.000	0.883	-0.002	[MN/m]				
1031	PXX	0.000	0.883	-0.003	[MN/m]				
1032	PXX	0.000	0.883	-0.002	[MN/m]				
1032	PXX	0.000	0.883	-0.003	[MN/m]				
1033	PXX	0.000	0.883	-0.002	[MN/m]				
1033	PXX	0.000	0.883	-0.003	[MN/m]				
1034	PXX	0.000	0.883	-0.002	[MN/m]				
1034	PXX	0.000	0.883	-0.003	[MN/m]				
1035	PXX	0.000	0.883	-0.002	[MN/m]				
1035	PXX	0.000	0.883	-0.003	[MN/m]				
1036	PXX	0.000	0.883	-0.002	[MN/m]				
1036	PXX	0.000	0.883	-0.003	[MN/m]				
1037	PXX	0.000	0.883	-0.002	[MN/m]				
1037	PXX	0.000	0.883	-0.003	[MN/m]				
1038	PXX	0.000	0.883	-0.002	[MN/m]				
1038	PXX	0.000	0.883	-0.003	[MN/m]				
1039	PXX	0.000	0.883	-0.002	[MN/m]				
1039	PXX	0.000	0.883	-0.003	[MN/m]				
1040	PXX	0.000	0.883	-0.002	[MN/m]				
1040	PXX	0.000	0.883	-0.003	[MN/m]				
1041	PXX	0.000	0.883	-0.002	[MN/m]				
1041	PXX	0.000	0.883	-0.003	[MN/m]				
1042	PXX	0.000	0.883	-0.002	[MN/m]				
1042	PXX	0.000	0.883	-0.003	[MN/m]				
1043	PXX	0.000	0.883	-0.002	[MN/m]				
1043	PXX	0.000	0.883	-0.003	[MN/m]				
1044	PXX	0.000	0.883	-0.002	[MN/m]				
1044	PXX	0.000	0.883	-0.003	[MN/m]				
1045	PXX	0.000	0.883	-0.002	[MN/m]				
1045	PXX	0.000	0.883	-0.003	[MN/m]				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΣΕΙΣΜΙΚΕΣ ΜΕΜΟΝΩΜΕΝΕΣ ΦΟΡΤΙΣΕΙΣ

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
1046	PXX	0.000	0.883	-0.002	[MN/m]				
1046	PXX	0.000	0.883	-0.003	[MN/m]				
1047	PXX	0.000	0.883	-0.002	[MN/m]				
1047	PXX	0.000	0.883	-0.003	[MN/m]				
1048	PXX	0.000	0.883	-0.002	[MN/m]				
1048	PXX	0.000	0.883	-0.003	[MN/m]				
1049	PXX	0.000	0.883	-0.002	[MN/m]				
1049	PXX	0.000	0.883	-0.003	[MN/m]				
1050	PXX	0.000	0.883	-0.002	[MN/m]				
1050	PXX	0.000	0.883	-0.003	[MN/m]				
1051	PXX	0.000	0.883	-0.002	[MN/m]				
1051	PXX	0.000	0.883	-0.003	[MN/m]				
1052	PXX	0.000	0.883	-0.002	[MN/m]				
1052	PXX	0.000	0.883	-0.003	[MN/m]				
1053	PXX	0.000	0.883	-0.002	[MN/m]				
1053	PXX	0.000	0.883	-0.003	[MN/m]				
1054	PXX	0.000	0.883	-0.002	[MN/m]				
1054	PXX	0.000	0.883	-0.003	[MN/m]				
1055	PXX	0.000	0.883	-0.002	[MN/m]				
1055	PXX	0.000	0.883	-0.003	[MN/m]				
1056	PXX	0.000	0.883	-0.002	[MN/m]				
1056	PXX	0.000	0.883	-0.003	[MN/m]				
1057	PXX	0.000	0.883	-0.002	[MN/m]				
1057	PXX	0.000	0.883	-0.003	[MN/m]				
1058	PXX	0.000	0.883	-0.002	[MN/m]				
1058	PXX	0.000	0.883	-0.003	[MN/m]				
1059	PXX	0.000	0.883	-0.002	[MN/m]				
1059	PXX	0.000	0.883	-0.003	[MN/m]				
1060	PXX	0.000	0.883	-0.002	[MN/m]				
1060	PXX	0.000	0.883	-0.003	[MN/m]				
11001	PXX	0.000	0.550	-0.032	[MN/m]				
11002	PXX	0.000	0.550	-0.032	[MN/m]				
11003	PXX	0.000	0.550	-0.032	[MN/m]				
11004	PXX	0.000	0.550	-0.032	[MN/m]				
11005	PXX	0.000	0.550	-0.032	[MN/m]				
11006	PXX	0.000	0.550	-0.032	[MN/m]				
12001	PXX	0.000	1.000	-0.032	[MN/m]				
12002	PXX	0.000	1.000	-0.032	[MN/m]				
12003	PXX	0.000	1.000	-0.032	[MN/m]				
12004	PXX	0.000	1.000	-0.032	[MN/m]				
12005	PXX	0.000	1.000	-0.032	[MN/m]				
12006	PXX	0.000	1.000	-0.032	[MN/m]				
12007	PXX	0.000	1.000	-0.032	[MN/m]				
12008	PXX	0.000	1.000	-0.032	[MN/m]				
12009	PXX	0.000	1.000	-0.032	[MN/m]				
12010	PXX	0.000	1.000	-0.032	[MN/m]				
12011	PXX	0.000	1.000	-0.032	[MN/m]				
12012	PXX	0.000	1.000	-0.032	[MN/m]				
12013	PXX	0.000	1.000	-0.032	[MN/m]				
12014	PXX	0.000	1.000	-0.032	[MN/m]				
12015	PXX	0.000	1.000	-0.032	[MN/m]				
12016	PXX	0.000	1.000	-0.032	[MN/m]				
12017	PXX	0.000	1.000	-0.032	[MN/m]				
12018	PXX	0.000	1.000	-0.032	[MN/m]				

Loads acting on QUAD-elements

Elements from	to	inc	Load Prim Type LC/CC	Load val.	Dimension	Variation dP/dX	dP/dY	dP/dZ
3000	3999	1	PXX	-0.001	[MN/m2]			
4000	4999	1	PXX	-0.001	[MN/m2]			

Load Case 714 +EY

Factor forces and moments 1.000
Factor dead weight DL-XX 0.000
Factor dead weight DL-YY 0.360
Factor dead weight DL-ZZ 0.000
Loads partially copied from load case 202 with factor 1.000

Meshfree Loading

Kind	Referenceto	Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
Area			0.000	2.000	0.000	PYY	0.004	[MN/m2]
			11.800	2.000	0.000		0.004	[MN/m2]
			11.800	-1.000	0.000		0.002	[MN/m2]
			0.000	-1.000	0.000		0.002	[MN/m2]
	QGRP	3	3.000	(--)	activated		89.83	percent
Area			0.000	2.000	0.000	PYY	0.004	[MN/m2]
			11.800	2.000	0.000		0.004	[MN/m2]
			11.800	-1.000	0.000		0.002	[MN/m2]

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΣΕΙΣΜΙΚΕΣ ΜΕΜΟΝΩΜΕΝΕΣ ΦΟΡΤΙΣΕΙΣ

Meshfree Loading

Kind	Referenceto		Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
				0.000	-1.000	0.000		0.002	[MN/m2]
	QGRP	4	3.000	(--)	activated			10.17	percent
Area				11.800	-2.000	0.000	PYY	0.006	[MN/m2]
				0.000	-2.000	0.000		0.006	[MN/m2]
				0.000	-1.000	0.000		0.006	[MN/m2]
				11.800	-1.000	0.000		0.006	[MN/m2]
	QGRP	3	3.000	(--)	activated			89.83	percent
Area				11.800	-2.000	0.000	PYY	0.006	[MN/m2]
				0.000	-2.000	0.000		0.006	[MN/m2]
				0.000	-1.000	0.000		0.006	[MN/m2]
				11.800	-1.000	0.000		0.006	[MN/m2]
	QGRP	4	3.000	(--)	activated			10.17	percent
Area				0.000	2.000	0.000	PYY	0.000	[MN/m2]
				11.800	2.000	0.000		0.000	[MN/m2]
				11.800	-1.000	0.000		0.000	[MN/m2]
				0.000	-1.000	0.000		0.000	[MN/m2]
	QGRP	3	3.000	(--)	activated			89.83	percent
Area				0.000	2.000	0.000	PYY	0.000	[MN/m2]
				11.800	2.000	0.000		0.000	[MN/m2]
				11.800	-1.000	0.000		0.000	[MN/m2]
				0.000	-1.000	0.000		0.000	[MN/m2]
	QGRP	4	3.000	(--)	activated			10.17	percent

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
1001	PYY	0.000	0.883	0.002	[MN/m]				
1001	PYY	0.000	0.883	0.003	[MN/m]				
1002	PYY	0.000	0.883	0.002	[MN/m]				
1002	PYY	0.000	0.883	0.003	[MN/m]				
1003	PYY	0.000	0.883	0.002	[MN/m]				
1003	PYY	0.000	0.883	0.003	[MN/m]				
1004	PYY	0.000	0.883	0.002	[MN/m]				
1004	PYY	0.000	0.883	0.003	[MN/m]				
1005	PYY	0.000	0.883	0.002	[MN/m]				
1005	PYY	0.000	0.883	0.003	[MN/m]				
1006	PYY	0.000	0.883	0.002	[MN/m]				
1006	PYY	0.000	0.883	0.003	[MN/m]				
1007	PYY	0.000	0.883	0.002	[MN/m]				
1007	PYY	0.000	0.883	0.003	[MN/m]				
1008	PYY	0.000	0.883	0.002	[MN/m]				
1008	PYY	0.000	0.883	0.003	[MN/m]				
1009	PYY	0.000	0.883	0.002	[MN/m]				
1009	PYY	0.000	0.883	0.003	[MN/m]				
1010	PYY	0.000	0.883	0.002	[MN/m]				
1010	PYY	0.000	0.883	0.003	[MN/m]				
1011	PYY	0.000	0.883	0.002	[MN/m]				
1011	PYY	0.000	0.883	0.003	[MN/m]				
1012	PYY	0.000	0.883	0.002	[MN/m]				
1012	PYY	0.000	0.883	0.003	[MN/m]				
1013	PYY	0.000	0.883	0.002	[MN/m]				
1013	PYY	0.000	0.883	0.003	[MN/m]				
1014	PYY	0.000	0.883	0.002	[MN/m]				
1014	PYY	0.000	0.883	0.003	[MN/m]				
1015	PYY	0.000	0.883	0.002	[MN/m]				
1015	PYY	0.000	0.883	0.003	[MN/m]				
1016	PYY	0.000	0.883	0.002	[MN/m]				
1016	PYY	0.000	0.883	0.003	[MN/m]				
1017	PYY	0.000	0.883	0.002	[MN/m]				
1017	PYY	0.000	0.883	0.003	[MN/m]				
1018	PYY	0.000	0.883	0.002	[MN/m]				
1018	PYY	0.000	0.883	0.003	[MN/m]				
1019	PYY	0.000	0.883	0.002	[MN/m]				
1019	PYY	0.000	0.883	0.003	[MN/m]				
1020	PYY	0.000	0.883	0.002	[MN/m]				
1020	PYY	0.000	0.883	0.003	[MN/m]				
1021	PYY	0.000	0.883	0.002	[MN/m]				
1021	PYY	0.000	0.883	0.003	[MN/m]				
1022	PYY	0.000	0.883	0.002	[MN/m]				
1022	PYY	0.000	0.883	0.003	[MN/m]				
1023	PYY	0.000	0.883	0.002	[MN/m]				
1023	PYY	0.000	0.883	0.003	[MN/m]				
1024	PYY	0.000	0.883	0.002	[MN/m]				
1024	PYY	0.000	0.883	0.003	[MN/m]				
1025	PYY	0.000	0.883	0.002	[MN/m]				
1025	PYY	0.000	0.883	0.003	[MN/m]				
1026	PYY	0.000	0.883	0.002	[MN/m]				
1026	PYY	0.000	0.883	0.003	[MN/m]				
1027	PYY	0.000	0.883	0.002	[MN/m]				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΣΕΙΣΜΙΚΕΣ ΜΕΜΟΝΩΜΕΝΕΣ ΦΟΡΤΙΣΕΙΣ

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
1027	PYY	0.000	0.883	0.003	[MN/m]				
1028	PYY	0.000	0.883	0.002	[MN/m]				
1028	PYY	0.000	0.883	0.003	[MN/m]				
1029	PYY	0.000	0.883	0.002	[MN/m]				
1029	PYY	0.000	0.883	0.003	[MN/m]				
1030	PYY	0.000	0.883	0.002	[MN/m]				
1030	PYY	0.000	0.883	0.003	[MN/m]				
1031	PYY	0.000	0.883	0.002	[MN/m]				
1031	PYY	0.000	0.883	0.003	[MN/m]				
1032	PYY	0.000	0.883	0.002	[MN/m]				
1032	PYY	0.000	0.883	0.003	[MN/m]				
1033	PYY	0.000	0.883	0.002	[MN/m]				
1033	PYY	0.000	0.883	0.003	[MN/m]				
1034	PYY	0.000	0.883	0.002	[MN/m]				
1034	PYY	0.000	0.883	0.003	[MN/m]				
1035	PYY	0.000	0.883	0.002	[MN/m]				
1035	PYY	0.000	0.883	0.003	[MN/m]				
1036	PYY	0.000	0.883	0.002	[MN/m]				
1036	PYY	0.000	0.883	0.003	[MN/m]				
1037	PYY	0.000	0.883	0.002	[MN/m]				
1037	PYY	0.000	0.883	0.003	[MN/m]				
1038	PYY	0.000	0.883	0.002	[MN/m]				
1038	PYY	0.000	0.883	0.003	[MN/m]				
1039	PYY	0.000	0.883	0.002	[MN/m]				
1039	PYY	0.000	0.883	0.003	[MN/m]				
1040	PYY	0.000	0.883	0.002	[MN/m]				
1040	PYY	0.000	0.883	0.003	[MN/m]				
1041	PYY	0.000	0.883	0.002	[MN/m]				
1041	PYY	0.000	0.883	0.003	[MN/m]				
1042	PYY	0.000	0.883	0.002	[MN/m]				
1042	PYY	0.000	0.883	0.003	[MN/m]				
1043	PYY	0.000	0.883	0.002	[MN/m]				
1043	PYY	0.000	0.883	0.003	[MN/m]				
1044	PYY	0.000	0.883	0.002	[MN/m]				
1044	PYY	0.000	0.883	0.003	[MN/m]				
1045	PYY	0.000	0.883	0.002	[MN/m]				
1045	PYY	0.000	0.883	0.003	[MN/m]				
1046	PYY	0.000	0.883	0.002	[MN/m]				
1046	PYY	0.000	0.883	0.003	[MN/m]				
1047	PYY	0.000	0.883	0.002	[MN/m]				
1047	PYY	0.000	0.883	0.003	[MN/m]				
1048	PYY	0.000	0.883	0.002	[MN/m]				
1048	PYY	0.000	0.883	0.003	[MN/m]				
1049	PYY	0.000	0.883	0.002	[MN/m]				
1049	PYY	0.000	0.883	0.003	[MN/m]				
1050	PYY	0.000	0.883	0.002	[MN/m]				
1050	PYY	0.000	0.883	0.003	[MN/m]				
1051	PYY	0.000	0.883	0.002	[MN/m]				
1051	PYY	0.000	0.883	0.003	[MN/m]				
1052	PYY	0.000	0.883	0.002	[MN/m]				
1052	PYY	0.000	0.883	0.003	[MN/m]				
1053	PYY	0.000	0.883	0.002	[MN/m]				
1053	PYY	0.000	0.883	0.003	[MN/m]				
1054	PYY	0.000	0.883	0.002	[MN/m]				
1054	PYY	0.000	0.883	0.003	[MN/m]				
1055	PYY	0.000	0.883	0.002	[MN/m]				
1055	PYY	0.000	0.883	0.003	[MN/m]				
1056	PYY	0.000	0.883	0.002	[MN/m]				
1056	PYY	0.000	0.883	0.003	[MN/m]				
1057	PYY	0.000	0.883	0.002	[MN/m]				
1057	PYY	0.000	0.883	0.003	[MN/m]				
1058	PYY	0.000	0.883	0.002	[MN/m]				
1058	PYY	0.000	0.883	0.003	[MN/m]				
1059	PYY	0.000	0.883	0.002	[MN/m]				
1059	PYY	0.000	0.883	0.003	[MN/m]				
1060	PYY	0.000	0.883	0.002	[MN/m]				
1060	PYY	0.000	0.883	0.003	[MN/m]				

Loads acting on QUAD-elements

Elements from	to	inc	Load Prim Type	LC/CC	Load val.	Dimension	Variation dP/dX	dP/dY	dP/dZ
3000	3999	1	PYY		0.001	[MN/m2]			
4000	4999	1	PYY		0.001	[MN/m2]			

Load Case 715 -EY

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΣΕΙΣΜΙΚΕΣ ΜΕΜΟΝΩΜΕΝΕΣ ΦΟΡΤΙΣΕΙΣ

Load Case 715 -EY

```
Factor forces and moments          1.000
Factor dead weight      DL-XX      0.000
Factor dead weight      DL-YY     -0.360
Factor dead weight      DL-ZZ      0.000
Loads partially  copied from load case   202 with factor   -1.000
```

Meshfree Loading

Kind	Referenceto		Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
Area				0.000	2.000	0.000	PYY	-0.004	[MN/m2]
				11.800	2.000	0.000		-0.004	[MN/m2]
				11.800	-1.000	0.000		-0.002	[MN/m2]
				0.000	-1.000	0.000		-0.002	[MN/m2]
	QGRP	3	3.000	(--)	activated			89.83	percent
Area				0.000	2.000	0.000	PYY	-0.004	[MN/m2]
				11.800	2.000	0.000		-0.004	[MN/m2]
				11.800	-1.000	0.000		-0.002	[MN/m2]
				0.000	-1.000	0.000		-0.002	[MN/m2]
	QGRP	4	3.000	(--)	activated			10.17	percent
Area				11.800	-2.000	0.000	PYY	-0.006	[MN/m2]
				0.000	-2.000	0.000		-0.006	[MN/m2]
				0.000	-1.000	0.000		-0.006	[MN/m2]
				11.800	-1.000	0.000		-0.006	[MN/m2]
	QGRP	3	3.000	(--)	activated			89.83	percent
Area				11.800	-2.000	0.000	PYY	-0.006	[MN/m2]
				0.000	-2.000	0.000		-0.006	[MN/m2]
				0.000	-1.000	0.000		-0.006	[MN/m2]
				11.800	-1.000	0.000		-0.006	[MN/m2]
	QGRP	4	3.000	(--)	activated			10.17	percent
Area				0.000	2.000	0.000	PYY	0.000	[MN/m2]
				11.800	2.000	0.000		0.000	[MN/m2]
				11.800	-1.000	0.000		0.000	[MN/m2]
				0.000	-1.000	0.000		0.000	[MN/m2]
	QGRP	3	3.000	(--)	activated			89.83	percent
Area				0.000	2.000	0.000	PYY	0.000	[MN/m2]
				11.800	2.000	0.000		0.000	[MN/m2]
				11.800	-1.000	0.000		0.000	[MN/m2]
				0.000	-1.000	0.000		0.000	[MN/m2]
	QGRP	4	3.000	(--)	activated			10.17	percent

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
1001	PYY	0.000	0.883	-0.002	[MN/m]				
1001	PYY	0.000	0.883	-0.003	[MN/m]				
1002	PYY	0.000	0.883	-0.002	[MN/m]				
1002	PYY	0.000	0.883	-0.003	[MN/m]				
1003	PYY	0.000	0.883	-0.002	[MN/m]				
1003	PYY	0.000	0.883	-0.003	[MN/m]				
1004	PYY	0.000	0.883	-0.002	[MN/m]				
1004	PYY	0.000	0.883	-0.003	[MN/m]				
1005	PYY	0.000	0.883	-0.002	[MN/m]				
1005	PYY	0.000	0.883	-0.003	[MN/m]				
1006	PYY	0.000	0.883	-0.002	[MN/m]				
1006	PYY	0.000	0.883	-0.003	[MN/m]				
1007	PYY	0.000	0.883	-0.002	[MN/m]				
1007	PYY	0.000	0.883	-0.003	[MN/m]				
1008	PYY	0.000	0.883	-0.002	[MN/m]				
1008	PYY	0.000	0.883	-0.003	[MN/m]				
1009	PYY	0.000	0.883	-0.002	[MN/m]				
1009	PYY	0.000	0.883	-0.003	[MN/m]				
1010	PYY	0.000	0.883	-0.002	[MN/m]				
1010	PYY	0.000	0.883	-0.003	[MN/m]				
1011	PYY	0.000	0.883	-0.002	[MN/m]				
1011	PYY	0.000	0.883	-0.003	[MN/m]				
1012	PYY	0.000	0.883	-0.002	[MN/m]				
1012	PYY	0.000	0.883	-0.003	[MN/m]				
1013	PYY	0.000	0.883	-0.002	[MN/m]				
1013	PYY	0.000	0.883	-0.003	[MN/m]				
1014	PYY	0.000	0.883	-0.002	[MN/m]				
1014	PYY	0.000	0.883	-0.003	[MN/m]				
1015	PYY	0.000	0.883	-0.002	[MN/m]				
1015	PYY	0.000	0.883	-0.003	[MN/m]				
1016	PYY	0.000	0.883	-0.002	[MN/m]				
1016	PYY	0.000	0.883	-0.003	[MN/m]				
1017	PYY	0.000	0.883	-0.002	[MN/m]				
1017	PYY	0.000	0.883	-0.003	[MN/m]				
1018	PYY	0.000	0.883	-0.002	[MN/m]				
1018	PYY	0.000	0.883	-0.003	[MN/m]				
1019	PYY	0.000	0.883	-0.002	[MN/m]				
1019	PYY	0.000	0.883	-0.003	[MN/m]				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΣΕΙΣΜΙΚΕΣ ΜΕΜΟΝΩΜΕΝΕΣ ΦΟΡΤΙΣΕΙΣ

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
1020	PYY	0.000	0.883	-0.002	[MN/m]				
1020	PYY	0.000	0.883	-0.003	[MN/m]				
1021	PYY	0.000	0.883	-0.002	[MN/m]				
1021	PYY	0.000	0.883	-0.003	[MN/m]				
1022	PYY	0.000	0.883	-0.002	[MN/m]				
1022	PYY	0.000	0.883	-0.003	[MN/m]				
1023	PYY	0.000	0.883	-0.002	[MN/m]				
1023	PYY	0.000	0.883	-0.003	[MN/m]				
1024	PYY	0.000	0.883	-0.002	[MN/m]				
1024	PYY	0.000	0.883	-0.003	[MN/m]				
1025	PYY	0.000	0.883	-0.002	[MN/m]				
1025	PYY	0.000	0.883	-0.003	[MN/m]				
1026	PYY	0.000	0.883	-0.002	[MN/m]				
1026	PYY	0.000	0.883	-0.003	[MN/m]				
1027	PYY	0.000	0.883	-0.002	[MN/m]				
1027	PYY	0.000	0.883	-0.003	[MN/m]				
1028	PYY	0.000	0.883	-0.002	[MN/m]				
1028	PYY	0.000	0.883	-0.003	[MN/m]				
1029	PYY	0.000	0.883	-0.002	[MN/m]				
1029	PYY	0.000	0.883	-0.003	[MN/m]				
1030	PYY	0.000	0.883	-0.002	[MN/m]				
1030	PYY	0.000	0.883	-0.003	[MN/m]				
1031	PYY	0.000	0.883	-0.002	[MN/m]				
1031	PYY	0.000	0.883	-0.003	[MN/m]				
1032	PYY	0.000	0.883	-0.002	[MN/m]				
1032	PYY	0.000	0.883	-0.003	[MN/m]				
1033	PYY	0.000	0.883	-0.002	[MN/m]				
1033	PYY	0.000	0.883	-0.003	[MN/m]				
1034	PYY	0.000	0.883	-0.002	[MN/m]				
1034	PYY	0.000	0.883	-0.003	[MN/m]				
1035	PYY	0.000	0.883	-0.002	[MN/m]				
1035	PYY	0.000	0.883	-0.003	[MN/m]				
1036	PYY	0.000	0.883	-0.002	[MN/m]				
1036	PYY	0.000	0.883	-0.003	[MN/m]				
1037	PYY	0.000	0.883	-0.002	[MN/m]				
1037	PYY	0.000	0.883	-0.003	[MN/m]				
1038	PYY	0.000	0.883	-0.002	[MN/m]				
1038	PYY	0.000	0.883	-0.003	[MN/m]				
1039	PYY	0.000	0.883	-0.002	[MN/m]				
1039	PYY	0.000	0.883	-0.003	[MN/m]				
1040	PYY	0.000	0.883	-0.002	[MN/m]				
1040	PYY	0.000	0.883	-0.003	[MN/m]				
1041	PYY	0.000	0.883	-0.002	[MN/m]				
1041	PYY	0.000	0.883	-0.003	[MN/m]				
1042	PYY	0.000	0.883	-0.002	[MN/m]				
1042	PYY	0.000	0.883	-0.003	[MN/m]				
1043	PYY	0.000	0.883	-0.002	[MN/m]				
1043	PYY	0.000	0.883	-0.003	[MN/m]				
1044	PYY	0.000	0.883	-0.002	[MN/m]				
1044	PYY	0.000	0.883	-0.003	[MN/m]				
1045	PYY	0.000	0.883	-0.002	[MN/m]				
1045	PYY	0.000	0.883	-0.003	[MN/m]				
1046	PYY	0.000	0.883	-0.002	[MN/m]				
1046	PYY	0.000	0.883	-0.003	[MN/m]				
1047	PYY	0.000	0.883	-0.002	[MN/m]				
1047	PYY	0.000	0.883	-0.003	[MN/m]				
1048	PYY	0.000	0.883	-0.002	[MN/m]				
1048	PYY	0.000	0.883	-0.003	[MN/m]				
1049	PYY	0.000	0.883	-0.002	[MN/m]				
1049	PYY	0.000	0.883	-0.003	[MN/m]				
1050	PYY	0.000	0.883	-0.002	[MN/m]				
1050	PYY	0.000	0.883	-0.003	[MN/m]				
1051	PYY	0.000	0.883	-0.002	[MN/m]				
1051	PYY	0.000	0.883	-0.003	[MN/m]				
1052	PYY	0.000	0.883	-0.002	[MN/m]				
1052	PYY	0.000	0.883	-0.003	[MN/m]				
1053	PYY	0.000	0.883	-0.002	[MN/m]				
1053	PYY	0.000	0.883	-0.003	[MN/m]				
1054	PYY	0.000	0.883	-0.002	[MN/m]				
1054	PYY	0.000	0.883	-0.003	[MN/m]				
1055	PYY	0.000	0.883	-0.002	[MN/m]				
1055	PYY	0.000	0.883	-0.003	[MN/m]				
1056	PYY	0.000	0.883	-0.002	[MN/m]				
1056	PYY	0.000	0.883	-0.003	[MN/m]				
1057	PYY	0.000	0.883	-0.002	[MN/m]				
1057	PYY	0.000	0.883	-0.003	[MN/m]				
1058	PYY	0.000	0.883	-0.002	[MN/m]				
1058	PYY	0.000	0.883	-0.003	[MN/m]				
1059	PYY	0.000	0.883	-0.002	[MN/m]				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΣΕΙΣΜΙΚΕΣ ΜΕΜΟΝΩΜΕΝΕΣ ΦΟΡΤΙΣΕΙΣ

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
1059	PYY	0.000	0.883	-0.003	[MN/m]				
1060	PYY	0.000	0.883	-0.002	[MN/m]				
1060	PYY	0.000	0.883	-0.003	[MN/m]				

Loads acting on QUAD-elements

Elements from	to	inc	Load Prim Type LC/CC	Load val.	Dimension	Variation dP/dX	dP/dY	dP/dZ
3000	3999	1	PYY	-0.001	[MN/m2]			
4000	4999	1	PYY	-0.001	[MN/m2]			

Load Case 716 EZ

Factor forces and moments 1.000
Factor dead weight DL-XX 0.000
Factor dead weight DL-YY 0.000
Factor dead weight DL-ZZ 0.432
Loads partially copied from load case 203 with factor 1.000

Meshfree Loading

Kind	Referenceto	Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue
Area			0.000	2.000	0.000	PG	0.004 [MN/m2]
			11.800	2.000	0.000		0.004 [MN/m2]
			11.800	-1.000	0.000		0.002 [MN/m2]
			0.000	-1.000	0.000		0.002 [MN/m2]
	QGRP	3	3.000	(--)	activated		89.83 percent
Area			0.000	2.000	0.000	PG	0.004 [MN/m2]
			11.800	2.000	0.000		0.004 [MN/m2]
			11.800	-1.000	0.000		0.002 [MN/m2]
			0.000	-1.000	0.000		0.002 [MN/m2]
	QGRP	4	3.000	(--)	activated		10.17 percent
Area			11.800	-2.000	0.000	PG	0.007 [MN/m2]
			0.000	-2.000	0.000		0.007 [MN/m2]
			0.000	-1.000	0.000		0.007 [MN/m2]
			11.800	-1.000	0.000		0.007 [MN/m2]
	QGRP	3	3.000	(--)	activated		89.83 percent
Area			11.800	-2.000	0.000	PG	0.007 [MN/m2]
			0.000	-2.000	0.000		0.007 [MN/m2]
			0.000	-1.000	0.000		0.007 [MN/m2]
			11.800	-1.000	0.000		0.007 [MN/m2]
	QGRP	4	3.000	(--)	activated		10.17 percent
Area			0.000	2.000	0.000	PG	0.000 [MN/m2]
			11.800	2.000	0.000		0.000 [MN/m2]
			11.800	-1.000	0.000		0.000 [MN/m2]
			0.000	-1.000	0.000		0.000 [MN/m2]
	QGRP	3	3.000	(--)	activated		89.83 percent
Area			0.000	2.000	0.000	PG	0.000 [MN/m2]
			11.800	2.000	0.000		0.000 [MN/m2]
			11.800	-1.000	0.000		0.000 [MN/m2]
			0.000	-1.000	0.000		0.000 [MN/m2]
	QGRP	4	3.000	(--)	activated		10.17 percent

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
1001	PG	0.000	0.883	0.002	[MN/m]				
1001	PG	0.000	0.883	0.003	[MN/m]				
1002	PG	0.000	0.883	0.002	[MN/m]				
1002	PG	0.000	0.883	0.003	[MN/m]				
1003	PG	0.000	0.883	0.002	[MN/m]				
1003	PG	0.000	0.883	0.003	[MN/m]				
1004	PG	0.000	0.883	0.002	[MN/m]				
1004	PG	0.000	0.883	0.003	[MN/m]				
1005	PG	0.000	0.883	0.002	[MN/m]				
1005	PG	0.000	0.883	0.003	[MN/m]				
1006	PG	0.000	0.883	0.002	[MN/m]				
1006	PG	0.000	0.883	0.003	[MN/m]				
1007	PG	0.000	0.883	0.002	[MN/m]				
1007	PG	0.000	0.883	0.003	[MN/m]				
1008	PG	0.000	0.883	0.002	[MN/m]				
1008	PG	0.000	0.883	0.003	[MN/m]				
1009	PG	0.000	0.883	0.002	[MN/m]				
1009	PG	0.000	0.883	0.003	[MN/m]				
1010	PG	0.000	0.883	0.002	[MN/m]				
1010	PG	0.000	0.883	0.003	[MN/m]				
1011	PG	0.000	0.883	0.002	[MN/m]				
1011	PG	0.000	0.883	0.003	[MN/m]				
1012	PG	0.000	0.883	0.002	[MN/m]				
1012	PG	0.000	0.883	0.003	[MN/m]				
1013	PG	0.000	0.883	0.002	[MN/m]				
1013	PG	0.000	0.883	0.003	[MN/m]				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΣΕΙΣΜΙΚΕΣ ΜΕΜΟΝΩΜΕΝΕΣ ΦΟΡΤΙΣΕΙΣ

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
1014	PG	0.000	0.883	0.002	[MN/m]				
1014	PG	0.000	0.883	0.003	[MN/m]				
1015	PG	0.000	0.883	0.002	[MN/m]				
1015	PG	0.000	0.883	0.003	[MN/m]				
1016	PG	0.000	0.883	0.002	[MN/m]				
1016	PG	0.000	0.883	0.003	[MN/m]				
1017	PG	0.000	0.883	0.002	[MN/m]				
1017	PG	0.000	0.883	0.003	[MN/m]				
1018	PG	0.000	0.883	0.002	[MN/m]				
1018	PG	0.000	0.883	0.003	[MN/m]				
1019	PG	0.000	0.883	0.002	[MN/m]				
1019	PG	0.000	0.883	0.003	[MN/m]				
1020	PG	0.000	0.883	0.002	[MN/m]				
1020	PG	0.000	0.883	0.003	[MN/m]				
1021	PG	0.000	0.883	0.002	[MN/m]				
1021	PG	0.000	0.883	0.003	[MN/m]				
1022	PG	0.000	0.883	0.002	[MN/m]				
1022	PG	0.000	0.883	0.003	[MN/m]				
1023	PG	0.000	0.883	0.002	[MN/m]				
1023	PG	0.000	0.883	0.003	[MN/m]				
1024	PG	0.000	0.883	0.002	[MN/m]				
1024	PG	0.000	0.883	0.003	[MN/m]				
1025	PG	0.000	0.883	0.002	[MN/m]				
1025	PG	0.000	0.883	0.003	[MN/m]				
1026	PG	0.000	0.883	0.002	[MN/m]				
1026	PG	0.000	0.883	0.003	[MN/m]				
1027	PG	0.000	0.883	0.002	[MN/m]				
1027	PG	0.000	0.883	0.003	[MN/m]				
1028	PG	0.000	0.883	0.002	[MN/m]				
1028	PG	0.000	0.883	0.003	[MN/m]				
1029	PG	0.000	0.883	0.002	[MN/m]				
1029	PG	0.000	0.883	0.003	[MN/m]				
1030	PG	0.000	0.883	0.002	[MN/m]				
1030	PG	0.000	0.883	0.003	[MN/m]				
1031	PG	0.000	0.883	0.002	[MN/m]				
1031	PG	0.000	0.883	0.003	[MN/m]				
1032	PG	0.000	0.883	0.002	[MN/m]				
1032	PG	0.000	0.883	0.003	[MN/m]				
1033	PG	0.000	0.883	0.002	[MN/m]				
1033	PG	0.000	0.883	0.003	[MN/m]				
1034	PG	0.000	0.883	0.002	[MN/m]				
1034	PG	0.000	0.883	0.003	[MN/m]				
1035	PG	0.000	0.883	0.002	[MN/m]				
1035	PG	0.000	0.883	0.003	[MN/m]				
1036	PG	0.000	0.883	0.002	[MN/m]				
1036	PG	0.000	0.883	0.003	[MN/m]				
1037	PG	0.000	0.883	0.002	[MN/m]				
1037	PG	0.000	0.883	0.003	[MN/m]				
1038	PG	0.000	0.883	0.002	[MN/m]				
1038	PG	0.000	0.883	0.003	[MN/m]				
1039	PG	0.000	0.883	0.002	[MN/m]				
1039	PG	0.000	0.883	0.003	[MN/m]				
1040	PG	0.000	0.883	0.002	[MN/m]				
1040	PG	0.000	0.883	0.003	[MN/m]				
1041	PG	0.000	0.883	0.002	[MN/m]				
1041	PG	0.000	0.883	0.003	[MN/m]				
1042	PG	0.000	0.883	0.002	[MN/m]				
1042	PG	0.000	0.883	0.003	[MN/m]				
1043	PG	0.000	0.883	0.002	[MN/m]				
1043	PG	0.000	0.883	0.003	[MN/m]				
1044	PG	0.000	0.883	0.002	[MN/m]				
1044	PG	0.000	0.883	0.003	[MN/m]				
1045	PG	0.000	0.883	0.002	[MN/m]				
1045	PG	0.000	0.883	0.003	[MN/m]				
1046	PG	0.000	0.883	0.002	[MN/m]				
1046	PG	0.000	0.883	0.003	[MN/m]				
1047	PG	0.000	0.883	0.002	[MN/m]				
1047	PG	0.000	0.883	0.003	[MN/m]				
1048	PG	0.000	0.883	0.002	[MN/m]				
1048	PG	0.000	0.883	0.003	[MN/m]				
1049	PG	0.000	0.883	0.002	[MN/m]				
1049	PG	0.000	0.883	0.003	[MN/m]				
1050	PG	0.000	0.883	0.002	[MN/m]				
1050	PG	0.000	0.883	0.003	[MN/m]				
1051	PG	0.000	0.883	0.002	[MN/m]				
1051	PG	0.000	0.883	0.003	[MN/m]				
1052	PG	0.000	0.883	0.002	[MN/m]				
1052	PG	0.000	0.883	0.003	[MN/m]				
1053	PG	0.000	0.883	0.002	[MN/m]				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΣΕΙΣΜΙΚΕΣ ΜΕΜΟΝΩΜΕΝΕΣ ΦΟΡΤΙΣΕΙΣ

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
1053	PG	0.000	0.883	0.003	[MN/m]				
1054	PG	0.000	0.883	0.002	[MN/m]				
1054	PG	0.000	0.883	0.003	[MN/m]				
1055	PG	0.000	0.883	0.002	[MN/m]				
1055	PG	0.000	0.883	0.003	[MN/m]				
1056	PG	0.000	0.883	0.002	[MN/m]				
1056	PG	0.000	0.883	0.003	[MN/m]				
1057	PG	0.000	0.883	0.002	[MN/m]				
1057	PG	0.000	0.883	0.003	[MN/m]				
1058	PG	0.000	0.883	0.002	[MN/m]				
1058	PG	0.000	0.883	0.003	[MN/m]				
1059	PG	0.000	0.883	0.002	[MN/m]				
1059	PG	0.000	0.883	0.003	[MN/m]				
1060	PG	0.000	0.883	0.002	[MN/m]				
1060	PG	0.000	0.883	0.003	[MN/m]				

Loads acting on QUAD-elements

Elements from	to	inc	Load Prim Type LC/CC	Load val.	Dimension	Variation dP/dX	dP/dY	dP/dZ
3000	3999	1	PG	0.002	[MN/m2]			
4000	4999	1	PG	0.002	[MN/m2]			

Load Case 717 -EZ

Factor forces and moments 1.000
Factor dead weight DL-XX 0.000
Factor dead weight DL-YY 0.000
Factor dead weight DL-ZZ -0.432
Loads partially copied from load case 203 with factor -1.000

Meshfree Loading

Kind	Referenceto	Projection W[m]	Coordinates X[m]	Y[m]	Z[m]	Type	Loadvalue	
Area			0.000	2.000	0.000	PG	-0.004	[MN/m2]
			11.800	2.000	0.000		-0.004	[MN/m2]
			11.800	-1.000	0.000		-0.002	[MN/m2]
			0.000	-1.000	0.000		-0.002	[MN/m2]
	QGRP	3	3.000	(--)	activated		89.83	percent
Area			0.000	2.000	0.000	PG	-0.004	[MN/m2]
			11.800	2.000	0.000		-0.004	[MN/m2]
			11.800	-1.000	0.000		-0.002	[MN/m2]
			0.000	-1.000	0.000		-0.002	[MN/m2]
	QGRP	4	3.000	(--)	activated		10.17	percent
Area			11.800	-2.000	0.000	PG	-0.007	[MN/m2]
			0.000	-2.000	0.000		-0.007	[MN/m2]
			0.000	-1.000	0.000		-0.007	[MN/m2]
			11.800	-1.000	0.000		-0.007	[MN/m2]
	QGRP	3	3.000	(--)	activated		89.83	percent
Area			11.800	-2.000	0.000	PG	-0.007	[MN/m2]
			0.000	-2.000	0.000		-0.007	[MN/m2]
			0.000	-1.000	0.000		-0.007	[MN/m2]
			11.800	-1.000	0.000		-0.007	[MN/m2]
	QGRP	4	3.000	(--)	activated		10.17	percent
Area			0.000	2.000	0.000	PG	0.000	[MN/m2]
			11.800	2.000	0.000		0.000	[MN/m2]
			11.800	-1.000	0.000		0.000	[MN/m2]
			0.000	-1.000	0.000		0.000	[MN/m2]
	QGRP	3	3.000	(--)	activated		89.83	percent
Area			0.000	2.000	0.000	PG	0.000	[MN/m2]
			11.800	2.000	0.000		0.000	[MN/m2]
			11.800	-1.000	0.000		0.000	[MN/m2]
			0.000	-1.000	0.000		0.000	[MN/m2]
	QGRP	4	3.000	(--)	activated		10.17	percent

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
1001	PG	0.000	0.883	-0.002	[MN/m]				
1001	PG	0.000	0.883	-0.003	[MN/m]				
1002	PG	0.000	0.883	-0.002	[MN/m]				
1002	PG	0.000	0.883	-0.003	[MN/m]				
1003	PG	0.000	0.883	-0.002	[MN/m]				
1003	PG	0.000	0.883	-0.003	[MN/m]				
1004	PG	0.000	0.883	-0.002	[MN/m]				
1004	PG	0.000	0.883	-0.003	[MN/m]				
1005	PG	0.000	0.883	-0.002	[MN/m]				
1005	PG	0.000	0.883	-0.003	[MN/m]				
1006	PG	0.000	0.883	-0.002	[MN/m]				
1006	PG	0.000	0.883	-0.003	[MN/m]				
1007	PG	0.000	0.883	-0.002	[MN/m]				
1007	PG	0.000	0.883	-0.003	[MN/m]				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΣΕΙΣΜΙΚΕΣ ΜΕΜΟΝΩΜΕΝΕΣ ΦΟΡΤΙΣΕΙΣ

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
1008	PG	0.000	0.883	-0.002	[MN/m]				
1008	PG	0.000	0.883	-0.003	[MN/m]				
1009	PG	0.000	0.883	-0.002	[MN/m]				
1009	PG	0.000	0.883	-0.003	[MN/m]				
1010	PG	0.000	0.883	-0.002	[MN/m]				
1010	PG	0.000	0.883	-0.003	[MN/m]				
1011	PG	0.000	0.883	-0.002	[MN/m]				
1011	PG	0.000	0.883	-0.003	[MN/m]				
1012	PG	0.000	0.883	-0.002	[MN/m]				
1012	PG	0.000	0.883	-0.003	[MN/m]				
1013	PG	0.000	0.883	-0.002	[MN/m]				
1013	PG	0.000	0.883	-0.003	[MN/m]				
1014	PG	0.000	0.883	-0.002	[MN/m]				
1014	PG	0.000	0.883	-0.003	[MN/m]				
1015	PG	0.000	0.883	-0.002	[MN/m]				
1015	PG	0.000	0.883	-0.003	[MN/m]				
1016	PG	0.000	0.883	-0.002	[MN/m]				
1016	PG	0.000	0.883	-0.003	[MN/m]				
1017	PG	0.000	0.883	-0.002	[MN/m]				
1017	PG	0.000	0.883	-0.003	[MN/m]				
1018	PG	0.000	0.883	-0.002	[MN/m]				
1018	PG	0.000	0.883	-0.003	[MN/m]				
1019	PG	0.000	0.883	-0.002	[MN/m]				
1019	PG	0.000	0.883	-0.003	[MN/m]				
1020	PG	0.000	0.883	-0.002	[MN/m]				
1020	PG	0.000	0.883	-0.003	[MN/m]				
1021	PG	0.000	0.883	-0.002	[MN/m]				
1021	PG	0.000	0.883	-0.003	[MN/m]				
1022	PG	0.000	0.883	-0.002	[MN/m]				
1022	PG	0.000	0.883	-0.003	[MN/m]				
1023	PG	0.000	0.883	-0.002	[MN/m]				
1023	PG	0.000	0.883	-0.003	[MN/m]				
1024	PG	0.000	0.883	-0.002	[MN/m]				
1024	PG	0.000	0.883	-0.003	[MN/m]				
1025	PG	0.000	0.883	-0.002	[MN/m]				
1025	PG	0.000	0.883	-0.003	[MN/m]				
1026	PG	0.000	0.883	-0.002	[MN/m]				
1026	PG	0.000	0.883	-0.003	[MN/m]				
1027	PG	0.000	0.883	-0.002	[MN/m]				
1027	PG	0.000	0.883	-0.003	[MN/m]				
1028	PG	0.000	0.883	-0.002	[MN/m]				
1028	PG	0.000	0.883	-0.003	[MN/m]				
1029	PG	0.000	0.883	-0.002	[MN/m]				
1029	PG	0.000	0.883	-0.003	[MN/m]				
1030	PG	0.000	0.883	-0.002	[MN/m]				
1030	PG	0.000	0.883	-0.003	[MN/m]				
1031	PG	0.000	0.883	-0.002	[MN/m]				
1031	PG	0.000	0.883	-0.003	[MN/m]				
1032	PG	0.000	0.883	-0.002	[MN/m]				
1032	PG	0.000	0.883	-0.003	[MN/m]				
1033	PG	0.000	0.883	-0.002	[MN/m]				
1033	PG	0.000	0.883	-0.003	[MN/m]				
1034	PG	0.000	0.883	-0.002	[MN/m]				
1034	PG	0.000	0.883	-0.003	[MN/m]				
1035	PG	0.000	0.883	-0.002	[MN/m]				
1035	PG	0.000	0.883	-0.003	[MN/m]				
1036	PG	0.000	0.883	-0.002	[MN/m]				
1036	PG	0.000	0.883	-0.003	[MN/m]				
1037	PG	0.000	0.883	-0.002	[MN/m]				
1037	PG	0.000	0.883	-0.003	[MN/m]				
1038	PG	0.000	0.883	-0.002	[MN/m]				
1038	PG	0.000	0.883	-0.003	[MN/m]				
1039	PG	0.000	0.883	-0.002	[MN/m]				
1039	PG	0.000	0.883	-0.003	[MN/m]				
1040	PG	0.000	0.883	-0.002	[MN/m]				
1040	PG	0.000	0.883	-0.003	[MN/m]				
1041	PG	0.000	0.883	-0.002	[MN/m]				
1041	PG	0.000	0.883	-0.003	[MN/m]				
1042	PG	0.000	0.883	-0.002	[MN/m]				
1042	PG	0.000	0.883	-0.003	[MN/m]				
1043	PG	0.000	0.883	-0.002	[MN/m]				
1043	PG	0.000	0.883	-0.003	[MN/m]				
1044	PG	0.000	0.883	-0.002	[MN/m]				
1044	PG	0.000	0.883	-0.003	[MN/m]				
1045	PG	0.000	0.883	-0.002	[MN/m]				
1045	PG	0.000	0.883	-0.003	[MN/m]				
1046	PG	0.000	0.883	-0.002	[MN/m]				
1046	PG	0.000	0.883	-0.003	[MN/m]				
1047	PG	0.000	0.883	-0.002	[MN/m]				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΣΕΙΣΜΙΚΕΣ ΜΕΜΟΝΩΜΕΝΕΣ ΦΟΡΤΙΣΕΙΣ

Loads acting on Beam-elements

Number	Type	a[m]	l[m]	Loadval	Loadval Dimens.	ya[m]	za[m]	ye[m]	ze[m]
1047	PG	0.000	0.883	-0.003	[MN/m]				
1048	PG	0.000	0.883	-0.002	[MN/m]				
1048	PG	0.000	0.883	-0.003	[MN/m]				
1049	PG	0.000	0.883	-0.002	[MN/m]				
1049	PG	0.000	0.883	-0.003	[MN/m]				
1050	PG	0.000	0.883	-0.002	[MN/m]				
1050	PG	0.000	0.883	-0.003	[MN/m]				
1051	PG	0.000	0.883	-0.002	[MN/m]				
1051	PG	0.000	0.883	-0.003	[MN/m]				
1052	PG	0.000	0.883	-0.002	[MN/m]				
1052	PG	0.000	0.883	-0.003	[MN/m]				
1053	PG	0.000	0.883	-0.002	[MN/m]				
1053	PG	0.000	0.883	-0.003	[MN/m]				
1054	PG	0.000	0.883	-0.002	[MN/m]				
1054	PG	0.000	0.883	-0.003	[MN/m]				
1055	PG	0.000	0.883	-0.002	[MN/m]				
1055	PG	0.000	0.883	-0.003	[MN/m]				
1056	PG	0.000	0.883	-0.002	[MN/m]				
1056	PG	0.000	0.883	-0.003	[MN/m]				
1057	PG	0.000	0.883	-0.002	[MN/m]				
1057	PG	0.000	0.883	-0.003	[MN/m]				
1058	PG	0.000	0.883	-0.002	[MN/m]				
1058	PG	0.000	0.883	-0.003	[MN/m]				
1059	PG	0.000	0.883	-0.002	[MN/m]				
1059	PG	0.000	0.883	-0.003	[MN/m]				
1060	PG	0.000	0.883	-0.002	[MN/m]				
1060	PG	0.000	0.883	-0.003	[MN/m]				

Loads acting on QUAD-elements

Elements from	to	inc	Load Prim Type LC/CC	Load val.	Dimension	Variation dP/dX	dP/dY	dP/dZ
3000	3999	1	PG	-0.002	[MN/m2]			
4000	4999	1	PG	-0.002	[MN/m2]			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CALCULATION OF SEISMIC COMBINATIONS (MAIN +Ex)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
8101	Unid.-Seismic Combinatio	2.220	0.402	4.205

Iteration sequence
Iteration 1 Residual 0.003 energy 401.7166 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 401.8509 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CALCULATION OF SEISMIC COMBINATIONS (MAIN +Ex)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
8103	Unid.-Seismic Combinatio	2.220	0.402	3.240

Iteration sequence
Iteration 1 Residual 0.002 energy 391.9405 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 392.0504 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CALCULATION OF SEISMIC COMBINATIONS (MAIN +Ex)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
8105	Unid.-Seismic Combinatio	2.220	-0.402	4.205

Iteration sequence
Iteration 1 Residual 0.003 energy 401.9994 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 402.1343 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CALCULATION OF SEISMIC COMBINATIONS (MAIN +Ex)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
8107	Unid.-Seismic Combinatio	2.220	-0.402	3.240

Iteration sequence
Iteration 1 Residual 0.002 energy 392.1588 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 392.2691 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CALCULATION OF SEISMIC COMBINATIONS (MAIN -Ex)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
8102	Unid.-Seismic Combinatio	-2.303	0.402	4.205

Iteration sequence
Iteration 1 Residual 0.003 energy 405.8246 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 405.9723 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CALCULATION OF SEISMIC COMBINATIONS (MAIN -Ex)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
8104	Unid.-Seismic Combinatio	-2.303	0.402	3.240

Iteration sequence
Iteration 1 Residual 0.003 energy 396.0448 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 396.1657 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CALCULATION OF SEISMIC COMBINATIONS (MAIN -Ex)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
8106	Unid.-Seismic Combinatio	-2.303	-0.402	4.205

Iteration sequence
Iteration 1 Residual 0.003 energy 406.1074 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 406.2556 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CALCULATION OF SEISMIC COMBINATIONS (MAIN -Ex)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
8108	Unid.-Seismic Combinatio	-2.303	-0.402	3.240

Iteration sequence
Iteration 1 Residual 0.003 energy 396.2631 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 396.3844 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CALCULATION OF SEISMIC COMBINATIONS (MAIN +Ey)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
8201	Unid.-Seismic Combinatio	0.637	1.340	4.205

Iteration sequence
Iteration 1 Residual 0.002 energy 369.8273 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 369.8826 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CALCULATION OF SEISMIC COMBINATIONS (MAIN +Ey)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
8203	Unid.-Seismic Combinatio	0.637	1.340	3.240

Iteration sequence
Iteration 1 Residual 0.002 energy 360.1228 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 360.1679 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CALCULATION OF SEISMIC COMBINATIONS (MAIN +Ey)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
8205	Unid.-Seismic Combinatio	0.637	-1.340	4.205

Iteration sequence
Iteration 1 Residual 0.002 energy 370.7699 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 370.8270 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CALCULATION OF SEISMIC COMBINATIONS (MAIN +Ey)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
8207	Unid.-Seismic Combinatio	0.637	-1.340	3.240

Iteration sequence
Iteration 1 Residual 0.002 energy 360.8504 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 360.8965 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CALCULATION OF SEISMIC COMBINATIONS (MAIN -Ey)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
8202	Unid.-Seismic Combinatio	-0.720	1.340	4.205

Iteration sequence
Iteration 1 Residual 0.002 energy 370.9828 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 371.0421 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CALCULATION OF SEISMIC COMBINATIONS (MAIN -Ey)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
8204	Unid.-Seismic Combinatio	-0.720	1.340	3.240

Iteration sequence
Iteration 1 Residual 0.002 energy 361.2783 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 361.3267 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CALCULATION OF SEISMIC COMBINATIONS (MAIN -Ey)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
8206	Unid.-Seismic Combinatio	-0.720	-1.340	4.205

Iteration sequence
Iteration 1 Residual 0.002 energy 371.9254 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 371.9865 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CALCULATION OF SEISMIC COMBINATIONS (MAIN -Ey)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
8208	Unid.-Seismic Combinatio	-0.720	-1.340	3.240

Iteration sequence
Iteration 1 Residual 0.002 energy 362.0059 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 362.0554 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CALCULATION OF SEISMIC COMBINATIONS (MAIN +Ez)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
8301	Unid.-Seismic Combinatio	0.637	0.402	5.330

Iteration sequence
Iteration 1 Residual 0.001 energy 363.2062 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 363.2245 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CALCULATION OF SEISMIC COMBINATIONS (MAIN +Ez)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
8303	Unid.-Seismic Combinatio	0.637	0.402	2.114

Iteration sequence
Iteration 1 Residual 0.000 energy 330.6069 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 330.6157 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CALCULATION OF SEISMIC COMBINATIONS (MAIN +Ez)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
8305	Unid.-Seismic Combinatio	0.637	-0.402	5.330

Iteration sequence
Iteration 1 Residual 0.001 energy 363.5642 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 363.5834 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CALCULATION OF SEISMIC COMBINATIONS (MAIN +Ez)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
8307	Unid.-Seismic Combinatio	0.637	-0.402	2.114

Iteration sequence
Iteration 1 Residual 0.000 energy 330.7499 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 330.7589 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CALCULATION OF SEISMIC COMBINATIONS (MAIN -Ez)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
8302	Unid.-Seismic Combinatio	-0.720	0.402	5.330

Iteration sequence
Iteration 1 Residual 0.001 energy 364.3616 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 364.3849 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CALCULATION OF SEISMIC COMBINATIONS (MAIN -Ez)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
8304	Unid.-Seismic Combinatio	-0.720	0.402	2.114

Iteration sequence
Iteration 1 Residual 0.001 energy 331.7624 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 331.7737 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CALCULATION OF SEISMIC COMBINATIONS (MAIN -Ez)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
8306	Unid.-Seismic Combinatio	-0.720	-0.402	5.330

Iteration sequence
Iteration 1 Residual 0.001 energy 364.7197 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 364.7438 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CALCULATION OF SEISMIC COMBINATIONS (MAIN -Ez)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
8308	Unid.-Seismic Combinatio	-0.720	-0.402	2.114

Iteration sequence
Iteration 1 Residual 0.001 energy 331.9054 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 331.9168 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΔΙΑΣΤΑΣΙΟΛΟΓΗΣΗ ΦΟΡΕΑ ΣΕΙΣΜΙΚΕΣ ΦΟΡΤΙΣΕΙΣ

Design according to DIN1045-1 2008
Loadcases have been calculated in the Ultimate Limit State
In BEMESS no additional load safety factor is applied.

Load Cases for the Design

```
Loadcase 8101   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 8102   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 8103   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 8104   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 8105   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 8106   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 8107   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 8108   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 8201   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 8202   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 8203   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 8204   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 8205   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 8206   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 8207   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 8301   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 8302   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 8303   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 8304   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 8305   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 8306   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 8307   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 8308   Unid.-Seismic Combinatio + Nodal reaction punching design
```

Material (DIN1045-1 2008)

Mat	f-ck [MPa]	f-cr [MPa]	f-yk [MPa]	f-tk [MPa]	f-ctm [MPa]	N [-]	minQ [-]	type
1	25.0	21.2			2.565	7.5	0.20	mainly static
3	25.0	21.2			2.565	7.5	0.20	mainly static
4	25.0	21.2			2.565	7.5	0.20	mainly static
5	25.0	21.2			2.565	7.5	0.20	mainly static
6	25.0	21.2			2.565	7.5	0.20	mainly static
7	25.0	21.2			2.565	7.5	0.20	mainly static
8	25.0	21.2			2.565	7.5	0.20	mainly static
9	25.0	21.2			2.565	7.5	0.20	mainly static
10	25.0	21.2			2.565	7.5	0.20	mainly static
12			500.0	525.0				

Minimum reinforcement: 0.00 p.c. of stat. req. section

Reduction of FC in case of transvers tension = 25.0 [o/o]

Material-safety-factors:

Mat	concr SC1	SC2	steel SS1	SS2
1	1.50	1.50		
3	1.50	1.50		
4	1.50	1.50		
5	1.50	1.50		
6	1.50	1.50		
7	1.50	1.50		
8	1.50	1.50		
9	1.50	1.50		
10	1.50	1.50		
12			1.15	1.15

Acc. the german DIN Fachberichten a minimum concrete shear capacity VRd,ct
is taken into account in the shear design without shear reinforcement.

In shear design the cotangens theta is limited to 1.750 .

At direct supports from the face of the support up to 1.0*d the shear force is reduced.
The maximum shear capacity is checked at the face of the support without reduction.

The punching design has been switched off and must be done separately.
Outside the punching area, the normal slab shear design may increase the,
longitudinal reinforcement up to 0.20% [input CTRL...RO_V].

Geometry (axial covers)

No	he-upper [mm]	hi-upper [mm]	he-lower [mm]	hi-lower [mm]	Elem. height [mm]
1	50	70	35	55	As saved

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΔΙΑΣΤΑΣΙΟΛΟΓΗΣΗ ΦΟΡΕΑ ΣΕΙΣΜΙΚΕΣ ΦΟΡΤΙΣΕΙΣ

Selection of elements

	from	to	inc	group	GEOMETRY
Element	3001	3999	1	-	1
Element	4001	4999	1	-	1
Element	8001	8999	1	-	1

Reinforcement is saved in the data base file
Number of stored reinforcement-distribution: 522

REINFORCEMENT ACC. TO DIN1045-1 2008 in [cm2/m] upper/lower
General load safety factor - as defined in BEMESS: Gamma-f = 1.00
Shear: stresses VEd/d and VRd,ct/d with d=effective depth = h-hm
Shear index 2m = minimum shear reinforcement

Grp	ELEM No	LC No	MAT No MBW	GEO No	h [m]	Reinforcement main	cross	dir	dphi deg	Shr zon	VEd/d [MPa] VRd,ct/d	Ass [cm2/m2]
3	3001	maximum			0.25	1.39	0.28	0		1	0.078	
						0.78	0.16	0			0.495	
3	3002	maximum			0.25	0.31	0.06	0		1	0.058	
						1.55	0.31	0			0.495	
3	3003	maximum			0.25		0.02	0		1	0.049	
						2.03	0.41	0			0.495	
3	3004	maximum			0.25		0.02	0		1	0.038	
						2.36	0.47	0			0.495	
3	3005	maximum			0.25		0.02	0		1	0.027	
						2.51	0.50	0			0.496	
3	3006	maximum			0.25		0.02	0		1	0.017	
						2.61	0.52	0			0.496	
3	3007	maximum			0.25	1.39	0.28	0		1	0.053	
						0.86	0.17	0			0.495	
3	3008	maximum			0.25	0.33	0.14	0		1	0.056	
						1.53	0.31	0			0.496	
3	3009	maximum			0.25	0.02	0.11	0		1	0.045	
						2.03	0.41	0			0.496	
3	3010	maximum			0.25	0.02	0.11	0		1	0.038	
						2.36	0.47	0			0.496	
3	3011	maximum			0.25	0.02	0.11	0		1	0.033	
						2.52	0.50	0			0.496	
3	3012	maximum			0.25	0.02	0.11	0		1	0.029	
						2.60	0.52	0			0.496	
3	3013	maximum			0.25	1.41	0.43	0		1	0.078	
						0.84	0.17	0			0.496	
3	3014	maximum			0.25	0.37	0.20	0		1	0.048	
						1.52	0.30	0			0.495	
3	3015	maximum			0.25	0.03	0.15	0		1	0.038	
						2.04	0.41	0			0.495	
3	3016	maximum			0.25	0.03	0.15	0		1	0.031	
						2.38	0.48	0			0.496	
3	3017	maximum			0.25	0.03	0.15	0		1	0.027	
						2.53	0.51	0			0.496	
3	3018	maximum			0.25	0.03	0.15	0		1	0.025	
						2.60	0.52	0			0.496	
3	3019	maximum			0.25	1.41	0.59	0		1	0.081	
						0.75	0.15	0			0.495	
3	3020	maximum			0.25	0.42	0.37	0		1	0.062	
						1.51	0.30	0			0.495	
3	3021	maximum			0.25	0.05	0.23	0		1	0.046	
						2.01	0.40	0			0.495	
3	3022	maximum			0.25	0.03	0.15	0		1	0.034	
						2.35	0.47	0			0.495	
3	3023	maximum			0.25	0.02	0.10	0		1	0.023	
						2.51	0.50	0			0.496	
3	3024	maximum			0.25	0.02	0.08	0		1	0.014	
						2.61	0.52	0			0.496	
3	3025	maximum			0.25	1.55	0.94	0		1	0.090	
						0.79	0.16	0			0.496	
3	3026	maximum			0.25	0.52	0.76	0		1	0.080	
						1.51	0.30	0			0.496	
3	3027	maximum			0.25	0.09	0.44	0		1	0.058	
						2.04	0.41	0			0.496	
3	3028	maximum			0.25	0.06	0.30	0		1	0.047	
						2.38	0.48	0			0.496	
3	3029	maximum			0.25	0.04	0.20	0		1	0.038	
						2.53	0.51	0			0.496	
3	3030	maximum			0.25	0.03	0.16	0		1	0.033	
						2.60	0.52	0			0.496	
3	3031	maximum			0.25	1.55	0.95	0		1	0.066	
						0.75	0.15	0			0.495	
3	3032	maximum			0.25	0.48	0.72	0		1	0.050	
						1.49	0.30	0			0.495	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΔΙΑΣΤΑΣΙΟΛΟΓΗΣΗ ΦΟΡΕΑ ΣΕΙΣΜΙΚΕΣ ΦΟΡΤΙΣΕΙΣ

REINFORCEMENT ACC. TO DIN1045-1 2008 in [cm2/m] upper/lower
General load safety factor - as defined in BEMESS: Gamma-f = 1.00
Shear: stresses VEd/d and VRd,ct/d with d=effective depth = h-hm
Shear index 2m = minimum shear reinforcement

Grp	ELEM No	LC No	MAT No	GEO No	h [m]	Reinforcement main	cross	dir	dphi deg	Shr zon	VEd/d [MPa] VRd,ct/d	Ass [cm2/m2]
		MBW										
3	3033	maximum			0.25	0.11	0.54	0		1	0.040	
						1.99	0.40	0			0.495	
3	3034	maximum			0.25	0.08	0.41	0		1	0.032	
						2.34	0.47	0			0.495	
3	3035	maximum			0.25	0.06	0.32	0		1	0.025	
						2.50	0.50	0			0.496	
3	3036	maximum			0.25	0.05	0.26	0		1	0.020	
						2.59	0.52	0			0.496	
3	3037	maximum			0.25	1.60	1.12	0		1	0.064	
						0.76	0.15	0			0.495	
3	3038	maximum			0.25	0.59	1.01	0		1	0.064	
						1.49	0.30	0			0.495	
3	3039	maximum			0.25	0.14	0.71	0		1	0.051	
						2.01	0.40	0			0.495	
3	3040	maximum			0.25	0.10	0.51	0		1	0.040	
						2.36	0.47	0			0.495	
3	3041	maximum			0.25	0.07	0.36	0		1	0.029	
						2.87	0.57	0			0.496	
3	3042	maximum			0.25	0.05	0.27	0		1	0.020	
						2.60	0.52	0			0.496	
3	3043	maximum			0.25	1.70	1.20	0		1	0.055	
						0.73	0.15	0			0.495	
3	3044	maximum			0.25	0.56	0.98	0		1	0.058	
						1.50	0.30	0			0.495	
3	3045	maximum			0.25	0.15	0.73	0		1	0.050	
						2.01	0.40	0			0.495	
3	3046	maximum			0.25	0.11	0.56	0		1	0.044	
						2.35	0.47	0			0.495	
3	3047	maximum			0.25	0.08	0.42	0		1	0.038	
						2.57	0.51	0			0.495	
3	3048	maximum			0.25	0.07	0.36	0		1	0.035	
						2.60	0.52	0			0.495	
3	3049	maximum			0.25	1.68	1.11	0		1	0.059	
						0.74	0.15	0			0.495	
3	3050	maximum			0.25	0.59	0.93	0		1	0.051	
						1.51	0.30	0			0.495	
3	3051	maximum			0.25	0.12	0.60	0		1	0.042	
						2.06	0.41	0			0.495	
3	3052	maximum			0.25	0.09	0.43	0		1	0.034	
						2.43	0.49	0			0.495	
3	3053	maximum			0.25	0.06	0.29	0		1	0.025	
						2.58	0.52	0			0.496	
3	3054	maximum			0.25	0.05	0.23	0		1	0.017	
						2.65	0.53	0			0.496	
3	3055	maximum			0.25	1.75	1.08	0		1	0.087	
						0.72	0.14	0			0.496	
3	3056	maximum			0.25	0.52	0.80	0		1	0.086	
						1.53	0.31	0			0.496	
3	3057	maximum			0.25	0.10	0.52	0		1	0.074	
						2.06	0.41	0			0.496	
3	3058	maximum			0.25	0.08	0.38	0		1	0.071	
						2.62	0.52	0			0.496	
3	3059	maximum			0.25	0.06	0.29	0		1	0.067	
						2.57	0.51	0			0.496	
3	3060	maximum			0.25	0.05	0.25	0		1	0.064	
						2.66	0.53	0			0.496	
3	3061	maximum			0.25	1.61	0.77	0		1	0.073	
						0.74	0.15	0			0.495	
3	3062	maximum			0.25	0.52	0.52	0		1	0.063	
						1.62	0.32	0			0.495	
3	3063	maximum			0.25	0.05	0.26	0		1	0.054	
						2.16	0.43	0			0.495	
3	3064	maximum			0.25	0.03	0.13	0		1	0.044	
						2.50	0.50	0			0.495	
3	3065	maximum			0.25	0.02	0.08	0		1	0.035	
						2.65	0.53	0			0.495	
3	3066	maximum			0.25	0.01	0.05	0		1	0.030	
						2.74	0.55	0			0.495	
3	3067	maximum			0.25	1.61	0.68	0		1	0.069	
						0.89	0.18	0			0.496	
3	3068	maximum			0.25	0.56	0.47	0		1	0.052	
						1.68	0.34	0			0.495	
3	3069	maximum			0.25	0.04	0.18	0		1	0.040	
						2.24	0.45	0			0.495	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΔΙΑΣΤΑΣΙΟΛΟΓΗΣΗ ΦΟΡΕΑ ΣΕΙΣΜΙΚΕΣ ΦΟΡΤΙΣΕΙΣ

REINFORCEMENT ACC. TO DIN1045-1 2008 in [cm2/m] upper/lower
General load safety factor - as defined in BEMESS: Gamma-f = 1.00
Shear: stresses VEd/d and VRd,ct/d with d=effective depth = h-hm
Shear index 2m = minimum shear reinforcement

Grp	ELEM No	LC No	MAT No	GEO No	h [m]	Reinforcement main	cross	dphi dir	deg	Shr zon	VEd/d [MPa] VRd,ct/d	Ass [cm2/m2]
			MBW									
3	3070	maximum			0.25	0.02	0.10	0		1	0.032	
						2.58	0.52	0			0.496	
3	3071	maximum			0.25	0.01	0.05	0		1	0.024	
						2.72	0.54	0			0.496	
3	3072	maximum			0.25		0.02	0		1	0.018	
						2.78	0.56	0			0.497	
3	3073	maximum			0.25	1.64	0.33	0		1	0.068	
						0.73	0.15	0			0.495	
3	3074	maximum			0.25	0.44	0.30	0		1	0.072	
						1.60	0.32	0			0.496	
3	3075	maximum			0.25	0.03	0.16	0		1	0.059	
						2.31	0.46	0			0.496	
3	3076	maximum			0.25	0.03	0.15	0		1	0.051	
						2.47	0.49	0			0.496	
3	3077	maximum			0.25	0.03	0.14	0		1	0.045	
						2.63	0.53	0			0.496	
3	3078	maximum			0.25	0.03	0.14	0		1	0.041	
						2.73	0.55	0			0.496	
3	3079	maximum			0.25	1.56	0.31	0		1	0.100	
						0.77	0.16	0			0.495	
3	3080	maximum			0.25	0.41	0.15	0		1	0.085	
						1.59	0.32	0			0.495	
3	3081	maximum			0.25	0.01	0.04	0		1	0.070	
						2.13	0.43	0			0.495	
3	3082	maximum			0.25	0.01	0.03	0		1	0.055	
						2.48	0.50	0			0.495	
3	3083	maximum			0.25	0.01	0.03	0		1	0.039	
						2.64	0.53	0			0.495	
3	3084	maximum			0.25	0.01	0.03	0		1	0.022	
						2.75	0.55	0			0.496	
3	3085	maximum			0.25		0.02	0		1	0.017	
						2.62	0.52	0			0.496	
3	3086	maximum			0.25		0.02	0		1	0.027	
						2.52	0.50	0			0.496	
3	3087	maximum			0.25		0.02	0		1	0.037	
						2.37	0.47	0			0.495	
3	3088	maximum			0.25		0.02	0		1	0.048	
						2.05	0.41	0			0.495	
3	3089	maximum			0.25	0.28	0.06	0		1	0.058	
						1.59	0.32	0			0.495	
3	3090	maximum			0.25	1.35	0.27	0		1	0.077	
						0.83	0.17	0			0.495	
3	3091	maximum			0.25	0.02	0.11	0		1	0.029	
						2.61	0.52	0			0.496	
3	3092	maximum			0.25	0.02	0.11	0		1	0.032	
						2.53	0.51	0			0.496	
3	3093	maximum			0.25	0.02	0.11	0		1	0.038	
						2.38	0.48	0			0.496	
3	3094	maximum			0.25	0.02	0.11	0		1	0.045	
						2.05	0.41	0			0.496	
3	3095	maximum			0.25	0.30	0.14	0		1	0.056	
						1.57	0.31	0			0.496	
3	3096	maximum			0.25	1.35	0.27	0		1	0.053	
						0.90	0.18	0			0.495	
3	3097	maximum			0.25	0.03	0.15	0		1	0.025	
						2.60	0.52	0			0.496	
3	3098	maximum			0.25	0.03	0.15	0		1	0.027	
						2.54	0.51	0			0.496	
3	3099	maximum			0.25	0.03	0.15	0		1	0.031	
						2.39	0.48	0			0.496	
3	3100	maximum			0.25	0.03	0.15	0		1	0.037	
						2.07	0.41	0			0.495	
3	3101	maximum			0.25	0.34	0.20	0		1	0.048	
						1.56	0.31	0			0.495	
3	3102	maximum			0.25	1.38	0.43	0		1	0.078	
						0.89	0.18	0			0.496	
3	3103	maximum			0.25	0.02	0.08	0		1	0.014	
						2.61	0.52	0			0.496	
3	3104	maximum			0.25	0.02	0.10	0		1	0.023	
						2.52	0.50	0			0.496	
3	3105	maximum			0.25	0.03	0.15	0		1	0.034	
						2.37	0.47	0			0.495	
3	3106	maximum			0.25	0.05	0.23	0		1	0.046	
						2.03	0.41	0			0.495	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΔΙΑΣΤΑΣΙΟΛΟΓΗΣΗ ΦΟΡΕΑ ΣΕΙΣΜΙΚΕΣ ΦΟΡΤΙΣΕΙΣ

REINFORCEMENT ACC. TO DIN1045-1 2008 in [cm2/m] upper/lower
General load safety factor - as defined in BEMESS: Gamma-f = 1.00
Shear: stresses VEd/d and VRd,ct/d with d=effective depth = h-hm
Shear index 2m = minimum shear reinforcement

Grp	ELEM No	LC No	MAT No	GEO No	h [m]	Reinforcement main	cross	dphi dir	deg	Shr zon	VEd/d [MPa] VRd,ct/d	Ass [cm2/m2]
		MBW										
3	3107	maximum			0.25	0.39	0.37	0		1	0.062	
						1.55	0.31	0			0.495	
3	3108	maximum			0.25	1.38	0.59	0		1	0.081	
						0.79	0.16	0			0.495	
3	3109	maximum			0.25	0.03	0.16	0		1	0.032	
						2.60	0.52	0			0.496	
3	3110	maximum			0.25	0.04	0.20	0		1	0.037	
						2.54	0.51	0			0.496	
3	3111	maximum			0.25	0.06	0.30	0		1	0.047	
						2.40	0.48	0			0.496	
3	3112	maximum			0.25	0.10	0.52	0		1	0.058	
						2.06	0.41	0			0.496	
3	3113	maximum			0.25	0.49	0.76	0		1	0.080	
						1.55	0.31	0			0.496	
3	3114	maximum			0.25	1.51	0.94	0		1	0.090	
						0.84	0.17	0			0.496	
3	3115	maximum			0.25	0.05	0.26	0		1	0.020	
						2.59	0.52	0			0.496	
3	3116	maximum			0.25	0.06	0.32	0		1	0.025	
						2.51	0.50	0			0.496	
3	3117	maximum			0.25	0.08	0.41	0		1	0.031	
						2.36	0.47	0			0.495	
3	3118	maximum			0.25	0.11	0.53	0		1	0.040	
						2.02	0.40	0			0.495	
3	3119	maximum			0.25	0.45	0.72	0		1	0.050	
						1.53	0.31	0			0.495	
3	3120	maximum			0.25	1.52	0.95	0		1	0.065	
						0.79	0.16	0			0.495	
3	3121	maximum			0.25	0.05	0.27	0		1	0.020	
						2.61	0.52	0			0.496	
3	3122	maximum			0.25	0.07	0.36	0		1	0.029	
						2.89	0.58	0			0.496	
3	3123	maximum			0.25	0.10	0.51	0		1	0.040	
						2.38	0.48	0			0.495	
3	3124	maximum			0.25	0.14	0.70	0		1	0.050	
						2.04	0.41	0			0.495	
3	3125	maximum			0.25	0.56	1.01	0		1	0.064	
						1.53	0.31	0			0.495	
3	3126	maximum			0.25	1.57	1.12	0		1	0.064	
						0.80	0.16	0			0.495	
3	3127	maximum			0.25	0.07	0.35	0		1	0.035	
						2.61	0.52	0			0.495	
3	3128	maximum			0.25	0.08	0.42	0		1	0.037	
						2.69	0.54	0			0.495	
3	3129	maximum			0.25	0.11	0.55	0		1	0.044	
						2.37	0.47	0			0.495	
3	3130	maximum			0.25	0.19	0.94	0		1	0.050	
						2.04	0.41	0			0.495	
3	3131	maximum			0.25	0.53	0.97	0		1	0.058	
						1.54	0.31	0			0.495	
3	3132	maximum			0.25	1.67	1.19	0		1	0.055	
						0.77	0.15	0			0.495	
3	3133	maximum			0.25	0.05	0.23	0		1	0.016	
						2.65	0.53	0			0.496	
3	3134	maximum			0.25	0.06	0.29	0		1	0.025	
						2.59	0.52	0			0.496	
3	3135	maximum			0.25	0.08	0.42	0		1	0.033	
						2.45	0.49	0			0.495	
3	3136	maximum			0.25	0.15	0.74	0		1	0.042	
						2.13	0.43	0			0.495	
3	3137	maximum			0.25	0.56	0.92	0		1	0.051	
						1.55	0.31	0			0.495	
3	3138	maximum			0.25	1.64	1.10	0		1	0.058	
						0.79	0.16	0			0.495	
3	3139	maximum			0.25	0.05	0.25	0		1	0.064	
						2.66	0.53	0			0.496	
3	3140	maximum			0.25	0.06	0.29	0		1	0.067	
						2.58	0.52	0			0.496	
3	3141	maximum			0.25	0.08	0.38	0		1	0.071	
						2.84	0.57	0			0.496	
3	3142	maximum			0.25	0.10	0.51	0		1	0.073	
						2.09	0.42	0			0.496	
3	3143	maximum			0.25	0.49	0.79	0		1	0.085	
						1.57	0.31	0			0.496	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
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Grp	ELEM No	LC No	MAT No MBW	GEO No	h [m]	Reinforcement main	cross	dphi dir	deg	Shr zon	VEd/d [MPa] VRd,ct/d	Ass [cm2/m2]
3	3144	maximum			0.25	1.71	1.07	0		1	0.087	
						0.77	0.15	0			0.496	
3	3145	maximum			0.25	0.01	0.05	0		1	0.030	
						2.74	0.55	0			0.495	
3	3146	maximum			0.25	0.02	0.08	0		1	0.035	
						2.66	0.53	0			0.495	
3	3147	maximum			0.25	0.03	0.13	0		1	0.044	
						2.52	0.50	0			0.495	
3	3148	maximum			0.25	0.05	0.25	0		1	0.053	
						2.18	0.44	0			0.495	
3	3149	maximum			0.25	0.49	0.52	0		1	0.063	
						1.66	0.33	0			0.495	
3	3150	maximum			0.25	1.58	0.76	0		1	0.072	
						0.79	0.16	0			0.495	
3	3151	maximum			0.25	0.01	0.03	0		1	0.018	
						2.78	0.56	0			0.497	
3	3152	maximum			0.25	0.01	0.05	0		1	0.023	
						2.73	0.55	0			0.496	
3	3153	maximum			0.25	0.02	0.10	0		1	0.031	
						2.60	0.52	0			0.496	
3	3154	maximum			0.25	0.04	0.18	0		1	0.040	
						2.27	0.45	0			0.495	
3	3155	maximum			0.25	0.53	0.47	0		1	0.051	
						1.71	0.34	0			0.495	
3	3156	maximum			0.25	1.58	0.68	0		1	0.070	
						0.93	0.19	0			0.495	
3	3157	maximum			0.25	0.03	0.14	0		1	0.040	
						2.73	0.55	0			0.496	
3	3158	maximum			0.25	0.03	0.14	0		1	0.044	
						2.63	0.53	0			0.496	
3	3159	maximum			0.25	0.03	0.15	0		1	0.051	
						2.49	0.50	0			0.496	
3	3160	maximum			0.25	0.03	0.16	0		1	0.058	
						2.16	0.43	0			0.496	
3	3161	maximum			0.25	0.41	0.30	0		1	0.071	
						1.64	0.33	0			0.496	
3	3162	maximum			0.25	1.61	0.33	0		1	0.068	
						0.77	0.15	0			0.495	
3	3163	maximum			0.25	0.01	0.03	0		1	0.022	
						2.76	0.55	0			0.496	
3	3164	maximum			0.25	0.01	0.03	0		1	0.038	
						2.66	0.53	0			0.495	
3	3165	maximum			0.25	0.01	0.03	0		1	0.054	
						2.50	0.50	0			0.495	
3	3166	maximum			0.25	0.01	0.04	0		1	0.070	
						2.16	0.43	0			0.495	
3	3167	maximum			0.25	0.38	0.15	0		1	0.084	
						1.63	0.33	0			0.495	
3	3168	maximum			0.25	1.52	0.30	0		1	0.100	
						0.82	0.16	0			0.495	
4	4001	maximum			0.25	6.74	1.35	0		1		
						0.48	0.11	0			0.497	
4	4002	maximum			0.25	2.63	0.53	0		1	0.333	
						0.23	0.16	0			0.495	
4	4003	maximum			0.25	5.24	1.45	0		1	0.402	
						0.18	0.42	0			0.495	
4	4004	maximum			0.25	2.31	0.46	0		1	0.282	
						0.09	0.43	0			0.496	
4	4005	maximum			0.25	5.31	1.67	0		2	0.658	12.63
						0.20	0.17	0			0.496	
4	4006	maximum			0.25	2.42	0.70	0		1	0.418	
						0.10	0.14	0			0.496	
4	4007	maximum			0.25	6.45	2.15	0		1		
						0.23	0.05	0			0.497	
4	4008	maximum			0.25	2.51	0.99	0		1	0.255	
						0.15	0.03	0			0.495	
4	4009	maximum			0.25	5.34	2.20	0		2	0.574	11.03
						0.01		0			0.495	
4	4010	maximum			0.25	2.63	1.25	0		1	0.313	
						0.05	0.01	0			0.496	
4	4011	maximum			0.25	5.66	2.47	0		2	0.497	9.55
						0.09	0.02	0			0.496	
4	4012	maximum			0.25	2.61	1.21	0		1	0.251	
						0.05	0.01	0			0.495	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
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REINFORCEMENT ACC. TO DIN1045-1 2008 in [cm2/m] upper/lower
General load safety factor - as defined in BEMESS: Gamma-f = 1.00
Shear: stresses VEd/d and VRd,ct/d with d=effective depth = h-hm
Shear index 2m = minimum shear reinforcement

Grp	ELEM No	LC No	MAT No MBW	GEO No	h [m]	Reinforcement main	cross	dir	dphi deg	Shr zon	VEd/d [MPa] VRd,ct/d	Ass [cm2/m2]
4	4013	maximum			0.25	5.66	2.51	0		1	0.491	
						0.04	0.01	0			0.495	
4	4014	maximum			0.25	2.71	1.40	0		1	0.223	
						0.05	0.01	0			0.495	
4	4015	maximum			0.25	5.63	2.28	0		2	0.521	10.00
						0.05	0.01	0			0.495	
4	4016	maximum			0.25	2.78	1.36	0		1	0.250	
						0.03	0.01	0			0.495	
4	4017	maximum			0.25	5.85	2.79	0		1	0.476	
								0			0.495	
4	4018	maximum			0.25	2.69	1.36	0		1	0.230	
						0.01		0			0.495	
4	4019	maximum			0.25	5.40	1.90	0		2	0.662	12.72
								0			0.495	
4	4020	maximum			0.25	2.76	1.22	0		1	0.350	
								0			0.496	
4	4021	maximum			0.25	6.79	2.32	0		1		
						0.08	0.02	0			0.497	
4	4022	maximum			0.25	2.74	1.15	0		1	0.254	
						0.07	0.01	0			0.495	
4	4023	maximum			0.25	5.72	2.23	0		2	0.608	11.68
								0			0.495	
4	4024	maximum			0.25	2.57	1.06	0		1	0.391	
								0			0.496	
4	4025	maximum			0.25	5.49	1.17	0		2	0.515	9.90
						0.14	0.72	0			0.495	
4	4026	maximum			0.25	2.48	0.50	0		1	0.308	
						0.11	0.55	0			0.496	
4	4027	maximum			0.25	7.27	1.45	0		1		
						0.25	0.18	0			0.497	
4	4028	maximum			0.25	2.81	0.56	0		1	0.376	
						0.22	0.21	0			0.495	
4	4029	maximum			0.25	2.58	0.52	0		1	0.324	
						0.29	0.16	0			0.495	
4	4030	maximum			0.25	6.56	1.31	0		1		
						0.64	0.13	0			0.497	
4	4031	maximum			0.25	2.27	0.45	0		1	0.273	
						0.14	0.41	0			0.496	
4	4032	maximum			0.25	5.12	1.43	0		1	0.389	
						0.30	0.40	0			0.496	
4	4033	maximum			0.25	2.38	0.70	0		1	0.408	
						0.15	0.14	0			0.496	
4	4034	maximum			0.25	5.19	1.63	0		2	0.645	12.39
						0.36	0.17	0			0.495	
4	4035	maximum			0.25	2.47	0.98	0		1	0.249	
						0.21	0.04	0			0.495	
4	4036	maximum			0.25	6.29	2.11	0		1		
						0.37	0.07	0			0.497	
4	4037	maximum			0.25	2.58	1.25	0		1	0.302	
						0.10	0.02	0			0.496	
4	4038	maximum			0.25	5.22	2.18	0		2	0.559	10.74
						0.11	0.02	0			0.496	
4	4039	maximum			0.25	2.57	1.21	0		1	0.245	
						0.10	0.02	0			0.495	
4	4040	maximum			0.25	5.54	2.44	0		1	0.485	
						0.18	0.04	0			0.495	
4	4041	maximum			0.25	2.66	1.39	0		1	0.217	
						0.14	0.03	0			0.495	
4	4042	maximum			0.25	5.54	2.48	0		1	0.478	
						0.14	0.03	0			0.495	
4	4043	maximum			0.25	2.74	1.36	0		1	0.243	
						0.09	0.02	0			0.495	
4	4044	maximum			0.25	5.50	2.24	0		2	0.507	9.74
						0.16	0.03	0			0.496	
4	4045	maximum			0.25	2.65	1.36	0		1	0.224	
						0.06	0.01	0			0.495	
4	4046	maximum			0.25	5.72	2.74	0		1	0.463	
						0.09	0.02	0			0.495	
4	4047	maximum			0.25	2.71	1.21	0		1	0.339	
						0.06	0.01	0			0.496	
4	4048	maximum			0.25	5.28	1.87	0		2	0.647	12.43
						0.10	0.02	0			0.495	
4	4049	maximum			0.25	2.69	1.14	0		1	0.248	
						0.13	0.03	0			0.495	

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Grp	ELEM No	LC No	MAT No MBW	GEO No	h [m]	Reinforcement main	cross	dir	dphi deg	Shr zon	VEd/d [MPa] VRd,ct/d	Ass [cm2/m2]
4	4050	maximum			0.25	6.63	2.27	0		1		
						0.22	0.04	0			0.497	
4	4051	maximum			0.25	2.53	1.06	0		1	0.381	
						0.03	0.01	0			0.496	
4	4052	maximum			0.25	5.60	2.19	0		2	0.594	11.41
						0.01		0			0.495	
4	4053	maximum			0.25	2.45	0.49	0		1	0.298	
						0.11	0.53	0			0.496	
4	4054	maximum			0.25	5.37	1.15	0		2	0.502	9.65
						0.18	0.71	0			0.495	
4	4055	maximum			0.25	2.77	0.55	0		1	0.367	
						0.28	0.21	0			0.495	
4	4056	maximum			0.25	7.09	1.42	0		1		
						0.40	0.20	0			0.497	
8	8001	maximum			1.20	2.43	5.27	0		1	0.101	
						3.67	13.11	0			0.294	
8	8002	maximum			1.20	2.44	12.22	0		1	0.160	
						5.97	29.87	0			0.317	
8	8003	maximum			1.20	6.27	10.04	0		1	0.178	
						23.55	55.08	0			0.360	
8	8004	maximum			1.20	4.47	6.75	0		1	0.206	
						5.05	25.26	0			0.313	
8	8005	maximum			1.20	9.57	5.40	0		1	0.207	
						23.64	49.98	0			0.362	
8	8006	maximum			1.20	0.36	1.80	0		1	0.240	
						3.63	18.13	0			0.345	
8	8007	maximum			1.20	7.06	3.03	0		1	0.080	
						6.35	9.74	0			0.316	
8	8008	maximum			1.20	0.33	1.64	0		1	0.282	
						3.59	17.97	0			0.358	
8	8009	maximum			1.20	7.98	6.35	0		1	0.137	
						25.66	47.15	0			0.350	
8	8010	maximum			1.20	0.33	1.64	0		1	0.323	
						3.50	17.50	0			0.372	
8	8011	maximum			1.20	8.81	6.21	0		1	0.132	
						19.61	37.01	0			0.330	
8	8012	maximum			1.20	0.55	2.77	0		1	0.203	
						4.10	20.51	0			0.391	
8	8013	maximum			1.20	6.90	5.26	0		1	0.113	
						17.70	34.74	0			0.325	
8	8014	maximum			1.20	1.03	5.14	0		1	0.185	
						4.77	23.87	0			0.398	
8	8015	maximum			1.20	6.94	5.19	0		1	0.125	
						17.32	34.42	0			0.328	
8	8016	maximum			1.20	1.00	5.02	0		1	0.187	
						4.82	24.11	0			0.399	
8	8017	maximum			1.20	9.26	6.22	0		1	0.126	
						20.29	38.07	0			0.333	
8	8018	maximum			1.20	0.49	2.46	0		1	0.211	
						4.19	20.93	0			0.393	
8	8019	maximum			1.20	7.61	5.73	0		1	0.140	
						25.67	47.76	0			0.352	
8	8020	maximum			1.20	0.26	1.30	0		1	0.330	
						3.57	17.85	0			0.373	
8	8021	maximum			1.20	8.16	2.83	0		1	0.097	
						5.78	9.70	0			0.311	
8	8022	maximum			1.20	0.23	1.16	0		1	0.301	
						3.65	18.27	0			0.357	
8	8023	maximum			1.20	10.53	4.83	0		1	0.219	
						25.23	52.53	0			0.374	
8	8024	maximum			1.20	0.22	1.11	0		1	0.261	
						3.70	18.51	0			0.341	
8	8025	maximum			1.20	5.72	8.20	0		1	0.188	
						24.39	57.11	0			0.371	
8	8026	maximum			1.20	4.75	6.02	0		1	0.224	
						5.07	25.34	0			0.311	
8	8027	maximum			1.20	2.39	4.74	0		1	0.107	
						3.72	13.15	0			0.295	
8	8028	maximum			1.20	2.27	11.34	0		1	0.195	
						6.03	30.14	0			0.317	
8	8029	maximum			1.20	2.45	5.42	0		1	0.098	
						3.64	12.84	0			0.291	
8	8030	maximum			1.20	6.90	11.71	0		1	0.175	
						22.93	53.32	0			0.356	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΔΙΑΣΤΑΣΙΟΛΟΓΗΣΗ ΦΟΡΕΑ ΣΕΙΣΜΙΚΕΣ ΦΟΡΤΙΣΕΙΣ

REINFORCEMENT ACC. TO DIN1045-1 2008 in [cm2/m] upper/lower
General load safety factor - as defined in BEMESS: Gamma-f = 1.00
Shear: stresses VEd/d and VRd,ct/d with d=effective depth = h-hm
Shear index 2m = minimum shear reinforcement

Grp	ELEM No	LC No	MAT No MBW	GEO No	h [m]	Reinforcement main	Reinforcement cross	dir	dphi deg	Shr zon	VEd/d [MPa] VRd,ct/d	Ass [cm2/m2]
8	8031	maximum			1.20	2.52	12.61	0		1	0.160	
						5.84	29.18	0			0.317	
8	8032	maximum			1.20	4.33	7.62	0		1	0.202	
						4.91	24.54	0			0.314	
8	8033	maximum			1.20	9.58	7.00	0		1	0.205	
						22.89	48.14	0			0.358	
8	8034	maximum			1.20	7.18	3.59	0		1	0.080	
						6.53	9.48	0			0.314	
8	8035	maximum			1.20	8.72	7.82	0		1	0.135	
						25.16	45.55	0			0.346	
8	8036	maximum			1.20	0.51	2.55	0		1	0.235	
						3.50	17.48	0			0.347	
8	8037	maximum			1.20	0.47	2.37	0		1	0.277	
						3.47	17.33	0			0.366	
8	8038	maximum			1.20	0.46	2.29	0		1	0.315	
						3.36	16.81	0			0.373	
8	8039	maximum			1.20	9.01	6.63	0		1	0.133	
						19.30	35.72	0			0.329	
8	8040	maximum			1.20	7.00	6.23	0		1	0.113	
						17.38	33.61	0			0.324	
8	8041	maximum			1.20	0.69	3.43	0		1	0.200	
						3.93	19.66	0			0.393	
8	8042	maximum			1.20	1.19	5.94	0		1	0.182	
						4.62	23.08	0			0.399	
8	8043	maximum			1.20	6.90	6.32	0		1	0.126	
						17.04	33.42	0			0.327	
8	8044	maximum			1.20	9.15	6.36	0		1	0.127	
						19.78	36.79	0			0.332	
8	8045	maximum			1.20	1.17	5.87	0		1	0.184	
						4.67	23.34	0			0.399	
8	8046	maximum			1.20	0.66	3.28	0		1	0.208	
						4.05	20.23	0			0.394	
8	8047	maximum			1.20	8.18	7.21	0		1	0.138	
						25.00	46.16	0			0.348	
8	8048	maximum			1.20	8.13	3.34	0		1	0.097	
						5.79	9.40	0			0.311	
8	8049	maximum			1.20	10.47	5.45	0		1	0.216	
						24.47	50.70	0			0.370	
8	8050	maximum			1.20	0.40	2.01	0		1	0.323	
						3.44	17.22	0			0.373	
8	8051	maximum			1.20	0.38	1.89	0		1	0.295	
						3.52	17.62	0			0.363	
8	8052	maximum			1.20	0.36	1.80	0		1	0.257	
						3.56	17.79	0			0.344	
8	8053	maximum			1.20	6.28	9.80	0		1	0.185	
						23.69	55.28	0			0.366	
8	8054	maximum			1.20	2.40	4.82	0		1	0.103	
						3.68	12.85	0			0.296	
8	8055	maximum			1.20	4.70	6.88	0		1	0.220	
						4.95	24.75	0			0.313	
8	8056	maximum			1.20	2.29	11.46	0		1	0.194	
						5.87	29.36	0			0.318	

Explanations shear state Shr zon:
1 = check without necessary shear reinforcement
2 = shear reinforcement required
m = minimum shear reinforcement
Acc. DIN 1045-1 10.3.4(2) the leverarm z was limited to d-2*nomc.

Maximum Reinforcement [cm2/m]
(stored in data base file with reinforcement-distribution-no. 522)

Grp	Element	upper:As	Ast	dir	lower:As	Ast	dir	Ass[cm2/m2]	AssE[cm2]
3	3001	1.39	0.28	0	0.78	0.16	0		
3	3002	0.31	0.06	0	1.55	0.31	0		
3	3003		0.02	0	2.03	0.41	0		
3	3004		0.02	0	2.36	0.47	0		
3	3005		0.02	0	2.51	0.50	0		
3	3006		0.02	0	2.61	0.52	0		
3	3007	1.39	0.28	0	0.86	0.17	0		
3	3008	0.33	0.14	0	1.53	0.31	0		
3	3009	0.02	0.11	0	2.03	0.41	0		
3	3010	0.02	0.11	0	2.36	0.47	0		
3	3011	0.02	0.11	0	2.52	0.50	0		
3	3012	0.02	0.11	0	2.60	0.52	0		

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
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Maximum Reinforcement [cm2/m]
(stored in data base file with reinforcement-distribution-no. 522)

Grp	Element	upper:As	Ast	dir	lower:As	Ast	dir	Ass[cm2/m2]	AssE[cm2]
3	3013	1.41	0.43	0	0.84	0.17	0		
3	3014	0.37	0.20	0	1.52	0.30	0		
3	3015	0.03	0.15	0	2.04	0.41	0		
3	3016	0.03	0.15	0	2.38	0.48	0		
3	3017	0.03	0.15	0	2.53	0.51	0		
3	3018	0.03	0.15	0	2.60	0.52	0		
3	3019	1.41	0.59	0	0.75	0.15	0		
3	3020	0.42	0.37	0	1.51	0.30	0		
3	3021	0.05	0.23	0	2.01	0.40	0		
3	3022	0.03	0.15	0	2.35	0.47	0		
3	3023	0.02	0.10	0	2.51	0.50	0		
3	3024	0.02	0.08	0	2.61	0.52	0		
3	3025	1.55	0.94	0	0.79	0.16	0		
3	3026	0.52	0.76	0	1.51	0.30	0		
3	3027	0.09	0.44	0	2.04	0.41	0		
3	3028	0.06	0.30	0	2.38	0.48	0		
3	3029	0.04	0.20	0	2.53	0.51	0		
3	3030	0.03	0.16	0	2.60	0.52	0		
3	3031	1.55	0.95	0	0.75	0.15	0		
3	3032	0.48	0.72	0	1.49	0.30	0		
3	3033	0.11	0.54	0	1.99	0.40	0		
3	3034	0.08	0.41	0	2.34	0.47	0		
3	3035	0.06	0.32	0	2.50	0.50	0		
3	3036	0.05	0.26	0	2.59	0.52	0		
3	3037	1.60	1.12	0	0.76	0.15	0		
3	3038	0.59	1.01	0	1.49	0.30	0		
3	3039	0.14	0.71	0	2.01	0.40	0		
3	3040	0.10	0.51	0	2.36	0.47	0		
3	3041	0.07	0.36	0	2.87	0.57	0		
3	3042	0.05	0.27	0	2.60	0.52	0		
3	3043	1.70	1.20	0	0.73	0.15	0		
3	3044	0.56	0.98	0	1.50	0.30	0		
3	3045	0.15	0.73	0	2.01	0.40	0		
3	3046	0.11	0.56	0	2.35	0.47	0		
3	3047	0.08	0.42	0	2.57	0.51	0		
3	3048	0.07	0.36	0	2.60	0.52	0		
3	3049	1.68	1.11	0	0.74	0.15	0		
3	3050	0.59	0.93	0	1.51	0.30	0		
3	3051	0.12	0.60	0	2.06	0.41	0		
3	3052	0.09	0.43	0	2.43	0.49	0		
3	3053	0.06	0.29	0	2.58	0.52	0		
3	3054	0.05	0.23	0	2.65	0.53	0		
3	3055	1.75	1.08	0	0.72	0.14	0		
3	3056	0.52	0.80	0	1.53	0.31	0		
3	3057	0.10	0.52	0	2.06	0.41	0		
3	3058	0.08	0.38	0	2.62	0.52	0		
3	3059	0.06	0.29	0	2.57	0.51	0		
3	3060	0.05	0.25	0	2.66	0.53	0		
3	3061	1.61	0.77	0	0.74	0.15	0		
3	3062	0.52	0.52	0	1.62	0.32	0		
3	3063	0.05	0.26	0	2.16	0.43	0		
3	3064	0.03	0.13	0	2.50	0.50	0		
3	3065	0.02	0.08	0	2.65	0.53	0		
3	3066	0.01	0.05	0	2.74	0.55	0		
3	3067	1.61	0.68	0	0.89	0.18	0		
3	3068	0.56	0.47	0	1.68	0.34	0		
3	3069	0.04	0.18	0	2.24	0.45	0		
3	3070	0.02	0.10	0	2.58	0.52	0		
3	3071	0.01	0.05	0	2.72	0.54	0		
3	3072		0.02	0	2.78	0.56	0		
3	3073	1.64	0.33	0	0.73	0.15	0		
3	3074	0.44	0.30	0	1.60	0.32	0		
3	3075	0.03	0.16	0	2.31	0.46	0		
3	3076	0.03	0.15	0	2.47	0.49	0		
3	3077	0.03	0.14	0	2.63	0.53	0		
3	3078	0.03	0.14	0	2.73	0.55	0		
3	3079	1.56	0.31	0	0.77	0.16	0		
3	3080	0.41	0.15	0	1.59	0.32	0		
3	3081	0.01	0.04	0	2.13	0.43	0		
3	3082	0.01	0.03	0	2.48	0.50	0		
3	3083	0.01	0.03	0	2.64	0.53	0		
3	3084	0.01	0.03	0	2.75	0.55	0		
3	3085		0.02	0	2.62	0.52	0		
3	3086		0.02	0	2.52	0.50	0		
3	3087		0.02	0	2.37	0.47	0		
3	3088		0.02	0	2.05	0.41	0		
3	3089	0.28	0.06	0	1.59	0.32	0		
3	3090	1.35	0.27	0	0.83	0.17	0		

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Maximum Reinforcement [cm2/m]
(stored in data base file with reinforcement-distribution-no. 522)

Grp	Element	upper:As	Ast	dir	lower:As	Ast	dir	Ass[cm2/m2]	AssE[cm2]
3	3091	0.02	0.11	0	2.61	0.52	0		
3	3092	0.02	0.11	0	2.53	0.51	0		
3	3093	0.02	0.11	0	2.38	0.48	0		
3	3094	0.02	0.11	0	2.05	0.41	0		
3	3095	0.30	0.14	0	1.57	0.31	0		
3	3096	1.35	0.27	0	0.90	0.18	0		
3	3097	0.03	0.15	0	2.60	0.52	0		
3	3098	0.03	0.15	0	2.54	0.51	0		
3	3099	0.03	0.15	0	2.39	0.48	0		
3	3100	0.03	0.15	0	2.07	0.41	0		
3	3101	0.34	0.20	0	1.56	0.31	0		
3	3102	1.38	0.43	0	0.89	0.18	0		
3	3103	0.02	0.08	0	2.61	0.52	0		
3	3104	0.02	0.10	0	2.52	0.50	0		
3	3105	0.03	0.15	0	2.37	0.47	0		
3	3106	0.05	0.23	0	2.03	0.41	0		
3	3107	0.39	0.37	0	1.55	0.31	0		
3	3108	1.38	0.59	0	0.79	0.16	0		
3	3109	0.03	0.16	0	2.60	0.52	0		
3	3110	0.04	0.20	0	2.54	0.51	0		
3	3111	0.06	0.30	0	2.40	0.48	0		
3	3112	0.10	0.52	0	2.06	0.41	0		
3	3113	0.49	0.76	0	1.55	0.31	0		
3	3114	1.51	0.94	0	0.84	0.17	0		
3	3115	0.05	0.26	0	2.59	0.52	0		
3	3116	0.06	0.32	0	2.51	0.50	0		
3	3117	0.08	0.41	0	2.36	0.47	0		
3	3118	0.11	0.53	0	2.02	0.40	0		
3	3119	0.45	0.72	0	1.53	0.31	0		
3	3120	1.52	0.95	0	0.79	0.16	0		
3	3121	0.05	0.27	0	2.61	0.52	0		
3	3122	0.07	0.36	0	2.89	0.58	0		
3	3123	0.10	0.51	0	2.38	0.48	0		
3	3124	0.14	0.70	0	2.04	0.41	0		
3	3125	0.56	1.01	0	1.53	0.31	0		
3	3126	1.57	1.12	0	0.80	0.16	0		
3	3127	0.07	0.35	0	2.61	0.52	0		
3	3128	0.08	0.42	0	2.69	0.54	0		
3	3129	0.11	0.55	0	2.37	0.47	0		
3	3130	0.19	0.94	0	2.04	0.41	0		
3	3131	0.53	0.97	0	1.54	0.31	0		
3	3132	1.67	1.19	0	0.77	0.15	0		
3	3133	0.05	0.23	0	2.65	0.53	0		
3	3134	0.06	0.29	0	2.59	0.52	0		
3	3135	0.08	0.42	0	2.45	0.49	0		
3	3136	0.15	0.74	0	2.13	0.43	0		
3	3137	0.56	0.92	0	1.55	0.31	0		
3	3138	1.64	1.10	0	0.79	0.16	0		
3	3139	0.05	0.25	0	2.66	0.53	0		
3	3140	0.06	0.29	0	2.58	0.52	0		
3	3141	0.08	0.38	0	2.84	0.57	0		
3	3142	0.10	0.51	0	2.09	0.42	0		
3	3143	0.49	0.79	0	1.57	0.31	0		
3	3144	1.71	1.07	0	0.77	0.15	0		
3	3145	0.01	0.05	0	2.74	0.55	0		
3	3146	0.02	0.08	0	2.66	0.53	0		
3	3147	0.03	0.13	0	2.52	0.50	0		
3	3148	0.05	0.25	0	2.18	0.44	0		
3	3149	0.49	0.52	0	1.66	0.33	0		
3	3150	1.58	0.76	0	0.79	0.16	0		
3	3151	0.01	0.03	0	2.78	0.56	0		
3	3152	0.01	0.05	0	2.73	0.55	0		
3	3153	0.02	0.10	0	2.60	0.52	0		
3	3154	0.04	0.18	0	2.27	0.45	0		
3	3155	0.53	0.47	0	1.71	0.34	0		
3	3156	1.58	0.68	0	0.93	0.19	0		
3	3157	0.03	0.14	0	2.73	0.55	0		
3	3158	0.03	0.14	0	2.63	0.53	0		
3	3159	0.03	0.15	0	2.49	0.50	0		
3	3160	0.03	0.16	0	2.16	0.43	0		
3	3161	0.41	0.30	0	1.64	0.33	0		
3	3162	1.61	0.33	0	0.77	0.15	0		
3	3163	0.01	0.03	0	2.76	0.55	0		
3	3164	0.01	0.03	0	2.66	0.53	0		
3	3165	0.01	0.03	0	2.50	0.50	0		
3	3166	0.01	0.04	0	2.16	0.43	0		
3	3167	0.38	0.15	0	1.63	0.33	0		
3	3168	1.52	0.30	0	0.82	0.16	0		

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΔΙΑΣΤΑΣΙΟΛΟΓΗΣΗ ΦΟΡΕΑ ΣΕΙΣΜΙΚΕΣ ΦΟΡΤΙΣΕΙΣ

Maximum Reinforcement [cm2/m]
(stored in data base file with reinforcement-distribution-no. 522)

Grp	Element	upper:As	Ast	dir	lower:As	Ast	dir	Ass[cm2/m2]	AssE[cm2]
4	4001	6.74	1.35	0	0.48	0.11	0		
4	4002	2.63	0.53	0	0.23	0.16	0		
4	4003	5.24	1.45	0	0.18	0.42	0		
4	4004	2.31	0.46	0	0.09	0.43	0		
4	4005	5.31	1.67	0	0.20	0.17	0	12.63	0.76
4	4006	2.42	0.70	0	0.10	0.14	0		
4	4007	6.45	2.15	0	0.23		0		
4	4008	2.51	0.99	0	0.15		0		
4	4009	5.34	2.20	0			0	11.03	0.66
4	4010	2.63	1.25	0	0.05		0		
4	4011	5.66	2.47	0	0.09		0	9.55	1.15
4	4012	2.61	1.21	0	0.05		0		
4	4013	5.66	2.51	0			0		
4	4014	2.71	1.40	0			0		
4	4015	5.63	2.28	0			0	10.00	1.20
4	4016	2.78	1.36	0			0		
4	4017	5.85	2.79	0			0		
4	4018	2.69	1.36	0			0		
4	4019	5.40	1.90	0			0	12.72	0.76
4	4020	2.76	1.22	0			0		
4	4021	6.79	2.32	0	0.08		0		
4	4022	2.74	1.15	0	0.07		0		
4	4023	5.72	2.23	0			0	11.68	0.70
4	4024	2.57	1.06	0			0		
4	4025	5.49	1.17	0	0.14	0.72	0	9.90	0.59
4	4026	2.48	0.50	0	0.11	0.55	0		
4	4027	7.27	1.45	0	0.25	0.18	0		
4	4028	2.81	0.56	0	0.22	0.21	0		
4	4029	2.58	0.52	0	0.29	0.16	0		
4	4030	6.56	1.31	0	0.64	0.13	0		
4	4031	2.27	0.45	0	0.14	0.41	0		
4	4032	5.12	1.43	0	0.30	0.40	0		
4	4033	2.38	0.70	0	0.15	0.14	0		
4	4034	5.19	1.63	0	0.36	0.17	0	12.39	0.74
4	4035	2.47	0.98	0	0.21		0		
4	4036	6.29	2.11	0	0.37	0.07	0		
4	4037	2.58	1.25	0	0.10		0		
4	4038	5.22	2.18	0	0.11		0	10.74	0.64
4	4039	2.57	1.21	0	0.10		0		
4	4040	5.54	2.44	0	0.18		0		
4	4041	2.66	1.39	0	0.14		0		
4	4042	5.54	2.48	0	0.14		0		
4	4043	2.74	1.36	0	0.09		0		
4	4044	5.50	2.24	0	0.16		0	9.74	1.17
4	4045	2.65	1.36	0	0.06		0		
4	4046	5.72	2.74	0	0.09		0		
4	4047	2.71	1.21	0	0.06		0		
4	4048	5.28	1.87	0	0.10		0	12.43	0.75
4	4049	2.69	1.14	0	0.13		0		
4	4050	6.63	2.27	0	0.22		0		
4	4051	2.53	1.06	0			0		
4	4052	5.60	2.19	0			0	11.41	0.68
4	4053	2.45	0.49	0	0.11	0.53	0		
4	4054	5.37	1.15	0	0.18	0.71	0	9.65	0.58
4	4055	2.77	0.55	0	0.28	0.21	0		
4	4056	7.09	1.42	0	0.40	0.20	0		
8	8001	2.43	5.27	0	3.67	13.11	0		
8	8002	2.44	12.22	0	5.97	29.87	0		
8	8003	6.27	10.04	0	23.55	55.08	0		
8	8004	4.47	6.75	0	5.05	25.26	0		
8	8005	9.57	5.40	0	23.64	49.98	0		
8	8006	0.36	1.80	0	3.63	18.13	0		
8	8007	7.06	3.03	0	6.35	9.74	0		
8	8008	0.33	1.64	0	3.59	17.97	0		
8	8009	7.98	6.35	0	25.66	47.15	0		
8	8010	0.33	1.64	0	3.50	17.50	0		
8	8011	8.81	6.21	0	19.61	37.01	0		
8	8012	0.55	2.77	0	4.10	20.51	0		
8	8013	6.90	5.26	0	17.70	34.74	0		
8	8014	1.03	5.14	0	4.77	23.87	0		
8	8015	6.94	5.19	0	17.32	34.42	0		
8	8016	1.00	5.02	0	4.82	24.11	0		
8	8017	9.26	6.22	0	20.29	38.07	0		
8	8018	0.49	2.46	0	4.19	20.93	0		
8	8019	7.61	5.73	0	25.67	47.76	0		
8	8020	0.26	1.30	0	3.57	17.85	0		
8	8021	8.16	2.83	0	5.78	9.70	0		
8	8022	0.23	1.16	0	3.65	18.27	0		

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΔΙΑΣΤΑΣΙΟΛΟΓΗΣΗ ΦΟΡΕΑ ΣΕΙΣΜΙΚΕΣ ΦΟΡΤΙΣΕΙΣ

Maximum Reinforcement [cm2/m]
(stored in data base file with reinforcement-distribution-no. 522)

Grp	Element	upper:As	Ast	dir	lower:As	Ast	dir	Ass[cm2/m2]	AssE[cm2]
8	8023	10.53	4.83	0	25.23	52.53	0		
8	8024	0.22	1.11	0	3.70	18.51	0		
8	8025	5.72	8.20	0	24.39	57.11	0		
8	8026	4.75	6.02	0	5.07	25.34	0		
8	8027	2.39	4.74	0	3.72	13.15	0		
8	8028	2.27	11.34	0	6.03	30.14	0		
8	8029	2.45	5.42	0	3.64	12.84	0		
8	8030	6.90	11.71	0	22.93	53.32	0		
8	8031	2.52	12.61	0	5.84	29.18	0		
8	8032	4.33	7.62	0	4.91	24.54	0		
8	8033	9.58	7.00	0	22.89	48.14	0		
8	8034	7.18	3.59	0	6.53	9.48	0		
8	8035	8.72	7.82	0	25.16	45.55	0		
8	8036	0.51	2.55	0	3.50	17.48	0		
8	8037	0.47	2.37	0	3.47	17.33	0		
8	8038	0.46	2.29	0	3.36	16.81	0		
8	8039	9.01	6.63	0	19.30	35.72	0		
8	8040	7.00	6.23	0	17.38	33.61	0		
8	8041	0.69	3.43	0	3.93	19.66	0		
8	8042	1.19	5.94	0	4.62	23.08	0		
8	8043	6.90	6.32	0	17.04	33.42	0		
8	8044	9.15	6.36	0	19.78	36.79	0		
8	8045	1.17	5.87	0	4.67	23.34	0		
8	8046	0.66	3.28	0	4.05	20.23	0		
8	8047	8.18	7.21	0	25.00	46.16	0		
8	8048	8.13	3.34	0	5.79	9.40	0		
8	8049	10.47	5.45	0	24.47	50.70	0		
8	8050	0.40	2.01	0	3.44	17.22	0		
8	8051	0.38	1.89	0	3.52	17.62	0		
8	8052	0.36	1.80	0	3.56	17.79	0		
8	8053	6.28	9.80	0	23.69	55.28	0		
8	8054	2.40	4.82	0	3.68	12.85	0		
8	8055	4.70	6.88	0	4.95	24.75	0		
8	8056	2.29	11.46	0	5.87	29.36	0		

REINFORCEMENT INDEX [kg netto]: 0.095 (Upper)
0.271 (Lower)
0.008 (Shear)

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΔΙΑΣΤΑΣΙΟΛΟΓΗΣΗ ΦΟΡΕΑ ΣΕΙΣΜΙΚΕΣ ΦΟΡΤΙΣΕΙΣ

Maximal values of the shear design
Only elements with shear reinforcement are printed.
At punching points punching reinforcement is printed.

element	ass [cm2/m2]	tau [MPa]	acc.VED/VRDmax	acc.cot_theta	min_z [m]
4005	12.63	0.66	0.210	1.75	0.182
4009	11.03	0.57	0.183	1.75	0.145
4011	9.55	0.50	0.159	1.75	0.145
4015	10.00	0.52	0.166	1.75	0.181
4019	12.72	0.66	0.212	1.75	0.182
4023	11.68	0.61	0.194	1.75	0.181
4025	9.90	0.52	0.164	1.75	0.180
4034	12.39	0.64	0.206	1.75	0.182
4038	10.74	0.56	0.179	1.75	0.145
4044	9.74	0.51	0.162	1.75	0.181
4048	12.43	0.65	0.207	1.75	0.182
4052	11.41	0.59	0.190	1.75	0.145
4054	9.65	0.50	0.160	1.75	0.145

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN_ΣΕΙΣΜΙΚΑ_ΠΑΣ/ΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Selected Beam Elements

FROM	TO	INC	X-VALUE	NC	MEMBER	CS0	CS1	CS2	CS3	CS4	CS5
10001											
10005											
10006											
10009											
10010											
10014											
10016											
10020											
10021											
10024											
10025											
10029											
12000	12150	1									

Default design code is DIN Fachbericht 102 Massivbrȯcken (2003) (Germany)
Klasse(Tab.4.118): D

Materials

No. 1 C 25/30 (DIN 1045-1)
No. 3 C 25/30 (DIN 1045-1)
No. 4 C 25/30 (DIN 1045-1)
No. 5 C 25/30 (DIN 1045-1)
No. 6 C 25/30 (DIN 1045-1)
No. 7 C 25/30 (DIN 1045-1)
No. 8 C 25/30 (DIN 1045-1)
No. 9 C 25/30 (DIN 1045-1)
No. 10 C 25/30 (DIN 1045-1)
No. 12 BSt 500 SA (DIN 1045-1)

Reinforcement will be accounted for sectional values as defined in AQUA
Reinforcements saved as design case LCR 509

Considered Load Cases

8101	8102	8103	8104	8105	8106
8107	8108	8201	8202	8203	8204
8205	8206	8207	8301	8302	8303
8304	8305	8306	8307	8308	

Ultimate Load Design

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Design for ultimate loads DIN Fachbericht 102 Massivbrȯcken (2003)
Biaxial bending, uniaxial stress calculated in y-z axis

Safety factors	SC-1	SC-2	SC-S	SS-1	SS-2	PIIa
	1.50	1.50	1.50	1.15	1.15	7
Strain limits	C1	C2	S1	S2	Z1	Z2
max	-3.50	-2.00	3.00	25.00	-3.50	25.00

parameters for reinforcements

Minimum reinforcements Bending.	Compress.	compression e/d	N/Npl	min. reinforcem. requ. section	maximum- reinforc.
0.00 [cm2]	0.30 [o/o]	3.50	0.0010	0.00	0.15	9.00

Tensile forces in the longitudinal reinforcements due to shear are NOT accounted for.
Material of sections uses Ultimate Limit strain-stress law with global safety factors
Material of reinforcements uses Ultimate Limit strain-stress law with global safety factors

MNo.	temp lev.	Material- safety [-]	max.compr stress [MPa]	at strain [o/oo]	max.tens stress [MPa]	at strain [o/oo]	tension- stiffening [MPa]
1	0	1.500	-14.17	-2.00	0.00	0.00	
3	0	1.500	-14.17	-2.00	0.00	0.00	
4	0	1.500	-14.17	-2.00	0.00	0.00	
5	0	1.500	-14.17	-2.00	0.00	0.00	
6	0	1.500	-14.17	-2.00	0.00	0.00	
7	0	1.500	-14.17	-2.00	0.00	0.00	
8	0	1.500	-14.17	-2.00	0.00	0.00	
9	0	1.500	-14.17	-2.00	0.00	0.00	
10	0	1.500	-14.17	-2.00	0.00	0.00	
12	0	1.150	-456.52	-25.00	456.52	25.00	

Shear Design

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Design for shear DIN 1045-1 (2003)
Minimum shear factor or tan of inclination of compressive struts 0.57 / 1.72

MNo	f-cd [MPa]	tau-rd [MPa]	sigIIQ [MPa]	sigIIT [MPa]	sigIIQ+ [MPa]	fyd [MPa]
1	14.17	0.10	10.62	7.44	10.62	
3	14.17	0.10	10.62	7.44	10.62	
4	14.17	0.10	10.62	7.44	10.62	
5	14.17	0.10	10.62	7.44	10.62	
6	14.17	0.10	10.62	7.44	10.62	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN_ΣΕΙΣΜΙΚΑ_ΠΑΣ/ΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Shear Design

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Design for shear DIN 1045-1 (2003)
Minimum shear factor or tan of inclination of compressive struts 0.57 / 1.72

MNo	f-cd [MPa]	tau-rd [MPa]	sigIIQ [MPa]	sigIIT [MPa]	sigIIQ+ [MPa]	fyd [MPa]
7	14.17	0.10	10.62	7.44	10.62	
8	14.17	0.10	10.62	7.44	10.62	
9	14.17	0.10	10.62	7.44	10.62	
10	14.17	0.10	10.62	7.44	10.62	
12						434.78

Tolerance for exceeding maximum shear or principal compression stress 0.0200

Longitudinal Reinforcements LCR 509
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
10001	0.000	8	0.29	38.75		19.10T	9.82		9.83	
10001	0.200	8	0.04	5.31		5.31T				
10005	0.000	8	0.64	84.66		53.50T	7.93		23.23	
10005	0.200	8	0.61	80.19		53.49T	5.36		21.34	
10006	0.000	8	0.02	3.04		2.15T			0.89	
10006	0.400	8	0.03	4.45		2.42T			2.02	
10009	0.000	8	0.03	4.10		2.09T			2.01	
10009	0.400	8	0.02	2.63		1.83T			0.80	
10010	0.000	8	0.61	80.02		53.22T	5.46		21.35	
10010	0.200	8	0.64	84.41		53.22T	7.98		23.21	
10014	0.000	8	0.04	5.36		5.36T				
10014	0.200	8	0.29	38.87		19.20T	9.83		9.83	
10016	0.000	8	0.29	38.50		18.82T	9.84		9.84	
10016	0.200	8	0.04	5.14		5.14T				
10020	0.000	8	0.63	83.69		52.31T	7.92		23.46	
10020	0.200	8	0.60	79.28		52.30T	5.34		21.64	
10021	0.000	8	0.01	1.93		1.93T				
10021	0.400	8	0.03	3.94		2.35T			1.59	
10024	0.000	8	0.03	4.17		2.02T			2.15	
10024	0.400	8	0.02	2.70		1.80T			0.90	
10025	0.000	8	0.60	78.78		51.91T	5.45		21.42	
10025	0.200	8	0.63	83.16		51.92T	7.98		23.26	
10029	0.000	8	0.04	5.37		5.37T				
10029	0.200	8	0.30	39.69		19.28T	10.20		10.21	
12001	0.000	9	2.18	109.57			109.57T			
12001	1.000	9	1.39	69.99			69.99T			
12002	0.000	9	1.39	70.03			70.03T			
12002	1.000	9	0.66	33.11			33.11T			
12003	0.000	9	0.66	33.21			33.21T			
12003	1.000	9	0.30	15.08			15.08T			
12004	0.000	9	2.19	110.07			110.07T			
12004	1.000	9	1.38	69.47			69.47T			
12005	0.000	9	1.38	69.51			69.51T			
12005	1.000	9	0.63	31.61			31.61T			
12006	0.000	9	0.63	31.73			31.73T			
12006	1.000	9	0.30	15.08			15.08T			
12007	0.000	9	2.19	110.10			110.10T			
12007	1.000	9	1.40	70.17			70.17T			
12008	0.000	9	1.40	70.21			70.21T			
12008	1.000	9	0.66	32.94			32.94T			
12009	0.000	9	0.66	33.04			33.04T			
12009	1.000	9	0.30	15.08			15.08T			
12010	0.000	9	2.09	105.05			105.05T			
12010	1.000	9	1.33	66.94			66.94T			
12011	0.000	9	1.33	66.97			66.97T			
12011	1.000	9	0.63	31.62			31.62T			
12012	0.000	9	0.63	31.70			31.70T			
12012	1.000	9	0.30	15.08			15.08T			
12013	0.000	9	2.10	105.40			105.40T			
12013	1.000	9	1.32	66.31			66.31T			
12014	0.000	9	1.32	66.35			66.35T			
12014	1.000	9	0.60	30.05			30.05T			
12015	0.000	9	0.60	30.16			30.16T			
12015	1.000	9	0.30	15.08			15.08T			
12016	0.000	9	2.10	105.57			105.57T			
12016	1.000	9	1.34	67.12			67.12T			
12017	0.000	9	1.34	67.15			67.15T			
12017	1.000	9	0.63	31.45			31.45T			
12018	0.000	9	0.63	31.54			31.54T			
12018	1.000	9	0.30	15.08			15.08T			
12019	0.000	9	0.30	15.08			15.08T			
12019	1.000	9	0.38	19.09			19.09T			
12020	0.000	9	0.38	19.09			19.09T			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN_ΣΕΙΣΜΙΚΑ_ΠΑΣ/ΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Longitudinal Reinforcements LCR 509
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
12020	1.000	9	0.46	22.90			22.90T			
12021	0.000	9	0.46	22.90			22.90T			
12021	1.000	9	0.49	24.62			24.62T			
12022	0.000	9	0.49	24.62			24.62T			
12022	1.000	9	0.45	22.50			22.50T			
12023	0.000	9	0.45	22.50			22.50T			
12023	1.000	9	0.36	18.29			18.29T			
12024	0.000	9	0.36	18.29			18.29T			
12024	1.000	9	0.30	15.08			15.08T			
12025	0.000	9	0.30	15.08			15.08T			
12025	1.000	9	0.30	15.08			15.08T			
12026	0.000	9	0.30	15.08			15.08T			
12026	1.000	9	0.30	15.08			15.08T			
12027	0.000	9	0.30	15.08			15.08T			
12027	1.000	9	0.30	15.08			15.08T			
12028	0.000	9	0.30	15.08			15.08T			
12028	1.000	9	0.30	15.08			15.08T			
12029	0.000	9	0.30	15.08			15.08T			
12029	1.000	9	0.30	15.08			15.08T			
12030	0.000	9	0.30	15.08			15.08T			
12030	1.000	9	0.30	15.08			15.08T			
12031	0.000	9	0.30	15.08			15.08T			
12031	1.000	9	0.30	15.08			15.08T			
12032	0.000	9	0.30	15.08			15.08T			
12032	1.000	9	0.30	15.08			15.08T			
12033	0.000	9	0.30	15.08			15.08T			
12033	1.000	9	0.30	15.08			15.08T			
12034	0.000	9	0.30	15.08			15.08T			
12034	1.000	9	0.30	15.08			15.08T			
12035	0.000	9	0.30	15.08			15.08T			
12035	1.000	9	0.30	15.08			15.08T			
12036	0.000	9	0.30	15.08			15.08T			
12036	1.000	9	0.30	15.08			15.08T			
12037	0.000	9	0.30	15.08			15.08T			
12037	1.000	9	0.30	15.08			15.08T			
12038	0.000	9	0.30	15.08			15.08T			
12038	1.000	9	0.30	15.08			15.08T			
12039	0.000	9	0.30	15.08			15.08T			
12039	1.000	9	0.30	15.08			15.08T			
12040	0.000	9	0.30	15.08			15.08'			
12040	1.000	9	0.30	15.08			15.08'			
12041	0.000	9	0.30	15.08			15.08T			
12041	1.000	9	0.30	15.08			15.08T			
12042	0.000	9	0.30	15.08			15.08T			
12042	1.000	9	0.39	19.85			19.85T			
12043	0.000	9	0.39	19.85			19.85T			
12043	1.000	9	0.43	21.55			21.55T			
12044	0.000	9	0.43	21.55			21.55T			
12044	1.000	9	0.39	19.40			19.40T			
12045	0.000	9	0.39	19.40			19.40T			
12045	1.000	9	0.30	15.16			15.16T			
12046	0.000	9	0.30	15.16			15.16T			
12046	1.000	9	0.30	15.08			15.08T			
12047	0.000	9	0.30	15.08			15.08T			
12047	1.000	9	0.30	15.08			15.08T			
12048	0.000	9	0.30	15.08			15.08T			
12048	1.000	9	0.30	15.08			15.08T			
12049	0.000	9	0.30	15.08			15.08T			
12049	1.000	9	0.30	15.08			15.08T			
12050	0.000	9	0.30	15.08			15.08T			
12050	1.000	9	0.30	15.08			15.08T			
12051	0.000	9	0.30	15.08			15.08T			
12051	1.000	9	0.30	15.08			15.08T			
12052	0.000	9	0.30	15.08			15.08T			
12052	1.000	9	0.30	15.08			15.08T			
12053	0.000	9	0.30	15.08			15.08T			
12053	1.000	9	0.30	15.08			15.08T			
12054	0.000	9	0.30	15.08			15.08T			
12054	1.000	9	0.30	15.08			15.08T			
12055	0.000	9	0.30	15.08			15.08T			
12055	1.000	9	0.30	15.08			15.08T			
12056	0.000	9	0.30	15.08			15.08T			
12056	1.000	9	0.30	15.08			15.08T			
12057	0.000	9	0.30	15.08			15.08T			
12057	1.000	9	0.30	15.08			15.08T			
12058	0.000	9	0.30	15.08			15.08T			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN_ΣΕΙΣΜΙΚΑ_ΠΑΣ/ΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Longitudinal Reinforcements LCR 509
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
12058	1.000	9	0.30	15.08			15.08T			
12059	0.000	9	0.30	15.08			15.08T			
12059	1.000	9	0.30	15.08			15.08T			
12060	0.000	9	0.30	15.08			15.08T			
12060	1.000	9	0.30	15.08			15.08T			
12061	0.000	9	0.30	15.08			15.08T			
12061	1.000	9	0.30	15.08			15.08T			
12062	0.000	9	0.30	15.08			15.08'			
12062	1.000	9	0.30	15.08			15.08'			
12063	0.000	9	0.30	15.08			15.08T			
12063	1.000	9	0.36	17.97			17.97T			
12064	0.000	9	0.36	17.97			17.97T			
12064	1.000	9	0.44	22.16			22.16T			
12065	0.000	9	0.44	22.16			22.16T			
12065	1.000	9	0.48	23.91			23.91T			
12066	0.000	9	0.48	23.91			23.91T			
12066	1.000	9	0.43	21.82			21.82T			
12067	0.000	9	0.43	21.82			21.82T			
12067	1.000	9	0.35	17.61			17.61T			
12068	0.000	9	0.35	17.61			17.61T			
12068	1.000	9	0.30	15.08			15.08T			
12069	0.000	9	0.30	15.08			15.08T			
12069	1.000	9	0.30	15.08			15.08T			
12070	0.000	9	0.30	15.08			15.08T			
12070	1.000	9	0.30	15.08			15.08T			
12071	0.000	9	0.30	15.08			15.08T			
12071	1.000	9	0.30	15.08			15.08T			
12072	0.000	9	0.30	15.08			15.08T			
12072	1.000	9	0.30	15.08			15.08T			
12073	0.000	9	0.30	15.08			15.08T			
12073	1.000	9	0.30	15.08			15.08T			
12074	0.000	9	0.30	15.08			15.08T			
12074	1.000	9	0.30	15.08			15.08T			
12075	0.000	9	0.30	15.08			15.08T			
12075	1.000	9	0.30	15.08			15.08T			
12076	0.000	9	0.30	15.08			15.08T			
12076	1.000	9	0.30	15.08			15.08T			
12077	0.000	9	0.30	15.08			15.08T			
12077	1.000	9	0.30	15.08			15.08T			
12078	0.000	9	0.30	15.08			15.08T			
12078	1.000	9	0.30	15.08			15.08T			
12079	0.000	9	0.30	15.08			15.08T			
12079	1.000	9	0.30	15.08			15.08T			
12080	0.000	9	0.30	15.08			15.08T			
12080	1.000	9	0.30	15.08			15.08T			
12081	0.000	9	0.30	15.08			15.08T			
12081	1.000	9	0.30	15.08			15.08T			
12082	0.000	9	0.30	15.08			15.08T			
12082	1.000	9	0.30	15.08			15.08T			
12083	0.000	9	0.30	15.08			15.08T			
12083	1.000	9	0.30	15.08			15.08T			
12084	0.000	9	0.30	15.08			15.08'			
12084	1.000	9	0.30	15.08			15.08'			
12085	0.000	9	0.30	15.08			15.08T			
12085	1.000	9	0.36	17.97			17.97T			
12086	0.000	9	0.36	17.97			17.97T			
12086	1.000	9	0.48	24.26			24.26T			
12087	0.000	9	0.48	24.26			24.26T			
12087	1.000	9	0.52	26.18			26.18T			
12088	0.000	9	0.52	26.18			26.18T			
12088	1.000	9	0.48	24.01			24.01T			
12089	0.000	9	0.48	24.01			24.01T			
12089	1.000	9	0.39	19.60			19.60T			
12090	0.000	9	0.39	19.60			19.60T			
12090	1.000	9	0.30	15.08			15.08T			
12091	0.000	9	0.30	15.08			15.08T			
12091	1.000	9	0.30	15.08			15.08T			
12092	0.000	9	0.30	15.08			15.08T			
12092	1.000	9	0.30	15.08			15.08T			
12093	0.000	9	0.30	15.08			15.08T			
12093	1.000	9	0.30	15.08			15.08T			
12094	0.000	9	0.30	15.08			15.08T			
12094	1.000	9	0.30	15.08			15.08T			
12095	0.000	9	0.30	15.08			15.08T			
12095	1.000	9	0.30	15.08			15.08T			
12096	0.000	9	0.30	15.08			15.08T			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN_ΣΕΙΣΜΙΚΑ_ΠΑΣ/ΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Longitudinal Reinforcements LCR 509
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
12096	1.000	9	0.30	15.08			15.08T			
12097	0.000	9	0.30	15.08			15.08T			
12097	1.000	9	0.30	15.08			15.08T			
12098	0.000	9	0.30	15.08			15.08T			
12098	1.000	9	0.30	15.08			15.08T			
12099	0.000	9	0.30	15.08			15.08T			
12099	1.000	9	0.30	15.08			15.08T			
12100	0.000	9	0.30	15.08			15.08T			
12100	1.000	9	0.30	15.08			15.08T			
12101	0.000	9	0.30	15.08			15.08T			
12101	1.000	9	0.30	15.08			15.08T			
12102	0.000	9	0.30	15.08			15.08T			
12102	1.000	9	0.30	15.08			15.08T			
12103	0.000	9	0.30	15.08			15.08T			
12103	1.000	9	0.30	15.08			15.08T			
12104	0.000	9	0.30	15.08			15.08T			
12104	1.000	9	0.30	15.08			15.08T			
12105	0.000	9	0.30	15.08			15.08T			
12105	1.000	9	0.30	15.08			15.08T			
12106	0.000	9	0.30	15.08			15.08'			
12106	1.000	9	0.30	15.08			15.08'			
12107	0.000	9	0.30	15.08			15.08T			
12107	1.000	9	0.30	15.08			15.08T			
12108	0.000	9	0.30	15.08			15.08T			
12108	1.000	9	0.42	21.23			21.23T			
12109	0.000	9	0.42	21.23			21.23T			
12109	1.000	9	0.46	23.13			23.13T			
12110	0.000	9	0.46	23.13			23.13T			
12110	1.000	9	0.42	20.94			20.94T			
12111	0.000	9	0.42	20.94			20.94T			
12111	1.000	9	0.33	16.50			16.50T			
12112	0.000	9	0.33	16.50			16.50T			
12112	1.000	9	0.30	15.08			15.08T			
12113	0.000	9	0.30	15.08			15.08T			
12113	1.000	9	0.30	15.08			15.08T			
12114	0.000	9	0.30	15.08			15.08T			
12114	1.000	9	0.30	15.08			15.08T			
12115	0.000	9	0.30	15.08			15.08T			
12115	1.000	9	0.30	15.08			15.08T			
12116	0.000	9	0.30	15.08			15.08T			
12116	1.000	9	0.30	15.08			15.08T			
12117	0.000	9	0.30	15.08			15.08T			
12117	1.000	9	0.30	15.08			15.08T			
12118	0.000	9	0.30	15.08			15.08T			
12118	1.000	9	0.30	15.08			15.08T			
12119	0.000	9	0.30	15.08			15.08T			
12119	1.000	9	0.30	15.08			15.08T			
12120	0.000	9	0.30	15.08			15.08T			
12120	1.000	9	0.30	15.08			15.08T			
12121	0.000	9	0.30	15.08			15.08T			
12121	1.000	9	0.30	15.08			15.08T			
12122	0.000	9	0.30	15.08			15.08T			
12122	1.000	9	0.30	15.08			15.08T			
12123	0.000	9	0.30	15.08			15.08T			
12123	1.000	9	0.30	15.08			15.08T			
12124	0.000	9	0.30	15.08			15.08T			
12124	1.000	9	0.30	15.08			15.08T			
12125	0.000	9	0.30	15.08			15.08T			
12125	1.000	9	0.30	15.08			15.08T			
12126	0.000	9	0.30	15.08			15.08T			
12126	1.000	9	0.30	15.08			15.08T			
12127	0.000	9	0.30	15.08			15.08T			
12127	1.000	9	0.30	15.08			15.08T			
12128	0.000	9	0.30	15.08			15.08'			
12128	1.000	9	0.30	15.08			15.08'			
12129	0.000	9	0.30	15.08			15.08T			
12129	1.000	9	0.37	18.52			18.52T			
12130	0.000	9	0.37	18.52			18.52T			
12130	1.000	9	0.47	23.53			23.53T			
12131	0.000	9	0.47	23.53			23.53T			
12131	1.000	9	0.51	25.48			25.48T			
12132	0.000	9	0.51	25.48			25.48T			
12132	1.000	9	0.46	23.34			23.34T			
12133	0.000	9	0.46	23.34			23.34T			
12133	1.000	9	0.38	18.93			18.93T			
12134	0.000	9	0.38	18.93			18.93T			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN_ΣΕΙΣΜΙΚΑ_ΠΑΣ/ΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Longitudinal Reinforcements LCR 509
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
12134	1.000	9	0.30	15.08			15.08T			
12135	0.000	9	0.30	15.08			15.08T			
12135	1.000	9	0.30	15.08			15.08T			
12136	0.000	9	0.30	15.08			15.08T			
12136	1.000	9	0.30	15.08			15.08T			
12137	0.000	9	0.30	15.08			15.08T			
12137	1.000	9	0.30	15.08			15.08T			
12138	0.000	9	0.30	15.08			15.08T			
12138	1.000	9	0.30	15.08			15.08T			
12139	0.000	9	0.30	15.08			15.08T			
12139	1.000	9	0.30	15.08			15.08T			
12140	0.000	9	0.30	15.08			15.08T			
12140	1.000	9	0.30	15.08			15.08T			
12141	0.000	9	0.30	15.08			15.08T			
12141	1.000	9	0.30	15.08			15.08T			
12142	0.000	9	0.30	15.08			15.08T			
12142	1.000	9	0.30	15.08			15.08T			
12143	0.000	9	0.30	15.08			15.08T			
12143	1.000	9	0.30	15.08			15.08T			
12144	0.000	9	0.30	15.08			15.08T			
12144	1.000	9	0.30	15.08			15.08T			
12145	0.000	9	0.30	15.08			15.08T			
12145	1.000	9	0.30	15.08			15.08T			
12146	0.000	9	0.30	15.08			15.08T			
12146	1.000	9	0.30	15.08			15.08T			
12147	0.000	9	0.30	15.08			15.08T			
12147	1.000	9	0.30	15.08			15.08T			
12148	0.000	9	0.30	15.08			15.08T			
12148	1.000	9	0.30	15.08			15.08T			
12149	0.000	9	0.30	15.08			15.08T			
12149	1.000	9	0.30	15.08			15.08T			
12150	0.000	9	0.30	15.08			15.08'			
12150	1.000	9	0.30	15.08			15.08'			

Shear Reinforcements per Cutted Part of Section LCR 509

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
10001	0.000	8	2.25	9.82				
10001	0.200	8	2.25	9.92				
10005	0.000	8	10.15	11.49				
10005	0.200	8	10.15	11.59				
10006	0.000	8	0.85	5.90				
10006	0.400	8	0.85	6.10				
10009	0.000	8	0.73	6.09				
10009	0.400	8	0.73	5.89				
10010	0.000	8	10.07	11.65				
10010	0.200	8	10.07	11.57				
10014	0.000	8	2.27	10.01				
10014	0.200	8	2.27	9.92				
10016	0.000	8	2.18	9.82				
10016	0.200	8	2.18	9.92				
10020	0.000	8	9.83	11.57				
10020	0.200	8	9.83	11.65				
10021	0.000	8	0.83	2.86				
10021	0.400	8	0.83	6.12				
10024	0.000	8	0.71	6.11				
10024	0.400	8	0.71	5.91				
10025	0.000	8	9.75	11.48				
10025	0.200	8	9.75	11.41				
10029	0.000	8	2.20	10.09				
10029	0.200	8	2.20	10.00				
12001	0.000	9	0.00	10.18				
12001	1.000	9	0.00	10.53				
12002	0.000	9	0.00	10.52				
12002	1.000	9	0.00	10.64				
12003	0.000	9	0.00	10.64				
12003	1.000	9	0.00	10.04				
12004	0.000	9	0.00	10.27				
12004	1.000	9	0.00	10.61				
12005	0.000	9	0.00	10.61				
12005	1.000	9	0.00	10.72				
12006	0.000	9	0.00	10.72				
12006	1.000	9	0.00	9.21				
12007	0.000	9	0.00	10.23				
12007	1.000	9	0.00	10.57				
12008	0.000	9	0.00	10.57				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN_ΣΕΙΣΜΙΚΑ_ΠΑΣ/ΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Shear Reinforcements per Cutted Part of Section LCR 509

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
12008	1.000	9	0.00	10.69				
12009	0.000	9	0.00	10.69				
12009	1.000	9	0.00	9.72				
12010	0.000	9	0.00	9.91				
12010	1.000	9	0.00	10.19				
12011	0.000	9	0.00	10.19				
12011	1.000	9	0.00	10.26				
12012	0.000	9	0.00	10.26				
12012	1.000	9	0.00	10.63				
12013	0.000	9	0.00	9.99				
12013	1.000	9	0.00	10.27				
12014	0.000	9	0.00	10.27				
12014	1.000	9	0.00	10.33				
12015	0.000	9	0.00	10.33				
12015	1.000	9	0.00	8.81				
12016	0.000	9	0.00	9.96				
12016	1.000	9	0.00	10.24				
12017	0.000	9	0.00	10.24				
12017	1.000	9	0.00	10.31				
12018	0.000	9	0.00	10.30				
12018	1.000	9	0.00	9.32				
12019	0.000	9	0.00	10.04				
12019	1.000	9	0.00	5.49				
12020	0.000	9	0.00	5.49				
12020	1.000	9	0.00	5.28				
12021	0.000	9	0.00	5.28				
12021	1.000	9	0.00	5.27				
12022	0.000	9	0.00	5.27				
12022	1.000	9	0.00	5.31				
12023	0.000	9	0.00	5.31				
12023	1.000	9	0.00	5.37				
12024	0.000	9	0.00	5.37				
12024	1.000	9	0.00	5.49				
12025	0.000	9	0.00	5.49				
12025	1.000	9	0.00	5.61				
12026	0.000	9	0.00	5.61				
12026	1.000	9	0.00	5.69				
12027	0.000	9	0.00	5.69				
12027	1.000	9	0.00	5.74				
12028	0.000	9	0.00	5.74				
12028	1.000	9	0.00	5.74				
12029	0.000	9	0.00	5.74				
12029	1.000	9	0.00	5.74				
12030	0.000	9	0.00	5.74				
12030	1.000	9	0.00	5.74				
12031	0.000	9	0.00	5.74				
12031	1.000	9	0.00	5.74				
12032	0.000	9	0.00	5.74				
12032	1.000	9	0.00	5.74				
12033	0.000	9	0.00	5.74				
12033	1.000	9	0.00	5.74				
12034	0.000	9	0.00	5.74				
12034	1.000	9	0.00	5.74				
12035	0.000	9	0.00	5.74				
12035	1.000	9	0.00	5.74				
12036	0.000	9	0.00	5.74				
12036	1.000	9	0.00	5.74				
12037	0.000	9	0.00	5.74				
12037	1.000	9	0.00	5.74				
12038	0.000	9	0.00	5.74				
12038	1.000	9	0.00	5.74				
12039	0.000	9	0.00	5.74				
12039	1.000	9	0.00	4.58				
12040	0.000	9	0.00	4.58				
12040	1.000	9	0.00	4.58				
12041	0.000	9	0.00	9.23				
12041	1.000	9	0.00	5.22				
12042	0.000	9	0.00	5.22				
12042	1.000	9	0.00	5.16				
12043	0.000	9	0.00	5.16				
12043	1.000	9	0.00	5.14				
12044	0.000	9	0.00	5.14				
12044	1.000	9	0.00	5.16				
12045	0.000	9	0.00	5.16				
12045	1.000	9	0.00	5.21				
12046	0.000	9	0.00	5.21				
12046	1.000	9	0.00	5.17				
12047	0.000	9	0.00	5.17				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN_ΣΕΙΣΜΙΚΑ_ΠΑΣ/ΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Shear Reinforcements per Cutted Part of Section LCR 509

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
12047	1.000	9	0.00	5.11				
12048	0.000	9	0.00	5.11				
12048	1.000	9	0.00	4.97				
12049	0.000	9	0.00	4.97				
12049	1.000	9	0.00	4.74				
12050	0.000	9	0.00	4.74				
12050	1.000	9	0.00	4.62				
12051	0.000	9	0.00	4.62				
12051	1.000	9	0.00	4.58				
12052	0.000	9	0.00	4.58				
12052	1.000	9	0.00	4.58				
12053	0.000	9	0.00	4.58				
12053	1.000	9	0.00	4.58				
12054	0.000	9	0.00	4.58				
12054	1.000	9	0.00	4.58				
12055	0.000	9	0.00	4.58				
12055	1.000	9	0.00	4.58				
12056	0.000	9	0.00	4.58				
12056	1.000	9	0.00	4.58				
12057	0.000	9	0.00	4.58				
12057	1.000	9	0.00	4.58				
12058	0.000	9	0.00	4.58				
12058	1.000	9	0.00	4.58				
12059	0.000	9	0.00	4.58				
12059	1.000	9	0.00	4.58				
12060	0.000	9	0.00	4.58				
12060	1.000	9	0.00	4.58				
12061	0.000	9	0.00	4.58				
12061	1.000	9	0.00	4.58				
12062	0.000	9	0.00	4.58				
12062	1.000	9	0.00	4.58				
12063	0.000	9	0.00	9.74				
12063	1.000	9	0.00	5.41				
12064	0.000	9	0.00	5.41				
12064	1.000	9	0.00	5.29				
12065	0.000	9	0.00	5.29				
12065	1.000	9	0.00	5.28				
12066	0.000	9	0.00	5.28				
12066	1.000	9	0.00	5.31				
12067	0.000	9	0.00	5.31				
12067	1.000	9	0.00	5.37				
12068	0.000	9	0.00	5.37				
12068	1.000	9	0.00	5.49				
12069	0.000	9	0.00	5.49				
12069	1.000	9	0.00	5.61				
12070	0.000	9	0.00	5.61				
12070	1.000	9	0.00	5.69				
12071	0.000	9	0.00	5.69				
12071	1.000	9	0.00	5.74				
12072	0.000	9	0.00	5.74				
12072	1.000	9	0.00	5.74				
12073	0.000	9	0.00	5.74				
12073	1.000	9	0.00	5.74				
12074	0.000	9	0.00	5.74				
12074	1.000	9	0.00	5.74				
12075	0.000	9	0.00	5.74				
12075	1.000	9	0.00	5.74				
12076	0.000	9	0.00	5.74				
12076	1.000	9	0.00	5.74				
12077	0.000	9	0.00	5.74				
12077	1.000	9	0.00	5.74				
12078	0.000	9	0.00	5.74				
12078	1.000	9	0.00	5.74				
12079	0.000	9	0.00	5.74				
12079	1.000	9	0.00	5.74				
12080	0.000	9	0.00	5.74				
12080	1.000	9	0.00	5.74				
12081	0.000	9	0.00	5.74				
12081	1.000	9	0.00	4.64				
12082	0.000	9	0.00	4.64				
12082	1.000	9	0.00	4.58				
12083	0.000	9	0.00	4.58				
12083	1.000	9	0.00	4.58				
12084	0.000	9	0.00	4.58				
12084	1.000	9	0.00	4.58				
12085	0.000	9	0.00	10.63				
12085	1.000	9	0.00	5.55				
12086	0.000	9	0.00	5.55				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN_ΣΕΙΣΜΙΚΑ_ΠΑΣ/ΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Shear Reinforcements per Cutted Part of Section LCR 509

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
12086	1.000	9	0.00	5.29				
12087	0.000	9	0.00	5.29				
12087	1.000	9	0.00	5.28				
12088	0.000	9	0.00	5.28				
12088	1.000	9	0.00	5.29				
12089	0.000	9	0.00	5.29				
12089	1.000	9	0.00	5.35				
12090	0.000	9	0.00	5.35				
12090	1.000	9	0.00	5.46				
12091	0.000	9	0.00	5.46				
12091	1.000	9	0.00	5.60				
12092	0.000	9	0.00	5.60				
12092	1.000	9	0.00	5.68				
12093	0.000	9	0.00	5.68				
12093	1.000	9	0.00	5.73				
12094	0.000	9	0.00	5.73				
12094	1.000	9	0.00	5.74				
12095	0.000	9	0.00	5.74				
12095	1.000	9	0.00	5.74				
12096	0.000	9	0.00	5.74				
12096	1.000	9	0.00	5.74				
12097	0.000	9	0.00	5.74				
12097	1.000	9	0.00	5.09				
12098	0.000	9	0.00	5.09				
12098	1.000	9	0.00	4.94				
12099	0.000	9	0.00	4.94				
12099	1.000	9	0.00	4.76				
12100	0.000	9	0.00	4.76				
12100	1.000	9	0.00	4.66				
12101	0.000	9	0.00	4.66				
12101	1.000	9	0.00	4.61				
12102	0.000	9	0.00	4.61				
12102	1.000	9	0.00	4.58				
12103	0.000	9	0.00	4.58				
12103	1.000	9	0.00	4.58				
12104	0.000	9	0.00	4.58				
12104	1.000	9	0.00	4.58				
12105	0.000	9	0.00	4.58				
12105	1.000	9	0.00	4.58				
12106	0.000	9	0.00	4.58				
12106	1.000	9	0.00	4.58				
12107	0.000	9	0.00	8.83				
12107	1.000	9	0.00	5.22				
12108	0.000	9	0.00	5.22				
12108	1.000	9	0.00	5.16				
12109	0.000	9	0.00	5.16				
12109	1.000	9	0.00	5.15				
12110	0.000	9	0.00	5.15				
12110	1.000	9	0.00	5.15				
12111	0.000	9	0.00	5.15				
12111	1.000	9	0.00	5.19				
12112	0.000	9	0.00	5.19				
12112	1.000	9	0.00	5.18				
12113	0.000	9	0.00	5.18				
12113	1.000	9	0.00	5.13				
12114	0.000	9	0.00	5.13				
12114	1.000	9	0.00	5.01				
12115	0.000	9	0.00	5.01				
12115	1.000	9	0.00	4.78				
12116	0.000	9	0.00	4.78				
12116	1.000	9	0.00	4.63				
12117	0.000	9	0.00	4.63				
12117	1.000	9	0.00	4.58				
12118	0.000	9	0.00	4.58				
12118	1.000	9	0.00	4.58				
12119	0.000	9	0.00	4.58				
12119	1.000	9	0.00	4.58				
12120	0.000	9	0.00	4.58				
12120	1.000	9	0.00	4.58				
12121	0.000	9	0.00	4.58				
12121	1.000	9	0.00	4.58				
12122	0.000	9	0.00	4.58				
12122	1.000	9	0.00	4.58				
12123	0.000	9	0.00	4.58				
12123	1.000	9	0.00	4.58				
12124	0.000	9	0.00	4.58				
12124	1.000	9	0.00	4.58				
12125	0.000	9	0.00	4.58				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN_ΣΕΙΣΜΙΚΑ_ΠΑΣ/ΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Shear Reinforcements per Cutted Part of Section LCR 509

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
12125	1.000	9	0.00	4.58				
12126	0.000	9	0.00	4.58				
12126	1.000	9	0.00	4.58				
12127	0.000	9	0.00	4.58				
12127	1.000	9	0.00	4.58				
12128	0.000	9	0.00	4.58				
12128	1.000	9	0.00	4.58				
12129	0.000	9	0.00	9.33				
12129	1.000	9	0.00	5.39				
12130	0.000	9	0.00	5.39				
12130	1.000	9	0.00	5.30				
12131	0.000	9	0.00	5.30				
12131	1.000	9	0.00	5.28				
12132	0.000	9	0.00	5.28				
12132	1.000	9	0.00	5.30				
12133	0.000	9	0.00	5.30				
12133	1.000	9	0.00	5.36				
12134	0.000	9	0.00	5.36				
12134	1.000	9	0.00	5.48				
12135	0.000	9	0.00	5.48				
12135	1.000	9	0.00	5.60				
12136	0.000	9	0.00	5.60				
12136	1.000	9	0.00	5.68				
12137	0.000	9	0.00	5.68				
12137	1.000	9	0.00	5.73				
12138	0.000	9	0.00	5.73				
12138	1.000	9	0.00	5.74				
12139	0.000	9	0.00	5.74				
12139	1.000	9	0.00	5.74				
12140	0.000	9	0.00	5.74				
12140	1.000	9	0.00	5.74				
12141	0.000	9	0.00	5.74				
12141	1.000	9	0.00	5.74				
12142	0.000	9	0.00	5.74				
12142	1.000	9	0.00	5.74				
12143	0.000	9	0.00	5.74				
12143	1.000	9	0.00	5.74				
12144	0.000	9	0.00	5.74				
12144	1.000	9	0.00	5.74				
12145	0.000	9	0.00	5.74				
12145	1.000	9	0.00	5.74				
12146	0.000	9	0.00	5.74				
12146	1.000	9	0.00	5.74				
12147	0.000	9	0.00	5.74				
12147	1.000	9	0.00	5.74				
12148	0.000	9	0.00	5.74				
12148	1.000	9	0.00	4.58				
12149	0.000	9	0.00	4.58				
12149	1.000	9	0.00	4.58				
12150	0.000	9	0.00	4.58				
12150	1.000	9	0.00	4.58				

Maximum Degree of Utilization

		N sig-c	Vy sig-t	Vz tau	Mt sig-*	My tend.	Mz As-l	Mb As-v	Mt2 crack	Total sigdyn	lamda tau-*
Cross sect.	8	0.000	0.000	0.120	0.429	0.000	0.000	0.000	0.000	1.000	0.000
DOKOS-4		0.000	0.000	0.000	0.000	0.000	1.000	0.000	0.000	0.000	0.000
Cross sect.	9	0.000	0.000	0.291	0.000	0.000	0.000	0.000	0.000	1.002	0.000
section pile		0.000	0.000	0.000	0.000	0.000	1.000	0.000	0.000	0.000	0.000
Total System		0.000	0.000	0.291	0.429	0.000	0.000	0.000	0.000	1.002	0.000
		0.000	0.000	0.000	0.000	0.000	1.000	0.000	0.000	0.000	0.000

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN_ΣΕΙΣΜΙΚΑ_ΔΟΚΟΙ

Selected Beam Elements

FROM	TO	INC	X-VALUE	NC	MEMBER	CS0	CS1	CS2	CS3	CS4	CS5
1000	1060	1		1	bending	10	40				
2000	2020	1									

Default design code is DIN Fachbericht 102 Massivbrȯcken (2003) (Germany)
Klasse(Tab.4.118): D

Materials

No. 1 C 25/30 (DIN 1045-1)
No. 3 C 25/30 (DIN 1045-1)
No. 4 C 25/30 (DIN 1045-1)
No. 5 C 25/30 (DIN 1045-1)
No. 6 C 25/30 (DIN 1045-1)
No. 7 C 25/30 (DIN 1045-1)
No. 8 C 25/30 (DIN 1045-1)
No. 9 C 25/30 (DIN 1045-1)
No. 10 C 25/30 (DIN 1045-1)
No. 12 BSt 500 SA (DIN 1045-1)

Reinforcement will be accounted for sectional values as defined in AQUA
Reinforcements saved as design case LCR 510

Considered Load Cases

8101	8102	8103	8104	8105	8106
8107	8108	8201	8202	8203	8204
8205	8206	8207	8301	8302	8303
8304	8305	8306	8307	8308	

Ultimate Load Design

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Design for ultimate loads DIN Fachbericht 102 Massivbrȯcken (2003)
Uniaxial bending due to symmetry

Safety factors	SC-1	SC-2	SC-S	SS-1	SS-2	PIIa
	1.50	1.50	1.50	1.15	1.15	7
Strain limits	C1	C2	S1	S2	Z1	Z2
max	-3.50	-2.00	3.00	25.00	-3.50	25.00

parameters for reinforcements

Minimum reinforcements		compression		min. reinforcem.	maximum-
Bending.	Compress.	e/d	N/Npl	requ. section	reinforc.
0.00 [cm2]	0.30 [o/o]	3.50	0.0010	0.00	0.15	9.00

Tensile forces in the longitudinal reinforcements due to shear are NOT accounted for.
Material of sections uses Ultimate Limit strain-stress law with global safety factors
Material of reinforcements uses Ultimate Limit strain-stress law with global safety factors

MNo.	temp lev.	Material-safety [-]	max.compr stress [MPa]	at strain [o/oo]	max.tens stress [MPa]	at strain [o/oo]	tension-stiffening [MPa]
1	0	1.500	-14.17	-2.00	0.00	0.00	
3	0	1.500	-14.17	-2.00	0.00	0.00	
4	0	1.500	-14.17	-2.00	0.00	0.00	
5	0	1.500	-14.17	-2.00	0.00	0.00	
6	0	1.500	-14.17	-2.00	0.00	0.00	
7	0	1.500	-14.17	-2.00	0.00	0.00	
8	0	1.500	-14.17	-2.00	0.00	0.00	
9	0	1.500	-14.17	-2.00	0.00	0.00	
10	0	1.500	-14.17	-2.00	0.00	0.00	
12	0	1.150	-456.52	-25.00	456.52	25.00	

Shear Design

==========

Design for shear DIN 1045-1 (2003)
Minimum shear factor or tan of inclination of compressive struts 0.57 / 1.72

MNo	f-cd [MPa]	tau-rd [MPa]	sigIIQ [MPa]	sigIIT [MPa]	sigIIQ+ [MPa]	fyd [MPa]
1	14.17	0.10	10.62	7.44	10.62	
3	14.17	0.10	10.62	7.44	10.62	
4	14.17	0.10	10.62	7.44	10.62	
5	14.17	0.10	10.62	7.44	10.62	
6	14.17	0.10	10.62	7.44	10.62	
7	14.17	0.10	10.62	7.44	10.62	
8	14.17	0.10	10.62	7.44	10.62	
9	14.17	0.10	10.62	7.44	10.62	
10	14.17	0.10	10.62	7.44	10.62	
12						434.78

Tolerance for exceeding maximum shear or principal compression stress 0.0200

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN_ΣΕΙΣΜΙΚΑ_ΔΟΚΟΙ

Longitudinal Reinforcements LCR 510
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
1001	0.000	1	0.50	24.88		2.37T	3.14		19.37	
1001	0.883	1	0.42	21.14		2.37T	3.48		15.29	
1002	0.000	1	0.43	21.19		2.37T	3.45		15.36	
1002	0.883	1	0.36	17.76		2.37T	3.59		11.79	
1003	0.000	1	0.36	17.82		2.37T	3.58		11.87	
1003	0.883	1	0.29	14.24		2.37T	3.25		8.62	
1004	0.000	1	0.29	14.30		2.37T	3.23		8.70	
1004	0.883	1	0.22	10.77		2.37T	2.42	0.12	5.79	0.04
						0.03				
1005	0.000	1	0.22	10.82		2.37T	2.40	0.12	5.86	0.04
						0.03				
1005	0.883	1	0.15	7.68		2.37T	1.11	0.05	3.98	0.09
						0.09				
1006	0.000	1	0.15	7.68		2.37T	1.09	0.04	4.00	0.09
						0.09				
1006	0.883	1	0.13	6.44		2.37T	0.80		3.27	
1007	0.000	1	0.13	6.44		2.37T	0.80		3.27	
1007	0.883	1	0.15	7.44		2.37T	1.17		3.90	
1008	0.000	1	0.15	7.62		2.37T	1.18		3.88	0.05
						0.14				
1008	0.883	1	0.21	10.21		2.37T	2.19	0.03	5.38	0.11
						0.12				
1009	0.000	1	0.20	10.16		2.37T	2.21	0.04	5.31	0.11
						0.12				
1009	0.883	1	0.28	13.73		2.37T	3.16		8.20	
1010	0.000	1	0.27	13.68		2.37T	3.18		8.13	
1010	0.883	1	0.35	17.44		2.37T	3.68		11.38	
1011	0.000	1	0.35	17.38		2.37T	3.70		11.31	
1011	0.883	1	0.42	21.03		2.37T	3.74		14.91	
1012	0.000	1	0.42	20.99		2.37T	3.77		14.84	
1012	0.883	1	0.50	24.78		2.37T	3.72		18.69	
1013	0.000	1	0.49	24.60		2.37T	2.72		19.51	
1013	0.883	1	0.41	20.46		2.37T	3.08		15.00	
1014	0.000	1	0.41	20.51		2.37T	3.06		15.07	
1014	0.883	1	0.34	16.88		2.37T	3.20		11.31	
1015	0.000	1	0.34	16.94		2.37T	3.18		11.38	
1015	0.883	1	0.27	13.27		2.37T	2.85		8.05	
1016	0.000	1	0.27	13.33		2.37T	2.83		8.12	
1016	0.883	1	0.19	9.59		2.37T	2.02		5.19	
1017	0.000	1	0.19	9.65		2.37T	2.01		5.26	
1017	0.883	1	0.13	6.49		2.37T	0.72		3.39	
1018	0.000	1	0.13	6.50		2.37T	0.71		3.42	
1018	0.883	1	0.11	5.46		2.37T	0.40		2.69	
1019	0.000	1	0.11	5.46		2.37T	0.40		2.69	
1019	0.883	1	0.13	6.46		2.37T	0.77		3.31	
1020	0.000	1	0.13	6.44		2.37T	0.78		3.29	
1020	0.883	1	0.18	8.96		2.37T	1.80		4.78	
1021	0.000	1	0.18	8.90		2.37T	1.82		4.71	
1021	0.883	1	0.26	12.77		2.37T	2.77		7.63	
1022	0.000	1	0.26	12.72		2.37T	2.78		7.56	
1022	0.883	1	0.33	16.57		2.37T	3.29		10.91	
1023	0.000	1	0.33	16.51		2.37T	3.31		10.83	
1023	0.883	1	0.41	20.38		2.37T	3.36		14.64	
1024	0.000	1	0.41	20.33		2.37T	3.39		14.57	
1024	0.883	1	0.49	24.49		2.37T	3.30		18.81	
1025	0.000	1	0.50	24.76		2.37T	2.43		19.96	
1025	0.883	1	0.41	20.25		2.37T	2.76		15.12	
1026	0.000	1	0.41	20.29		2.37T	2.73		15.18	
1026	0.883	1	0.33	16.46		2.37T	2.92		11.17	
1027	0.000	1	0.33	16.52		2.37T	2.90		11.25	
1027	0.883	1	0.26	12.76		2.37T	2.62		7.77	
1028	0.000	1	0.26	12.81		2.37T	2.60		7.84	
1028	0.883	1	0.18	9.04		2.37T	1.83		4.84	
1029	0.000	1	0.18	9.10		2.37T	1.82		4.91	
1029	0.883	1	0.12	5.98		2.37T	0.56		3.05	
1030	0.000	1	0.12	6.00		2.37T	0.55		3.07	
1030	0.883	1	0.10	4.98		2.37T	0.27		2.34	
1031	0.000	1	0.10	4.98		2.37T	0.27		2.34	
1031	0.883	1	0.12	5.97		2.37T	0.63		2.97	
1032	0.000	1	0.12	5.96		2.37T	0.63		2.95	
1032	0.883	1	0.17	8.42		2.37T	1.61		4.43	
1033	0.000	1	0.17	8.36		2.37T	1.63		4.36	
1033	0.883	1	0.25	12.26		2.37T	2.54		7.35	
1034	0.000	1	0.25	12.21		2.37T	2.55		7.28	
1034	0.883	1	0.32	16.16		2.37T	3.01		10.78	
1035	0.000	1	0.32	16.10		2.37T	3.03		10.70	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN_ΣΕΙΣΜΙΚΑ_ΔΟΚΟΙ

Longitudinal Reinforcements LCR 510
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
1035	0.883	1	0.41	20.17		2.37T	3.04		14.76	
1036	0.000	1	0.40	20.13		2.37T	3.07		14.69	
1036	0.883	1	0.49	24.62		2.37T	3.03		19.22	
1037	0.000	1	0.50	24.78		2.37T	2.55		19.86	
1037	0.883	1	0.41	20.50		2.37T	2.98		15.15	
1038	0.000	1	0.41	20.56		2.37T	2.95		15.23	
1038	0.883	1	0.34	16.94		2.37T	3.21		11.35	
1039	0.000	1	0.34	17.00		2.37T	3.19		11.44	
1039	0.883	1	0.27	13.34		2.37T	2.94		8.03	
1040	0.000	1	0.27	13.40		2.37T	2.92		8.11	
1040	0.883	1	0.19	9.67		2.37T	2.15		5.14	
1041	0.000	1	0.20	9.73		2.37T	2.14		5.21	
1041	0.883	1	0.13	6.67		2.37T	0.88		3.41	
1042	0.000	1	0.13	6.69		2.37T	0.88		3.44	
1042	0.883	1	0.11	5.70		2.37T	0.62		2.71	
1043	0.000	1	0.11	5.70		2.37T	0.62		2.71	
1043	0.883	1	0.13	6.69		2.37T	0.98		3.33	
1044	0.000	1	0.13	6.67		2.37T	0.98		3.31	
1044	0.883	1	0.18	9.03		2.37T	1.93		4.73	
1045	0.000	1	0.18	8.97		2.37T	1.94		4.65	
1045	0.883	1	0.26	12.84		2.37T	2.85		7.62	
1046	0.000	1	0.26	12.78		2.37T	2.87		7.54	
1046	0.883	1	0.33	16.64		2.37T	3.30		10.96	
1047	0.000	1	0.33	16.57		2.37T	3.32		10.88	
1047	0.883	1	0.41	20.44		2.37T	3.26		14.80	
1048	0.000	1	0.41	20.39		2.37T	3.29		14.72	
1048	0.883	1	0.50	24.67		2.37T	3.14		19.16	
1049	0.000	1	0.51	25.28		2.37T	2.88		20.02	
1049	0.883	1	0.43	21.25		2.37T	3.40		15.47	
1050	0.000	1	0.43	21.32		2.37T	3.37		15.58	
1050	0.883	1	0.36	17.91		2.37T	3.72		11.81	
1051	0.000	1	0.36	17.99		2.37T	3.69		11.93	
1051	0.883	1	0.29	14.41		2.37T	3.49		8.55	
1052	0.000	1	0.29	14.49		2.37T	3.46		8.66	
1052	0.883	1	0.22	11.08		2.37T	2.71	0.05	5.68	0.13
						0.14				
1053	0.000	1	0.22	11.15		2.37T	2.69	0.04	5.78	0.13
						0.13				
1053	0.883	1	0.16	7.90		2.37T	1.51		4.02	
1054	0.000	1	0.16	7.92		2.37T	1.50		4.05	
1054	0.883	1	0.14	7.15		2.37T	1.24	0.07	3.32	0.08
						0.08				
1055	0.000	1	0.14	7.15		2.37T	1.24	0.07	3.32	0.08
						0.08				
1055	0.883	1	0.16	7.92		2.37T	1.60		3.95	
1056	0.000	1	0.16	7.90		2.37T	1.61		3.92	
1056	0.883	1	0.20	10.12		2.37T	2.46		5.28	
1057	0.000	1	0.20	10.04		2.37T	2.49		5.18	
1057	0.883	1	0.28	14.08		2.37T	3.38	0.18	8.15	
1058	0.000	1	0.28	14.02		2.37T	3.41	0.19	8.04	
1058	0.883	1	0.35	17.60		2.37T	3.79		11.44	
1059	0.000	1	0.35	17.52		2.37T	3.82		11.32	
1059	0.883	1	0.43	21.19		2.37T	3.67		15.14	
1060	0.000	1	0.42	21.12		2.37T	3.71		15.04	
1060	0.883	1	0.51	25.18		2.37T	3.46		19.34	
2001	0.000	2	0.32	25.63		0.79T	3.03		21.81	
2001	0.300	2	0.31	24.61		0.79T	3.12		20.70	
2002	0.000	2	0.31	24.48		0.58T	3.12		20.78	
2002	0.300	2	0.29	23.14		0.58T	3.22		19.34	
2003	0.000	2	0.29	23.30		0.60T	3.77		18.93	
2003	0.300	2	0.31	24.70		0.60T	3.75		20.35	
2004	0.000	2	0.31	24.81		0.79T	3.75		20.27	
2004	0.300	2	0.32	25.48		0.79T	3.66		21.03	
2005	0.000	2	0.32	25.89		0.78T	2.52		22.59	
2005	0.300	2	0.31	24.59		0.78T	2.65		21.16	
2006	0.000	2	0.31	24.49		0.60T	2.65		21.24	
2006	0.300	2	0.29	22.88		0.60T	2.80		19.48	
2007	0.000	2	0.29	23.04		0.60T	3.36		19.09	
2007	0.300	2	0.31	24.70		0.59T	3.30		20.81	
2008	0.000	2	0.31	24.80		0.78T	3.30		20.72	
2008	0.300	2	0.32	25.72		0.78T	3.16		21.79	
2009	0.000	2	0.33	26.45		0.78T	2.31		23.36	
2009	0.300	2	0.31	24.93		0.78T	2.39		21.76	
2010	0.000	2	0.31	24.91		0.67T	2.39		21.85	
2010	0.300	2	0.29	23.12		0.67T	2.52		19.93	
2011	0.000	2	0.29	23.28		0.67T	3.09		19.52	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN_ΣΕΙΣΜΙΚΑ_ΔΟΚΟΙ

Longitudinal Reinforcements LCR 510
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
2011	0.300	2	0.31	25.13		0.67T	3.06		21.40	
2012	0.000	2	0.31	25.16		0.79T	3.06		21.31	
2012	0.300	2	0.33	26.27		0.78T	2.97		22.52	
2013	0.000	2	0.33	26.27		0.89T	2.29		23.09	
2013	0.300	2	0.31	24.93		0.89T	2.44		21.60	
2014	0.000	2	0.31	24.95		0.83T	2.44		21.68	
2014	0.300	2	0.29	23.28		0.84T	2.62		19.83	
2015	0.000	2	0.29	23.47		0.83T	3.19		19.45	
2015	0.300	2	0.31	25.18		0.83T	3.10		21.25	
2016	0.000	2	0.31	25.16		0.89T	3.10		21.17	
2016	0.300	2	0.33	26.10		0.89T	2.94		22.27	
2017	0.000	2	0.33	26.50		1.04T	2.57		22.89	
2017	0.300	2	0.32	25.35		1.04T	2.74		21.58	
2018	0.000	2	0.32	25.33		0.92T	2.74		21.67	
2018	0.300	2	0.30	23.85		0.93T	2.95		19.98	
2019	0.000	2	0.30	24.04		0.93T	3.51		19.60	
2019	0.300	2	0.32	25.54		0.93T	3.39		21.23	
2020	0.000	2	0.32	25.57		1.04T	3.38		21.14	
2020	0.300	2	0.33	26.34		1.04T	3.19		22.11	

Shear Reinforcements per Cutted Part of Section LCR 510

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
1001	0.000	1	0.55	3.28				
1001	0.883	1	0.55	3.09				
1002	0.000	1	0.35	3.03				
1002	0.883	1	0.35	2.85				
1003	0.000	1	0.24	2.73				
1003	0.883	1	0.24	2.54				
1004	0.000	1	0.17	2.42				
1004	0.883	1	0.17	2.23				
1005	0.000	1	0.10	2.09				
1005	0.883	1	0.10	1.90				
1006	0.000	1	0.04	1.75				
1006	0.883	1	0.04	1.56				
1007	0.000	1	0.03	1.50				
1007	0.883	1	0.03	1.70				
1008	0.000	1	0.09	1.85				
1008	0.883	1	0.09	2.06				
1009	0.000	1	0.16	2.20				
1009	0.883	1	0.16	2.40				
1010	0.000	1	0.23	2.53				
1010	0.883	1	0.23	2.74				
1011	0.000	1	0.33	2.85				
1011	0.883	1	0.33	3.05				
1012	0.000	1	0.54	3.11				
1012	0.883	1	0.54	3.36				
1013	0.000	1	0.53	3.52				
1013	0.883	1	0.53	3.33				
1014	0.000	1	0.36	3.22				
1014	0.883	1	0.36	3.03				
1015	0.000	1	0.36	2.92				
1015	0.883	1	0.36	2.73				
1016	0.000	1	0.31	2.58				
1016	0.883	1	0.31	2.39				
1017	0.000	1	0.24	2.21				
1017	0.883	1	0.24	2.02				
1018	0.000	1	0.14	1.81				
1018	0.883	1	0.14	1.62				
1019	0.000	1	0.14	1.57				
1019	0.883	1	0.14	1.77				
1020	0.000	1	0.24	1.98				
1020	0.883	1	0.24	2.18				
1021	0.000	1	0.31	2.37				
1021	0.883	1	0.31	2.57				
1022	0.000	1	0.36	2.73				
1022	0.883	1	0.36	2.94				
1023	0.000	1	0.37	3.04				
1023	0.883	1	0.37	3.24				
1024	0.000	1	0.52	3.36				
1024	0.883	1	0.52	3.64				
1025	0.000	1	0.73	3.90				
1025	0.883	1	0.73	3.71				
1026	0.000	1	0.55	3.50				
1026	0.883	1	0.55	3.31				
1027	0.000	1	0.50	3.10				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN_ΣΕΙΣΜΙΚΑ_ΔΟΚΟΙ

Shear Reinforcements per Cutted Part of Section LCR 510

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
1027	0.883	1	0.50	2.92				
1028	0.000	1	0.40	2.68				
1028	0.883	1	0.40	2.49				
1029	0.000	1	0.27	2.25				
1029	0.883	1	0.27	2.06				
1030	0.000	1	0.12	1.80				
1030	0.883	1	0.12	1.62				
1031	0.000	1	0.12	1.56				
1031	0.883	1	0.12	1.77				
1032	0.000	1	0.27	2.02				
1032	0.883	1	0.27	2.22				
1033	0.000	1	0.40	2.47				
1033	0.883	1	0.40	2.67				
1034	0.000	1	0.49	2.91				
1034	0.883	1	0.49	3.11				
1035	0.000	1	0.55	3.32				
1035	0.883	1	0.55	3.52				
1036	0.000	1	0.73	3.73				
1036	0.883	1	0.73	4.02				
1037	0.000	1	1.02	4.04				
1037	0.883	1	1.02	3.85				
1038	0.000	1	0.76	3.56				
1038	0.883	1	0.76	3.37				
1039	0.000	1	0.66	3.13				
1039	0.883	1	0.66	2.95				
1040	0.000	1	0.52	2.70				
1040	0.883	1	0.52	2.51				
1041	0.000	1	0.33	2.26				
1041	0.883	1	0.33	2.07				
1042	0.000	1	0.11	1.80				
1042	0.883	1	0.11	1.61				
1043	0.000	1	0.11	1.56				
1043	0.883	1	0.11	1.76				
1044	0.000	1	0.32	2.03				
1044	0.883	1	0.32	2.23				
1045	0.000	1	0.52	2.49				
1045	0.883	1	0.52	2.69				
1046	0.000	1	0.66	2.94				
1046	0.883	1	0.66	3.15				
1047	0.000	1	0.75	3.38				
1047	0.883	1	0.75	3.58				
1048	0.000	1	1.02	3.87				
1048	0.883	1	1.02	4.14				
1049	0.000	1	1.30	4.03				
1049	0.883	1	1.30	3.84				
1050	0.000	1	1.13	3.73				
1050	0.883	1	1.13	3.54				
1051	0.000	1	0.96	3.34				
1051	0.883	1	0.96	3.15				
1052	0.000	1	0.75	2.90				
1052	0.883	1	0.75	2.71				
1053	0.000	1	0.48	2.41				
1053	0.883	1	0.48	2.22				
1054	0.000	1	0.22	1.91				
1054	0.883	1	0.22	1.72				
1055	0.000	1	0.21	1.66				
1055	0.883	1	0.21	1.86				
1056	0.000	1	0.48	2.18				
1056	0.883	1	0.48	2.39				
1057	0.000	1	0.74	2.68				
1057	0.883	1	0.74	2.89				
1058	0.000	1	0.95	3.14				
1058	0.883	1	0.95	3.35				
1059	0.000	1	1.12	3.55				
1059	0.883	1	1.12	3.76				
1060	0.000	1	1.30	3.87				
1060	0.883	1	1.30	4.13				
2001	0.000	2	0.35	1.80				
2001	0.300	2	0.35	1.80				
2002	0.000	2	0.25	1.52				
2002	0.300	2	0.25	1.52				
2003	0.000	2	0.27	1.69				
2003	0.300	2	0.27	3.30				
2004	0.000	2	0.35	1.76				
2004	0.300	2	0.35	1.76				
2005	0.000	2	0.34	2.20				
2005	0.300	2	0.34	2.20				
2006	0.000	2	0.27	3.35				

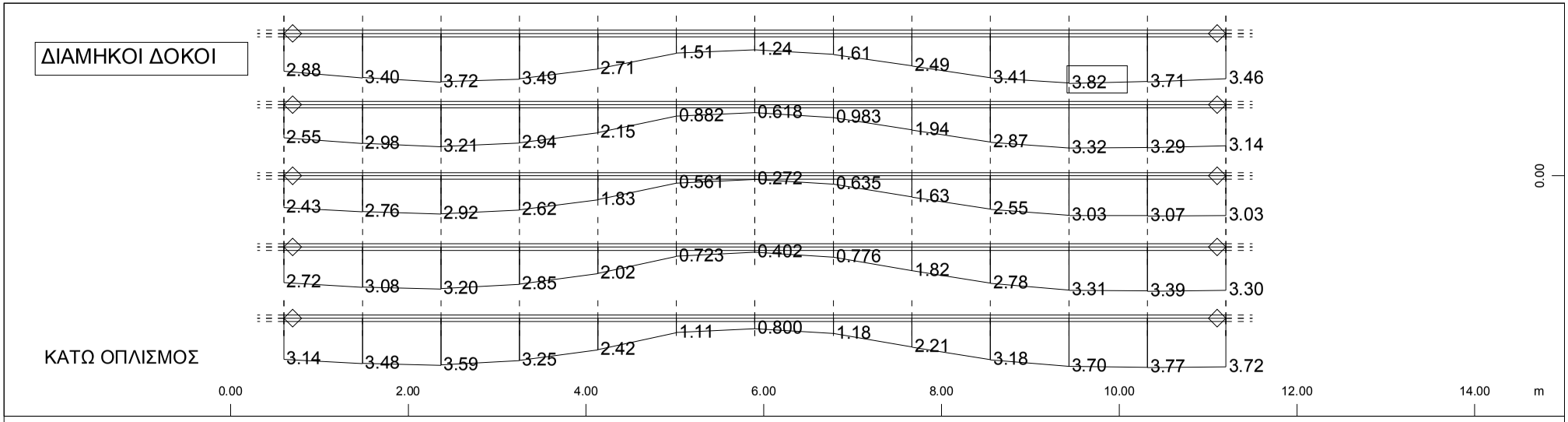
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN_ΣΕΙΣΜΙΚΑ_ΔΟΚΟΙ

Shear Reinforcements per Cutted Part of Section LCR 510

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
2006	0.300	2	0.27	3.35				
2007	0.000	2	0.26	3.48				
2007	0.300	2	0.26	3.82				
2008	0.000	2	0.34	3.16				
2008	0.300	2	0.34	3.42				
2009	0.000	2	0.34	2.33				
2009	0.300	2	0.34	2.33				
2010	0.000	2	0.29	3.56				
2010	0.300	2	0.29	3.56				
2011	0.000	2	0.29	3.69				
2011	0.300	2	0.29	4.06				
2012	0.000	2	0.34	3.48				
2012	0.300	2	0.34	3.74				
2013	0.000	2	0.39	2.32				
2013	0.300	2	0.39	2.32				
2014	0.000	2	0.37	3.54				
2014	0.300	2	0.37	3.54				
2015	0.000	2	0.37	3.66				
2015	0.300	2	0.37	4.02				
2016	0.000	2	0.39	3.28				
2016	0.300	2	0.39	3.55				
2017	0.000	2	0.45	2.16				
2017	0.300	2	0.45	2.16				
2018	0.000	2	0.40	3.27				
2018	0.300	2	0.40	3.27				
2019	0.000	2	0.40	3.39				
2019	0.300	2	0.40	3.72				
2020	0.000	2	0.45	2.12				
2020	0.300	2	0.45	3.22				

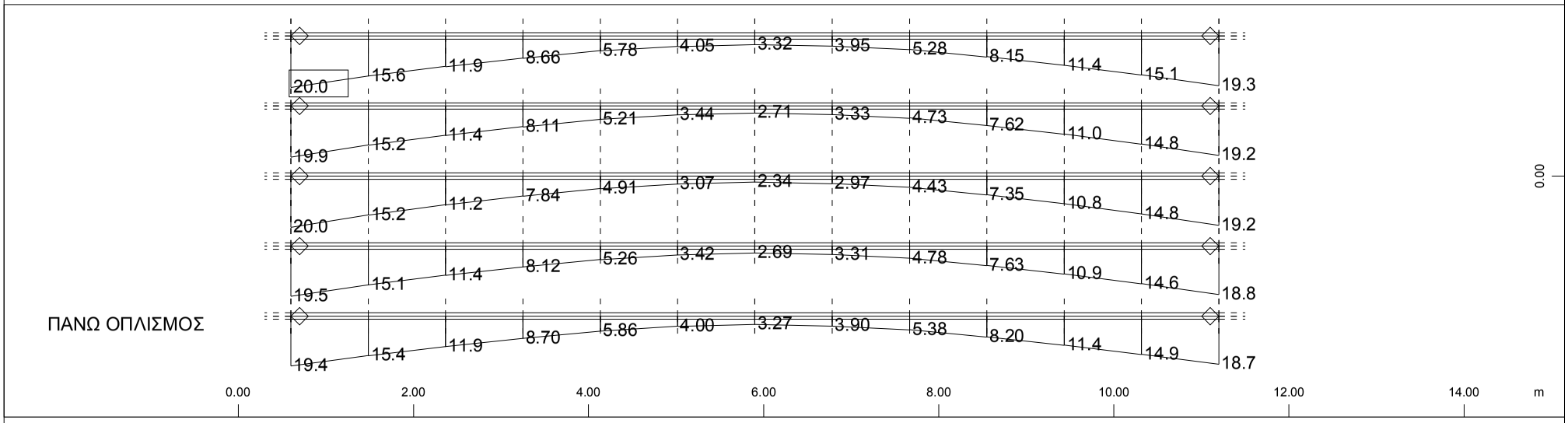
Maximum Degree of Utilization

		N sig-c	Vy sig-t	Vz tau	Mt sig-*	My tend.	Mz As-l	Mb As-v	Mt2 crack	Total sigdyn	lamda tau-*
Cross sect.	1	0.000	0.000	0.249	0.102	0.000	0.000	0.000	0.000	1.001	0.000
		0.000	0.000	0.000	0.000	0.000	1.001	0.000	0.000	0.000	0.000
Cross sect.	2	0.000	0.000	0.083	0.026	0.000	0.000	0.000	0.000	1.001	0.000
DOKOS-2		0.000	0.000	0.000	0.000	0.000	1.000	0.000	0.000	0.000	0.000
Total System		0.000	0.000	0.249	0.102	0.000	0.000	0.000	0.000	1.001	0.000
		0.000	0.000	0.000	0.000	0.000	1.001	0.000	0.000	0.000	0.000



Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case 510 , 1 cm 3D = 4.48 cm2 (Max=3.82)

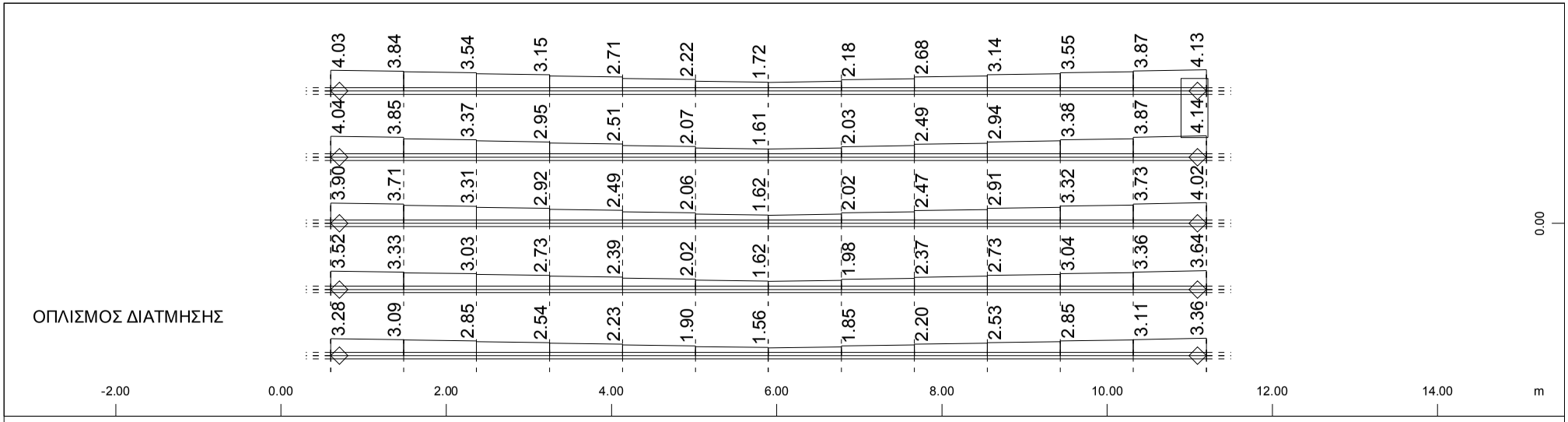
M 1 : 66



Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case 510 , 1 cm 3D = 22.4 cm2 (Max=20.0)

M 1 : 67

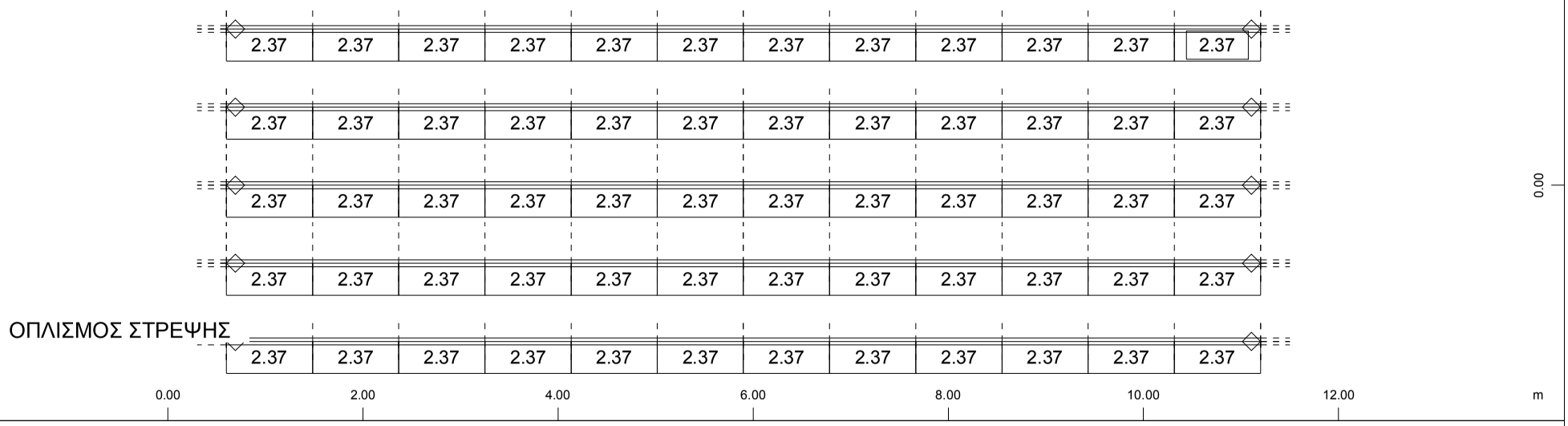
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΣΕΙΣΜΙΚΑ_Q=1.50



Z X
Y

Sector of system Beam Elements Group 1
Beam Elements , Shear reinforcements (maximum), Design Case 510 , 1 cm 3D = 11.2 cm2/m (Max=4.14)

M 1 : 71



Z X
Y

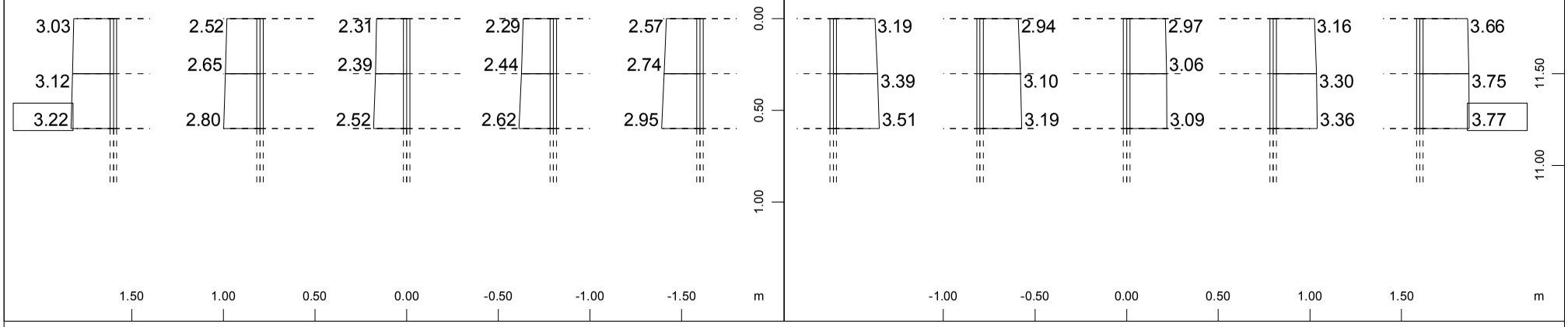
Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case 510 , 1 cm 3D = 4.48 cm2 (Max=2.37)

M 1 : 62

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΣΕΙΣΜΙΚΑ_Q=1.50

ΔΙΑΜΗΚΗ ΔΟΚΟΙ-ΣΤΗΡΙΞΗΣ

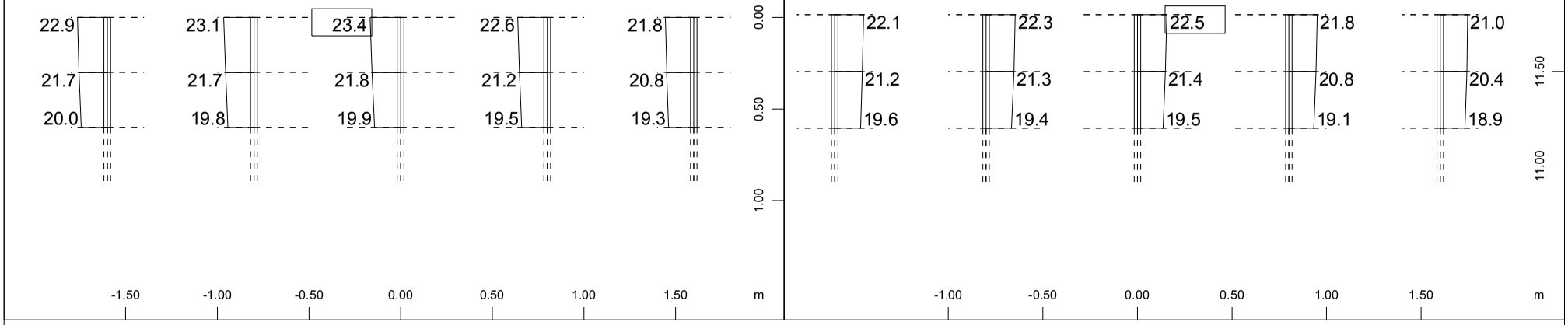
ΚΑΤΩ ΟΠΛΙΣΜΟΣ



Y Z X
Sector of system Beam Elements Group 2
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
510 , 1 cm 3D = 4.48 cm2 (Max=3.22)
M 1 : 32

X Z Y
Sector of system Beam Elements Group 2
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
510 , 1 cm 3D = 4.48 cm2 (Max=3.77)
M 1 : 32

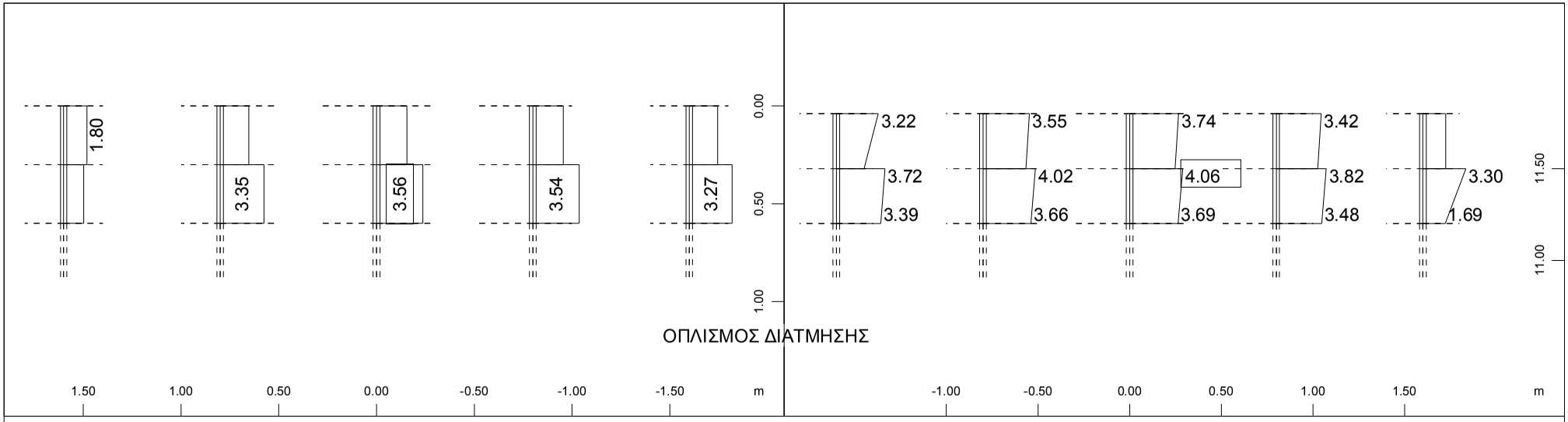
ΠΑΝΩ ΟΠΛΙΣΜΟΣ



Z Y X
Sector of system Beam Elements Group 2
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
510 , 1 cm 3D = 44.8 cm2 (Max=23.4)
M 1 : 32

X Z Y
Sector of system Beam Elements Group 2
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
510 , 1 cm 3D = 44.8 cm2 (Max=22.5)
M 1 : 31

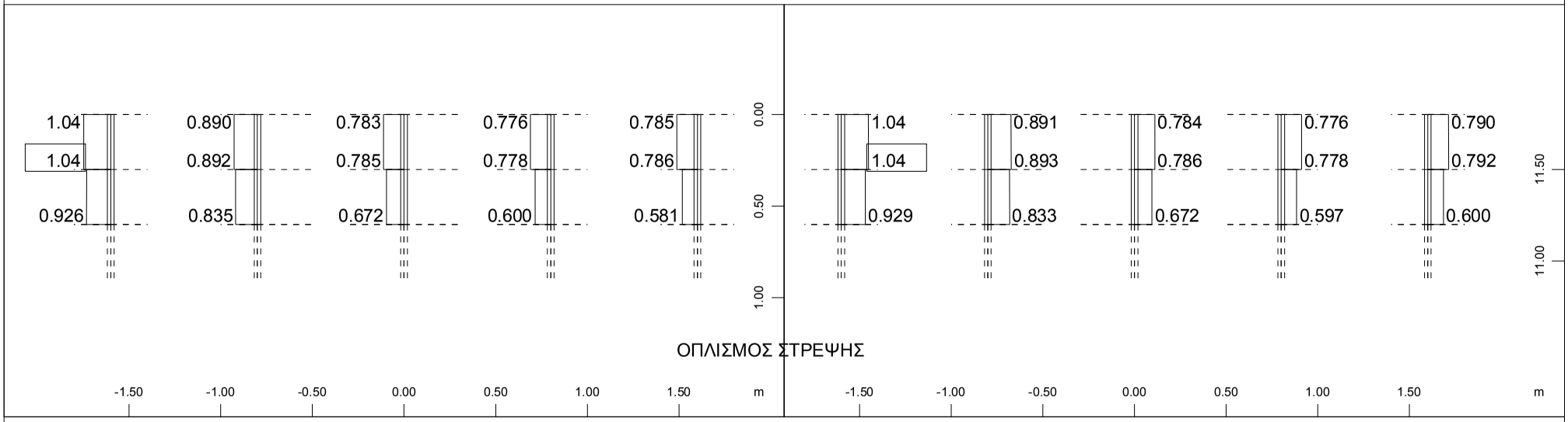
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΣΕΙΣΜΙΚΑ_Q=1.50



ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ

Y Z X Sector of system Beam Elements Group 2 M 1 : 30
Beam Elements , Shear reinforcements (maximum), Design Case 510
, 1 cm 3D = 4.48 cm2/m (Max=3.56)

X Z Y Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Shear reinforcements (maximum), Design Case 510
, 1 cm 3D = 4.48 cm2/m (Max=4.06)



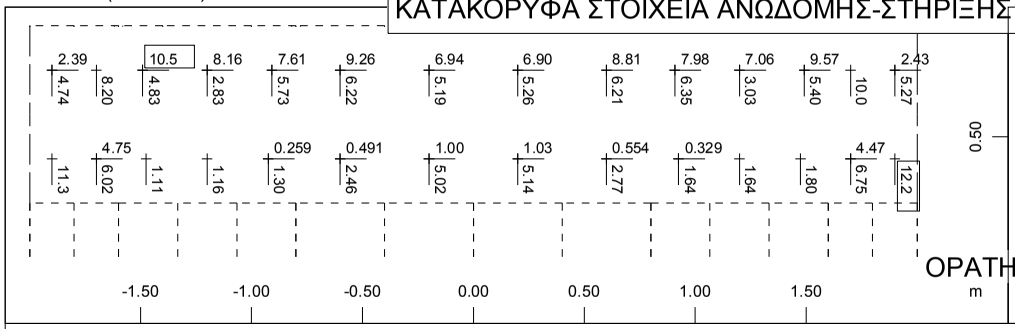
ΟΠΛΙΣΜΟΣ ΣΤΡΕΨΗΣ

Z Y X Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
510 , 1 cm 3D = 2.24 cm2 (Max=1.04)

X Z Y Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
510 , 1 cm 3D = 2.24 cm2 (Max=1.04)

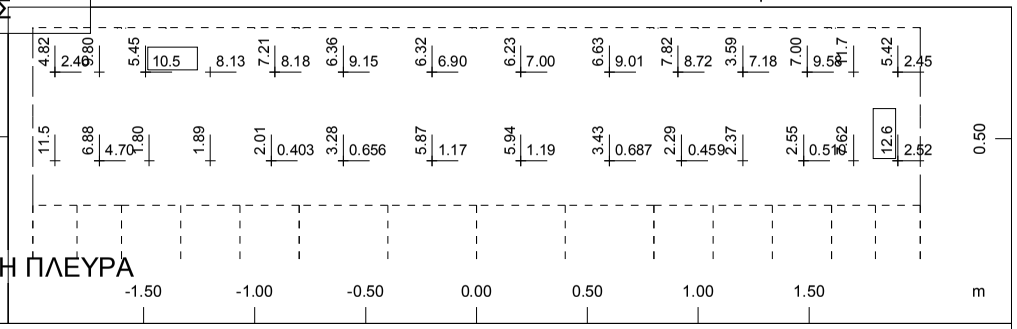
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΣΕΙΣΜΙΚΑ_Q=1.50

ΚΑΤΑΚΟΡΥΦΑ ΣΤΟΙΧΕΙΑ ΑΝΩΔΟΜΗΣ-ΣΤΗΡΙΞΗΣ



Sector of system Quadrilateral Elements Group 8
upper Reinforcements in Elements in cm2/m, Design Case 522 ULS
design (Max=12.2)

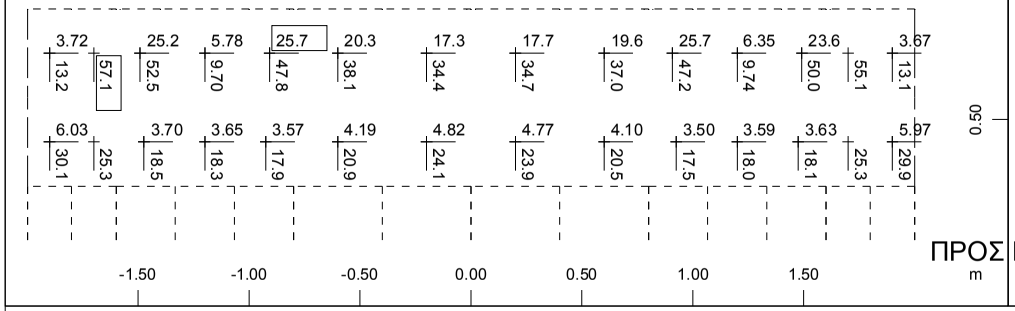
M 1 : 34



Sector of system Quadrilateral Elements Group 8
upper Reinforcements in Elements in cm2/m, Design Case 522 ULS
design (Max=12.6)

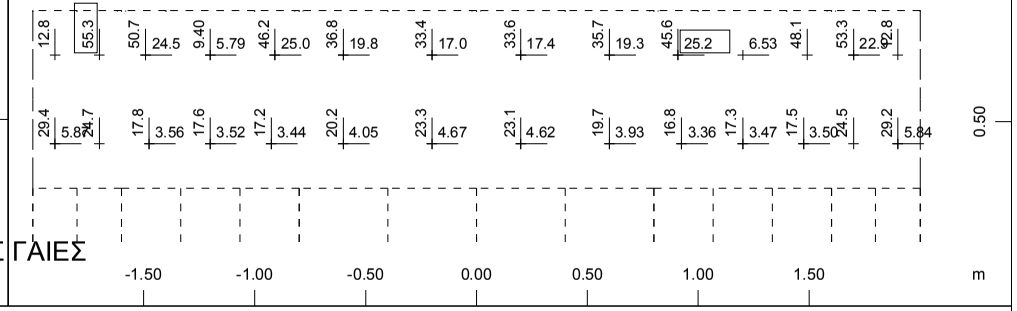
M 1 : 34

ΟΡΑΤΗ ΠΛΕΥΡΑ



Sector of system Quadrilateral Elements Group 8
lower Reinforcements in Elements in cm2/m, Design Case 522 ULS
design (Max=57.1)

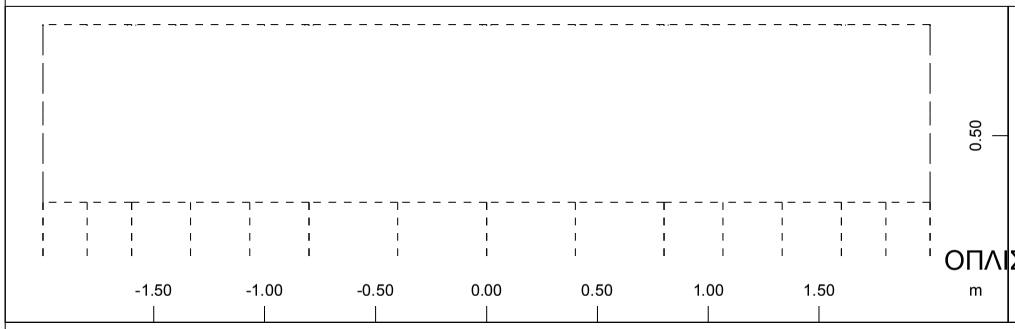
M 1 : 34



Sector of system Quadrilateral Elements Group 8
lower Reinforcements in Elements in cm2/m, Design Case 522 ULS
design (Max=55.3)

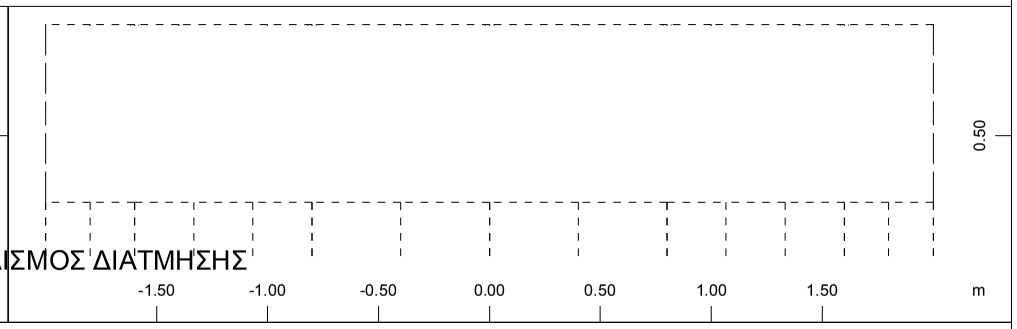
M 1 : 34

ΠΡΟΣ ΓΑΙΕΣ



Sector of system Quadrilateral Elements Group 8
Shear reinforcement from middle of element in cm2/m2, Design
Case 522 ULS design (Max=0)

M 1 : 34



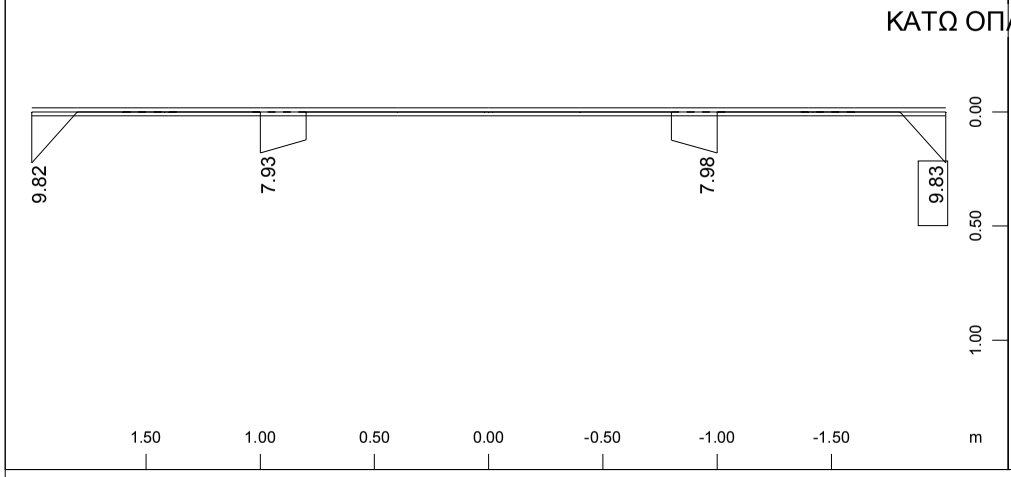
Sector of system Quadrilateral Elements Group 8
Shear reinforcement from middle of element in cm2/m2, Design
Case 522 ULS design (Max=0)

M 1 : 34

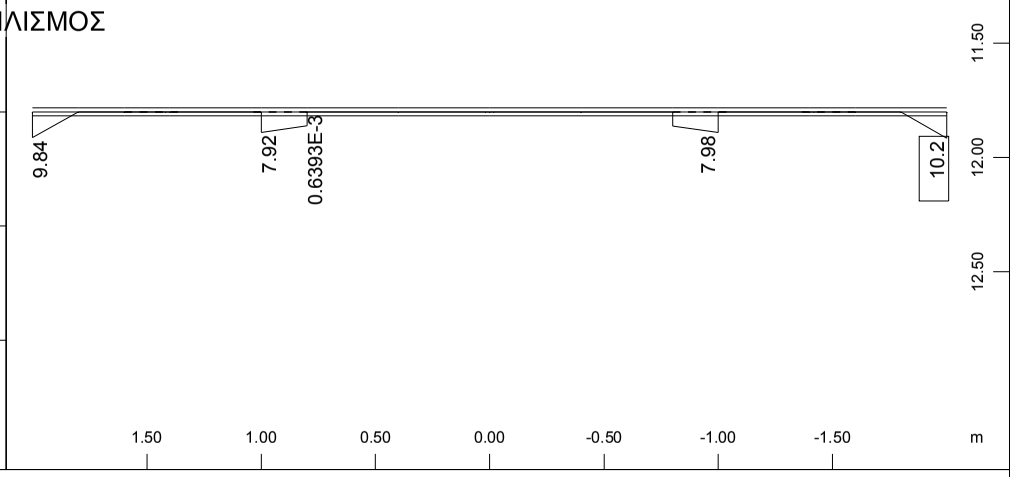
ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ

ΠΑΣΣΑΛΟΔΕΣΜΟΣ

ΚΑΤΩ ΟΠΛΙΣΜΟΣ

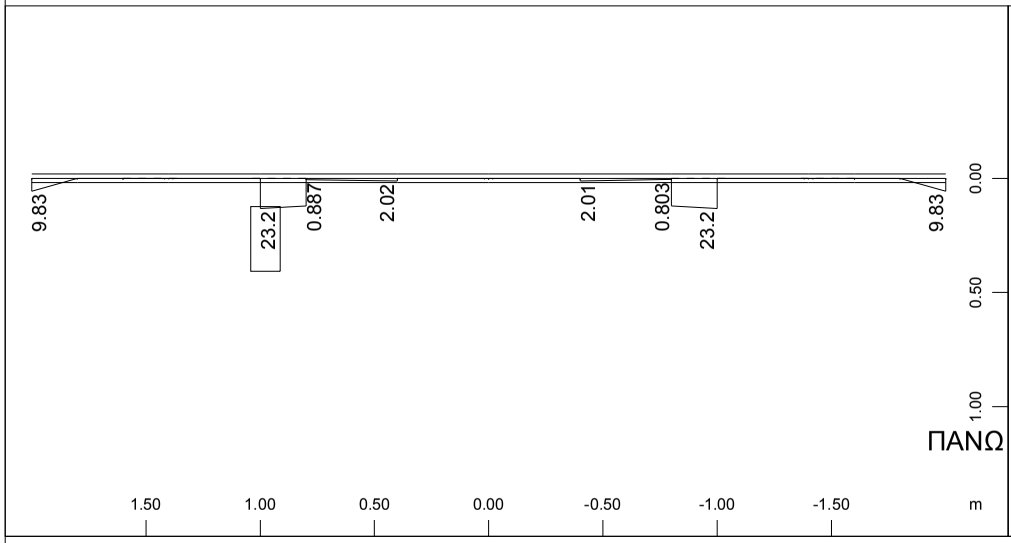


Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
509 , 1 cm 3D = 14.5 cm2 (Max=9.83)

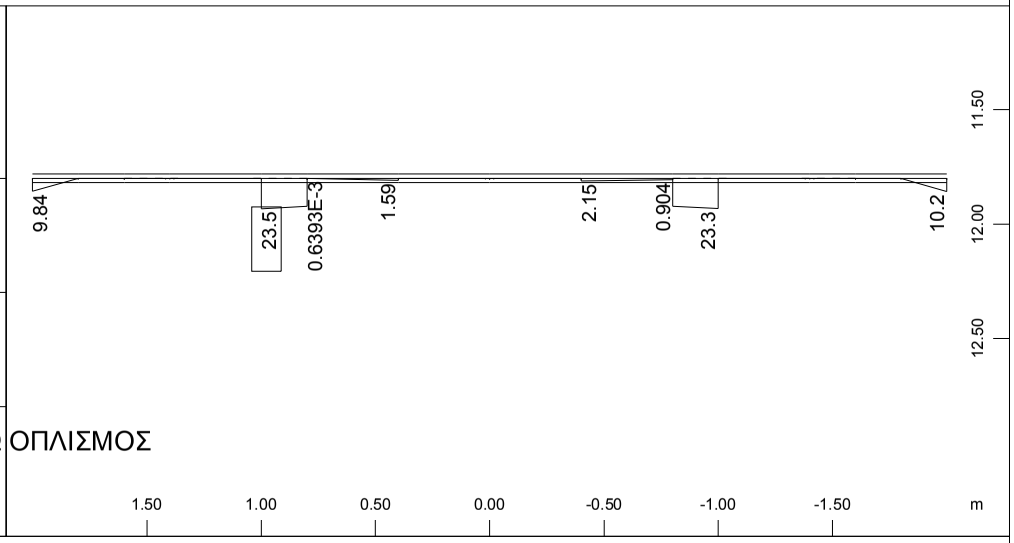


Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
509 , 1 cm 3D = 29.1 cm2 (Max=10.2)

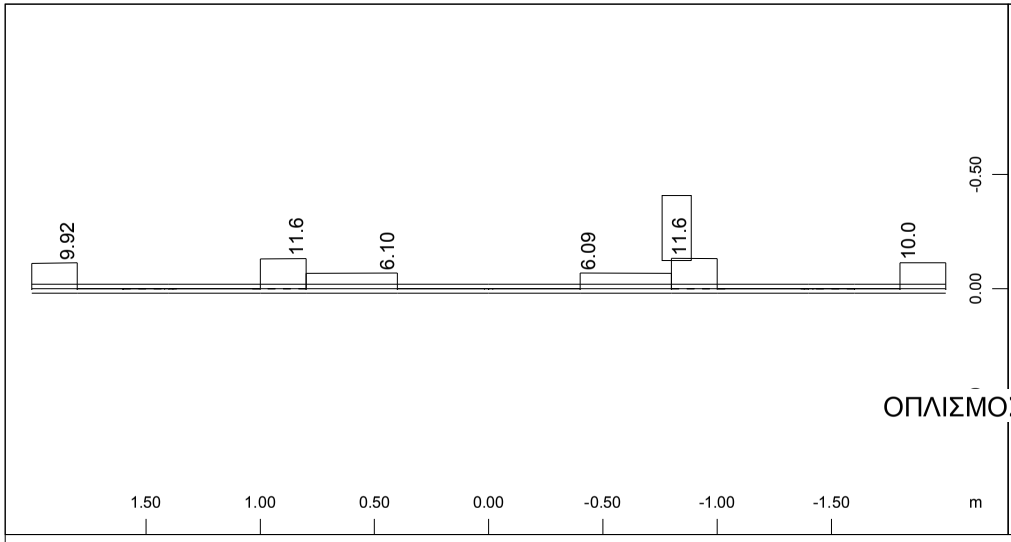
ΠΑΝΩ ΟΠΛΙΣΜΟΣ



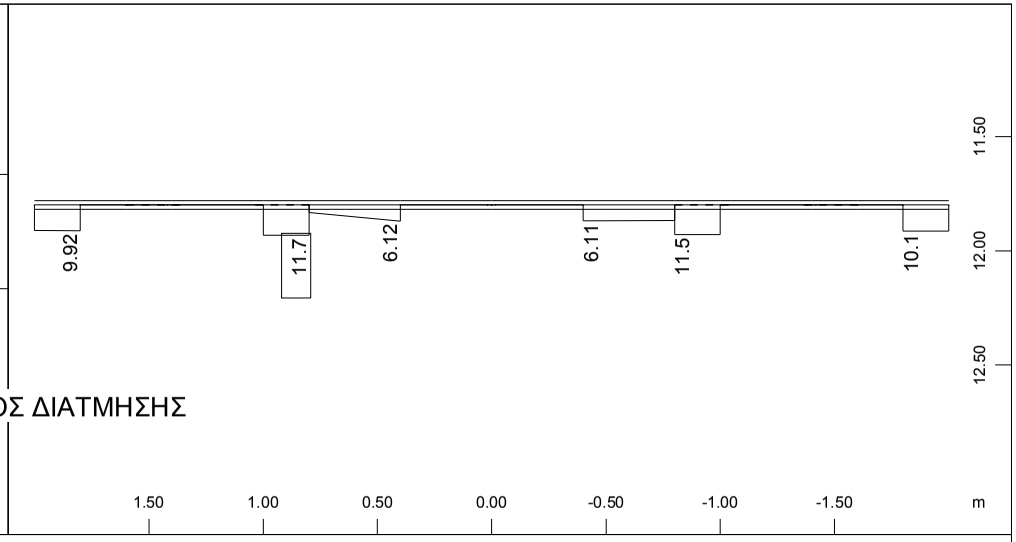
Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
509 , 1 cm 3D = 58.1 cm2 (Max=23.2)



Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
509 , 1 cm 3D = 58.1 cm2 (Max=23.5)

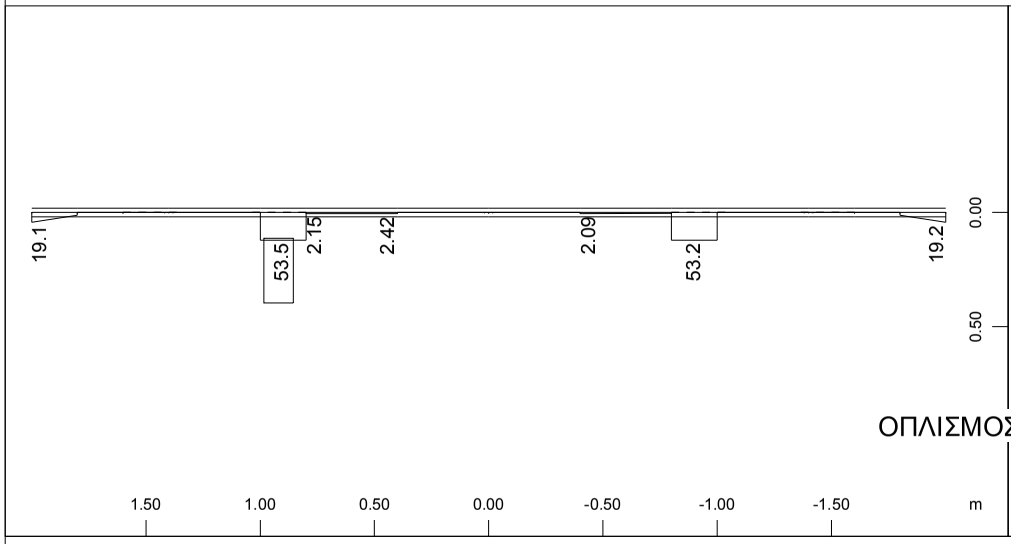


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Shear reinforcements (maximum), Design Case 509
, 1 cm 3D = 29.1 cm2/m (Max=11.6)

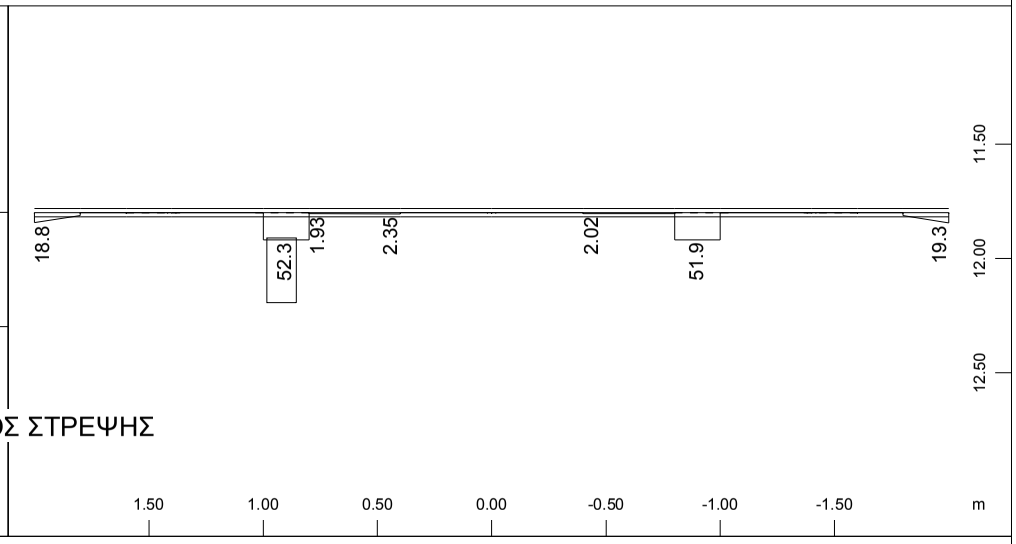


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Shear reinforcements (maximum), Design Case 509
, 1 cm 3D = 29.1 cm2/m (Max=11.7)

ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ

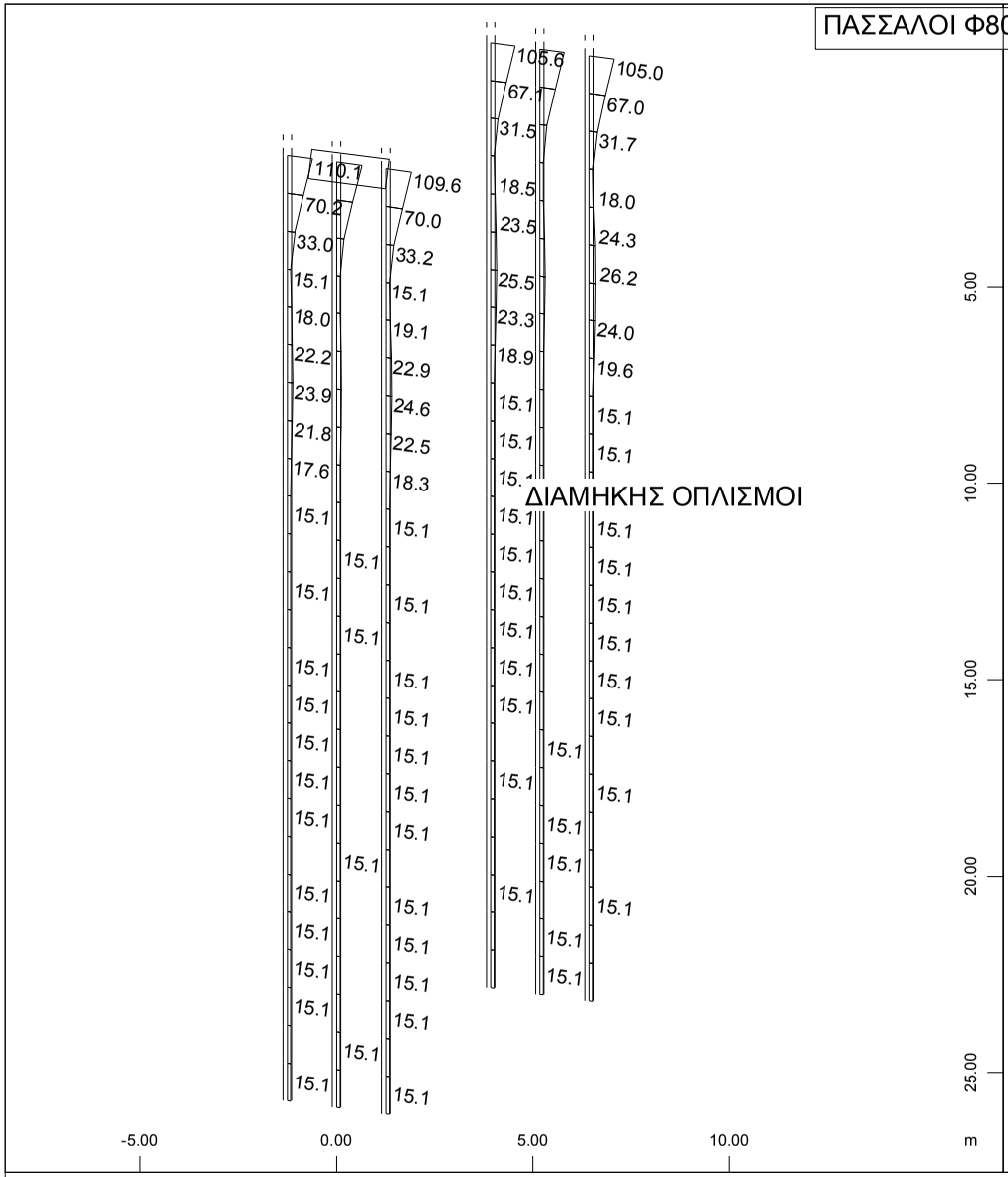


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
509 , 1 cm 3D = 145.3 cm2 (Max=53.5)



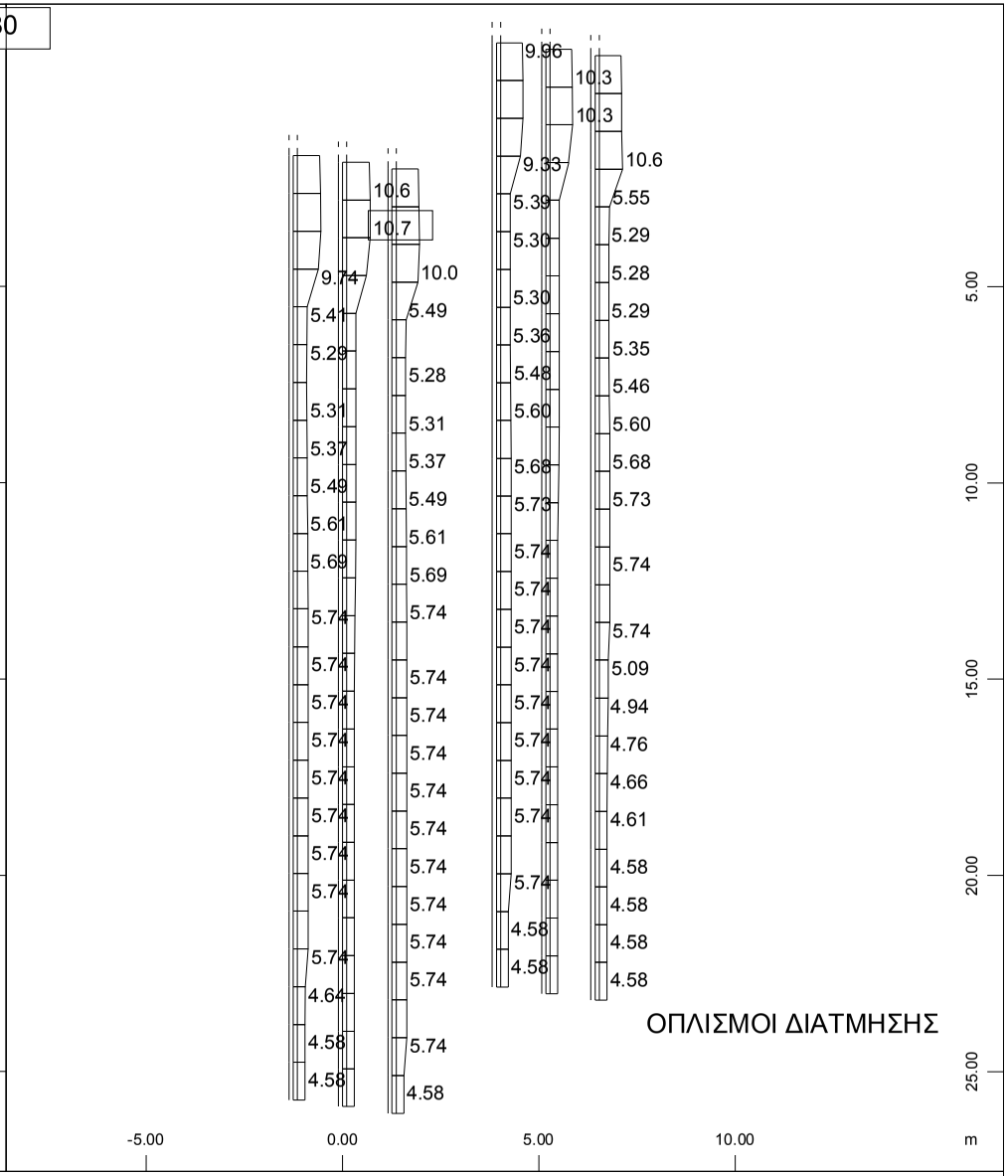
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
509 , 1 cm 3D = 145.3 cm2 (Max=52.3)

ΟΠΛΙΣΜΟΣ ΣΤΡΕΨΗΣ



Sector of system Beam Elements Group 12
Beam Elements , Longitudinal Reinforcements Lay. 1, Design
Case 509 , 1 cm 3D = 290.6 cm2 (Max=110.1)

M 1 : 191
X * 0.502
Y * 0.906
Z * 0.962

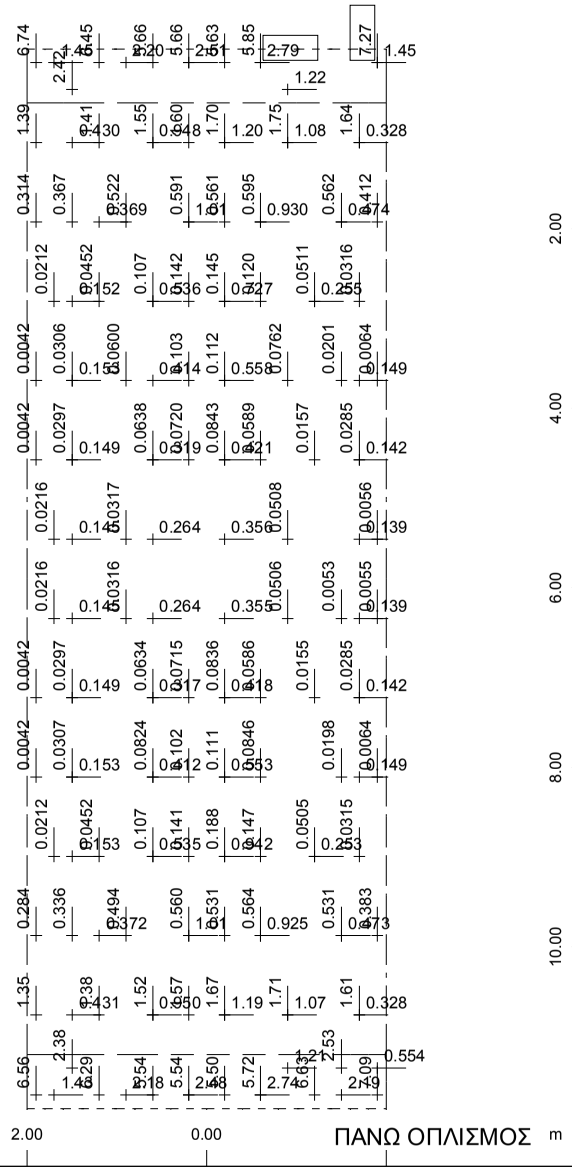


Sector of system Beam Elements Group 12
Beam Elements , Shear reinforcements (maximum), Design Case
509 , 1 cm 3D = 29.1 cm2/m (Max=10.7)

M 1 : 191
X * 0.502
Y * 0.906
Z * 0.962

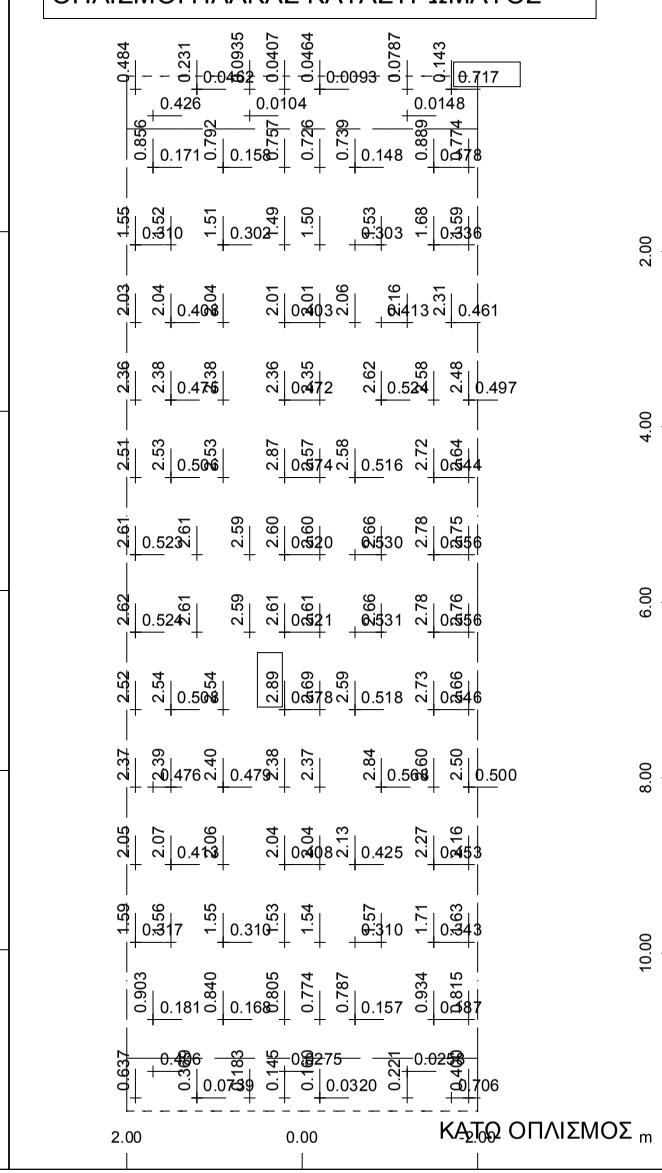
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΣΕΙΣΜΙΚΑ_Q=1.50

ΟΠΛΙΣΜΟΙ ΠΛΑΚΑΣ ΚΑΤΑΣΤΡΩΜΑΤΟΣ



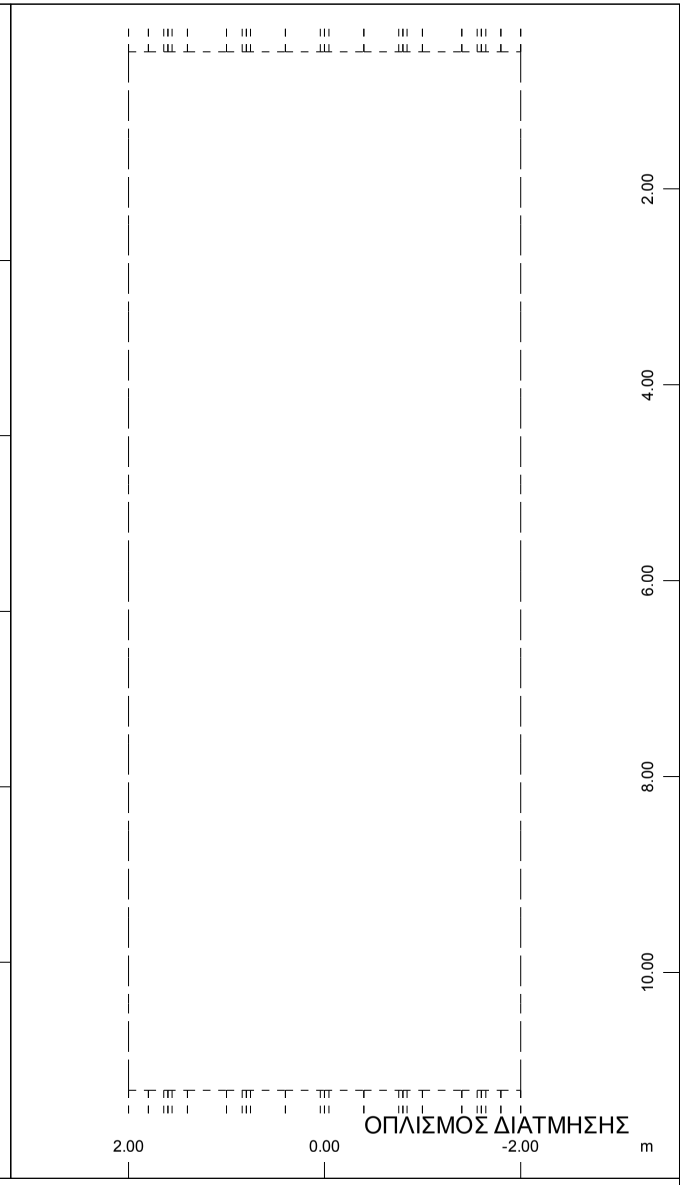
Sector of system Quadrilateral Elements Group 3 4
upper Reinforcements in Elements in cm2/m, Design Case
522 ULS design (Max=7.27)

M 1 : 84



Sector of system Quadrilateral Elements Group 3 4
lower Reinforcements in Elements in cm2/m, Design Case
522 ULS design (Max=2.89)

M 1 : 86



Sector of system Quadrilateral Elements Group 3
Shear reinforcement from middle of element in cm2/m2,
Design Case 522 ULS design (Max=0)

M 1 : 77

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΣΕΙΣΜΙΚΑ_Q=1.50

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00

11) ΦΑΣΗ-2 ΕΛΕΓΧΟΣ ΦΟΡΕΑ ULS-ΣΕΙΣΜΙΚΑ (Q=1.00, γBd1=1.25)

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
SEISMIC COMBINATIONS (MAIN +Ex) (Q=1.0)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
9101	Unid.-Seismic Combinatio	3.351	0.603	4.205

Iteration sequence

```
Iteration  1 Residual        0.005  energy 477.4897 e/f  0.000  1.000

Iteration  2 Residual        0.000  energy 477.7957 e/f  0.000  1.001
```

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
SEISMIC COMBINATIONS (MAIN +Ex) (Q=1.0)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
9103	Unid.-Seismic Combinatio	3.351	0.603	3.240

Iteration sequence

```
Iteration  1 Residual        0.004  energy 467.7261 e/f  0.000  1.000

Iteration  2 Residual        0.000  energy 467.9768 e/f  0.000  1.001
```

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
SEISMIC COMBINATIONS (MAIN +Ex) (Q=1.0)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
9105	Unid.-Seismic Combinatio	3.351	-0.603	4.205

Iteration sequence

```
Iteration  1 Residual        0.005  energy 477.9139 e/f  0.000  1.000

Iteration  2 Residual        0.000  energy 478.2209 e/f  0.000  1.001
```

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: **6**
Number of longitudinal springs: **6**
Number of torsional springs: **0**
No nonlinear effects detected

Statistic beam elements: number of checked elements : **0**
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
SEISMIC COMBINATIONS (MAIN +Ex) (Q=1.0)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
9107	Unid.-Seismic Combinatio	3.351	-0.603	3.240

Iteration sequence

```
Iteration  1 Residual        0.005  energy 468.0535 e/f  0.000  1.000

Iteration  2 Residual        0.000  energy 468.3049 e/f  0.000  1.001
```

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
SEISMIC COMBINATIONS (MAIN -Ex) (Q=1.0)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
9102	Unid.-Seismic Combinatio	-3.433	0.603	4.205

Iteration sequence

```
Iteration  1 Residual        0.005  energy 483.2671 e/f  0.000  1.000

Iteration  2 Residual        0.000  energy 483.5935 e/f  0.000  1.001
```

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: **6**
Number of longitudinal springs: **6**
Number of torsional springs: **0**
No nonlinear effects detected

Statistic beam elements: number of checked elements : **0**
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
SEISMIC COMBINATIONS (MAIN -Ex) (Q=1.0)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
9104	Unid.-Seismic Combinatio	-3.433	0.603	3.240

Iteration sequence

```
Iteration  1 Residual        0.005  energy 473.5034 e/f  0.000  1.000

Iteration  2 Residual        0.000  energy 473.7709 e/f  0.000  1.001
```

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
SEISMIC COMBINATIONS (MAIN -Ex) (Q=1.0)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
9106	Unid.-Seismic Combinatio	-3.433	-0.603	4.205

Iteration sequence
Iteration 1 Residual 0.005 energy 483.6913 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 484.0187 e/f 0.000 1.001

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
SEISMIC COMBINATIONS (MAIN -Ex) (Q=1.0)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
9108	Unid.-Seismic Combinatio	-3.433	-0.603	3.240

Iteration sequence

```
Iteration  1 Residual        0.005  energy 473.8309 e/f  0.000  1.000

Iteration  2 Residual        0.000  energy 474.0990 e/f  0.000  1.001
```

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: **6**
Number of longitudinal springs: **6**
Number of torsional springs: **0**
No nonlinear effects detected

Statistic beam elements: number of checked elements : **0**
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
SEISMIC COMBINATIONS (MAIN +Ey) (Q=1.0)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
9201	Unid.-Seismic Combinatio	0.976	2.010	4.205

Iteration sequence
Iteration 1 Residual 0.004 energy 404.3980 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 404.5235 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
SEISMIC COMBINATIONS (MAIN +Ey) (Q=1.0)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
9203	Unid.-Seismic Combinatio	0.976	2.010	3.240

Iteration sequence
Iteration 1 Residual 0.004 energy 394.7473 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 394.8497 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
SEISMIC COMBINATIONS (MAIN +Ey) (Q=1.0)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
9205	Unid.-Seismic Combinatio	0.976	-2.010	4.205

Iteration sequence

```
Iteration  1 Residual        0.004  energy 405.8119 e/f  0.000  1.000

Iteration  2 Residual        0.000  energy 405.9402 e/f  0.000  1.000
```

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
SEISMIC COMBINATIONS (MAIN +Ey) (Q=1.0)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
9207	Unid.-Seismic Combinatio	0.976	-2.010	3.240

Iteration sequence

```
Iteration  1 Residual        0.004  energy 395.8386 e/f  0.000  1.000

Iteration  2 Residual        0.000  energy 395.9428 e/f  0.000  1.000
```

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: **6**
Number of longitudinal springs: **6**
Number of torsional springs: **0**
No nonlinear effects detected

Statistic beam elements: number of checked elements : **0**
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
SEISMIC COMBINATIONS (MAIN -Ey) (Q=1.0)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
9202	Unid.-Seismic Combinatio	-1.059	2.010	4.205

Iteration sequence
Iteration 1 Residual 0.004 energy 406.1313 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 406.2629 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
SEISMIC COMBINATIONS (MAIN -Ey) (Q=1.0)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
9204	Unid.-Seismic Combinatio	-1.059	2.010	3.240

Iteration sequence
Iteration 1 Residual 0.004 energy 396.4805 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 396.5880 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
SEISMIC COMBINATIONS (MAIN -Ey) (Q=1.0)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
9206	Unid.-Seismic Combinatio	-1.059	-2.010	4.205

Iteration sequence
Iteration 1 Residual 0.004 energy 407.5451 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 407.6795 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
SEISMIC COMBINATIONS (MAIN -Ey) (Q=1.0)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
9208	Unid.-Seismic Combinatio	-1.059	-2.010	3.240

Iteration sequence
Iteration 1 Residual 0.004 energy 397.5718 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 397.6810 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
SEISMIC COMBINATIONS (MAIN +Ez) (Q=1.0)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
9301	Unid.-Seismic Combinatio	0.976	0.603	5.330

Iteration sequence
Iteration 1 Residual 0.001 energy 372.0461 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 372.0878 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
SEISMIC COMBINATIONS (MAIN +Ez) (Q=1.0)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
9303	Unid.-Seismic Combinatio	0.976	0.603	2.114

Iteration sequence
Iteration 1 Residual 0.001 energy 339.5006 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 339.5211 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
SEISMIC COMBINATIONS (MAIN +Ez) (Q=1.0)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
9305	Unid.-Seismic Combinatio	0.976	-0.603	5.330

Iteration sequence
Iteration 1 Residual 0.001 energy 372.5832 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 372.6261 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
SEISMIC COMBINATIONS (MAIN +Ez) (Q=1.0)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
9307	Unid.-Seismic Combinatio	0.976	-0.603	2.114

Iteration sequence
Iteration 1 Residual 0.001 energy 339.7151 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 339.7359 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CALCULATION OF SEISMIC COMBINATIONS (MAIN -Ez)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
9302	Unid.-Seismic Combinatio	-1.059	0.603	5.330

Iteration sequence
Iteration 1 Residual 0.002 energy 373.7794 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 373.8284 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CALCULATION OF SEISMIC COMBINATIONS (MAIN -Ez)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
9304	Unid.-Seismic Combinatio	-1.059	0.603	2.114

Iteration sequence
Iteration 1 Residual 0.001 energy 341.2339 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 341.2581 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CALCULATION OF SEISMIC COMBINATIONS (MAIN -Ez)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
9306	Unid.-Seismic Combinatio	-1.059	-0.603	5.330

Iteration sequence
Iteration 1 Residual 0.002 energy 374.3164 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 374.3668 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
CALCULATION OF SEISMIC COMBINATIONS (MAIN -Ez)

Analysis parameters
Calculation with nonlinear material properties

Nonlinear material properties are used for:
Springelements[CRAC,YIEL,MUE,GAP], pilebedding, QUAD-bedding
Only linear material properties are used for:
QUAD- and BRIQ-elements
Truss-, cable-, Beam-, pile- und boundaryelements
Beamelements

Sum of Loads

LC	Title	PXX[MN]	PYY[MN]	PZZ[MN]
9308	Unid.-Seismic Combinatio	-1.059	-0.603	2.114

Iteration sequence
Iteration 1 Residual 0.001 energy 341.4484 e/f 0.000 1.000

Iteration 2 Residual 0.000 energy 341.4729 e/f 0.000 1.000

Statistic nonlinear effects:

Statistic nonlinear effects of spring elements: no of elem.: 6
Number of longitudinal springs: 6
Number of torsional springs: 0
No nonlinear effects detected

Statistic beam elements: number of checked elements : 0
Number of yielding elements .....[DEHN KSV PL/PLD]: 0

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΔΙΑΣΤΑΣΙΟΛΟΓΗΣΗ ΦΟΡΕΑ ΣΕΙΣΜΙΚΕΣ ΦΟΡΤΙΣΕΙΣ-γBd1

Design according to DIN1045-1 2008
Loadcases have been calculated in the Ultimate Limit State
In BEMESS no additional load safety factor is applied.

Load Cases for the Design

```
Loadcase 9101   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 9102   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 9103   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 9104   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 9105   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 9106   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 9107   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 9108   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 9201   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 9202   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 9203   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 9204   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 9205   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 9206   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 9207   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 9301   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 9302   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 9303   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 9304   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 9305   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 9306   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 9307   Unid.-Seismic Combinatio + Nodal reaction punching design
Loadcase 9308   Unid.-Seismic Combinatio + Nodal reaction punching design
```

Material (DIN1045-1 2008)

Mat	f-ck [MPa]	f-cr [MPa]	f-yk [MPa]	f-tk [MPa]	f-ctm [MPa]	N [-]	minQ [-]	type
B1	25.0	21.2	500.0	500.0	2.565	10.5	0.20	mainly static

Minimum reinforcement: 0.00 p.c. of stat. req. section

Reduction of FC in case of transvers tension = 25.0 [o/o]

Material-safety-factors:

Mat	concr SC1	SC2	steel SS1	SS2
B1	1.88	1.88	1.15	1.15

Acc. the german DIN Fachberichten a minimum concrete shear capacity VRd,ct
is taken into account in the shear design without shear reinforcement.

In shear design the cotangens theta is limited to 1.750 .

At direct supports from the face of the support up to 1.0*d the shear force is reduced.
The maximum shear capacity is checked at the face of the support without reduction.

The punching design has been switched off and must be done separately.
Outside the punching area, the normal slab shear design may increase the,
longitudinal reinforcement up to 0.20% [input CTRL...RO_V].

Geometry (axial covers)

No	he-upper [mm]	hi-upper [mm]	he-lower [mm]	hi-lower [mm]	Elem. height [mm]
1	50	70	35	55	As saved

Selection of elements

	from	to	inc	group	GEOMETRY
Element	3001	3999	1	-	1
Element	4001	4999	1	-	1
Element	8001	8999	1	-	1

Reinforcement is saved in the data base file
Number of stored reinforcement-distribution: 523

REINFORCEMENT ACC. TO DIN1045-1 2008 in [cm2/m] upper/lower
General load safety factor - as defined in BEMESS: Gamma-f = 1.00
Shear: stresses VEd/d and VRd,ct/d with d=effective depth = h-hm
Shear index 2m = minimum shear reinforcement

Grp	ELEM No	LC No	MAT No	GEO No	h [m]	Reinforcement main	cross	dir	dphi deg	Shr zon	VEd/d [MPa] VRd,ct/d	Ass [cm2/m2]
3	3001	maximum			0.25	2.04	0.41	0		1	0.085	
						1.36	0.27	0			0.495	
3	3002	maximum			0.25	0.82	0.16	0		1	0.064	
						2.08	0.42	0			0.495	
3	3003	maximum			0.25		0.02	0		1	0.054	
						2.49	0.50	0			0.495	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΔΙΑΣΤΑΣΙΟΛΟΓΗΣΗ ΦΟΡΕΑ ΣΕΙΣΜΙΚΕΣ ΦΟΡΤΙΣΕΙΣ-γBd1

REINFORCEMENT ACC. TO DIN1045-1 2008 in [cm2/m] upper/lower
General load safety factor - as defined in BEMESS: Gamma-f = 1.00
Shear: stresses VEd/d and VRd,ct/d with d=effective depth = h-hm
Shear index 2m = minimum shear reinforcement

Grp	ELEM No	LC No	MAT No	GEO No	h [m]	Reinforcement main	cross	dir	dphi deg	Shr zon	VEd/d [MPa] VRd,ct/d	Ass [cm2/m2]
3	3004	maximum			0.25		0.02	0		1	0.043	
						2.74	0.55	0			0.495	
3	3005	maximum			0.25		0.02	0		1	0.032	
						2.81	0.56	0			0.496	
3	3006	maximum			0.25		0.02	0		1	0.022	
						2.78	0.56	0			0.496	
3	3007	maximum			0.25	2.04	0.41	0		1	0.058	
						1.46	0.29	0			0.495	
3	3008	maximum			0.25	0.87	0.19	0		1	0.061	
						2.06	0.41	0			0.496	
3	3009	maximum			0.25	0.02	0.11	0		1	0.048	
						2.48	0.50	0			0.496	
3	3010	maximum			0.25	0.02	0.11	0		1	0.041	
						2.74	0.55	0			0.496	
3	3011	maximum			0.25	0.02	0.11	0		1	0.034	
						2.81	0.56	0			0.496	
3	3012	maximum			0.25	0.02	0.11	0		1	0.029	
						2.76	0.55	0			0.496	
3	3013	maximum			0.25	2.09	0.61	0		1	0.095	
						1.48	0.30	0			0.496	
3	3014	maximum			0.25	0.91	0.25	0		1	0.054	
						2.07	0.41	0			0.495	
3	3015	maximum			0.25	0.04	0.19	0		1	0.043	
						2.51	0.50	0			0.495	
3	3016	maximum			0.25	0.04	0.20	0		1	0.034	
						2.76	0.55	0			0.496	
3	3017	maximum			0.25	0.04	0.20	0		1	0.029	
						2.83	0.57	0			0.496	
3	3018	maximum			0.25	0.04	0.19	0		1	0.026	
						2.76	0.55	0			0.496	
3	3019	maximum			0.25	2.05	0.72	0		1	0.094	
						1.31	0.26	0			0.495	
3	3020	maximum			0.25	0.95	0.45	0		1	0.070	
						2.05	0.41	0			0.495	
3	3021	maximum			0.25	0.06	0.28	0		1	0.053	
						2.48	0.50	0			0.495	
3	3022	maximum			0.25	0.04	0.19	0		1	0.039	
						2.74	0.55	0			0.496	
3	3023	maximum			0.25	0.03	0.13	0		1	0.028	
						2.80	0.56	0			0.496	
3	3024	maximum			0.25	0.02	0.11	0		1	0.018	
						2.77	0.55	0			0.496	
3	3025	maximum			0.25	2.27	1.19	0		1	0.103	
						1.50	0.30	0			0.496	
3	3026	maximum			0.25	1.10	0.96	0		1	0.091	
						2.07	0.41	0			0.496	
3	3027	maximum			0.25	0.12	0.60	0		1	0.064	
						2.52	0.50	0			0.496	
3	3028	maximum			0.25	0.08	0.38	0		1	0.053	
						2.77	0.55	0			0.496	
3	3029	maximum			0.25	0.05	0.25	0		1	0.041	
						2.82	0.56	0			0.496	
3	3030	maximum			0.25	0.03	0.17	0		1	0.034	
						2.76	0.55	0			0.496	
3	3031	maximum			0.25	2.27	1.17	0		1	0.077	
						1.35	0.27	0			0.495	
3	3032	maximum			0.25	1.04	0.87	0		1	0.056	
						2.07	0.41	0			0.495	
3	3033	maximum			0.25	0.13	0.67	0		1	0.046	
						2.48	0.50	0			0.495	
3	3034	maximum			0.25	0.10	0.51	0		1	0.036	
						2.74	0.55	0			0.495	
3	3035	maximum			0.25	0.08	0.38	0		1	0.029	
						2.79	0.56	0			0.496	
3	3036	maximum			0.25	0.06	0.30	0		1	0.024	
						2.74	0.55	0			0.496	
3	3037	maximum			0.25	2.32	1.39	0		1	0.072	
						1.43	0.29	0			0.495	
3	3038	maximum			0.25	1.17	1.25	0		1	0.072	
						2.09	0.42	0			0.496	
3	3039	maximum			0.25	0.19	0.95	0		1	0.057	
						2.54	0.51	0			0.496	
3	3040	maximum			0.25	0.13	0.64	0		1	0.046	
						2.79	0.56	0			0.496	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΔΙΑΣΤΑΣΙΟΛΟΓΗΣΗ ΦΟΡΕΑ ΣΕΙΣΜΙΚΕΣ ΦΟΡΤΙΣΕΙΣ-γBd1

REINFORCEMENT ACC. TO DIN1045-1 2008 in [cm2/m] upper/lower
General load safety factor - as defined in BEMESS: Gamma-f = 1.00
Shear: stresses VEd/d and VRd,ct/d with d=effective depth = h-hm
Shear index 2m = minimum shear reinforcement

Grp	ELEM No	LC No	MAT No	GEO No	h [m]	Reinforcement main	Reinforcement cross	dphi dir	dphi deg	Shr zon	VEd/d [MPa] VRd,ct/d	Ass [cm2/m2]
3	3041	maximum			0.25	0.09	0.44	0		1	0.034	
						2.95	0.59	0			0.496	
3	3042	maximum			0.25	0.06	0.29	0		1	0.024	
						2.76	0.55	0			0.496	
3	3043	maximum			0.25	2.44	1.49	0		1	0.060	
						1.33	0.27	0			0.495	
3	3044	maximum			0.25	1.13	1.19	0		1	0.066	
						2.12	0.42	0			0.496	
3	3045	maximum			0.25	0.18	0.91	0		1	0.056	
						2.53	0.51	0			0.496	
3	3046	maximum			0.25	0.14	0.68	0		1	0.050	
						2.77	0.55	0			0.496	
3	3047	maximum			0.25	0.10	0.50	0		1	0.042	
						3.34	0.67	0			0.496	
3	3048	maximum			0.25	0.08	0.38	0		1	0.037	
						2.76	0.55	0			0.496	
3	3049	maximum			0.25	2.38	1.33	0		1	0.067	
						1.37	0.27	0			0.495	
3	3050	maximum			0.25	1.16	1.12	0		1	0.057	
						2.11	0.42	0			0.495	
3	3051	maximum			0.25	0.16	0.78	0		1	0.048	
						2.59	0.52	0			0.495	
3	3052	maximum			0.25	0.11	0.53	0		1	0.039	
						2.85	0.57	0			0.496	
3	3053	maximum			0.25	0.07	0.36	0		1	0.030	
						2.90	0.58	0			0.497	
3	3054	maximum			0.25	0.06	0.29	0		1	0.021	
						2.81	0.56	0			0.497	
3	3055	maximum			0.25	2.48	1.32	0		1	0.102	
						1.35	0.27	0			0.496	
3	3056	maximum			0.25	1.06	0.94	0		1	0.096	
						2.13	0.43	0			0.497	
3	3057	maximum			0.25	0.16	0.81	0		1	0.078	
						2.55	0.51	0			0.497	
3	3058	maximum			0.25	0.09	0.46	0		1	0.073	
						3.09	0.62	0			0.497	
3	3059	maximum			0.25	0.07	0.33	0		1	0.068	
						2.86	0.57	0			0.496	
3	3060	maximum			0.25	0.05	0.27	0		1	0.065	
						2.82	0.56	0			0.496	
3	3061	maximum			0.25	2.28	0.91	0		1	0.085	
						1.34	0.27	0			0.495	
3	3062	maximum			0.25	1.06	0.61	0		1	0.071	
						2.18	0.44	0			0.495	
3	3063	maximum			0.25	0.07	0.36	0		1	0.060	
						2.64	0.53	0			0.495	
3	3064	maximum			0.25	0.04	0.18	0		1	0.050	
						2.90	0.58	0			0.495	
3	3065	maximum			0.25	0.02	0.12	0		1	0.039	
						2.95	0.59	0			0.495	
3	3066	maximum			0.25	0.02	0.09	0		1	0.031	
						2.90	0.58	0			0.495	
3	3067	maximum			0.25	2.27	0.82	0		1	0.084	
						1.52	0.30	0			0.496	
3	3068	maximum			0.25	1.12	0.55	0		1	0.056	
						2.23	0.45	0			0.496	
3	3069	maximum			0.25	0.06	0.31	0		1	0.045	
						2.72	0.54	0			0.496	
3	3070	maximum			0.25	0.03	0.17	0		1	0.035	
						2.97	0.59	0			0.496	
3	3071	maximum			0.25	0.02	0.11	0		1	0.027	
						3.02	0.60	0			0.496	
3	3072	maximum			0.25	0.02	0.08	0		1	0.021	
						2.95	0.59	0			0.497	
3	3073	maximum			0.25	2.34	0.47	0		1	0.071	
						1.30	0.26	0			0.495	
3	3074	maximum			0.25	0.97	0.32	0		1	0.075	
						2.13	0.43	0			0.497	
3	3075	maximum			0.25	0.03	0.17	0		1	0.060	
						2.57	0.51	0			0.497	
3	3076	maximum			0.25	0.03	0.16	0		1	0.052	
						2.83	0.57	0			0.497	
3	3077	maximum			0.25	0.03	0.15	0		1	0.045	
						2.91	0.58	0			0.497	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΔΙΑΣΤΑΣΙΟΛΟΓΗΣΗ ΦΟΡΕΑ ΣΕΙΣΜΙΚΕΣ ΦΟΡΤΙΣΕΙΣ-γBd1

REINFORCEMENT ACC. TO DIN1045-1 2008 in [cm2/m] upper/lower
General load safety factor - as defined in BEMESS: Gamma-f = 1.00
Shear: stresses VEd/d and VRd,ct/d with d=effective depth = h-hm
Shear index 2m = minimum shear reinforcement

Grp	ELEM No	LC No	MAT No	GEO No	h [m]	Reinforcement main	cross	dir	dphi deg	Shr zon	VEd/d [MPa] VRd,ct/d	Ass [cm2/m2]
3	3078	maximum			0.25	0.03	0.15	0		1	0.041	
						2.90	0.58	0			0.497	
3	3079	maximum			0.25	2.23	0.45	0		1	0.109	
						1.31	0.26	0			0.495	
3	3080	maximum			0.25	0.93	0.19	0		1	0.090	
						2.15	0.43	0			0.495	
3	3081	maximum			0.25	0.01	0.06	0		1	0.076	
						2.60	0.52	0			0.495	
3	3082	maximum			0.25	0.01	0.03	0		1	0.061	
						2.87	0.57	0			0.495	
3	3083	maximum			0.25	0.01	0.03	0		1	0.044	
						2.93	0.59	0			0.496	
3	3084	maximum			0.25	0.01	0.03	0		1	0.028	
						2.92	0.58	0			0.496	
3	3085	maximum			0.25		0.02	0		1	0.021	
						2.78	0.56	0			0.496	
3	3086	maximum			0.25		0.02	0		1	0.031	
						2.82	0.56	0			0.496	
3	3087	maximum			0.25		0.02	0		1	0.042	
						2.76	0.55	0			0.496	
3	3088	maximum			0.25		0.02	0		1	0.053	
						2.52	0.50	0			0.495	
3	3089	maximum			0.25	0.77	0.15	0		1	0.063	
						2.13	0.43	0			0.495	
3	3090	maximum			0.25	1.98	0.40	0		1	0.085	
						1.42	0.28	0			0.495	
3	3091	maximum			0.25	0.02	0.11	0		1	0.029	
						2.77	0.55	0			0.496	
3	3092	maximum			0.25	0.02	0.11	0		1	0.034	
						2.83	0.57	0			0.496	
3	3093	maximum			0.25	0.02	0.11	0		1	0.041	
						2.77	0.55	0			0.496	
3	3094	maximum			0.25	0.02	0.11	0		1	0.048	
						2.52	0.50	0			0.496	
3	3095	maximum			0.25	0.82	0.19	0		1	0.061	
						2.11	0.42	0			0.496	
3	3096	maximum			0.25	1.98	0.40	0		1	0.057	
						1.51	0.30	0			0.495	
3	3097	maximum			0.25	0.04	0.19	0		1	0.026	
						2.76	0.55	0			0.496	
3	3098	maximum			0.25	0.04	0.20	0		1	0.029	
						2.84	0.57	0			0.496	
3	3099	maximum			0.25	0.04	0.20	0		1	0.034	
						2.79	0.56	0			0.496	
3	3100	maximum			0.25	0.04	0.20	0		1	0.043	
						2.55	0.51	0			0.495	
3	3101	maximum			0.25	0.85	0.25	0		1	0.054	
						2.11	0.42	0			0.495	
3	3102	maximum			0.25	2.03	0.61	0		1	0.094	
						1.54	0.31	0			0.496	
3	3103	maximum			0.25	0.02	0.11	0		1	0.018	
						2.77	0.55	0			0.496	
3	3104	maximum			0.25	0.03	0.13	0		1	0.028	
						2.82	0.56	0			0.496	
3	3105	maximum			0.25	0.04	0.19	0		1	0.039	
						2.77	0.55	0			0.496	
3	3106	maximum			0.25	0.06	0.28	0		1	0.052	
						2.52	0.50	0			0.495	
3	3107	maximum			0.25	0.90	0.45	0		1	0.070	
						2.10	0.42	0			0.495	
3	3108	maximum			0.25	2.00	0.71	0		1	0.094	
						1.37	0.27	0			0.495	
3	3109	maximum			0.25	0.03	0.17	0		1	0.034	
						2.76	0.55	0			0.496	
3	3110	maximum			0.25	0.05	0.25	0		1	0.040	
						2.83	0.57	0			0.496	
3	3111	maximum			0.25	0.08	0.38	0		1	0.052	
						2.80	0.56	0			0.496	
3	3112	maximum			0.25	0.12	0.59	0		1	0.064	
						2.56	0.51	0			0.496	
3	3113	maximum			0.25	1.05	0.96	0		1	0.090	
						2.12	0.42	0			0.496	
3	3114	maximum			0.25	2.20	1.19	0		1	0.104	
						1.57	0.31	0			0.496	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΔΙΑΣΤΑΣΙΟΛΟΓΗΣΗ ΦΟΡΕΑ ΣΕΙΣΜΙΚΕΣ ΦΟΡΤΙΣΕΙΣ-γBd1

REINFORCEMENT ACC. TO DIN1045-1 2008 in [cm2/m] upper/lower
General load safety factor - as defined in BEMESS: Gamma-f = 1.00
Shear: stresses VEd/d and VRd,ct/d with d=effective depth = h-hm
Shear index 2m = minimum shear reinforcement

Grp	ELEM No	LC No	MAT No	GEO No	h [m]	Reinforcement main	cross	dir	dphi deg	Shr zon	VEd/d [MPa] VRd,ct/d	Ass [cm2/m2]
3	3115	maximum			0.25	0.06	0.30	0		1	0.024	
						2.75	0.55	0			0.496	
3	3116	maximum			0.25	0.08	0.38	0		1	0.029	
						2.81	0.56	0			0.496	
3	3117	maximum			0.25	0.10	0.50	0		1	0.036	
						2.76	0.55	0			0.495	
3	3118	maximum			0.25	0.13	0.66	0		1	0.046	
						2.52	0.50	0			0.495	
3	3119	maximum			0.25	0.99	0.87	0		1	0.056	
						2.12	0.42	0			0.495	
3	3120	maximum			0.25	2.21	1.17	0		1	0.076	
						1.40	0.28	0			0.495	
3	3121	maximum			0.25	0.06	0.29	0		1	0.024	
						2.77	0.55	0			0.496	
3	3122	maximum			0.25	0.09	0.43	0		1	0.034	
						2.97	0.59	0			0.496	
3	3123	maximum			0.25	0.13	0.63	0		1	0.045	
						2.82	0.56	0			0.496	
3	3124	maximum			0.25	0.18	0.91	0		1	0.056	
						2.58	0.52	0			0.496	
3	3125	maximum			0.25	1.11	1.24	0		1	0.072	
						2.14	0.43	0			0.496	
3	3126	maximum			0.25	2.26	1.38	0		1	0.072	
						1.49	0.30	0			0.495	
3	3127	maximum			0.25	0.08	0.38	0		1	0.037	
						2.77	0.55	0			0.496	
3	3128	maximum			0.25	0.10	0.49	0		1	0.042	
						3.36	0.67	0			0.496	
3	3129	maximum			0.25	0.13	0.67	0		1	0.049	
						2.80	0.56	0			0.496	
3	3130	maximum			0.25	0.18	0.90	0		1	0.055	
						2.57	0.51	0			0.496	
3	3131	maximum			0.25	1.08	1.18	0		1	0.065	
						2.18	0.44	0			0.496	
3	3132	maximum			0.25	2.37	1.48	0		1	0.060	
						1.39	0.28	0			0.495	
3	3133	maximum			0.25	0.06	0.28	0		1	0.021	
						2.82	0.56	0			0.497	
3	3134	maximum			0.25	0.07	0.36	0		1	0.030	
						2.92	0.58	0			0.497	
3	3135	maximum			0.25	0.10	0.52	0		1	0.039	
						2.88	0.58	0			0.496	
3	3136	maximum			0.25	0.15	0.76	0		1	0.047	
						2.63	0.53	0			0.495	
3	3137	maximum			0.25	1.11	1.11	0		1	0.056	
						2.16	0.43	0			0.495	
3	3138	maximum			0.25	2.32	1.31	0		1	0.066	
						1.44	0.29	0			0.495	
3	3139	maximum			0.25	0.05	0.27	0		1	0.065	
						2.83	0.57	0			0.496	
3	3140	maximum			0.25	0.07	0.33	0		1	0.068	
						2.87	0.57	0			0.496	
3	3141	maximum			0.25	0.09	0.45	0		1	0.073	
						3.12	0.62	0			0.497	
3	3142	maximum			0.25	0.13	0.63	0		1	0.077	
						2.59	0.52	0			0.497	
3	3143	maximum			0.25	1.01	0.93	0		1	0.095	
						2.18	0.44	0			0.497	
3	3144	maximum			0.25	2.42	1.31	0		1	0.101	
						1.41	0.28	0			0.496	
3	3145	maximum			0.25	0.02	0.09	0		1	0.031	
						2.91	0.58	0			0.495	
3	3146	maximum			0.25	0.02	0.12	0		1	0.039	
						2.96	0.59	0			0.495	
3	3147	maximum			0.25	0.04	0.18	0		1	0.049	
						2.93	0.59	0			0.495	
3	3148	maximum			0.25	0.07	0.35	0		1	0.059	
						2.67	0.53	0			0.495	
3	3149	maximum			0.25	1.01	0.61	0		1	0.070	
						2.23	0.45	0			0.495	
3	3150	maximum			0.25	2.22	0.90	0		1	0.085	
						1.40	0.28	0			0.495	
3	3151	maximum			0.25	0.02	0.09	0		1	0.020	
						2.96	0.59	0			0.497	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΔΙΑΣΤΑΣΙΟΛΟΓΗΣΗ ΦΟΡΕΑ ΣΕΙΣΜΙΚΕΣ ΦΟΡΤΙΣΕΙΣ-γBd1

REINFORCEMENT ACC. TO DIN1045-1 2008 in [cm2/m] upper/lower
General load safety factor - as defined in BEMESS: Gamma-f = 1.00
Shear: stresses VEd/d and VRd,ct/d with d=effective depth = h-hm
Shear index 2m = minimum shear reinforcement

Grp	ELEM No	LC No	MAT No	GEO No	h [m]	Reinforcement main	cross	dir	dphi deg	Shr zon	VEd/d [MPa] VRd,ct/d	Ass [cm2/m2]
3	3152	maximum			0.25	0.02	0.11	0		1	0.027	
						3.03	0.61	0			0.496	
3	3153	maximum			0.25	0.03	0.17	0		1	0.035	
						3.00	0.60	0			0.496	
3	3154	maximum			0.25	0.06	0.30	0		1	0.045	
						2.75	0.55	0			0.496	
3	3155	maximum			0.25	1.07	0.55	0		1	0.056	
						2.28	0.46	0			0.496	
3	3156	maximum			0.25	2.21	0.80	0		1	0.085	
						1.57	0.31	0			0.496	
3	3157	maximum			0.25	0.03	0.15	0		1	0.041	
						2.91	0.58	0			0.497	
3	3158	maximum			0.25	0.03	0.15	0		1	0.045	
						2.93	0.59	0			0.497	
3	3159	maximum			0.25	0.03	0.16	0		1	0.052	
						2.86	0.57	0			0.497	
3	3160	maximum			0.25	0.03	0.17	0		1	0.059	
						2.61	0.52	0			0.497	
3	3161	maximum			0.25	0.91	0.31	0		1	0.074	
						2.18	0.44	0			0.497	
3	3162	maximum			0.25	2.27	0.45	0		1	0.072	
						1.35	0.27	0			0.495	
3	3163	maximum			0.25	0.01	0.03	0		1	0.027	
						2.93	0.59	0			0.496	
3	3164	maximum			0.25	0.01	0.03	0		1	0.044	
						2.95	0.59	0			0.496	
3	3165	maximum			0.25	0.01	0.03	0		1	0.060	
						2.90	0.58	0			0.495	
3	3166	maximum			0.25	0.01	0.05	0		1	0.076	
						2.64	0.53	0			0.495	
3	3167	maximum			0.25	0.88	0.18	0		1	0.089	
						2.19	0.44	0			0.495	
3	3168	maximum			0.25	2.17	0.43	0		1	0.109	
						1.36	0.27	0			0.495	
4	4001	maximum			0.25	9.18	1.84	0		1		
						2.18	0.44	0			0.497	
4	4002	maximum			0.25	3.54	0.71	0		1	0.412	
						0.92	0.23	0			0.495	
4	4003	maximum			0.25	7.04	1.99	0		2	0.513	9.85
						1.51	0.64	0			0.495	
4	4004	maximum			0.25	3.07	0.61	0		1	0.369	
						0.68	0.62	0			0.496	
4	4005	maximum			0.25	7.14	2.26	0		2	0.822	15.80
						1.51	0.44	0			0.495	
4	4006	maximum			0.25	3.22	0.98	0		2	0.534	10.26
						0.73	0.34	0			0.496	
4	4007	maximum			0.25	8.72	2.83	0		1		
						1.80	0.36	0			0.497	
4	4008	maximum			0.25	3.37	1.23	0		1	0.309	
						0.79	0.16	0			0.495	
4	4009	maximum			0.25	7.15	2.87	0		2	0.723	13.89
						1.36	0.27	0			0.495	
4	4010	maximum			0.25	3.50	1.58	0		1	0.410	
						0.71	0.14	0			0.496	
4	4011	maximum			0.25	7.62	3.26	0		2	0.621	11.94
						1.49	0.30	0			0.495	
4	4012	maximum			0.25	3.47	1.50	0		1	0.318	
						0.69	0.14	0			0.496	
4	4013	maximum			0.25	7.61	3.27	0		2	0.616	11.84
						1.42	0.28	0			0.495	
4	4014	maximum			0.25	3.61	1.74	0		1	0.284	
						0.72	0.14	0			0.495	
4	4015	maximum			0.25	7.56	3.00	0		2	0.648	12.45
						1.45	0.29	0			0.495	
4	4016	maximum			0.25	3.69	1.71	0		1	0.313	
						0.69	0.14	0			0.495	
4	4017	maximum			0.25	7.83	3.62	0		2	0.598	11.50
						1.36	0.27	0			0.495	
4	4018	maximum			0.25	3.53	1.64	0		1	0.294	
						0.65	0.13	0			0.495	
4	4019	maximum			0.25	7.22	2.53	0		2	0.815	15.65
						1.33	0.30	0			0.495	
4	4020	maximum			0.25	3.64	1.54	0		1	0.451	
						0.75	0.15	0			0.496	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΔΙΑΣΤΑΣΙΟΛΟΓΗΣΗ ΦΟΡΕΑ ΣΕΙΣΜΙΚΕΣ ΦΟΡΤΙΣΕΙΣ-γBd1

REINFORCEMENT ACC. TO DIN1045-1 2008 in [cm2/m] upper/lower
General load safety factor - as defined in BEMESS: Gamma-f = 1.00
Shear: stresses VEd/d and VRd,ct/d with d=effective depth = h-hm
Shear index 2m = minimum shear reinforcement

Grp	ELEM No	LC No	MAT No	GEO No	h [m]	Reinforcement main	cross	dir	dphi deg	Shr zon	VEd/d [MPa] VRd,ct/d	Ass [cm2/m2]
4	4021	maximum			0.25	9.10	3.01	0		1		
						1.65	0.33	0			0.497	
4	4022	maximum			0.25	3.62	1.41	0		1	0.308	
						0.73	0.15	0			0.496	
4	4023	maximum			0.25	7.62	2.92	0		2	0.773	14.84
						1.41	0.28	0			0.495	
4	4024	maximum			0.25	3.37	1.38	0		2	0.507	9.75
						0.64	0.18	0			0.496	
4	4025	maximum			0.25	7.34	1.69	0		2	0.634	12.18
						1.64	0.99	0			0.495	
4	4026	maximum			0.25	3.23	0.65	0		1	0.398	
						0.68	0.76	0			0.496	
4	4027	maximum			0.25	9.79	1.96	0		1		
						1.97	0.42	0			0.498	
4	4028	maximum			0.25	3.73	0.75	0		1	0.457	
						0.94	0.28	0			0.495	
4	4029	maximum			0.25	3.46	0.69	0		1	0.403	
						0.99	0.22	0			0.495	
4	4030	maximum			0.25	8.97	1.79	0		1		
						2.35	0.47	0			0.497	
4	4031	maximum			0.25	3.01	0.60	0		1	0.360	
						0.74	0.60	0			0.496	
4	4032	maximum			0.25	6.89	1.95	0		2	0.501	9.63
						1.64	0.63	0			0.495	
4	4033	maximum			0.25	3.15	0.98	0		2	0.524	10.07
						0.80	0.34	0			0.496	
4	4034	maximum			0.25	6.99	2.22	0		2	0.809	15.54
						1.65	0.48	0			0.495	
4	4035	maximum			0.25	3.29	1.22	0		1	0.304	
						0.86	0.17	0			0.495	
4	4036	maximum			0.25	8.53	2.78	0		1		
						1.95	0.39	0			0.497	
4	4037	maximum			0.25	3.42	1.58	0		1	0.400	
						0.78	0.16	0			0.496	
4	4038	maximum			0.25	7.00	2.84	0		2	0.708	13.61
						1.48	0.30	0			0.495	
4	4039	maximum			0.25	3.40	1.49	0		1	0.312	
						0.75	0.15	0			0.496	
4	4040	maximum			0.25	7.46	3.21	0		2	0.610	11.71
						1.63	0.33	0			0.495	
4	4041	maximum			0.25	3.53	1.73	0		1	0.279	
						0.79	0.16	0			0.495	
4	4042	maximum			0.25	7.45	3.23	0		2	0.605	11.61
						1.56	0.31	0			0.495	
4	4043	maximum			0.25	3.61	1.69	0		1	0.307	
						0.75	0.15	0			0.495	
4	4044	maximum			0.25	7.40	2.96	0		2	0.635	12.20
						1.58	0.32	0			0.495	
4	4045	maximum			0.25	3.46	1.62	0		1	0.288	
						0.72	0.14	0			0.495	
4	4046	maximum			0.25	7.67	3.56	0		2	0.586	11.26
						1.50	0.30	0			0.495	
4	4047	maximum			0.25	3.57	1.52	0		1	0.441	
						0.72	0.14	0			0.496	
4	4048	maximum			0.25	7.07	2.48	0		2	0.800	15.37
						1.46	0.34	0			0.495	
4	4049	maximum			0.25	3.54	1.39	0		1	0.302	
						0.79	0.16	0			0.496	
4	4050	maximum			0.25	8.90	2.96	0		1		
						1.81	0.36	0			0.497	
4	4051	maximum			0.25	3.30	1.38	0		2	0.497	9.54
						0.72	0.18	0			0.496	
4	4052	maximum			0.25	7.46	2.88	0		2	0.757	14.55
						1.41	0.28	0			0.495	
4	4053	maximum			0.25	3.17	0.63	0		1	0.390	
						0.72	0.75	0			0.496	
4	4054	maximum			0.25	7.18	1.65	0		2	0.623	11.96
						1.50	0.98	0			0.495	
4	4055	maximum			0.25	3.65	0.73	0		1	0.449	
						1.01	0.28	0			0.495	
4	4056	maximum			0.25	9.57	1.91	0		1		
						2.14	0.44	0			0.498	
8	8001	maximum			1.20	3.41	9.34	0		1	0.135	
						4.52	17.05	0			0.294	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΔΙΑΣΤΑΣΙΟΛΟΓΗΣΗ ΦΟΡΕΑ ΣΕΙΣΜΙΚΕΣ ΦΟΡΤΙΣΕΙΣ-γBd1

REINFORCEMENT ACC. TO DIN1045-1 2008 in [cm2/m] upper/lower
General load safety factor - as defined in BEMESS: Gamma-f = 1.00
Shear: stresses VEd/d and VRd,ct/d with d=effective depth = h-hm
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Grp	ELEM No	LC No	MAT No	GEO No	h [m]	Reinforcement main	cross	dir	dphi deg	Shr zon	VEd/d [MPa] VRd,ct/d	Ass [cm2/m2]
8	8002	maximum			1.20	4.22	21.09	0		1	0.196	
						7.90	39.48	0			0.316	
8	8003	maximum			1.20	13.20	27.76	0		1	0.221	
						31.93	76.82	0			0.406	
8	8004	maximum			1.20	5.92	15.91	0		1	0.251	
						6.73	33.64	0			0.308	
8	8005	maximum			1.20	13.85	23.04	0		1	0.269	
						33.19	71.75	0			0.409	
8	8006	maximum			1.20	1.90	9.48	0		1	0.296	
						5.23	26.13	0			0.348	
8	8007	maximum			1.20	9.39	6.51	0		1	0.120	
						6.98	13.20	0			0.317	
8	8008	maximum			1.20	1.85	9.25	0		2m	0.347	4.92
						5.19	25.96	0			0.375	
8	8009	maximum			1.20	15.19	22.62	0		1	0.165	
						34.04	66.34	0			0.395	
8	8010	maximum			1.20	1.81	9.06	0		2	0.402	5.64
						5.02	25.09	0			0.383	
8	8011	maximum			1.20	14.30	18.85	0		1	0.178	
						25.66	52.13	0			0.353	
8	8012	maximum			1.20	2.21	11.03	0		1	0.242	
						5.86	29.31	0			0.406	
8	8013	maximum			1.20	11.76	17.94	0		1	0.154	
						23.30	48.82	0			0.338	
8	8014	maximum			1.20	2.83	14.13	0		1	0.235	
						6.65	33.25	0			0.412	
8	8015	maximum			1.20	11.40	17.85	0		1	0.167	
						22.79	48.40	0			0.342	
8	8016	maximum			1.20	2.80	14.01	0		1	0.237	
						6.71	33.56	0			0.411	
8	8017	maximum			1.20	14.72	18.52	0		1	0.173	
						26.52	53.42	0			0.360	
8	8018	maximum			1.20	2.14	10.70	0		1	0.251	
						5.97	29.84	0			0.406	
8	8019	maximum			1.20	14.66	21.97	0		1	0.168	
						33.98	67.05	0			0.396	
8	8020	maximum			1.20	1.74	8.71	0		2	0.408	5.73
						5.10	25.52	0			0.384	
8	8021	maximum			1.20	11.24	6.37	0		1	0.140	
						6.23	13.15	0			0.313	
8	8022	maximum			1.20	1.76	8.81	0		2	0.369	5.19
						5.24	26.21	0			0.374	
8	8023	maximum			1.20	14.42	21.80	0		1	0.285	
						35.43	75.15	0			0.421	
8	8024	maximum			1.20	1.77	8.84	0		1	0.320	
						5.32	26.59	0			0.348	
8	8025	maximum			1.20	12.71	26.06	0		1	0.234	
						32.98	79.43	0			0.417	
8	8026	maximum			1.20	6.24	15.21	0		1	0.270	
						6.79	33.94	0			0.316	
8	8027	maximum			1.20	3.61	8.72	0		1	0.143	
						4.57	17.12	0			0.298	
8	8028	maximum			1.20	4.03	20.17	0		1	0.243	
						7.98	39.88	0			0.312	
8	8029	maximum			1.20	3.51	9.80	0		1	0.132	
						4.48	16.77	0			0.293	
8	8030	maximum			1.20	13.90	29.60	0		1	0.218	
						31.27	74.92	0			0.402	
8	8031	maximum			1.20	4.42	22.11	0		1	0.196	
						7.75	38.75	0			0.316	
8	8032	maximum			1.20	5.85	16.85	0		1	0.247	
						6.58	32.89	0			0.306	
8	8033	maximum			1.20	14.56	24.77	0		1	0.267	
						32.39	69.77	0			0.405	
8	8034	maximum			1.20	9.60	6.95	0		1	0.119	
						7.16	12.94	0			0.316	
8	8035	maximum			1.20	16.01	24.22	0		1	0.164	
						33.53	64.66	0			0.391	
8	8036	maximum			1.20	2.06	10.32	0		1	0.291	
						5.09	25.43	0			0.348	
8	8037	maximum			1.20	2.01	10.06	0		1	0.342	
						5.05	25.25	0			0.376	
8	8038	maximum			1.20	1.96	9.78	0		2	0.395	5.54
						4.87	24.37	0			0.384	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΔΙΑΣΤΑΣΙΟΛΟΓΗΣΗ ΦΟΡΕΑ ΣΕΙΣΜΙΚΕΣ ΦΟΡΤΙΣΕΙΣ-γBd1

REINFORCEMENT ACC. TO DIN1045-1 2008 in [cm2/m] upper/lower
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Grp	ELEM No	LC No	MAT No	GEO No	h [m]	Reinforcement main	Reinforcement cross	dphi dir	dphi deg	Shr zon	VEd/d [MPa] VRd,ct/d	Ass [cm2/m2]
8	8039	maximum			1.20	14.96	20.11	0		1	0.178	
						25.35	50.80	0			0.349	
8	8040	maximum			1.20	12.29	19.12	0		1	0.154	
						22.95	47.57	0			0.335	
8	8041	maximum			1.20	2.35	11.77	0		1	0.239	
						5.68	28.39	0			0.408	
8	8042	maximum			1.20	3.00	15.00	0		1	0.232	
						6.48	32.40	0			0.413	
8	8043	maximum			1.20	11.96	19.14	0		1	0.167	
						22.47	47.25	0			0.339	
8	8044	maximum			1.20	15.19	19.81	0		1	0.173	
						26.01	52.10	0			0.356	
8	8045	maximum			1.20	2.99	14.93	0		1	0.234	
						6.54	32.72	0			0.412	
8	8046	maximum			1.20	2.32	11.60	0		1	0.248	
						5.82	29.08	0			0.406	
8	8047	maximum			1.20	15.30	23.59	0		1	0.166	
						33.28	65.35	0			0.392	
8	8048	maximum			1.20	11.12	6.73	0		1	0.140	
						6.18	12.84	0			0.312	
8	8049	maximum			1.20	15.12	23.55	0		1	0.283	
						34.61	73.17	0			0.417	
8	8050	maximum			1.20	1.90	9.49	0		2	0.400	5.62
						4.97	24.86	0			0.384	
8	8051	maximum			1.20	1.92	9.61	0		2	0.364	5.11
						5.10	25.51	0			0.377	
8	8052	maximum			1.20	1.92	9.60	0		1	0.315	
						5.16	25.82	0			0.348	
8	8053	maximum			1.20	13.34	27.81	0		1	0.231	
						32.24	77.45	0			0.413	
8	8054	maximum			1.20	3.75	9.14	0		1	0.139	
						4.53	16.80	0			0.297	
8	8055	maximum			1.20	6.18	16.14	0		1	0.266	
						6.65	33.24	0			0.315	
8	8056	maximum			1.20	4.21	21.07	0		1	0.243	
						7.81	39.05	0			0.312	

Explanations shear state Shr zon:
1 = check without necessary shear reinforcement
2 = shear reinforcement required
m = minimum shear reinforcement
Acc. DIN 1045-1 10.3.4(2) the leverarm z was limited to d-2*nomc.

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΔΙΑΣΤΑΣΙΟΛΟΓΗΣΗ ΦΟΡΕΑ ΣΕΙΣΜΙΚΕΣ ΦΟΡΤΙΣΕΙΣ-γBd1

Maximal values of the shear design
Only elements with shear reinforcement are printed.
At punching points punching reinforcement is printed.

element	ass [cm2/m2]	tau [MPa]	acc.VED/VRDmax	acc.cot_theta	min_z [m]
4003	9.85	0.51	0.205	1.75	0.178
4005	15.80	0.82	0.328	1.75	0.177
4006	10.26	0.53	0.213	1.75	0.182
4009	13.89	0.72	0.289	1.75	0.177
4011	11.94	0.62	0.248	1.75	0.175
4013	11.84	0.62	0.246	1.75	0.175
4015	12.45	0.65	0.259	1.75	0.175
4017	11.50	0.60	0.239	1.75	0.175
4019	15.65	0.81	0.325	1.75	0.177
4023	14.84	0.77	0.308	1.75	0.145
4024	9.75	0.51	0.203	1.75	0.145
4025	12.18	0.63	0.253	1.75	0.145
4032	9.63	0.50	0.200	1.75	0.178
4033	10.07	0.52	0.209	1.75	0.182
4034	15.54	0.81	0.323	1.75	0.177
4038	13.61	0.71	0.283	1.75	0.177
4040	11.71	0.61	0.243	1.75	0.176
4042	11.61	0.60	0.241	1.75	0.176
4044	12.20	0.63	0.253	1.75	0.176
4046	11.26	0.59	0.234	1.75	0.175
4048	15.37	0.80	0.319	1.75	0.177
4051	9.54	0.50	0.198	1.75	0.145
4052	14.55	0.76	0.302	1.75	0.177
4054	11.96	0.62	0.249	1.75	0.145
8008	4.92	0.35	0.098	1.75	1.066
8010	5.64	0.40	0.112	1.75	1.067
8020	5.73	0.41	0.113	1.75	1.065
8022	5.19	0.37	0.104	1.75	1.063
8038	5.54	0.39	0.109	1.75	1.068
8050	5.62	0.40	0.111	1.75	1.067
8051	5.11	0.36	0.102	1.75	1.065

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
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Selected Beam Elements

FROM	TO	INC	X-VALUE	NC	MEMBER	CS0	CS1	CS2	CS3	CS4	CS5
10001											
10005											
10006											
10009											
10010											
10014											
10016											
10020											
10021											
10024											
10025											
10029											
12000	12150	1									

Default design code is DIN Fachbericht 102 Massivbrȯcken (2003) (Germany)
Klasse(Tab.4.118): D

Materials

No. 1 C 25/30 (DIN 1045-1)
No. 3 C 25/30 (DIN 1045-1)
No. 4 C 25/30 (DIN 1045-1)
No. 5 C 25/30 (DIN 1045-1)
No. 6 C 25/30 (DIN 1045-1)
No. 7 C 25/30 (DIN 1045-1)
No. 8 C 25/30 (DIN 1045-1)
No. 9 C 25/30 (DIN 1045-1)
No. 10 C 25/30 (DIN 1045-1)
No. 12 BSt 500 SA (DIN 1045-1)

Reinforcement will be accounted for sectional values as defined in AQUA
Reinforcements saved as design case LCR 511

Considered Load Cases

9101	9102	9103	9104	9105	9106
9107	9108	9201	9202	9203	9204
9205	9206	9207	9301	9302	9303
9304	9305	9306	9307	9308	

Ultimate Load Design

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Design for ultimate loads DIN Fachbericht 102 Massivbrȯcken (2003)
Biaxial bending, uniaxial stress calculated in y-z axis

Safety factors	SC-1	SC-2	SC-S	SS-1	SS-2	PIIa
	1.50	1.50	1.88	1.15	1.15	7
Strain limits	C1	C2	S1	S2	Z1	Z2
max	-3.50	-2.00	3.00	25.00	-3.50	25.00

parameters for reinforcements

Minimum reinforcements Bending.	Compress.	compression e/d	N/Npl	min. reinforcem. requ. section	maximum- reinforc.
0.00 [cm2]	0.30 [o/o]	3.50	0.0010	0.00	0.15	9.00

Tensile forces in the longitudinal reinforcements due to shear are NOT accounted for.
Material of sections uses Ultimate Limit strain-stress law with global safety factors
Material of reinforcements uses Ultimate Limit strain-stress law with global safety factors

MNo.	temp lev.	Material- safety [-]	max.compr stress [MPa]	at strain [o/oo]	max.tens stress [MPa]	at strain [o/oo]	tension- stiffening [MPa]
1	0	1.500	-14.17	-2.00	0.00	0.00	
3	0	1.500	-14.17	-2.00	0.00	0.00	
4	0	1.500	-14.17	-2.00	0.00	0.00	
5	0	1.500	-14.17	-2.00	0.00	0.00	
6	0	1.500	-14.17	-2.00	0.00	0.00	
7	0	1.500	-14.17	-2.00	0.00	0.00	
8	0	1.500	-14.17	-2.00	0.00	0.00	
9	0	1.500	-14.17	-2.00	0.00	0.00	
10	0	1.500	-14.17	-2.00	0.00	0.00	
12	0	1.150	-456.52	-25.00	456.52	25.00	

Shear Design

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Design for shear DIN 1045-1 (2003)
Minimum shear factor or tan of inclination of compressive struts 0.57 / 1.72

MNo	f-cd [MPa]	tau-rd [MPa]	sigIIQ [MPa]	sigIIT [MPa]	sigIIQ+ [MPa]	fyd [MPa]
1	11.33	0.08	8.50	5.95	8.50	
3	11.33	0.08	8.50	5.95	8.50	
4	11.33	0.08	8.50	5.95	8.50	
5	11.33	0.08	8.50	5.95	8.50	
6	11.33	0.08	8.50	5.95	8.50	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
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Shear Design

==========

Design for shear DIN 1045-1 (2003)
Minimum shear factor or tan of inclination of compressive struts 0.57 / 1.72

MNo	f-cd [MPa]	tau-rd [MPa]	sigIIQ [MPa]	sigIIT [MPa]	sigIIQ+ [MPa]	fyd [MPa]
7	11.33	0.08	8.50	5.95	8.50	
8	11.33	0.08	8.50	5.95	8.50	
9	11.33	0.08	8.50	5.95	8.50	
10	11.33	0.08	8.50	5.95	8.50	
12						434.78

Tolerance for exceeding maximum shear or principal compression stress 0.0200

Longitudinal Reinforcements LCR 511
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
10001	0.000	8	0.31	41.55		21.86T	9.84		9.84	
10001	0.200	8	0.05	6.87		6.87T				
10005	0.000	8	0.78	103.07		65.92T	10.10		27.05	
10005	0.200	8	0.72	95.43		65.91T	6.63		22.88	
10006	0.000	8	0.03	4.28		2.68T			1.60	
10006	0.400	8	0.04	5.93		3.13T			2.80	
10009	0.000	8	0.04	5.61		2.83T			2.78	
10009	0.400	8	0.03	3.90		2.39T			1.50	
10010	0.000	8	0.72	95.47		65.75T	6.75		22.97	
10010	0.200	8	0.78	102.94		65.76T	10.16		27.03	
10014	0.000	8	0.05	6.93		6.93T				
10014	0.200	8	0.32	41.68		21.97T	9.85		9.85	
10016	0.000	8	0.31	41.31		21.59T	9.86		9.86	
10016	0.200	8	0.05	6.71		6.71T				
10020	0.000	8	0.77	102.15		64.79T	10.08		27.27	
10020	0.200	8	0.72	94.60		64.78T	6.61		23.20	
10021	0.000	8	0.02	2.51		2.51T				
10021	0.400	8	0.04	5.43		3.06T			2.37	
10024	0.000	8	0.04	5.69		2.76T			2.92	
10024	0.400	8	0.03	3.99		2.38T			1.61	
10025	0.000	8	0.71	94.27		64.51T	6.75		23.01	
10025	0.200	8	0.77	101.76		64.52T	10.16		27.08	
10029	0.000	8	0.05	7.00		7.00T				
10029	0.200	8	0.32	42.51		22.06T	10.22		10.22	
12001	0.000	9	3.05	153.41			153.41T			
12001	1.000	9	2.00	100.44			100.44T			
12002	0.000	9	2.00	100.52			100.52T			
12002	1.000	9	0.98	49.02			49.02T			
12003	0.000	9	0.98	49.18			49.18T			
12003	1.000	9	0.40	20.00			20.00T			
12004	0.000	9	3.08	154.57			154.57T			
12004	1.000	9	1.99	100.06			100.06T			
12005	0.000	9	1.99	100.18			100.18T			
12005	1.000	9	0.93	46.92			46.92T			
12006	0.000	9	0.94	47.17			47.17T			
12006	1.000	9	0.30	15.08			15.08T			
12007	0.000	9	3.07	154.39			154.39T			
12007	1.000	9	2.01	100.94			100.94T			
12008	0.000	9	2.01	101.03			101.03T			
12008	1.000	9	0.98	49.05			49.05T			
12009	0.000	9	0.98	49.22			49.22T			
12009	1.000	9	0.38	18.90			18.90T			
12010	0.000	9	2.96	148.88			148.88T			
12010	1.000	9	1.94	97.37			97.37T			
12011	0.000	9	1.94	97.44			97.44T			
12011	1.000	9	0.95	47.55			47.55T			
12012	0.000	9	0.95	47.70			47.70T			
12012	1.000	9	0.38	19.20			19.20T			
12013	0.000	9	2.98	149.88			149.88T			
12013	1.000	9	1.93	96.87			96.87T			
12014	0.000	9	1.93	96.98			96.98T			
12014	1.000	9	0.90	45.38			45.38T			
12015	0.000	9	0.91	45.61			45.61T			
12015	1.000	9	0.30	15.08			15.08T			
12016	0.000	9	2.98	149.85			149.85T			
12016	1.000	9	1.95	97.87			97.87T			
12017	0.000	9	1.95	97.95			97.95T			
12017	1.000	9	0.95	47.57			47.57T			
12018	0.000	9	0.95	47.74			47.74T			
12018	1.000	9	0.38	18.98			18.98T			
12019	0.000	9	0.40	20.16			20.16T			
12019	1.000	9	0.65	32.46			32.46T			
12020	0.000	9	0.65	32.46			32.46T			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
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Longitudinal Reinforcements LCR 511
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
12020	1.000	9	0.78	39.18			39.18T			
12021	0.000	9	0.78	39.18			39.18T			
12021	1.000	9	0.86	43.08			43.08T			
12022	0.000	9	0.86	43.08			43.08T			
12022	1.000	9	0.80	40.44			40.44T			
12023	0.000	9	0.80	40.44			40.44T			
12023	1.000	9	0.68	34.09			34.09T			
12024	0.000	9	0.68	34.09			34.09T			
12024	1.000	9	0.52	26.36			26.36T			
12025	0.000	9	0.52	26.36			26.36T			
12025	1.000	9	0.38	18.97			18.97T			
12026	0.000	9	0.38	18.97			18.97T			
12026	1.000	9	0.30	15.08			15.08T			
12027	0.000	9	0.30	15.08			15.08T			
12027	1.000	9	0.30	15.08			15.08T			
12028	0.000	9	0.30	15.08			15.08T			
12028	1.000	9	0.30	15.08			15.08T			
12029	0.000	9	0.30	15.08			15.08T			
12029	1.000	9	0.30	15.08			15.08T			
12030	0.000	9	0.30	15.08			15.08T			
12030	1.000	9	0.30	15.08			15.08T			
12031	0.000	9	0.30	15.08			15.08T			
12031	1.000	9	0.30	15.08			15.08T			
12032	0.000	9	0.30	15.08			15.08T			
12032	1.000	9	0.30	15.08			15.08T			
12033	0.000	9	0.30	15.08			15.08T			
12033	1.000	9	0.30	15.08			15.08T			
12034	0.000	9	0.30	15.08			15.08T			
12034	1.000	9	0.30	15.08			15.08T			
12035	0.000	9	0.30	15.08			15.08T			
12035	1.000	9	0.30	15.08			15.08T			
12036	0.000	9	0.30	15.08			15.08T			
12036	1.000	9	0.30	15.08			15.08T			
12037	0.000	9	0.30	15.08			15.08T			
12037	1.000	9	0.30	15.08			15.08T			
12038	0.000	9	0.30	15.08			15.08T			
12038	1.000	9	0.30	15.08			15.08T			
12039	0.000	9	0.30	15.08			15.08T			
12039	1.000	9	0.30	15.08			15.08T			
12040	0.000	9	0.30	15.08			15.08			
12040	1.000	9	0.30	15.08			15.08			
12041	0.000	9	0.30	15.08			15.08T			
12041	1.000	9	0.43	21.52			21.52T			
12042	0.000	9	0.43	21.52			21.52T			
12042	1.000	9	0.69	34.66			34.66T			
12043	0.000	9	0.69	34.66			34.66T			
12043	1.000	9	0.77	38.65			38.65T			
12044	0.000	9	0.77	38.65			38.65T			
12044	1.000	9	0.72	35.99			35.99T			
12045	0.000	9	0.72	35.99			35.99T			
12045	1.000	9	0.59	29.57			29.57T			
12046	0.000	9	0.59	29.57			29.57T			
12046	1.000	9	0.43	21.76			21.76T			
12047	0.000	9	0.43	21.76			21.76T			
12047	1.000	9	0.30	15.08			15.08T			
12048	0.000	9	0.30	15.08			15.08T			
12048	1.000	9	0.30	15.08			15.08T			
12049	0.000	9	0.30	15.08			15.08T			
12049	1.000	9	0.30	15.08			15.08T			
12050	0.000	9	0.30	15.08			15.08T			
12050	1.000	9	0.30	15.08			15.08T			
12051	0.000	9	0.30	15.08			15.08T			
12051	1.000	9	0.30	15.08			15.08T			
12052	0.000	9	0.30	15.08			15.08T			
12052	1.000	9	0.30	15.08			15.08T			
12053	0.000	9	0.30	15.08			15.08T			
12053	1.000	9	0.30	15.08			15.08T			
12054	0.000	9	0.30	15.08			15.08T			
12054	1.000	9	0.30	15.08			15.08T			
12055	0.000	9	0.30	15.08			15.08T			
12055	1.000	9	0.30	15.08			15.08T			
12056	0.000	9	0.30	15.08			15.08T			
12056	1.000	9	0.30	15.08			15.08T			
12057	0.000	9	0.30	15.08			15.08T			
12057	1.000	9	0.30	15.08			15.08T			
12058	0.000	9	0.30	15.08			15.08T			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
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Longitudinal Reinforcements LCR 511
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
12058	1.000	9	0.30	15.08			15.08T			
12059	0.000	9	0.30	15.08			15.08T			
12059	1.000	9	0.30	15.08			15.08T			
12060	0.000	9	0.30	15.08			15.08T			
12060	1.000	9	0.30	15.08			15.08T			
12061	0.000	9	0.30	15.08			15.08T			
12061	1.000	9	0.30	15.08			15.08T			
12062	0.000	9	0.30	15.08			15.08'			
12062	1.000	9	0.30	15.08			15.08'			
12063	0.000	9	0.38	19.04			19.04T			
12063	1.000	9	0.62	31.24			31.24T			
12064	0.000	9	0.62	31.24			31.24T			
12064	1.000	9	0.76	38.40			38.40T			
12065	0.000	9	0.76	38.40			38.40T			
12065	1.000	9	0.84	42.37			42.37T			
12066	0.000	9	0.84	42.37			42.37T			
12066	1.000	9	0.79	39.76			39.76T			
12067	0.000	9	0.79	39.76			39.76T			
12067	1.000	9	0.66	33.41			33.41T			
12068	0.000	9	0.66	33.41			33.41T			
12068	1.000	9	0.51	25.68			25.68T			
12069	0.000	9	0.51	25.68			25.68T			
12069	1.000	9	0.36	18.28			18.28T			
12070	0.000	9	0.36	18.28			18.28T			
12070	1.000	9	0.30	15.08			15.08T			
12071	0.000	9	0.30	15.08			15.08T			
12071	1.000	9	0.30	15.08			15.08T			
12072	0.000	9	0.30	15.08			15.08T			
12072	1.000	9	0.30	15.08			15.08T			
12073	0.000	9	0.30	15.08			15.08T			
12073	1.000	9	0.30	15.08			15.08T			
12074	0.000	9	0.30	15.08			15.08T			
12074	1.000	9	0.30	15.08			15.08T			
12075	0.000	9	0.30	15.08			15.08T			
12075	1.000	9	0.30	15.08			15.08T			
12076	0.000	9	0.30	15.08			15.08T			
12076	1.000	9	0.30	15.08			15.08T			
12077	0.000	9	0.30	15.08			15.08T			
12077	1.000	9	0.30	15.08			15.08T			
12078	0.000	9	0.30	15.08			15.08T			
12078	1.000	9	0.30	15.08			15.08T			
12079	0.000	9	0.30	15.08			15.08T			
12079	1.000	9	0.30	15.08			15.08T			
12080	0.000	9	0.30	15.08			15.08T			
12080	1.000	9	0.30	15.08			15.08T			
12081	0.000	9	0.30	15.08			15.08T			
12081	1.000	9	0.30	15.08			15.08T			
12082	0.000	9	0.30	15.08			15.08T			
12082	1.000	9	0.30	15.08			15.08T			
12083	0.000	9	0.30	15.08			15.08T			
12083	1.000	9	0.30	15.08			15.08T			
12084	0.000	9	0.30	15.08			15.08			
12084	1.000	9	0.30	15.08			15.08			
12085	0.000	9	0.38	19.25			19.25T			
12085	1.000	9	0.62	31.31			31.31T			
12086	0.000	9	0.62	31.31			31.31T			
12086	1.000	9	0.81	40.67			40.67T			
12087	0.000	9	0.81	40.67			40.67T			
12087	1.000	9	0.89	44.79			44.79T			
12088	0.000	9	0.89	44.79			44.79T			
12088	1.000	9	0.84	42.10			42.10T			
12089	0.000	9	0.84	42.10			42.10T			
12089	1.000	9	0.71	35.53			35.53T			
12090	0.000	9	0.71	35.53			35.53T			
12090	1.000	9	0.55	27.52			27.52T			
12091	0.000	9	0.55	27.52			27.52T			
12091	1.000	9	0.39	19.72			19.72T			
12092	0.000	9	0.39	19.72			19.72T			
12092	1.000	9	0.30	15.08			15.08T			
12093	0.000	9	0.30	15.08			15.08T			
12093	1.000	9	0.30	15.08			15.08T			
12094	0.000	9	0.30	15.08			15.08T			
12094	1.000	9	0.30	15.08			15.08T			
12095	0.000	9	0.30	15.08			15.08T			
12095	1.000	9	0.30	15.08			15.08T			
12096	0.000	9	0.30	15.08			15.08T			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN-ΣΕΙΣΜΙΚΑ-BEAM-γBd1_ΠΑΣ/ΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Longitudinal Reinforcements LCR 511
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
12096	1.000	9	0.30	15.08			15.08T			
12097	0.000	9	0.30	15.08			15.08T			
12097	1.000	9	0.30	15.08			15.08T			
12098	0.000	9	0.30	15.08			15.08T			
12098	1.000	9	0.30	15.08			15.08T			
12099	0.000	9	0.30	15.08			15.08T			
12099	1.000	9	0.30	15.08			15.08T			
12100	0.000	9	0.30	15.08			15.08T			
12100	1.000	9	0.30	15.08			15.08T			
12101	0.000	9	0.30	15.08			15.08T			
12101	1.000	9	0.30	15.08			15.08T			
12102	0.000	9	0.30	15.08			15.08T			
12102	1.000	9	0.30	15.08			15.08T			
12103	0.000	9	0.30	15.08			15.08T			
12103	1.000	9	0.30	15.08			15.08T			
12104	0.000	9	0.30	15.08			15.08T			
12104	1.000	9	0.30	15.08			15.08T			
12105	0.000	9	0.30	15.08			15.08T			
12105	1.000	9	0.30	15.08			15.08T			
12106	0.000	9	0.30	15.08			15.08			
12106	1.000	9	0.30	15.08			15.08			
12107	0.000	9	0.30	15.08			15.08T			
12107	1.000	9	0.45	22.42			22.42T			
12108	0.000	9	0.45	22.42			22.42T			
12108	1.000	9	0.72	36.17			36.17T			
12109	0.000	9	0.72	36.17			36.17T			
12109	1.000	9	0.80	40.39			40.39T			
12110	0.000	9	0.80	40.39			40.39T			
12110	1.000	9	0.75	37.68			37.68T			
12111	0.000	9	0.75	37.68			37.68T			
12111	1.000	9	0.62	31.04			31.04T			
12112	0.000	9	0.62	31.04			31.04T			
12112	1.000	9	0.46	22.93			22.93T			
12113	0.000	9	0.46	22.93			22.93T			
12113	1.000	9	0.30	15.08			15.08T			
12114	0.000	9	0.30	15.08			15.08T			
12114	1.000	9	0.30	15.08			15.08T			
12115	0.000	9	0.30	15.08			15.08T			
12115	1.000	9	0.30	15.08			15.08T			
12116	0.000	9	0.30	15.08			15.08T			
12116	1.000	9	0.30	15.08			15.08T			
12117	0.000	9	0.30	15.08			15.08T			
12117	1.000	9	0.30	15.08			15.08T			
12118	0.000	9	0.30	15.08			15.08T			
12118	1.000	9	0.30	15.08			15.08T			
12119	0.000	9	0.30	15.08			15.08T			
12119	1.000	9	0.30	15.08			15.08T			
12120	0.000	9	0.30	15.08			15.08T			
12120	1.000	9	0.30	15.08			15.08T			
12121	0.000	9	0.30	15.08			15.08T			
12121	1.000	9	0.30	15.08			15.08T			
12122	0.000	9	0.30	15.08			15.08T			
12122	1.000	9	0.30	15.08			15.08T			
12123	0.000	9	0.30	15.08			15.08T			
12123	1.000	9	0.30	15.08			15.08T			
12124	0.000	9	0.30	15.08			15.08T			
12124	1.000	9	0.30	15.08			15.08T			
12125	0.000	9	0.30	15.08			15.08T			
12125	1.000	9	0.30	15.08			15.08T			
12126	0.000	9	0.30	15.08			15.08T			
12126	1.000	9	0.30	15.08			15.08T			
12127	0.000	9	0.30	15.08			15.08T			
12127	1.000	9	0.30	15.08			15.08T			
12128	0.000	9	0.30	15.08			15.08'			
12128	1.000	9	0.30	15.08			15.08'			
12129	0.000	9	0.38	19.12			19.12T			
12129	1.000	9	0.63	31.75			31.75T			
12130	0.000	9	0.63	31.75			31.75T			
12130	1.000	9	0.79	39.90			39.90T			
12131	0.000	9	0.79	39.90			39.90T			
12131	1.000	9	0.88	44.09			44.09T			
12132	0.000	9	0.88	44.09			44.09T			
12132	1.000	9	0.82	41.43			41.43T			
12133	0.000	9	0.82	41.43			41.43T			
12133	1.000	9	0.69	34.86			34.86T			
12134	0.000	9	0.69	34.86			34.86T			

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN-ΣΕΙΣΜΙΚΑ-BEAM-γBd1_ΠΑΣ/ΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Longitudinal Reinforcements LCR 511
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
12134	1.000	9	0.53	26.83			26.83T			
12135	0.000	9	0.53	26.83			26.83T			
12135	1.000	9	0.38	19.02			19.02T			
12136	0.000	9	0.38	19.02			19.02T			
12136	1.000	9	0.30	15.08			15.08T			
12137	0.000	9	0.30	15.08			15.08T			
12137	1.000	9	0.30	15.08			15.08T			
12138	0.000	9	0.30	15.08			15.08T			
12138	1.000	9	0.30	15.08			15.08T			
12139	0.000	9	0.30	15.08			15.08T			
12139	1.000	9	0.30	15.08			15.08T			
12140	0.000	9	0.30	15.08			15.08T			
12140	1.000	9	0.30	15.08			15.08T			
12141	0.000	9	0.30	15.08			15.08T			
12141	1.000	9	0.30	15.08			15.08T			
12142	0.000	9	0.30	15.08			15.08T			
12142	1.000	9	0.30	15.08			15.08T			
12143	0.000	9	0.30	15.08			15.08T			
12143	1.000	9	0.30	15.08			15.08T			
12144	0.000	9	0.30	15.08			15.08T			
12144	1.000	9	0.30	15.08			15.08T			
12145	0.000	9	0.30	15.08			15.08T			
12145	1.000	9	0.30	15.08			15.08T			
12146	0.000	9	0.30	15.08			15.08T			
12146	1.000	9	0.30	15.08			15.08T			
12147	0.000	9	0.30	15.08			15.08T			
12147	1.000	9	0.30	15.08			15.08T			
12148	0.000	9	0.30	15.08			15.08T			
12148	1.000	9	0.30	15.08			15.08T			
12149	0.000	9	0.30	15.08			15.08T			
12149	1.000	9	0.30	15.08			15.08T			
12150	0.000	9	0.30	15.08			15.08			
12150	1.000	9	0.30	15.08			15.08			

Shear Reinforcements per Cutted Part of Section LCR 511

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
10001	0.000	8	2.92	10.40				
10001	0.200	8	2.92	10.50				
10005	0.000	8	13.12	14.79				
10005	0.200	8	13.12	14.89				
10006	0.000	8	1.10	8.22				
10006	0.400	8	1.10	8.42				
10009	0.000	8	0.99	8.41				
10009	0.400	8	0.99	8.21				
10010	0.000	8	13.07	14.94				
10010	0.200	8	13.07	14.87				
10014	0.000	8	2.94	10.60				
10014	0.200	8	2.94	10.50				
10016	0.000	8	2.85	10.41				
10016	0.200	8	2.85	10.51				
10020	0.000	8	12.82	14.87				
10020	0.200	8	12.82	14.94				
10021	0.000	8	1.08	8.24				
10021	0.400	8	1.08	8.43				
10024	0.000	8	0.97	8.43				
10024	0.400	8	0.97	8.23				
10025	0.000	8	12.76	14.78				
10025	0.200	8	12.76	14.71				
10029	0.000	8	2.87	10.68				
10029	0.200	8	2.87	10.58				
12001	0.000	9	0.00	12.62				
12001	1.000	9	0.00	13.53				
12002	0.000	9	0.00	13.53				
12002	1.000	9	0.00	14.25				
12003	0.000	9	0.00	14.25				
12003	1.000	9	0.00	16.16				
12004	0.000	9	0.00	12.74				
12004	1.000	9	0.00	13.64				
12005	0.000	9	0.00	13.64				
12005	1.000	9	0.00	14.35				
12006	0.000	9	0.00	14.35				
12006	1.000	9	0.00	13.17				
12007	0.000	9	0.00	12.69				
12007	1.000	9	0.00	13.61				
12008	0.000	9	0.00	13.61				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN-ΣΕΙΣΜΙΚΑ-BEAM-γBd1_ΠΑΣ/ΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Shear Reinforcements per Cutted Part of Section LCR 511

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
12008	1.000	9	0.00	14.32				
12009	0.000	9	0.00	14.32				
12009	1.000	9	0.00	16.22				
12010	0.000	9	0.00	12.36				
12010	1.000	9	0.00	13.21				
12011	0.000	9	0.00	13.21				
12011	1.000	9	0.00	13.88				
12012	0.000	9	0.00	13.88				
12012	1.000	9	0.00	16.76				
12013	0.000	9	0.00	12.48				
12013	1.000	9	0.00	13.32				
12014	0.000	9	0.00	13.32				
12014	1.000	9	0.00	13.97				
12015	0.000	9	0.00	13.97				
12015	1.000	9	0.00	12.88				
12016	0.000	9	0.00	12.44				
12016	1.000	9	0.00	13.29				
12017	0.000	9	0.00	13.29				
12017	1.000	9	0.00	13.95				
12018	0.000	9	0.00	13.95				
12018	1.000	9	0.00	16.81				
12019	0.000	9	0.00	16.16				
12019	1.000	9	0.00	8.76				
12020	0.000	9	0.00	8.76				
12020	1.000	9	0.00	5.36				
12021	0.000	9	0.00	5.36				
12021	1.000	9	0.00	5.36				
12022	0.000	9	0.00	5.36				
12022	1.000	9	0.00	5.39				
12023	0.000	9	0.00	5.39				
12023	1.000	9	0.00	5.48				
12024	0.000	9	0.00	5.48				
12024	1.000	9	0.00	5.58				
12025	0.000	9	0.00	5.58				
12025	1.000	9	0.00	5.66				
12026	0.000	9	0.00	5.66				
12026	1.000	9	0.00	5.71				
12027	0.000	9	0.00	5.71				
12027	1.000	9	0.00	5.74				
12028	0.000	9	0.00	5.74				
12028	1.000	9	0.00	5.74				
12029	0.000	9	0.00	5.74				
12029	1.000	9	0.00	5.74				
12030	0.000	9	0.00	5.74				
12030	1.000	9	0.00	5.74				
12031	0.000	9	0.00	5.74				
12031	1.000	9	0.00	5.74				
12032	0.000	9	0.00	5.74				
12032	1.000	9	0.00	5.74				
12033	0.000	9	0.00	5.74				
12033	1.000	9	0.00	5.74				
12034	0.000	9	0.00	5.74				
12034	1.000	9	0.00	5.74				
12035	0.000	9	0.00	5.74				
12035	1.000	9	0.00	5.74				
12036	0.000	9	0.00	5.74				
12036	1.000	9	0.00	5.74				
12037	0.000	9	0.00	5.74				
12037	1.000	9	0.00	5.74				
12038	0.000	9	0.00	5.74				
12038	1.000	9	0.00	5.74				
12039	0.000	9	0.00	5.74				
12039	1.000	9	0.00	11.07				
12040	0.000	9	0.00	4.58				
12040	1.000	9	0.00	4.58				
12041	0.000	9	0.00	13.21				
12041	1.000	9	0.00	7.92				
12042	0.000	9	0.00	7.92				
12042	1.000	9	0.00	5.20				
12043	0.000	9	0.00	5.20				
12043	1.000	9	0.00	5.20				
12044	0.000	9	0.00	5.20				
12044	1.000	9	0.00	5.18				
12045	0.000	9	0.00	5.18				
12045	1.000	9	0.00	5.16				
12046	0.000	9	0.00	5.16				
12046	1.000	9	0.00	5.16				
12047	0.000	9	0.00	5.16				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN-ΣΕΙΣΜΙΚΑ-BEAM-γBd1_ΠΑΣ/ΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Shear Reinforcements per Cutted Part of Section LCR 511

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
12047	1.000	9	0.00	5.23				
12048	0.000	9	0.00	5.23				
12048	1.000	9	0.00	5.19				
12049	0.000	9	0.00	5.19				
12049	1.000	9	0.00	5.08				
12050	0.000	9	0.00	5.08				
12050	1.000	9	0.00	4.80				
12051	0.000	9	0.00	4.80				
12051	1.000	9	0.00	4.59				
12052	0.000	9	0.00	4.59				
12052	1.000	9	0.00	4.58				
12053	0.000	9	0.00	4.58				
12053	1.000	9	0.00	4.59				
12054	0.000	9	0.00	4.59				
12054	1.000	9	0.00	4.60				
12055	0.000	9	0.00	4.60				
12055	1.000	9	0.00	4.60				
12056	0.000	9	0.00	4.60				
12056	1.000	9	0.00	4.58				
12057	0.000	9	0.00	4.58				
12057	1.000	9	0.00	4.58				
12058	0.000	9	0.00	4.58				
12058	1.000	9	0.00	4.58				
12059	0.000	9	0.00	4.58				
12059	1.000	9	0.00	4.58				
12060	0.000	9	0.00	4.58				
12060	1.000	9	0.00	4.58				
12061	0.000	9	0.00	4.58				
12061	1.000	9	0.00	4.58				
12062	0.000	9	0.00	4.58				
12062	1.000	9	0.00	4.58				
12063	0.000	9	0.00	16.22				
12063	1.000	9	0.00	8.73				
12064	0.000	9	0.00	8.73				
12064	1.000	9	0.00	5.37				
12065	0.000	9	0.00	5.37				
12065	1.000	9	0.00	5.36				
12066	0.000	9	0.00	5.36				
12066	1.000	9	0.00	5.39				
12067	0.000	9	0.00	5.39				
12067	1.000	9	0.00	5.48				
12068	0.000	9	0.00	5.48				
12068	1.000	9	0.00	5.58				
12069	0.000	9	0.00	5.58				
12069	1.000	9	0.00	5.66				
12070	0.000	9	0.00	5.66				
12070	1.000	9	0.00	5.71				
12071	0.000	9	0.00	5.71				
12071	1.000	9	0.00	5.74				
12072	0.000	9	0.00	5.74				
12072	1.000	9	0.00	5.74				
12073	0.000	9	0.00	5.74				
12073	1.000	9	0.00	5.74				
12074	0.000	9	0.00	5.74				
12074	1.000	9	0.00	5.74				
12075	0.000	9	0.00	5.74				
12075	1.000	9	0.00	5.74				
12076	0.000	9	0.00	5.74				
12076	1.000	9	0.00	5.74				
12077	0.000	9	0.00	5.74				
12077	1.000	9	0.00	5.74				
12078	0.000	9	0.00	5.74				
12078	1.000	9	0.00	5.74				
12079	0.000	9	0.00	5.74				
12079	1.000	9	0.00	5.74				
12080	0.000	9	0.00	5.74				
12080	1.000	9	0.00	5.74				
12081	0.000	9	0.00	5.74				
12081	1.000	9	0.00	5.74				
12082	0.000	9	0.00	5.74				
12082	1.000	9	0.00	5.74				
12083	0.000	9	0.00	5.74				
12083	1.000	9	0.00	5.74				
12084	0.000	9	0.00	5.74				
12084	1.000	9	0.00	4.58				
12085	0.000	9	0.00	16.76				
12085	1.000	9	0.00	8.48				
12086	0.000	9	0.00	8.48				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN-ΣΕΙΣΜΙΚΑ-BEAM-γBd1_ΠΑΣ/ΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Shear Reinforcements per Cutted Part of Section LCR 511

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
12086	1.000	9	0.00	5.34				
12087	0.000	9	0.00	5.34				
12087	1.000	9	0.00	5.33				
12088	0.000	9	0.00	5.33				
12088	1.000	9	0.00	5.36				
12089	0.000	9	0.00	5.36				
12089	1.000	9	0.00	5.44				
12090	0.000	9	0.00	5.44				
12090	1.000	9	0.00	5.56				
12091	0.000	9	0.00	5.56				
12091	1.000	9	0.00	5.64				
12092	0.000	9	0.00	5.64				
12092	1.000	9	0.00	5.70				
12093	0.000	9	0.00	5.70				
12093	1.000	9	0.00	5.74				
12094	0.000	9	0.00	5.74				
12094	1.000	9	0.00	5.74				
12095	0.000	9	0.00	5.74				
12095	1.000	9	0.00	5.74				
12096	0.000	9	0.00	5.74				
12096	1.000	9	0.00	5.74				
12097	0.000	9	0.00	5.74				
12097	1.000	9	0.00	5.74				
12098	0.000	9	0.00	5.74				
12098	1.000	9	0.00	5.74				
12099	0.000	9	0.00	5.74				
12099	1.000	9	0.00	5.74				
12100	0.000	9	0.00	5.74				
12100	1.000	9	0.00	5.74				
12101	0.000	9	0.00	5.74				
12101	1.000	9	0.00	5.74				
12102	0.000	9	0.00	5.74				
12102	1.000	9	0.00	5.74				
12103	0.000	9	0.00	5.74				
12103	1.000	9	0.00	5.74				
12104	0.000	9	0.00	5.74				
12104	1.000	9	0.00	5.74				
12105	0.000	9	0.00	5.74				
12105	1.000	9	0.00	4.58				
12106	0.000	9	0.00	4.58				
12106	1.000	9	0.00	4.58				
12107	0.000	9	0.00	12.88				
12107	1.000	9	0.00	7.65				
12108	0.000	9	0.00	7.65				
12108	1.000	9	0.00	5.22				
12109	0.000	9	0.00	5.22				
12109	1.000	9	0.00	5.21				
12110	0.000	9	0.00	5.21				
12110	1.000	9	0.00	5.20				
12111	0.000	9	0.00	5.20				
12111	1.000	9	0.00	5.17				
12112	0.000	9	0.00	5.17				
12112	1.000	9	0.00	5.15				
12113	0.000	9	0.00	5.15				
12113	1.000	9	0.00	5.24				
12114	0.000	9	0.00	5.24				
12114	1.000	9	0.00	5.20				
12115	0.000	9	0.00	5.20				
12115	1.000	9	0.00	5.11				
12116	0.000	9	0.00	5.11				
12116	1.000	9	0.00	4.84				
12117	0.000	9	0.00	4.84				
12117	1.000	9	0.00	4.60				
12118	0.000	9	0.00	4.60				
12118	1.000	9	0.00	4.58				
12119	0.000	9	0.00	4.58				
12119	1.000	9	0.00	4.60				
12120	0.000	9	0.00	4.60				
12120	1.000	9	0.00	4.61				
12121	0.000	9	0.00	4.61				
12121	1.000	9	0.00	4.60				
12122	0.000	9	0.00	4.60				
12122	1.000	9	0.00	4.59				
12123	0.000	9	0.00	4.59				
12123	1.000	9	0.00	4.58				
12124	0.000	9	0.00	4.58				
12124	1.000	9	0.00	4.58				
12125	0.000	9	0.00	4.58				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN-ΣΕΙΣΜΙΚΑ-BEAM-γBd1_ΠΑΣ/ΔΕΣΜΟΣ-ΠΑΣΣΑΛΟΙ

Shear Reinforcements per Cutted Part of Section LCR 511

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
12125	1.000	9	0.00	4.58				
12126	0.000	9	0.00	4.58				
12126	1.000	9	0.00	4.58				
12127	0.000	9	0.00	4.58				
12127	1.000	9	0.00	4.58				
12128	0.000	9	0.00	4.58				
12128	1.000	9	0.00	4.58				
12129	0.000	9	0.00	16.81				
12129	1.000	9	0.00	8.45				
12130	0.000	9	0.00	8.45				
12130	1.000	9	0.00	5.34				
12131	0.000	9	0.00	5.34				
12131	1.000	9	0.00	5.33				
12132	0.000	9	0.00	5.33				
12132	1.000	9	0.00	5.36				
12133	0.000	9	0.00	5.36				
12133	1.000	9	0.00	5.44				
12134	0.000	9	0.00	5.44				
12134	1.000	9	0.00	5.56				
12135	0.000	9	0.00	5.56				
12135	1.000	9	0.00	5.64				
12136	0.000	9	0.00	5.64				
12136	1.000	9	0.00	5.70				
12137	0.000	9	0.00	5.70				
12137	1.000	9	0.00	5.74				
12138	0.000	9	0.00	5.74				
12138	1.000	9	0.00	5.74				
12139	0.000	9	0.00	5.74				
12139	1.000	9	0.00	5.74				
12140	0.000	9	0.00	5.74				
12140	1.000	9	0.00	5.74				
12141	0.000	9	0.00	5.74				
12141	1.000	9	0.00	5.74				
12142	0.000	9	0.00	5.74				
12142	1.000	9	0.00	5.74				
12143	0.000	9	0.00	5.74				
12143	1.000	9	0.00	5.74				
12144	0.000	9	0.00	5.74				
12144	1.000	9	0.00	5.74				
12145	0.000	9	0.00	5.74				
12145	1.000	9	0.00	5.74				
12146	0.000	9	0.00	5.74				
12146	1.000	9	0.00	5.74				
12147	0.000	9	0.00	5.74				
12147	1.000	9	0.00	5.74				
12148	0.000	9	0.00	5.74				
12148	1.000	9	0.00	5.74				
12149	0.000	9	0.00	5.74				
12149	1.000	9	0.00	5.74				
12150	0.000	9	0.00	5.74				
12150	1.000	9	0.00	4.58				

Maximum Degree of Utilization

		N sig-c	Vy sig-t	Vz tau	Mt sig-*	My tend.	Mz As-l	Mb As-v	Mt2 crack	Total sigdyn	lamda tau-*
Cross sect.	8	0.000	0.000	0.197	0.694	0.000	0.000	0.000	0.000	1.000	0.000
DOKOS-4		0.000	0.000	0.000	0.000	0.000	1.000	0.000	0.000	0.000	0.000
Cross sect.	9	0.000	0.000	0.499	0.000	0.000	0.000	0.000	0.000	1.003	0.000
section pile		0.000	0.000	0.000	0.000	0.000	1.000	0.000	0.000	0.000	0.000
Total System		0.000	0.000	0.499	0.694	0.000	0.000	0.000	0.000	1.003	0.000
		0.000	0.000	0.000	0.000	0.000	1.000	0.000	0.000	0.000	0.000

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN-ΣΕΙΣΜΙΚΑ-BEAM-γBd1_ΔΟΚΟΙ

Selected Beam Elements

FROM	TO	INC	X-VALUE	NC	MEMBER	CS0	CS1	CS2	CS3	CS4	CS5
1000	1060	1		1	bending	10	40				
2000	2020	1									

Default design code is DIN Fachbericht 102 Massivbrȯcken (2003) (Germany)
Klasse(Tab.4.118): D

Materials

No. 1 C 25/30 (DIN 1045-1)
No. 3 C 25/30 (DIN 1045-1)
No. 4 C 25/30 (DIN 1045-1)
No. 5 C 25/30 (DIN 1045-1)
No. 6 C 25/30 (DIN 1045-1)
No. 7 C 25/30 (DIN 1045-1)
No. 8 C 25/30 (DIN 1045-1)
No. 9 C 25/30 (DIN 1045-1)
No. 10 C 25/30 (DIN 1045-1)
No. 12 BSt 500 SA (DIN 1045-1)

Reinforcement will be accounted for sectional values as defined in AQUA
Reinforcements saved as design case LCR 512

Considered Load Cases

9101	9102	9103	9104	9105	9106
9107	9108	9201	9202	9203	9204
9205	9206	9207	9301	9302	9303
9304	9305	9306	9307	9308	

Ultimate Load Design

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Design for ultimate loads DIN Fachbericht 102 Massivbrȯcken (2003)
Uniaxial bending due to symmetry

Safety factors	SC-1	SC-2	SC-S	SS-1	SS-2	PIIa
	1.50	1.50	1.88	1.15	1.15	7

Strain limits	C1	C2	S1	S2	Z1	Z2
max	-3.50	-2.00	3.00	25.00	-3.50	25.00

parameters for reinforcements

Minimum reinforcements Bending.	Compress.	compression e/d	N/Npl	min. reinforcem. requ. section	maximum- reinforc.
0.00 [cm2]	0.30 [o/o]	3.50	0.0010	0.00	0.15	9.00

Tensile forces in the longitudinal reinforcements due to shear are NOT accounted for.
Material of sections uses Ultimate Limit strain-stress law with global safety factors
Material of reinforcements uses Ultimate Limit strain-stress law with global safety factors

MNo.	temp lev.	Material- safety [-]	max.compr stress [MPa]	at strain [o/oo]	max.tens stress [MPa]	at strain [o/oo]	tension- stiffening [MPa]
1	0	1.500	-14.17	-2.00	0.00	0.00	
3	0	1.500	-14.17	-2.00	0.00	0.00	
4	0	1.500	-14.17	-2.00	0.00	0.00	
5	0	1.500	-14.17	-2.00	0.00	0.00	
6	0	1.500	-14.17	-2.00	0.00	0.00	
7	0	1.500	-14.17	-2.00	0.00	0.00	
8	0	1.500	-14.17	-2.00	0.00	0.00	
9	0	1.500	-14.17	-2.00	0.00	0.00	
10	0	1.500	-14.17	-2.00	0.00	0.00	
12	0	1.150	-456.52	-25.00	456.52	25.00	

Shear Design

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Design for shear DIN 1045-1 (2003)
Minimum shear factor or tan of inclination of compressive struts 0.57 / 1.72

MNo	f-cd [MPa]	tau-rd [MPa]	sigIIQ [MPa]	sigIIT [MPa]	sigIIQ+ [MPa]	fyd [MPa]
1	11.33	0.08	8.50	5.95	8.50	
3	11.33	0.08	8.50	5.95	8.50	
4	11.33	0.08	8.50	5.95	8.50	
5	11.33	0.08	8.50	5.95	8.50	
6	11.33	0.08	8.50	5.95	8.50	
7	11.33	0.08	8.50	5.95	8.50	
8	11.33	0.08	8.50	5.95	8.50	
9	11.33	0.08	8.50	5.95	8.50	
10	11.33	0.08	8.50	5.95	8.50	
12						434.78

Tolerance for exceeding maximum shear or principal compression stress 0.0200

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN-ΣΕΙΣΜΙΚΑ-BEAM-γBd1_ΔΟΚΟΙ

Longitudinal Reinforcements LCR 512
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
1001	0.000	1	0.76	37.73		2.37T	9.40		25.96	
1001	0.883	1	0.63	31.43		2.37T	8.58		20.47	
1002	0.000	1	0.63	31.50		2.37T	8.55		20.57	
1002	0.883	1	0.53	26.58		2.37T	7.62	0.63	15.91	0.04
1003	0.000	1	0.54	26.65		2.37T	7.60	0.62	16.02	0.04
1003	0.883	1	0.43	21.25		2.37T	6.19	0.30	11.71	0.34
						0.34				
1004	0.000	1	0.43	21.32		2.37T	6.17	0.29	11.81	0.34
						0.34				
1004	0.883	1	0.31	15.64		2.37T	4.30	0.17	7.88	0.45
						0.48				
1005	0.000	1	0.32	15.70		2.37T	4.28	0.14	7.98	0.45
						0.48				
1005	0.883	1	0.20	9.75		2.37T	1.95	0.19	4.44	0.39
						0.41				
1006	0.000	1	0.20	9.81		2.37T	1.93	0.18	4.53	0.39
						0.41				
1006	0.883	1	0.15	7.58		2.37T	1.03	0.41	3.45	0.17
						0.15				
1007	0.000	1	0.15	7.58		2.37T	1.03	0.41	3.45	0.17
						0.15				
1007	0.883	1	0.20	9.85		2.37T	2.03	0.13	4.43	0.43
						0.45				
1008	0.000	1	0.20	9.79		2.37T	2.05	0.15	4.34	0.43
						0.45				
1008	0.883	1	0.33	16.32		2.37T	4.48	0.60	7.78	0.49
						0.60				
1009	0.000	1	0.33	16.29		2.37T	4.50	0.61	7.67	0.53
						0.60				
1009	0.883	1	0.44	21.89		2.37T	6.48	0.63	11.50	0.44
						0.47				
1010	0.000	1	0.44	21.83		2.37T	6.50	0.64	11.39	0.45
						0.47				
1010	0.883	1	0.54	26.83		2.37T	8.01	0.46	15.59	0.21
						0.18				
1011	0.000	1	0.54	26.75		2.37T	8.03	0.47	15.48	0.21
						0.18				
1011	0.883	1	0.63	31.47		2.37T	9.06		20.03	
1012	0.000	1	0.63	31.40		2.37T	9.09		19.93	
1012	0.883	1	0.76	37.64		2.37T	10.01		25.25	
1013	0.000	1	0.76	37.66		2.37T	9.03		26.25	
1013	0.883	1	0.62	30.68		2.37T	8.16		20.15	
1014	0.000	1	0.62	30.76		2.37T	8.12		20.26	
1014	0.883	1	0.50	24.85		2.37T	7.13		15.34	
1015	0.000	1	0.50	24.94		2.37T	7.12		15.45	
1015	0.883	1	0.38	19.08		2.37T	5.67		11.03	
1016	0.000	1	0.39	19.16		2.37T	5.65		11.14	
1016	0.883	1	0.27	13.31		2.37T	3.77		7.17	
1017	0.000	1	0.27	13.40		2.37T	3.75		7.27	
1017	0.883	1	0.15	7.56		2.37T	1.43		3.75	
1018	0.000	1	0.15	7.61		2.37T	1.42		3.82	
1018	0.883	1	0.11	5.64		2.37T	0.50		2.77	
1019	0.000	1	0.11	5.64		2.37T	0.50		2.77	
1019	0.883	1	0.15	7.61		2.37T	1.52		3.72	
1020	0.000	1	0.15	7.56		2.37T	1.53		3.65	
1020	0.883	1	0.27	13.40		2.37T	3.96		7.06	
1021	0.000	1	0.27	13.31		2.37T	3.97		6.96	
1021	0.883	1	0.39	19.16		2.37T	5.96		10.82	
1022	0.000	1	0.38	19.07		2.37T	5.98		10.72	
1022	0.883	1	0.50	24.93		2.37T	7.53		15.02	
1023	0.000	1	0.50	24.83		2.37T	7.55		14.91	
1023	0.883	1	0.62	30.72		2.37T	8.64		19.70	
1024	0.000	1	0.62	30.65		2.37T	8.68		19.60	
1024	0.883	1	0.76	37.56		2.37T	9.65		25.54	
1025	0.000	1	0.76	38.06		2.37T	8.80		26.88	
1025	0.883	1	0.61	30.54		2.37T	7.82		20.35	
1026	0.000	1	0.62	30.60		2.37T	7.79		20.44	
1026	0.883	1	0.49	24.34		2.37T	6.79		15.18	
1027	0.000	1	0.49	24.43		2.37T	6.77		15.29	
1027	0.883	1	0.37	18.41		2.37T	5.35		10.68	
1028	0.000	1	0.37	18.49		2.37T	5.33		10.79	
1028	0.883	1	0.25	12.59		2.37T	3.48		6.73	
1029	0.000	1	0.25	12.68		2.37T	3.47		6.84	
1029	0.883	1	0.14	6.87		2.37T	1.17		3.33	
1030	0.000	1	0.14	6.89		2.37T	1.16		3.36	
1030	0.883	1	0.10	4.99		2.37T	0.28		2.34	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN-ΣΕΙΣΜΙΚΑ-BEAM-γBd1_ΔΟΚΟΙ

Longitudinal Reinforcements LCR 512
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
1031	0.000	1	0.10	4.99		2.37T	0.28		2.34	
1031	0.883	1	0.14	6.89		2.37T	1.26		3.26	
1032	0.000	1	0.14	6.87		2.37T	1.27		3.23	
1032	0.883	1	0.25	12.67		2.37T	3.67		6.63	
1033	0.000	1	0.25	12.59		2.37T	3.68		6.53	
1033	0.883	1	0.37	18.49		2.37T	5.64		10.47	
1034	0.000	1	0.37	18.40		2.37T	5.66		10.37	
1034	0.883	1	0.49	24.42		2.37T	7.19		14.85	
1035	0.000	1	0.49	24.32		2.37T	7.21		14.74	
1035	0.883	1	0.61	30.54		2.37T	8.31		19.85	
1036	0.000	1	0.61	30.48		2.37T	8.35		19.76	
1036	0.883	1	0.76	37.96		2.37T	9.44		26.14	
1037	0.000	1	0.76	37.92		2.37T	8.89		26.66	
1037	0.883	1	0.62	30.79		2.37T	8.08		20.33	
1038	0.000	1	0.62	30.86		2.37T	8.05		20.44	
1038	0.883	1	0.50	24.93		2.37T	7.16		15.40	
1039	0.000	1	0.50	25.03		2.37T	7.14		15.51	
1039	0.883	1	0.39	19.16		2.37T	5.77		11.02	
1040	0.000	1	0.39	19.25		2.37T	5.75		11.13	
1040	0.883	1	0.27	13.39		2.37T	3.90		7.12	
1041	0.000	1	0.27	13.48		2.37T	3.88		7.22	
1041	0.883	1	0.15	7.71		2.37T	1.57		3.77	
1042	0.000	1	0.16	7.73		2.37T	1.55		3.80	
1042	0.883	1	0.12	5.87		2.37T	0.71		2.79	
1043	0.000	1	0.12	5.87		2.37T	0.71		2.79	
1043	0.883	1	0.16	7.73		2.37T	1.66		3.70	
1044	0.000	1	0.15	7.71		2.37T	1.67		3.67	
1044	0.883	1	0.27	13.48		2.37T	4.09		7.02	
1045	0.000	1	0.27	13.39		2.37T	4.10		6.91	
1045	0.883	1	0.39	19.24		2.37T	6.05		10.81	
1046	0.000	1	0.38	19.15		2.37T	6.07		10.70	
1046	0.883	1	0.50	25.01		2.37T	7.55		15.09	
1047	0.000	1	0.50	24.92		2.37T	7.57		14.97	
1047	0.883	1	0.62	30.82		2.37T	8.57		19.88	
1048	0.000	1	0.62	30.74		2.37T	8.60		19.77	
1048	0.883	1	0.76	37.83		2.37T	9.51		25.94	
1049	0.000	1	0.78	38.79		2.85T	9.23		26.72	
1049	0.883	1	0.65	32.14		2.88T	8.60		20.66	
1050	0.000	1	0.64	31.73		2.37T	8.55		20.81	
1050	0.883	1	0.55	27.54		3.02T	7.81	0.66	15.92	0.09
						0.03				
1051	0.000	1	0.55	27.18		2.58T	7.77	0.65	16.08	0.08
						0.03				
1051	0.883	1	0.44	21.95		2.45T	6.47	0.56	11.62	0.41
						0.43				
1052	0.000	1	0.44	21.90		2.37T	6.44	0.48	11.78	0.41
						0.42				
1052	0.883	1	0.33	16.57		2.37T	4.62	0.64	7.75	0.56
						0.62				
1053	0.000	1	0.34	16.67		2.37T	4.58	0.63	7.91	0.56
						0.63				
1053	0.883	1	0.21	10.21		2.37T	2.26	0.02	4.48	0.51
						0.56				
1054	0.000	1	0.21	10.21		2.37T	2.23		4.53	0.51
						0.56				
1054	0.883	1	0.17	8.24		2.37T	1.48	0.32	3.51	0.28
						0.28				
1055	0.000	1	0.17	8.24		2.37T	1.48	0.32	3.51	0.28
						0.28				
1055	0.883	1	0.20	9.96		2.37T	2.34		4.43	0.21
						0.61				
1056	0.000	1	0.21	10.33		2.37T	2.37		4.38	0.58
						0.63				
1056	0.883	1	0.30	14.86		2.37T	4.79		7.70	
1057	0.000	1	0.30	14.74		2.37T	4.82		7.54	
1057	0.883	1	0.44	21.76		2.37T	6.74	0.09	11.47	0.52
						0.56				
1058	0.000	1	0.44	21.68		2.39T	6.78	0.12	11.31	0.52
						0.56				
1058	0.883	1	0.55	27.28		2.52T	8.18	0.49	15.66	0.23
						0.21				
1059	0.000	1	0.56	27.62		2.96T	8.22	0.50	15.49	0.24
						0.21				
1059	0.883	1	0.64	31.70		2.37T	9.06		20.26	
1060	0.000	1	0.65	32.12		2.89T	9.11		20.11	
1060	0.883	1	0.78	38.69		2.85T	9.84		26.01	

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN-ΣΕΙΣΜΙΚΑ-BEAM-γBd1_ΔΟΚΟΙ

Longitudinal Reinforcements LCR 512
Note: Layer includes reinforcements for torsion if followed by T
Note: Layer has only compression reinforcements if followed by a quote

Beam	x[m]	NoS	mue [-]	As-Sum [cm2]	shift by [m]	Lay-0&5 [cm2]	Lay-1&6 [cm2]	Lay-2&7 [cm2]	Lay-3&8 [cm2]	Lay-4&9 [cm2]
2001	0.000	2	0.50	39.97		1.17T	9.73		29.07	
2001	0.300	2	0.48	38.55		1.17T	9.67		27.71	
2002	0.000	2	0.48	38.37		0.85T	9.69		27.82	
2002	0.300	2	0.45	36.23		0.85T	9.46		25.93	
2003	0.000	2	0.45	36.16		0.87T	10.07		25.22	
2003	0.300	2	0.48	38.28		0.87T	10.33		27.08	
2004	0.000	2	0.48	38.45		1.16T	10.32		26.97	
2004	0.300	2	0.50	39.86		1.16T	10.39		28.30	
2005	0.000	2	0.51	40.65		1.17T	9.37		30.11	
2005	0.300	2	0.49	38.83		1.17T	9.31		28.35	
2006	0.000	2	0.48	38.68		0.89T	9.33		28.46	
2006	0.300	2	0.45	36.21		0.89T	9.10		26.22	
2007	0.000	2	0.45	36.11		0.89T	9.72		25.50	
2007	0.300	2	0.48	38.57		0.89T	9.98		27.70	
2008	0.000	2	0.48	38.73		1.17T	9.97		27.59	
2008	0.300	2	0.51	40.54		1.17T	10.05		29.32	
2009	0.000	2	0.52	41.60		1.16T	9.31		31.13	
2009	0.300	2	0.49	39.46		1.17T	9.15		29.14	
2010	0.000	2	0.49	39.39		0.96T	9.17		29.26	
2010	0.300	2	0.46	36.70		0.97T	8.88		26.86	
2011	0.000	2	0.46	36.60		0.97T	9.52		26.11	
2011	0.300	2	0.49	39.28		0.97T	9.84		28.47	
2012	0.000	2	0.49	39.34		1.17T	9.82		28.35	
2012	0.300	2	0.52	41.48		1.17T	10.01		30.30	
2013	0.000	2	0.51	41.12		1.29T	9.17		30.66	
2013	0.300	2	0.49	39.27		1.30T	9.13		28.84	
2014	0.000	2	0.49	39.23		1.13T	9.15		28.95	
2014	0.300	2	0.46	36.71		1.13T	8.95		26.63	
2015	0.000	2	0.46	36.61		1.13T	9.58		25.90	
2015	0.300	2	0.49	39.11		1.13T	9.81		28.18	
2016	0.000	2	0.49	39.16		1.30T	9.79		28.07	
2016	0.300	2	0.51	41.00		1.30T	9.85		29.85	
2017	0.000	2	0.51	41.12		1.53T	9.37		30.23	
2017	0.300	2	0.49	39.60		1.53T	9.41		28.66	
2018	0.000	2	0.49	39.48		1.28T	9.42		28.78	
2018	0.300	2	0.47	37.24		1.29T	9.29		26.66	
2019	0.000	2	0.46	37.15		1.29T	9.91		25.95	
2019	0.300	2	0.49	39.38		1.29T	10.06		28.03	
2020	0.000	2	0.49	39.50		1.54T	10.05		27.91	
2020	0.300	2	0.51	41.01		1.53T	10.02		29.45	

Shear Reinforcements per Cutted Part of Section LCR 512

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
1001	0.000	1	0.78	4.01				
1001	0.883	1	0.78	3.82				
1002	0.000	1	0.47	3.76				
1002	0.883	1	0.47	3.57				
1003	0.000	1	0.33	3.46				
1003	0.883	1	0.33	3.27				
1004	0.000	1	0.23	3.14				
1004	0.883	1	0.23	2.95				
1005	0.000	1	0.14	2.81				
1005	0.883	1	0.14	2.62				
1006	0.000	1	0.05	2.47				
1006	0.883	1	0.05	2.28				
1007	0.000	1	0.04	2.20				
1007	0.883	1	0.04	2.39				
1008	0.000	1	0.13	2.55				
1008	0.883	1	0.13	2.73				
1009	0.000	1	0.22	2.88				
1009	0.883	1	0.22	3.06				
1010	0.000	1	0.32	3.19				
1010	0.883	1	0.32	3.38				
1011	0.000	1	0.46	3.49				
1011	0.883	1	0.46	3.68				
1012	0.000	1	0.77	3.74				
1012	0.883	1	0.77	3.93				
1013	0.000	1	0.75	4.29				
1013	0.883	1	0.75	4.10				
1014	0.000	1	0.45	3.96				
1014	0.883	1	0.45	3.78				
1015	0.000	1	0.43	3.68				
1015	0.883	1	0.43	3.49				
1016	0.000	1	0.37	3.33				
1016	0.883	1	0.37	3.15				

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN-ΣΕΙΣΜΙΚΑ-BEAM-γBd1_ΔΟΚΟΙ

Shear Reinforcements per Cutted Part of Section LCR 512

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
1017	0.000	1	0.29	2.95				
1017	0.883	1	0.29	2.77				
1018	0.000	1	0.18	2.56				
1018	0.883	1	0.18	2.37				
1019	0.000	1	0.18	2.30				
1019	0.883	1	0.18	2.49				
1020	0.000	1	0.29	2.69				
1020	0.883	1	0.29	2.88				
1021	0.000	1	0.37	3.07				
1021	0.883	1	0.37	3.26				
1022	0.000	1	0.44	3.42				
1022	0.883	1	0.44	3.61				
1023	0.000	1	0.46	3.71				
1023	0.883	1	0.46	3.89				
1024	0.000	1	0.75	4.03				
1024	0.883	1	0.75	4.22				
1025	0.000	1	0.96	4.73				
1025	0.883	1	0.96	4.54				
1026	0.000	1	0.64	4.28				
1026	0.883	1	0.64	4.09				
1027	0.000	1	0.56	3.86				
1027	0.883	1	0.56	3.68				
1028	0.000	1	0.45	3.43				
1028	0.883	1	0.45	3.24				
1029	0.000	1	0.30	2.98				
1029	0.883	1	0.30	2.79				
1030	0.000	1	0.15	2.53				
1030	0.883	1	0.15	2.35				
1031	0.000	1	0.15	2.27				
1031	0.883	1	0.15	2.46				
1032	0.000	1	0.30	2.72				
1032	0.883	1	0.30	2.91				
1033	0.000	1	0.45	3.16				
1033	0.883	1	0.45	3.35				
1034	0.000	1	0.56	3.60				
1034	0.883	1	0.56	3.79				
1035	0.000	1	0.64	4.02				
1035	0.883	1	0.64	4.20				
1036	0.000	1	0.96	4.46				
1036	0.883	1	0.96	4.65				
1037	0.000	1	1.26	4.85				
1037	0.883	1	1.26	4.66				
1038	0.000	1	0.85	4.31				
1038	0.883	1	0.85	4.12				
1039	0.000	1	0.72	3.87				
1039	0.883	1	0.72	3.68				
1040	0.000	1	0.56	3.43				
1040	0.883	1	0.56	3.24				
1041	0.000	1	0.35	2.98				
1041	0.883	1	0.35	2.79				
1042	0.000	1	0.13	2.52				
1042	0.883	1	0.13	2.34				
1043	0.000	1	0.12	2.26				
1043	0.883	1	0.12	2.45				
1044	0.000	1	0.35	2.72				
1044	0.883	1	0.35	2.91				
1045	0.000	1	0.56	3.17				
1045	0.883	1	0.56	3.36				
1046	0.000	1	0.71	3.61				
1046	0.883	1	0.71	3.79				
1047	0.000	1	0.85	4.05				
1047	0.883	1	0.85	4.24				
1048	0.000	1	1.26	4.58				
1048	0.883	1	1.26	4.77				
1049	0.000	1	1.60	4.79				
1049	0.883	1	1.60	4.61				
1050	0.000	1	1.29	4.49				
1050	0.883	1	1.29	4.30				
1051	0.000	1	1.07	4.10				
1051	0.883	1	1.07	3.91				
1052	0.000	1	0.83	3.66				
1052	0.883	1	0.83	3.47				
1053	0.000	1	0.53	3.18				
1053	0.883	1	0.53	2.99				
1054	0.000	1	0.27	2.67				
1054	0.883	1	0.27	2.48				
1055	0.000	1	0.26	2.40				
1055	0.883	1	0.26	2.59				

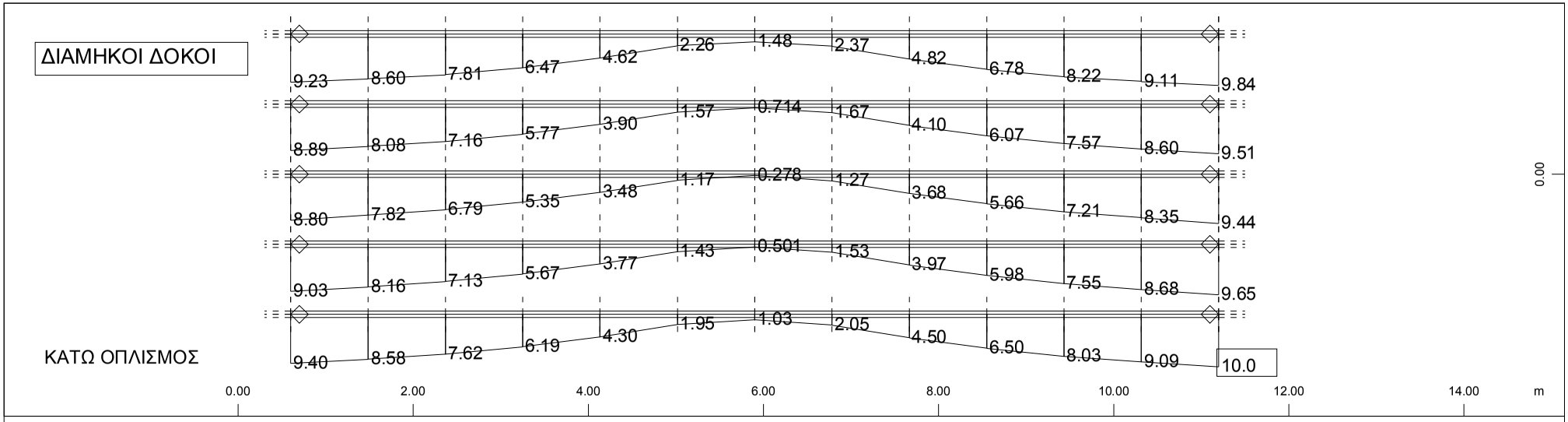
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
DESIGN-ΣΕΙΣΜΙΚΑ-BEAM-γBd1_ΔΟΚΟΙ

Shear Reinforcements per Cutted Part of Section LCR 512

Beam	x[m]	NoS	Asl-Mt [cm2/m]	SLay-0&5 [cm2/m]	SLay-1&6 [cm2/m]	SLay-2&7 [cm2/m]	SLay-3&8 [cm2/m]	SLay-4&9 [cm2/m]
1056	0.000	1	0.53	2.91				
1056	0.883	1	0.53	3.10				
1057	0.000	1	0.82	3.39				
1057	0.883	1	0.82	3.58				
1058	0.000	1	1.07	3.83				
1058	0.883	1	1.07	4.02				
1059	0.000	1	1.28	4.23				
1059	0.883	1	1.28	4.41				
1060	0.000	1	1.60	4.53				
1060	0.883	1	1.60	4.72				
2001	0.000	2	0.52	2.02				
2001	0.300	2	0.52	2.02				
2002	0.000	2	0.38	3.48				
2002	0.300	2	0.38	3.48				
2003	0.000	2	0.39	3.42				
2003	0.300	2	0.39	3.42				
2004	0.000	2	0.52	1.98				
2004	0.300	2	0.52	1.98				
2005	0.000	2	0.51	3.19				
2005	0.300	2	0.51	3.18				
2006	0.000	2	0.40	4.06				
2006	0.300	2	0.40	4.05				
2007	0.000	2	0.39	3.99				
2007	0.300	2	0.39	3.99				
2008	0.000	2	0.51	3.13				
2008	0.300	2	0.51	3.14				
2009	0.000	2	0.50	3.58				
2009	0.300	2	0.50	3.56				
2010	0.000	2	0.42	4.33				
2010	0.300	2	0.42	4.31				
2011	0.000	2	0.42	4.23				
2011	0.300	2	0.42	4.25				
2012	0.000	2	0.50	3.50				
2012	0.300	2	0.50	3.52				
2013	0.000	2	0.56	3.32				
2013	0.300	2	0.56	3.30				
2014	0.000	2	0.50	4.26				
2014	0.300	2	0.50	4.24				
2015	0.000	2	0.50	4.18				
2015	0.300	2	0.50	4.18				
2016	0.000	2	0.56	3.25				
2016	0.300	2	0.56	3.26				
2017	0.000	2	0.66	2.88				
2017	0.300	2	0.66	2.87				
2018	0.000	2	0.55	3.87				
2018	0.300	2	0.55	3.87				
2019	0.000	2	0.55	3.81				
2019	0.300	2	0.55	3.81				
2020	0.000	2	0.66	2.83				
2020	0.300	2	0.66	2.84				

Maximum Degree of Utilization

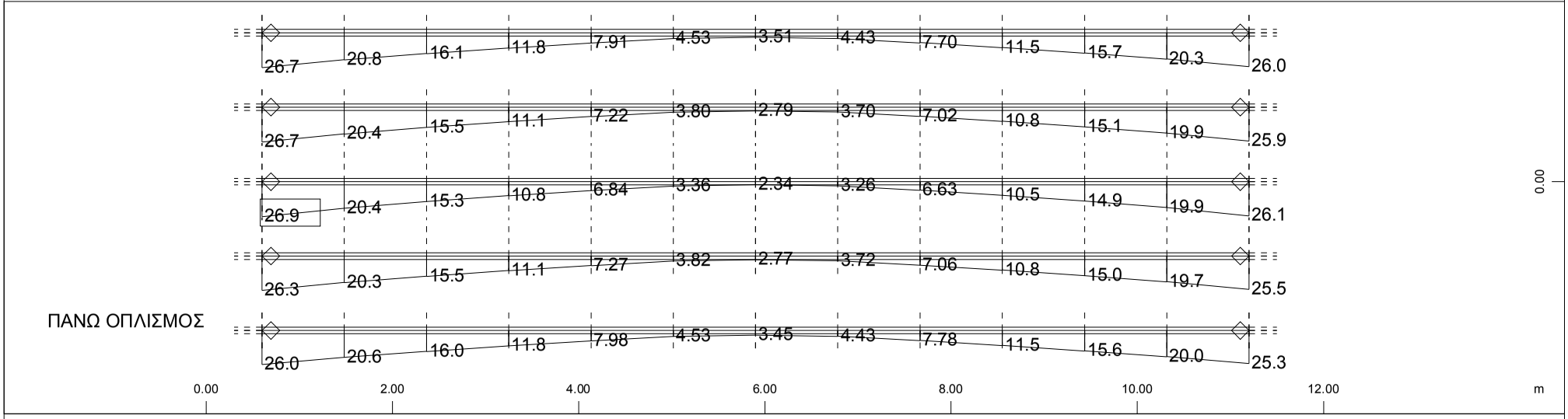
		N sig-c	Vy sig-t	Vz tau	Mt sig-*	My tend.	Mz As-l	Mb As-v	Mt2 crack	Total sigdyn	lamda tau-*
Cross sect.	1	0.000	0.000	0.366	0.156	0.000	0.000	0.000	0.000	1.003	0.000
		0.000	0.000	0.000	0.000	0.000	1.001	0.000	0.000	0.000	0.000
Cross sect.	2	0.000	0.000	0.110	0.048	0.000	0.000	0.000	0.000	1.001	0.000
DOKOS-2		0.000	0.000	0.000	0.000	0.000	1.000	0.000	0.000	0.000	0.000
Total System		0.000	0.000	0.366	0.156	0.000	0.000	0.000	0.000	1.003	0.000
		0.000	0.000	0.000	0.000	0.000	1.001	0.000	0.000	0.000	0.000



Z X
Y

Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case 512 , 1 cm 3D = 11.2 cm2 (Max=10.0)

M 1 : 67

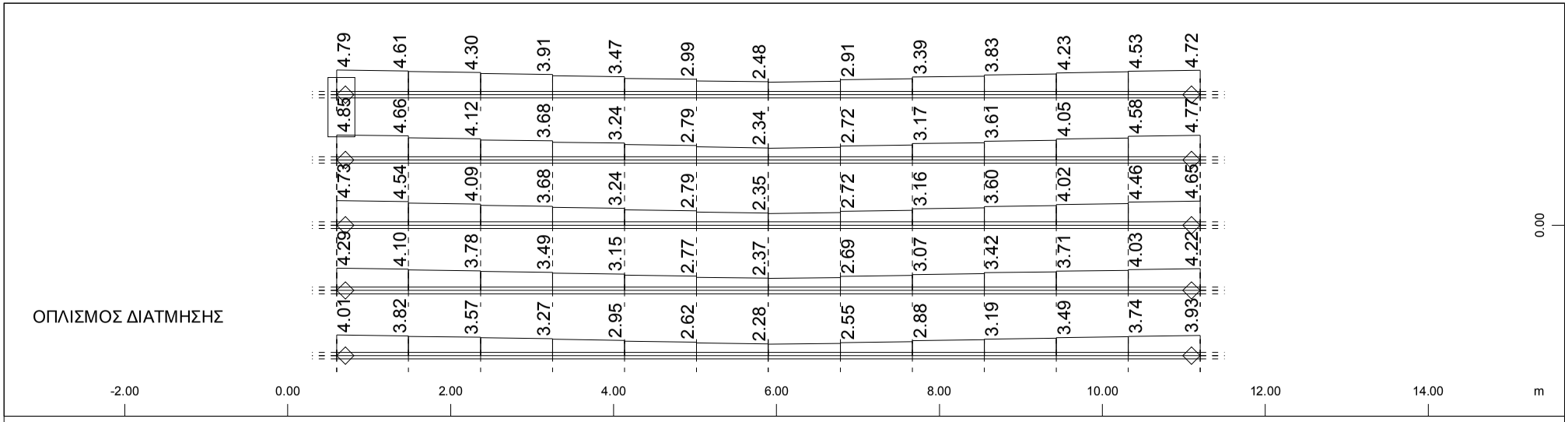


Z X
Y

Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case 512 , 1 cm 3D = 44.8 cm2 (Max=26.9)

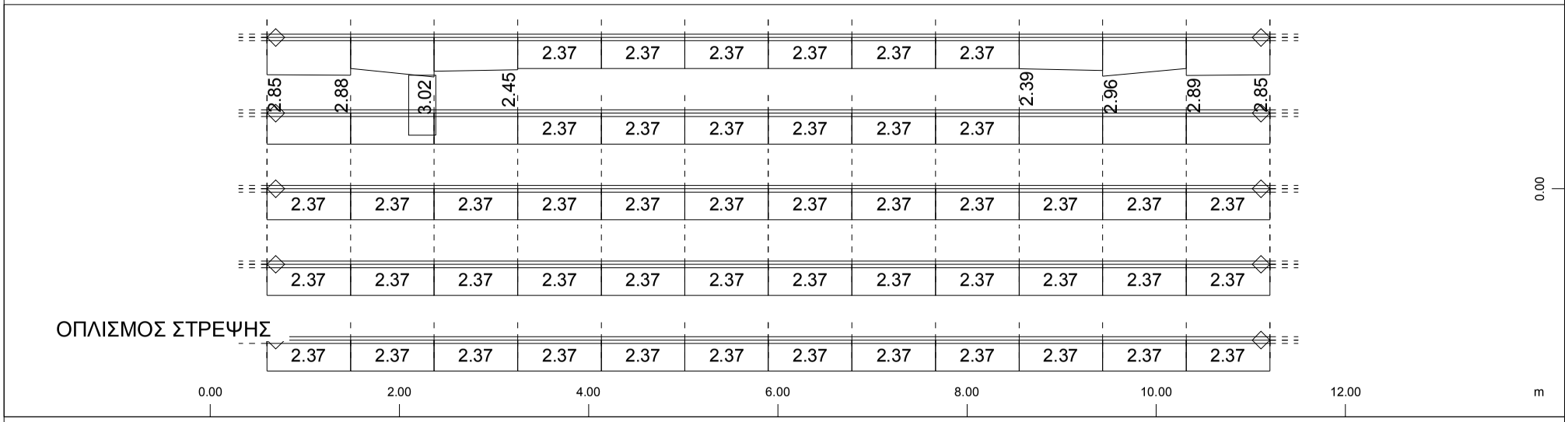
M 1 : 63

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΣΕΙΣΜΙΚΑ_Q=1.00



Sector of system Beam Elements Group 1
Beam Elements , Shear reinforcements (maximum), Design Case 512 , 1 cm 3D = 11.2 cm2/m (Max=4.85)

M 1 : 72



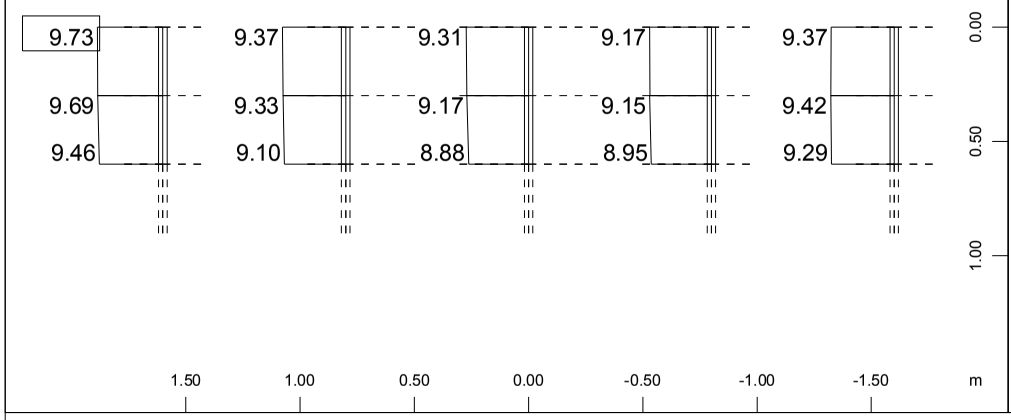
Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case 512 , 1 cm 3D = 4.48 cm2 (Max=3.02)

M 1 : 62

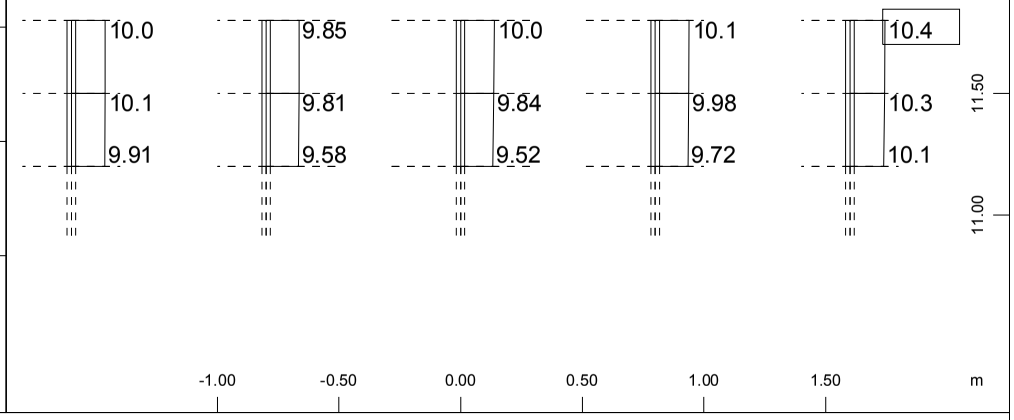
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΣΕΙΣΜΙΚΑ_Q=1.00

ΔΙΑΜΗΚΗ ΔΟΚΟΙ-ΣΤΗΡΙΞΗΣ

ΚΑΤΩ ΟΠΛΙΣΜΟΣ

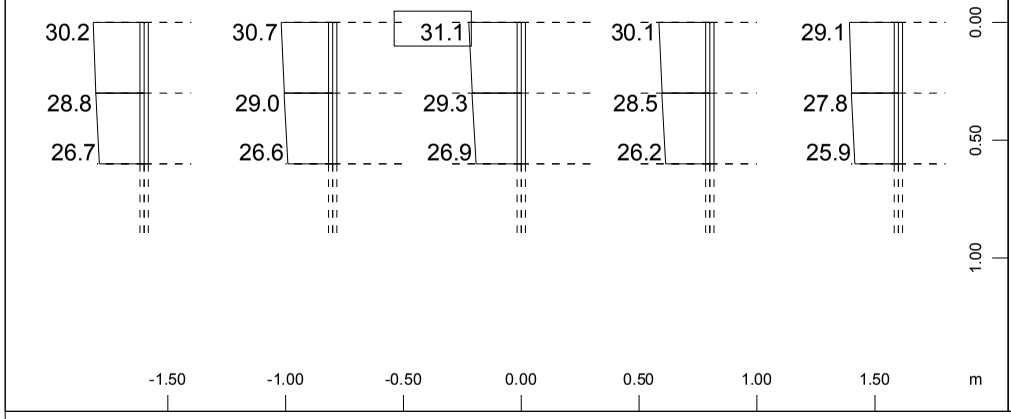


Y Z X
Sector of system Beam Elements Group 2 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
512 , 1 cm 3D = 11.2 cm2 (Max=9.73)

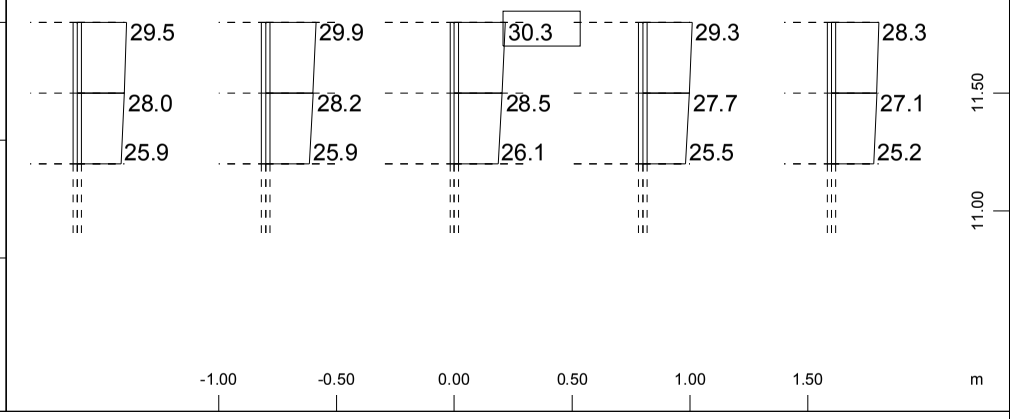


X Z Y
Sector of system Beam Elements Group 2 M 1 : 31
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
512 , 1 cm 3D = 22.4 cm2 (Max=10.4)

ΠΑΝΩ ΟΠΛΙΣΜΟΣ

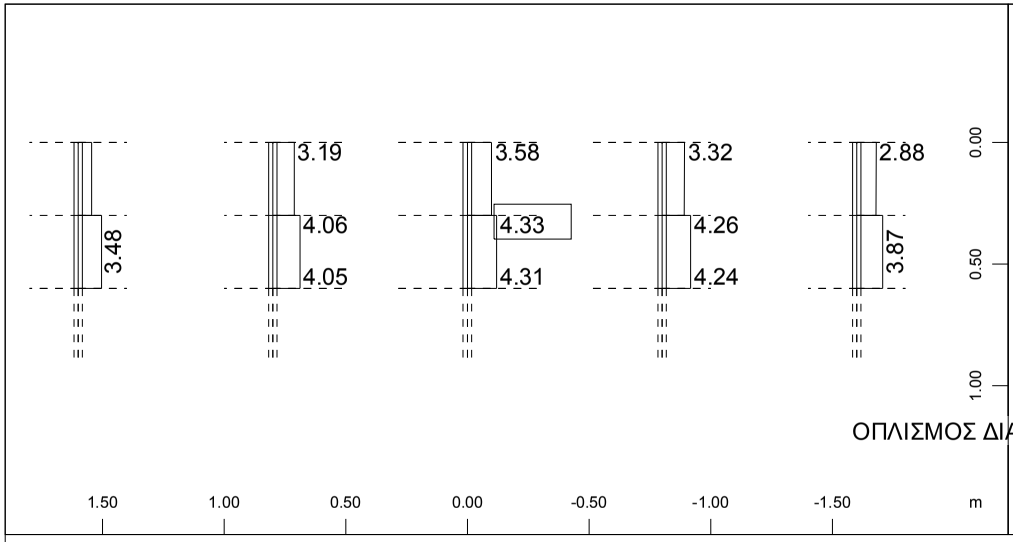


Z Y X
Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
512 , 1 cm 3D = 44.8 cm2 (Max=31.1)

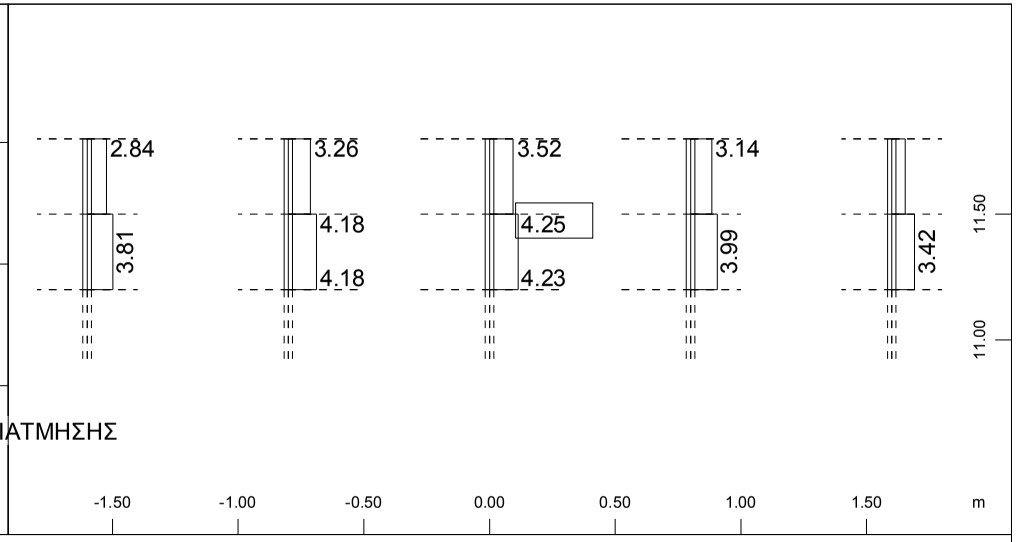


X Z Y
Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
512 , 1 cm 3D = 44.8 cm2 (Max=30.3)

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΣΕΙΣΜΙΚΑ_Q=1.00

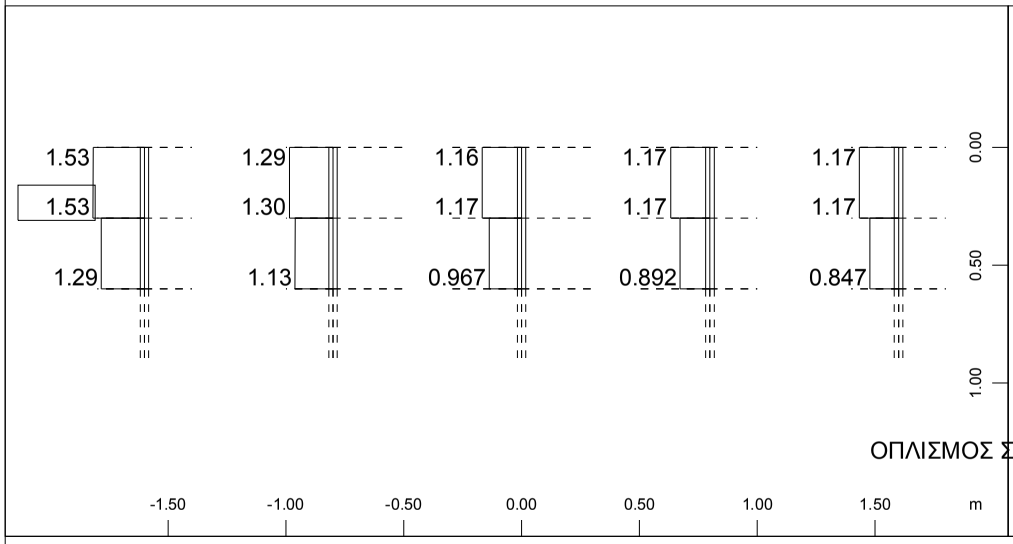


Sector of system Beam Elements Group 2 M 1 : 31
Beam Elements , Shear reinforcements (maximum), Design Case 512
, 1 cm 3D = 11.2 cm2/m (Max=4.33)

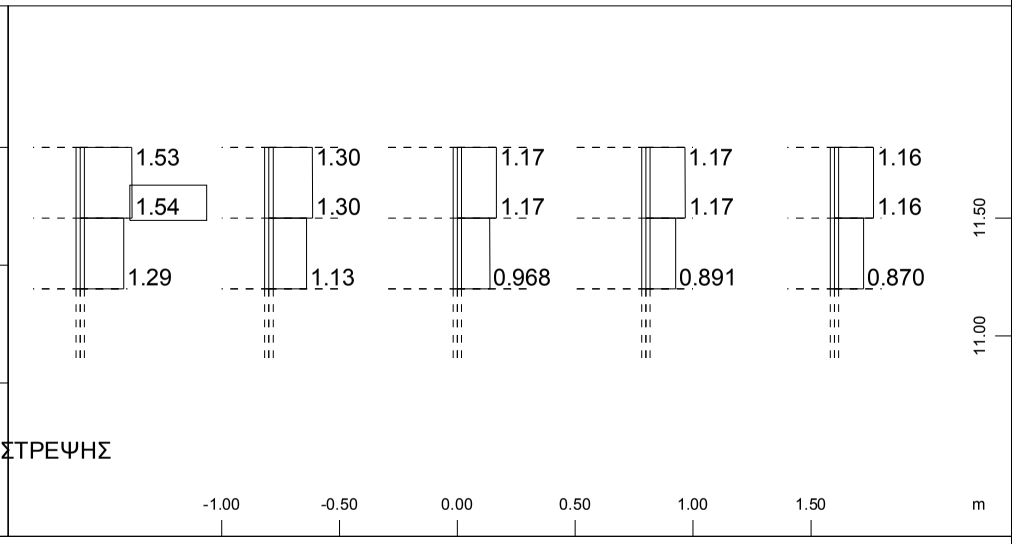


Sector of system Beam Elements Group 2 M 1 : 30
Beam Elements , Shear reinforcements (maximum), Design Case 512
, 1 cm 3D = 11.2 cm2/m (Max=4.25)

ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ



Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
512 , 1 cm 3D = 2.24 cm2 (Max=1.53)



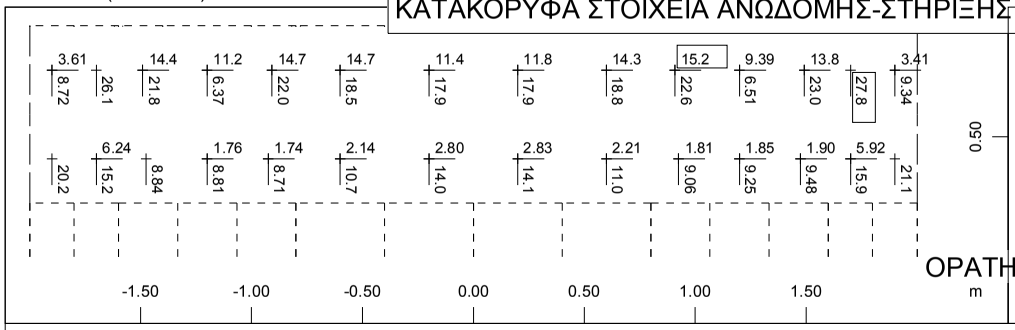
Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
512 , 1 cm 3D = 2.24 cm2 (Max=1.54)

ΟΠΛΙΣΜΟΣ ΣΤΡΕΨΗΣ

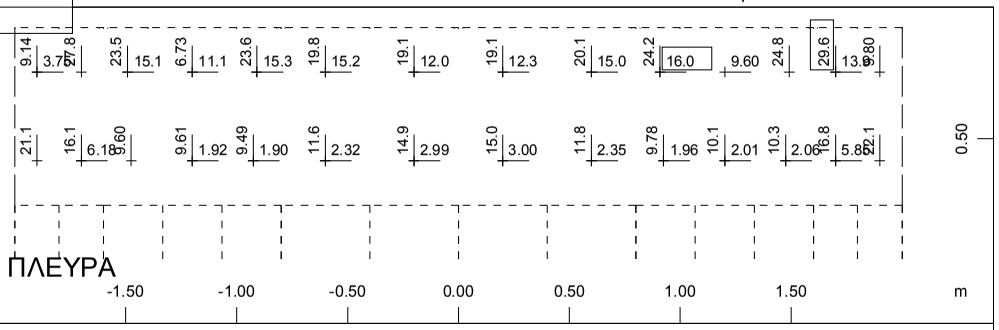
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΣΕΙΣΜΙΚΑ_Q=1.00

ΚΑΤΑΚΟΡΥΦΑ ΣΤΟΙΧΕΙΑ ΑΝΩΔΟΜΗΣ-ΣΤΗΡΙΞΗΣ

ΟΡΑΤΗ ΠΛΕΥΡΑ

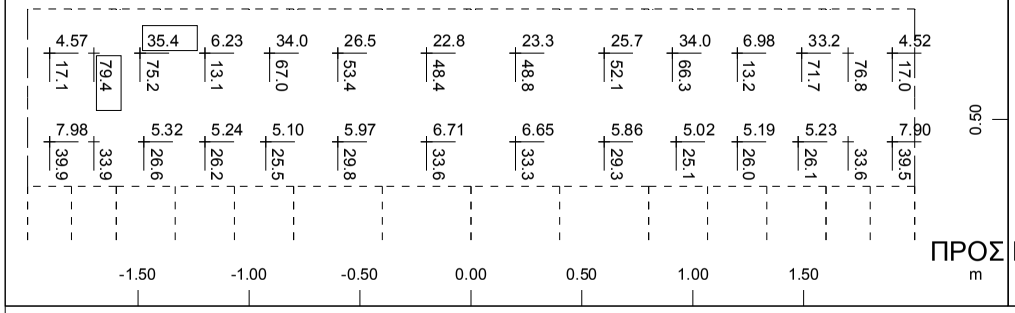


Sector of system Quadrilateral Elements Group 8 M 1 : 34
upper Reinforcements in Elements in cm2/m, Design Case 523 ULS
design (Max=27.8)

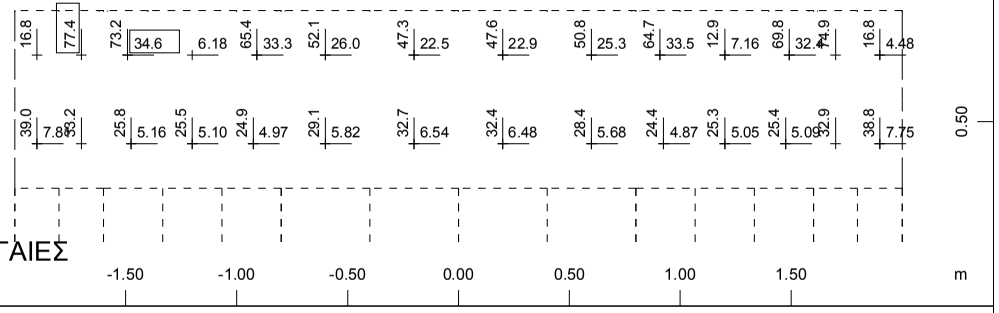


Sector of system Quadrilateral Elements Group 8 M 1 : 34
upper Reinforcements in Elements in cm2/m, Design Case 523 ULS
design (Max=29.6)

ΠΡΟΣ ΓΑΙΕΣ

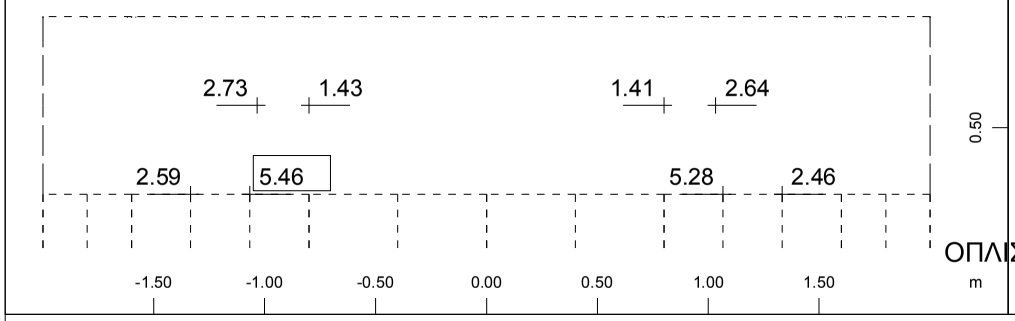


Sector of system Quadrilateral Elements Group 8 M 1 : 34
lower Reinforcements in Elements in cm2/m, Design Case 523 ULS
design (Max=79.4)

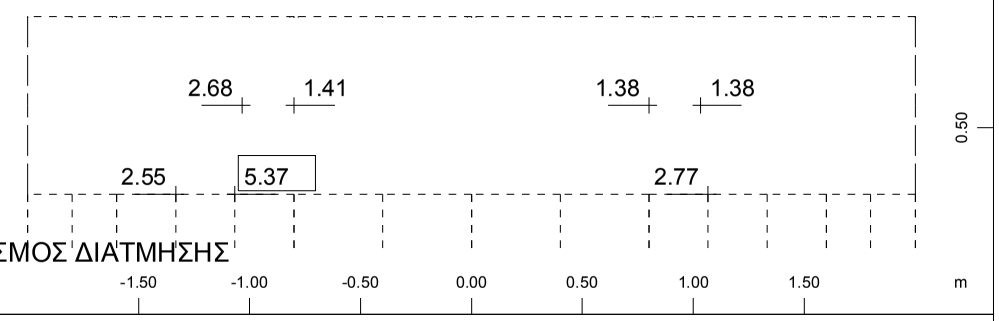


Sector of system Quadrilateral Elements Group 8 M 1 : 34
lower Reinforcements in Elements in cm2/m, Design Case 523 ULS
design (Max=77.4)

ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ



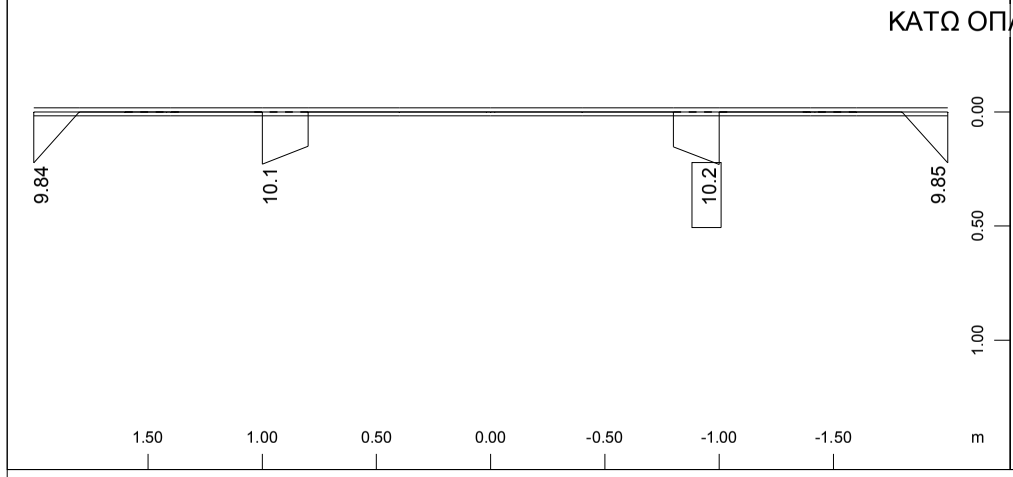
Sector of system Quadrilateral Elements Group 8 M 1 : 34
Shear reinforcement from middle of element in cm2/m2, Design
Case 523 ULS design (Max=5.46)



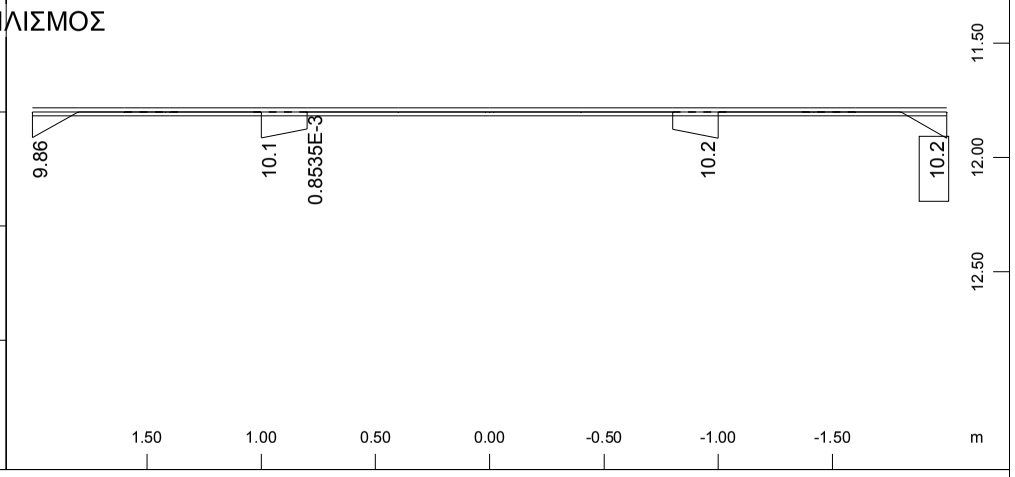
Sector of system Quadrilateral Elements Group 8 M 1 : 34
Shear reinforcement from middle of element in cm2/m2, Design
Case 523 ULS design (Max=5.37)

ΠΑΣΣΑΛΟΔΕΣΜΟΣ

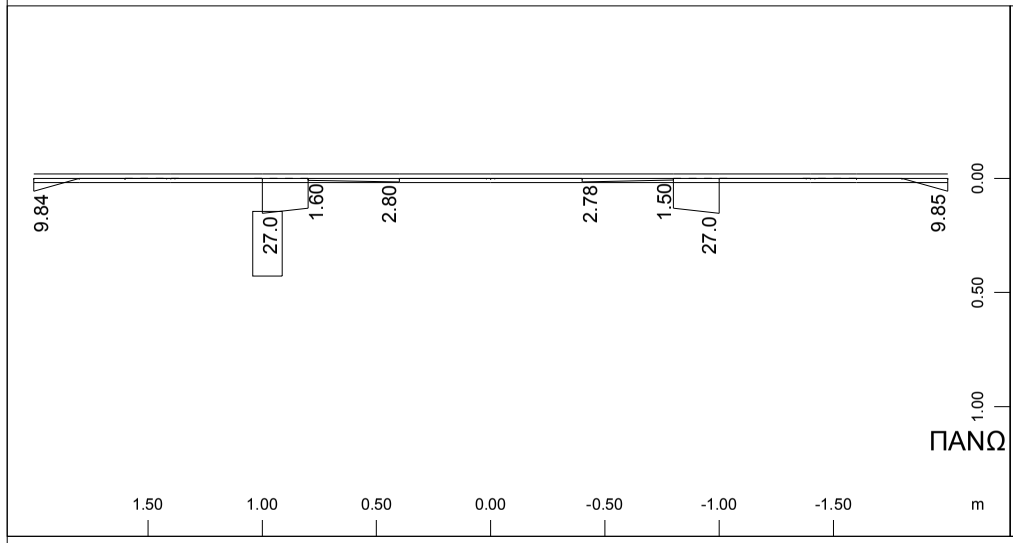
ΚΑΤΩ ΟΠΛΙΣΜΟΣ



Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
511 , 1 cm 3D = 14.5 cm2 (Max=10.2)

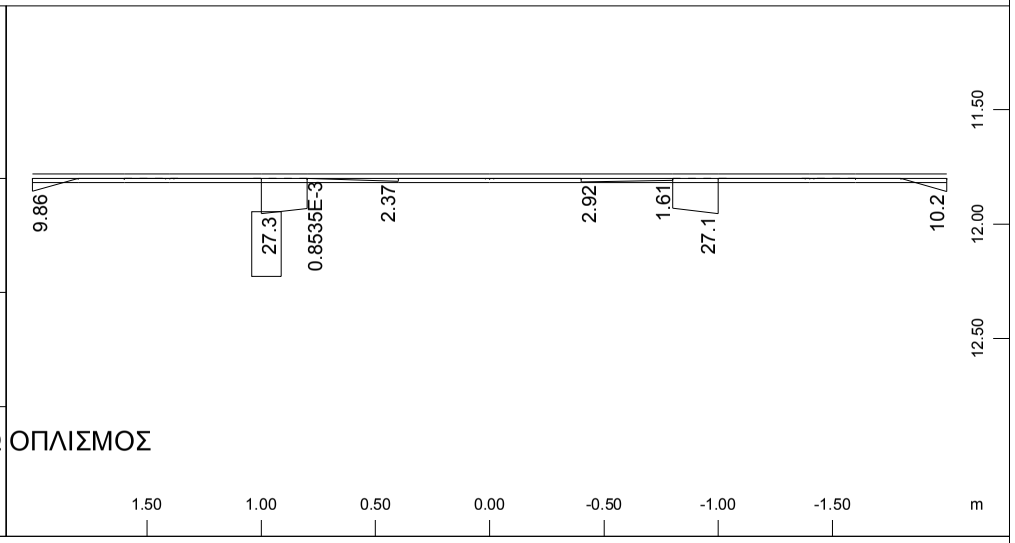


Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
511 , 1 cm 3D = 29.1 cm2 (Max=10.2)

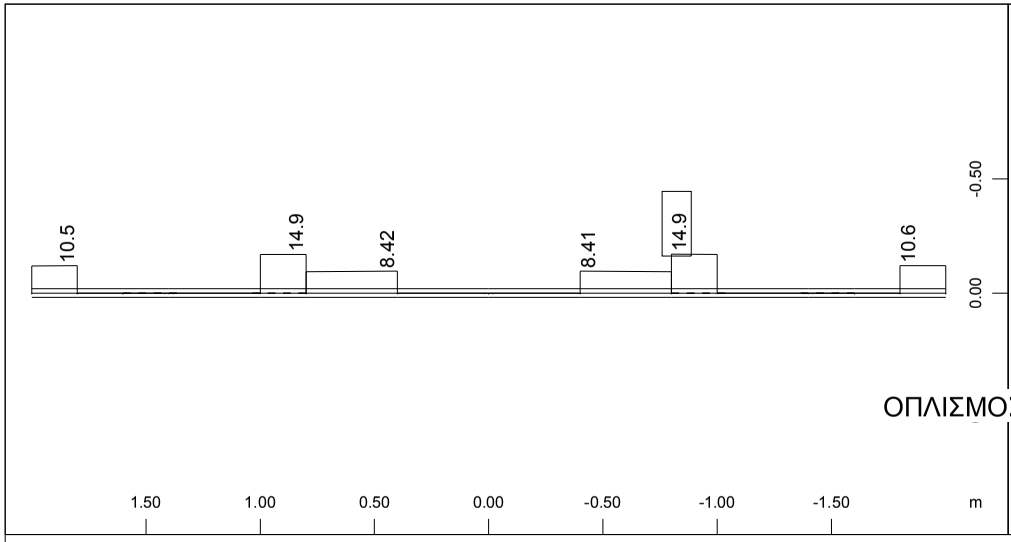


Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
511 , 1 cm 3D = 58.1 cm2 (Max=27.0)

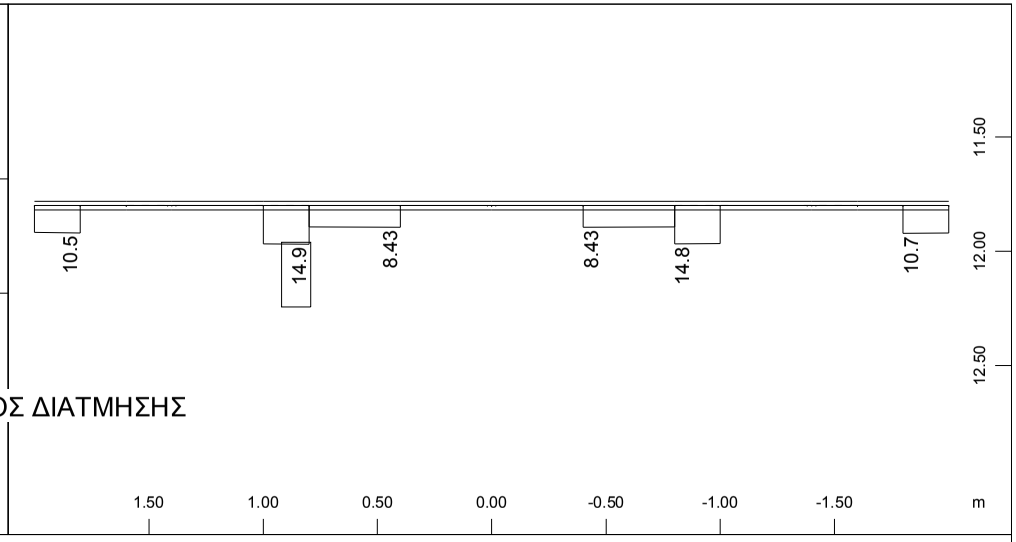
ΠΑΝΩ ΟΠΛΙΣΜΟΣ



Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
511 , 1 cm 3D = 58.1 cm2 (Max=27.3)

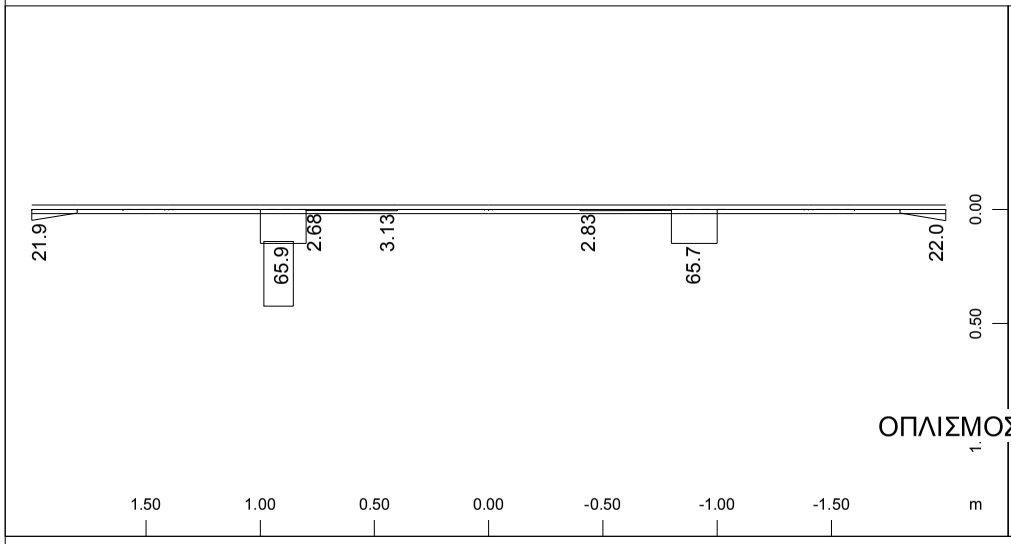


Sector of system Beam Elements Group 10
Beam Elements , Shear reinforcements (maximum), Design Case 511
, 1 cm 3D = 29.1 cm2/m (Max=14.9)
M 1 : 33

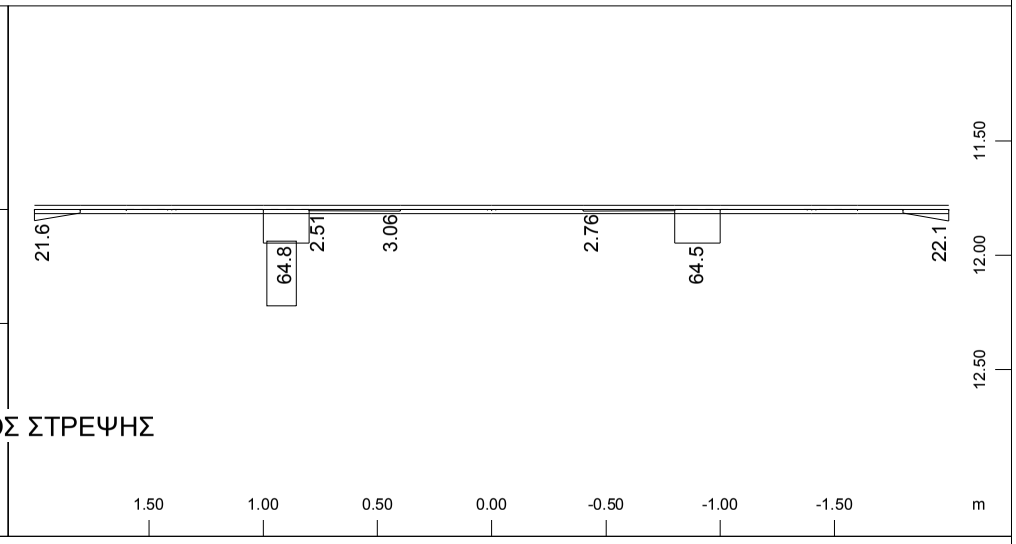


Sector of system Beam Elements Group 10
Beam Elements , Shear reinforcements (maximum), Design Case 511
, 1 cm 3D = 29.1 cm2/m (Max=14.9)
M 1 : 33

ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ

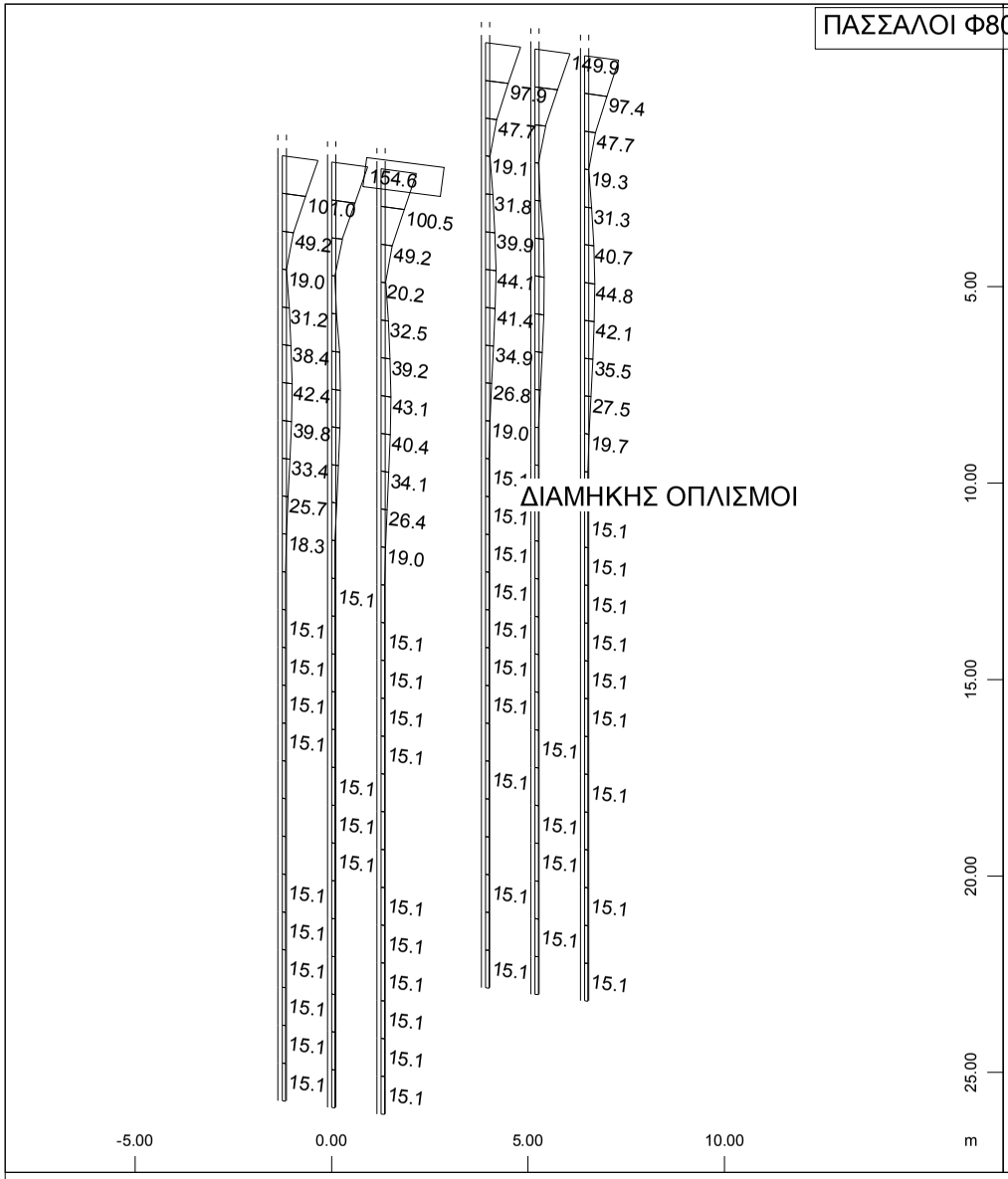


Sector of system Beam Elements Group 10
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
511 , 1 cm 3D = 145.3 cm2 (Max=65.9)
M 1 : 33



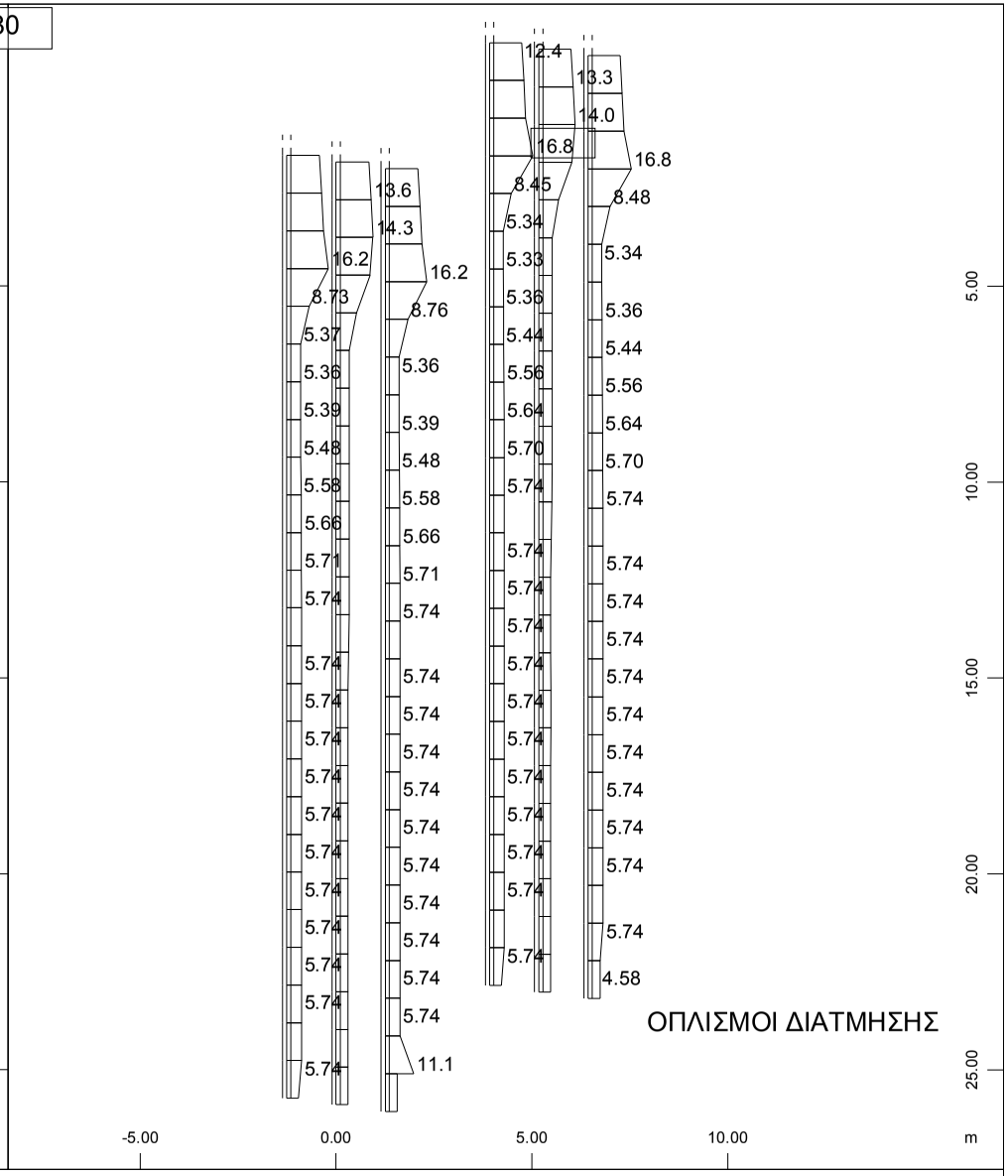
Sector of system Beam Elements Group 10
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
511 , 1 cm 3D = 145.3 cm2 (Max=64.8)
M 1 : 33

ΟΠΛΙΣΜΟΣ ΣΤΡΕΨΗΣ



Sector of system Beam Elements Group 12
Beam Elements , Longitudinal Reinforcements Lay. 1, Design
Case 511 , 1 cm 3D = 290.6 cm2 (Max=154.6)

M 1 : 191
X * 0.502
Y * 0.906
Z * 0.962

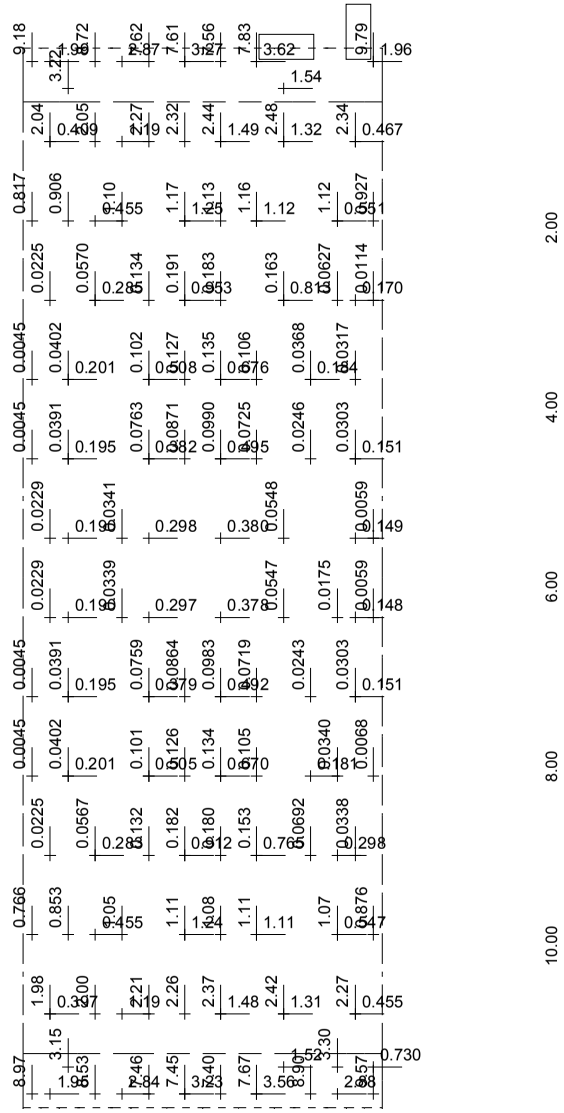


Sector of system Beam Elements Group 12
Beam Elements , Shear reinforcements (maximum), Design Case
511 , 1 cm 3D = 29.1 cm2/m (Max=16.8)

M 1 : 191
X * 0.502
Y * 0.906
Z * 0.962

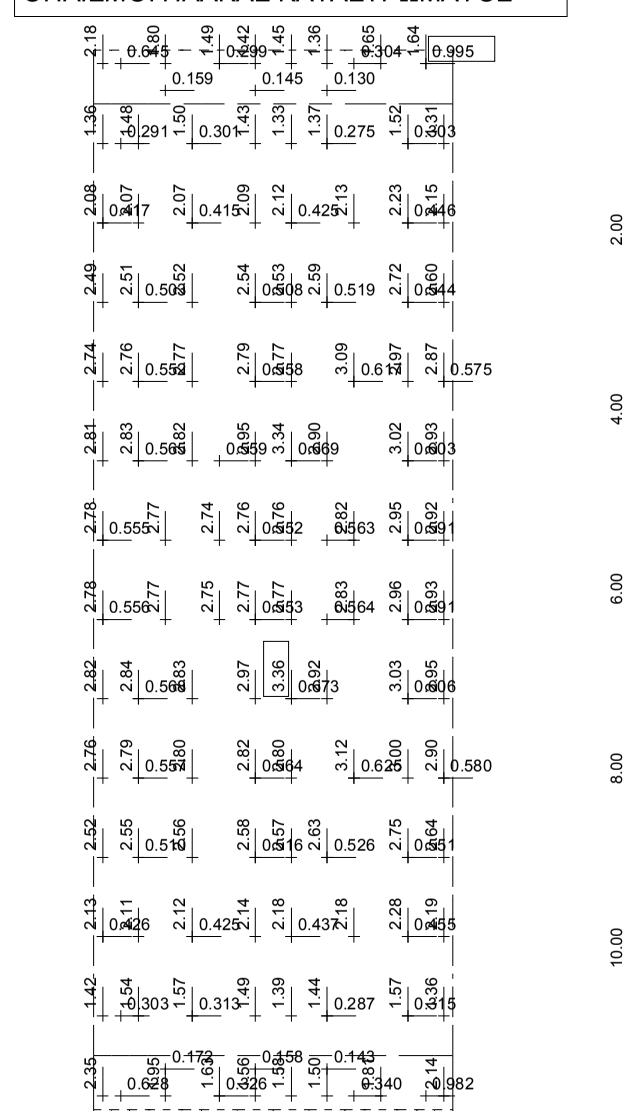
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΣΕΙΣΜΙΚΑ_Q=1.00

ΟΠΛΙΣΜΟΙ ΠΛΑΚΑΣ ΚΑΤΑΣΤΡΩΜΑΤΟΣ



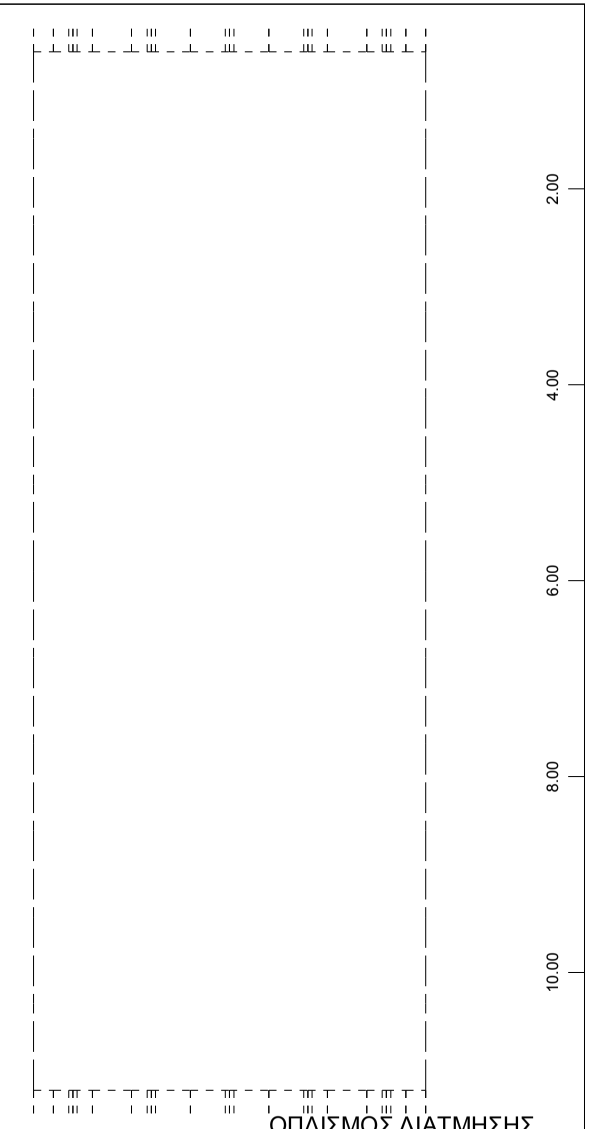
Sector of system Quadrilateral Elements Group 3 4
upper Reinforcements in Elements in cm2/m, Design Case
523 ULS design (Max=9.79)

M 1 : 84



Sector of system Quadrilateral Elements Group 3 4
lower Reinforcements in Elements in cm2/m, Design Case
523 ULS design (Max=3.36)

M 1 : 84



Sector of system Quadrilateral Elements Group 3
Shear reinforcement from middle of element in cm2/m2,
Design Case 523 ULS design (Max=0)

M 1 : 77

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΣΕΙΣΜΙΚΑ_Q=1.00

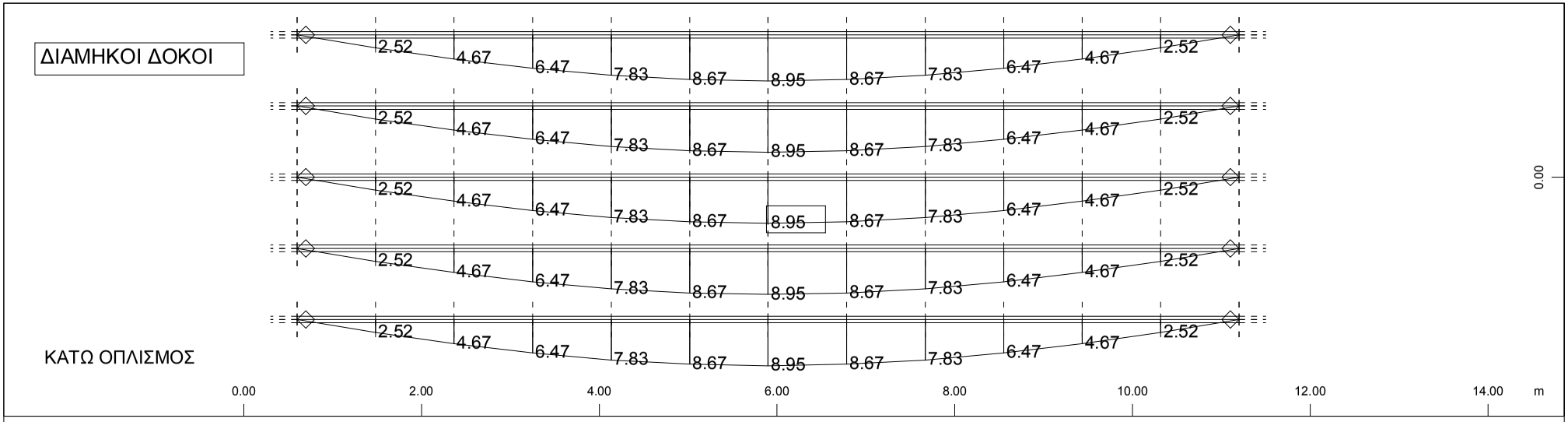
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
ΠΕΡΙΒΑΛΛΟΥΣΑ ΟΠΛΙΣΜΩΝ ΣΤΑΤΙΚΩΝ ΚΑΙ ΣΕΙΣΜΙΚΩΝ ΦΟΡΤΙΣΕΩΝ

Maximum of reinforcement-distributions
The reinforcement maximum was build out of the numbers of reinforcement-distributions:
521 , 522 , 523
and stored as new reinforcement-distribution 525 .

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00

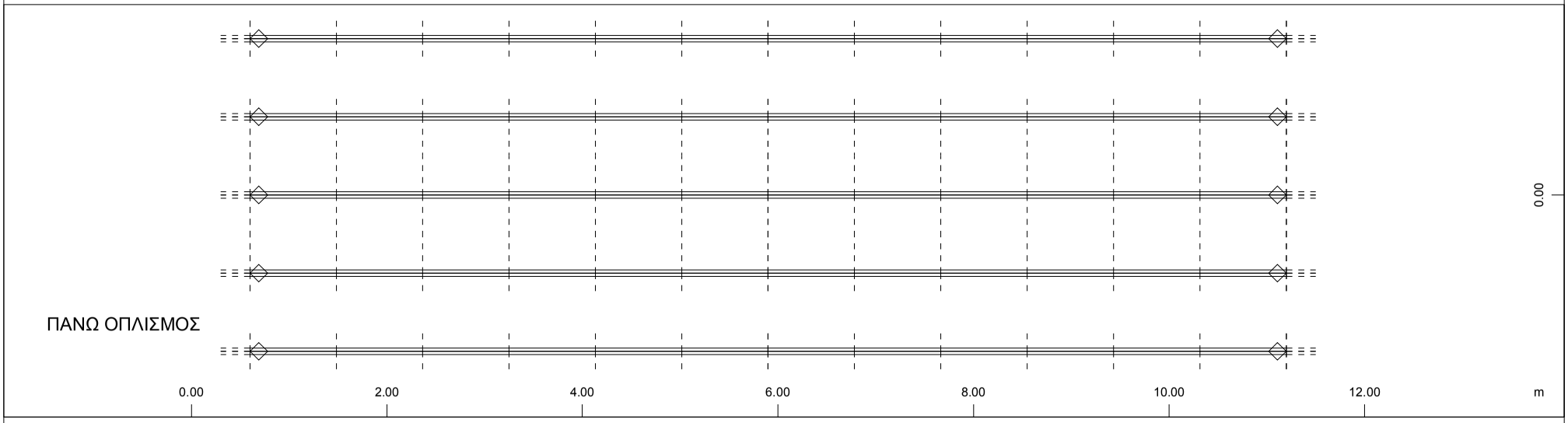
12) ΣΥΝΟΠΤΙΚΑ ΑΠΟΤΕΛΕΣΜΑΤΑ ΜΕ ΕΛΑΤΗΡΙΑΚΕΣ ΣΤΑΘΕΡΕΣ ΠΑΣΣΑΛΩΝ x5

5. 5) ΦΑΣΗ-1_ΕΛΕΓΧΟΣ ΦΟΡΕΑ ΣΕ ULS-ΣΤΑΤΙΚΑ



Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case 502 , 1 cm 3D = 11.2 cm2 (Max=8.95)

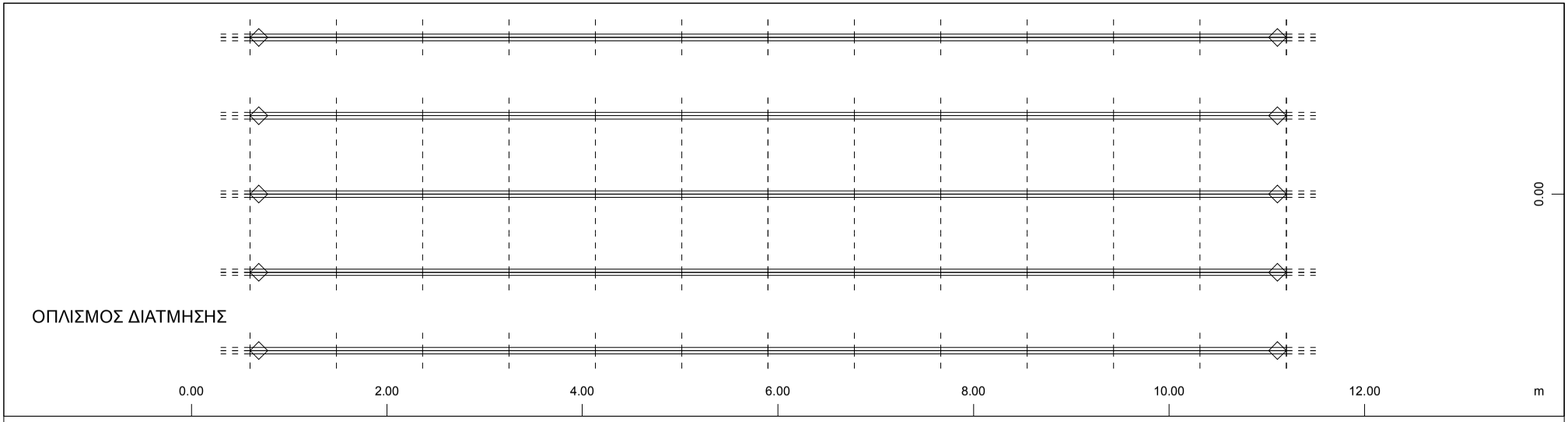
M 1 : 66



Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case 502 , 1 cm 3D = 0 cm2 (Max=0)

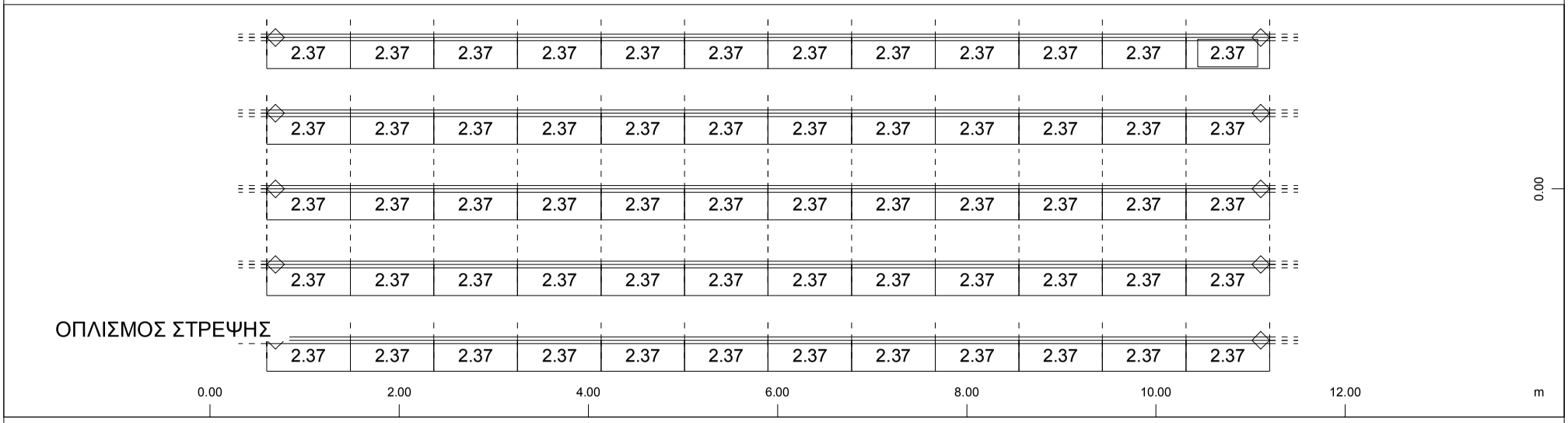
M 1 : 60

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΦΑΣΗ-1_ULS



Sector of system Beam Elements Group 1
Beam Elements , Shear reinforcements (maximum), Design Case 502 , 1 cm 3D = 0 cm2/m (Max=0)

M 1 : 60

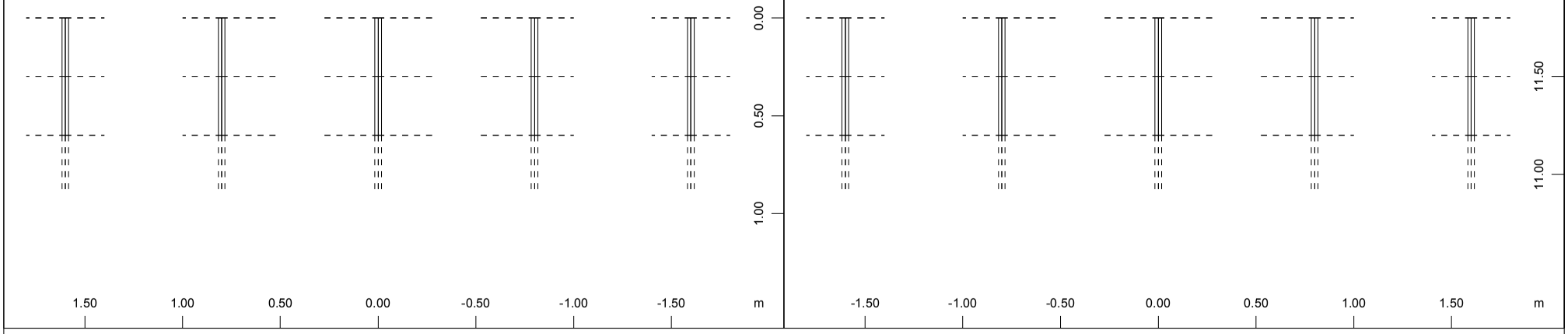


Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case 502 , 1 cm 3D = 4.48 cm2 (Max=2.37)

M 1 : 62

ΔΙΑΜΗΚΗ ΔΟΚΟΙ-ΣΤΗΡΙΞΗΣ

ΚΑΤΩ ΟΠΛΙΣΜΟΣ



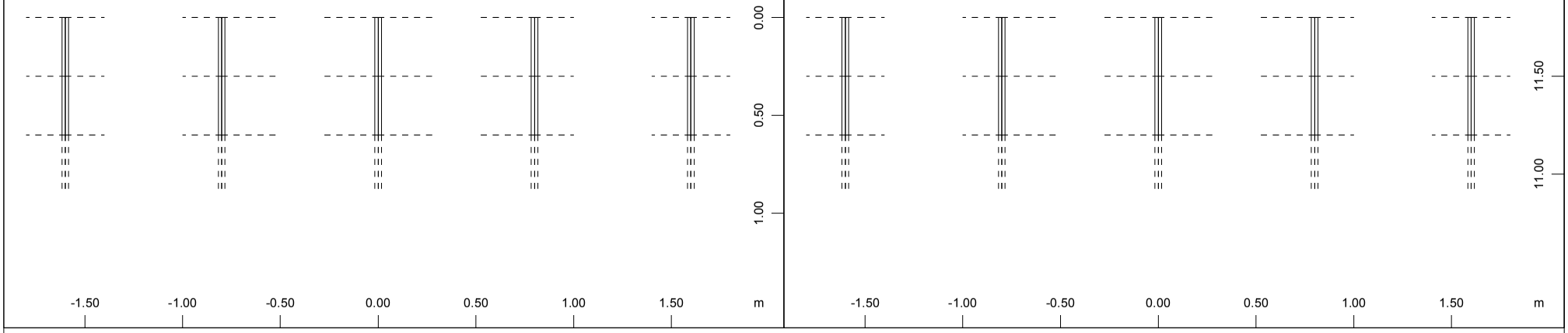
Y Z X

Sector of system Beam Elements Group 2 M 1 : 30
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
502 , 1 cm 3D = 0 cm2 (Max=0)

X Z Y

Sector of system Beam Elements Group 2 M 1 : 30
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
502 , 1 cm 3D = 0 cm2 (Max=0)

ΠΑΝΩ ΟΠΛΙΣΜΟΣ

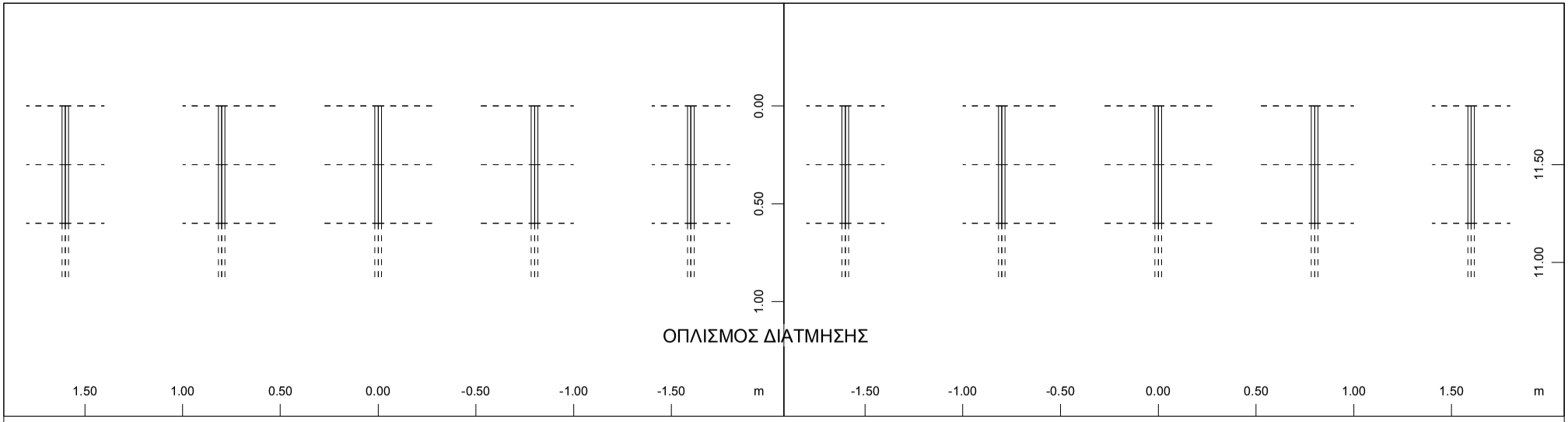


Z Y X

Sector of system Beam Elements Group 2 M 1 : 30
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
502 , 1 cm 3D = 0 cm2 (Max=0)

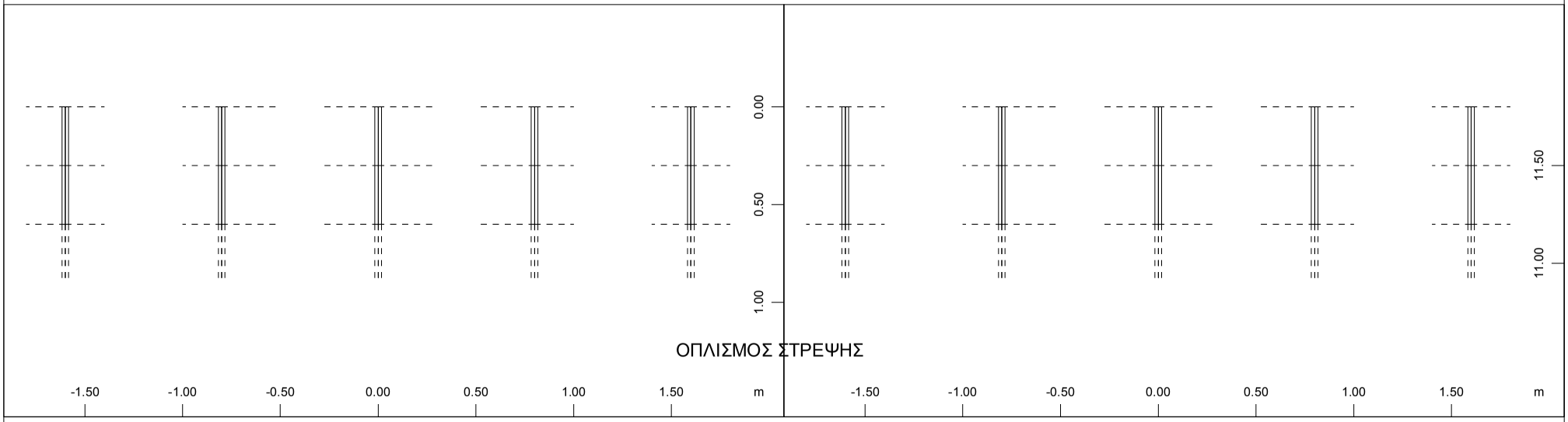
X Z Y

Sector of system Beam Elements Group 2 M 1 : 30
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
502 , 1 cm 3D = 0 cm2 (Max=0)



Y Z X
Sector of system Beam Elements Group 2 M 1 : 30
Beam Elements , Shear reinforcements (maximum), Design Case 502
, 1 cm 3D = 0 cm2/m (Max=0)

X Z Y
Sector of system Beam Elements Group 2 M 1 : 30
Beam Elements , Shear reinforcements (maximum), Design Case 502
, 1 cm 3D = 0 cm2/m (Max=0)

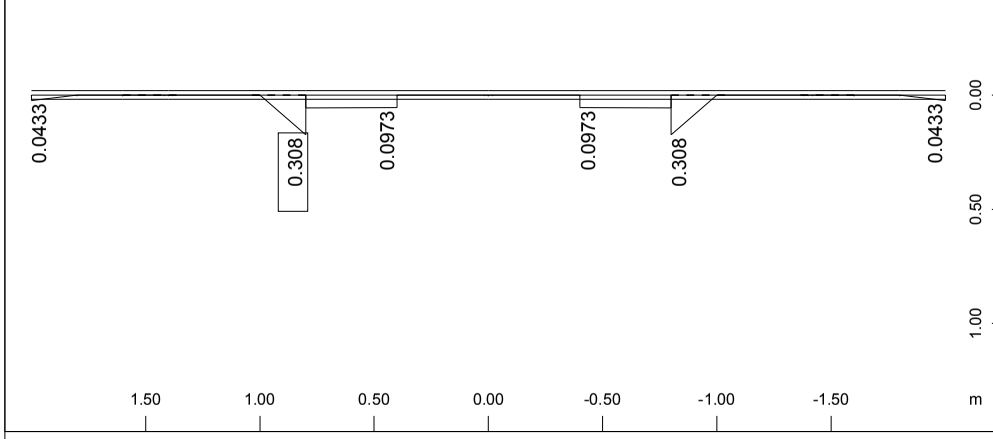


Z Y X
Sector of system Beam Elements Group 2 M 1 : 30
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
502 , 1 cm 3D = 0 cm2 (Max=0)

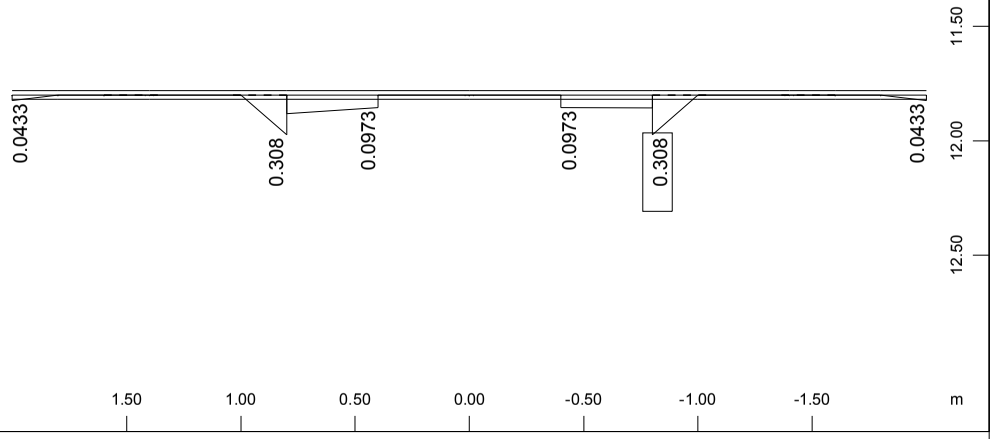
X Z Y
Sector of system Beam Elements Group 2 M 1 : 30
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
502 , 1 cm 3D = 0 cm2 (Max=0)

ΠΑΣΣΑΛΟΔΕΣΜΟΣ

ΚΑΤΩ ΟΠΛΙΣΜΟΣ

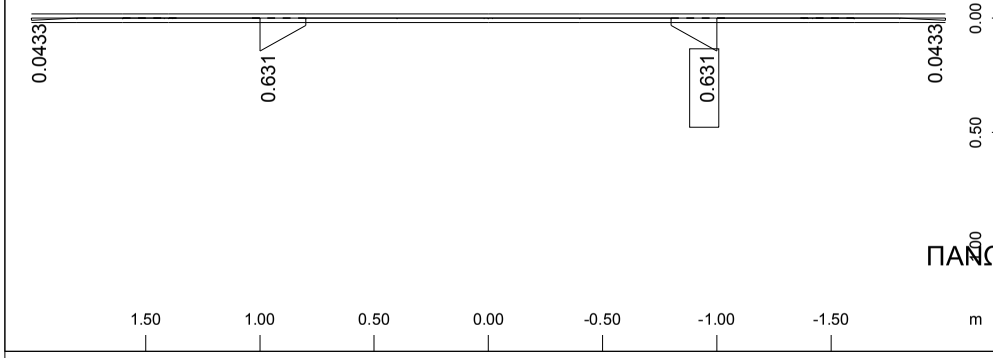


Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
501 , 1 cm 3D = 0.581 cm2 (Max=0.308)

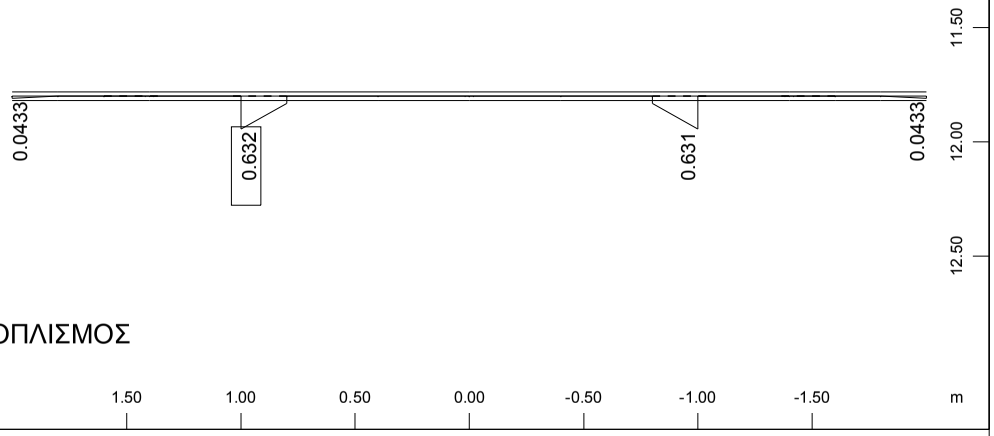


Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
501 , 1 cm 3D = 0.581 cm2 (Max=0.308)

ΠΑΝΩ ΟΠΛΙΣΜΟΣ

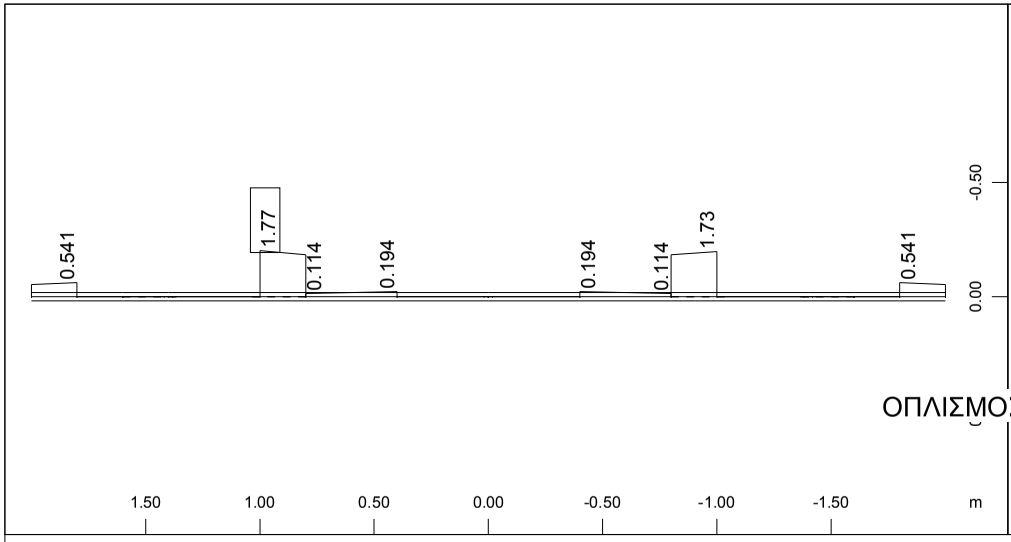


Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
501 , 1 cm 3D = 1.45 cm2 (Max=0.631)

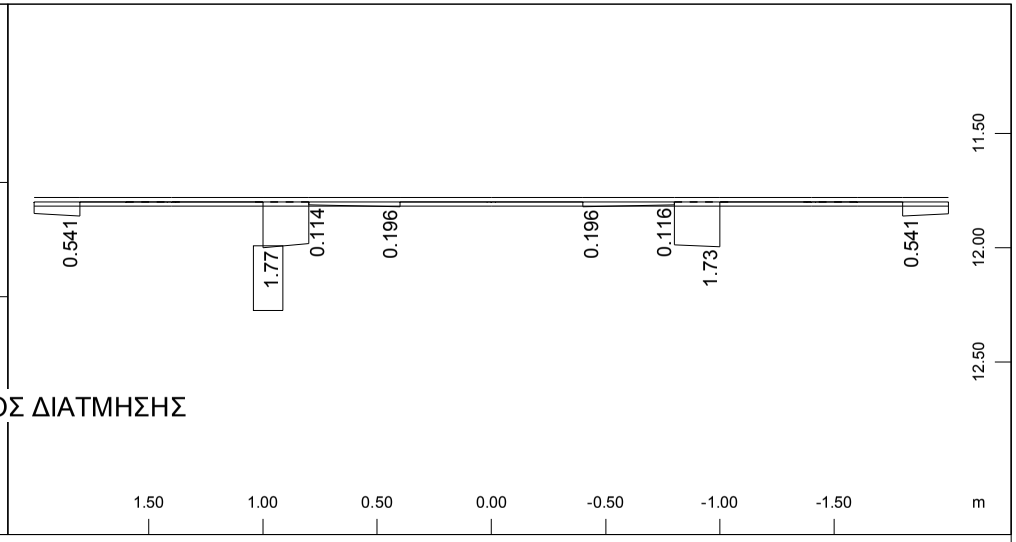


Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
501 , 1 cm 3D = 1.45 cm2 (Max=0.632)

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΦΑΣΗ-1_ULS

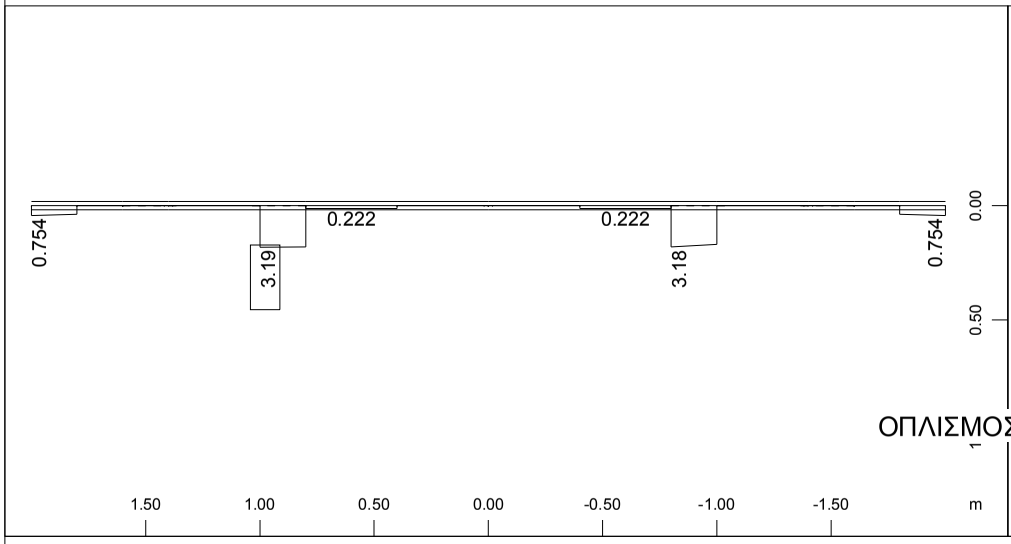


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Shear reinforcements (maximum), Design Case 501
, 1 cm 3D = 2.91 cm2/m (Max=1.77)

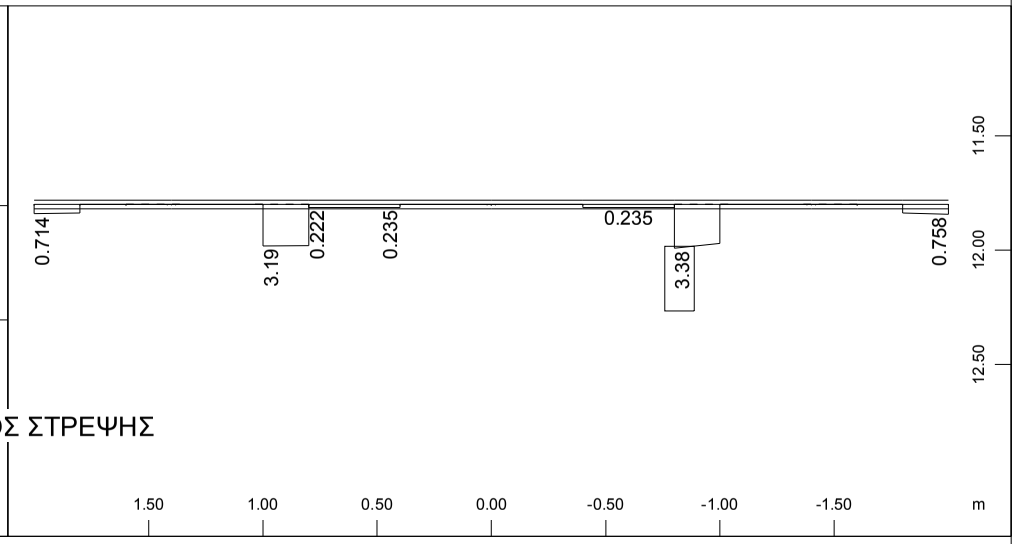


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Shear reinforcements (maximum), Design Case 501
, 1 cm 3D = 2.91 cm2/m (Max=1.77)

ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ

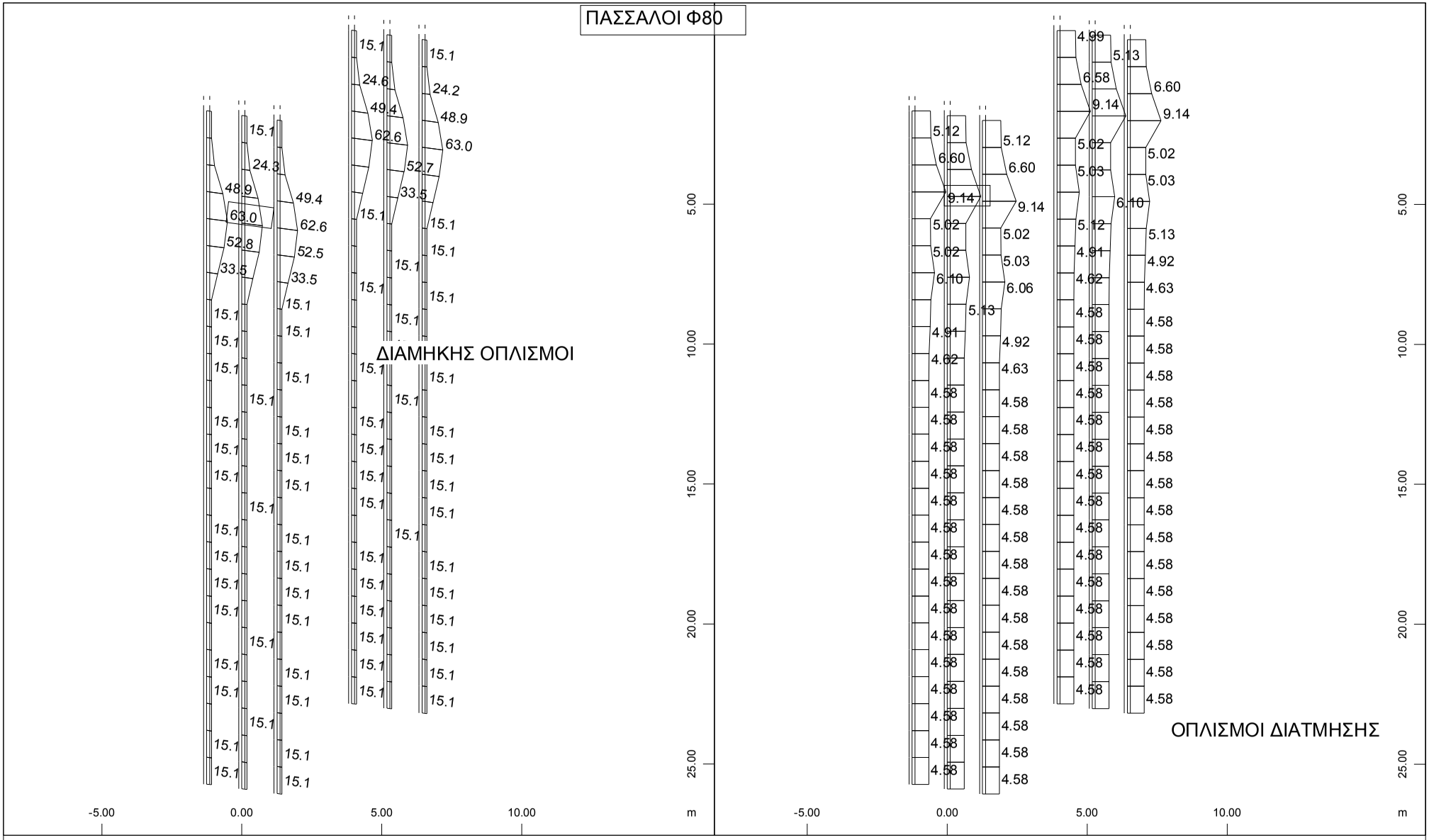


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
501 , 1 cm 3D = 5.81 cm2 (Max=3.19)



Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
501 , 1 cm 3D = 5.81 cm2 (Max=3.38)

ΟΠΛΙΣΜΟΣ ΣΤΡΕΨΗΣ



Sector of system Beam Elements Group 12
Beam Elements , Longitudinal Reinforcements Lay. 1, Design
Case 501 , 1 cm 3D = 145.3 cm2 (Max=63.0)

M 1 : 191
X * 0.502
Y * 0.906
Z * 0.962

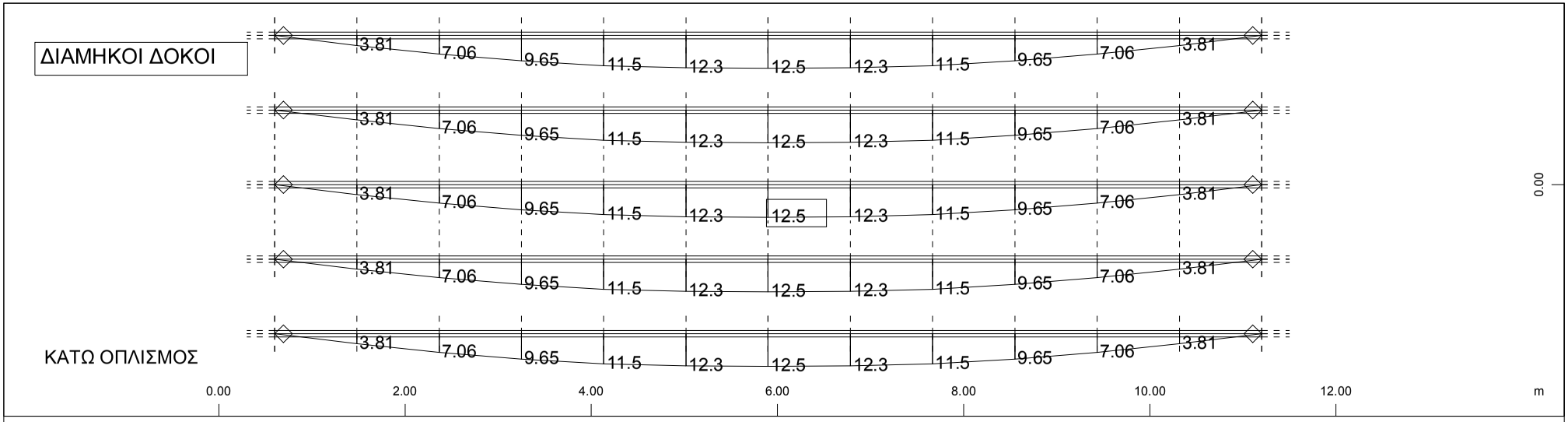
Sector of system Beam Elements Group 12
Beam Elements , Shear reinforcements (maximum), Design Case
501 , 1 cm 3D = 14.5 cm2/m (Max=9.14)

M 1 : 191
X * 0.502
Y * 0.906
Z * 0.962

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΦΑΣΗ-1_ULS

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00

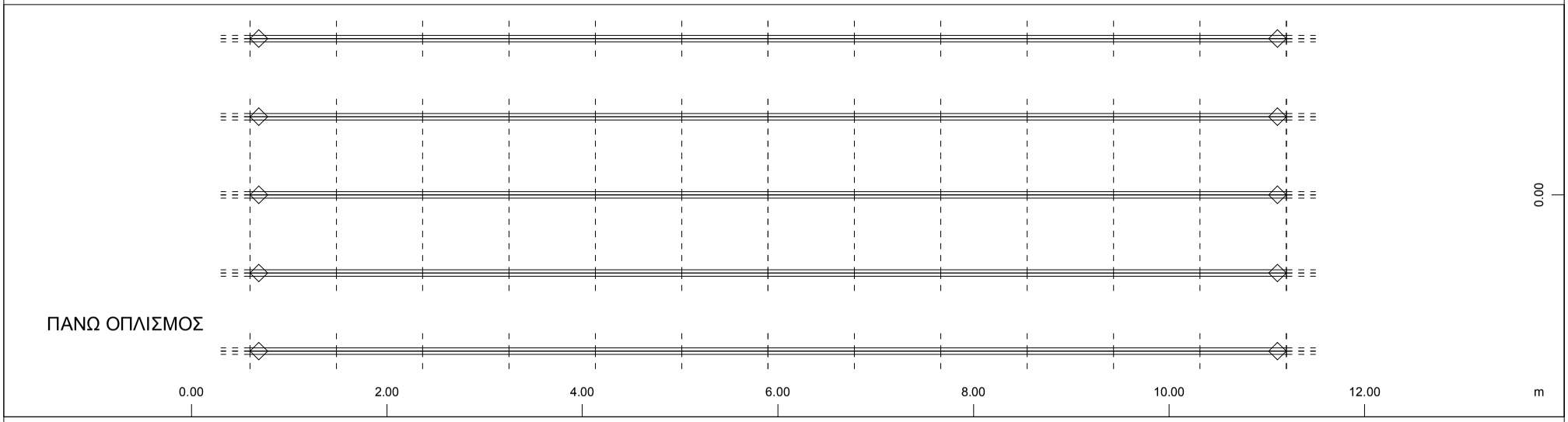
6) ΦΑΣΗ-1 ΕΛΕΓΧΟΣ ΦΟΡΕΑ ΣΕ SLS



Z—X
Y

Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case 503 , 1 cm 3D = 22.4 cm2 (Max=12.5)

M 1 : 63

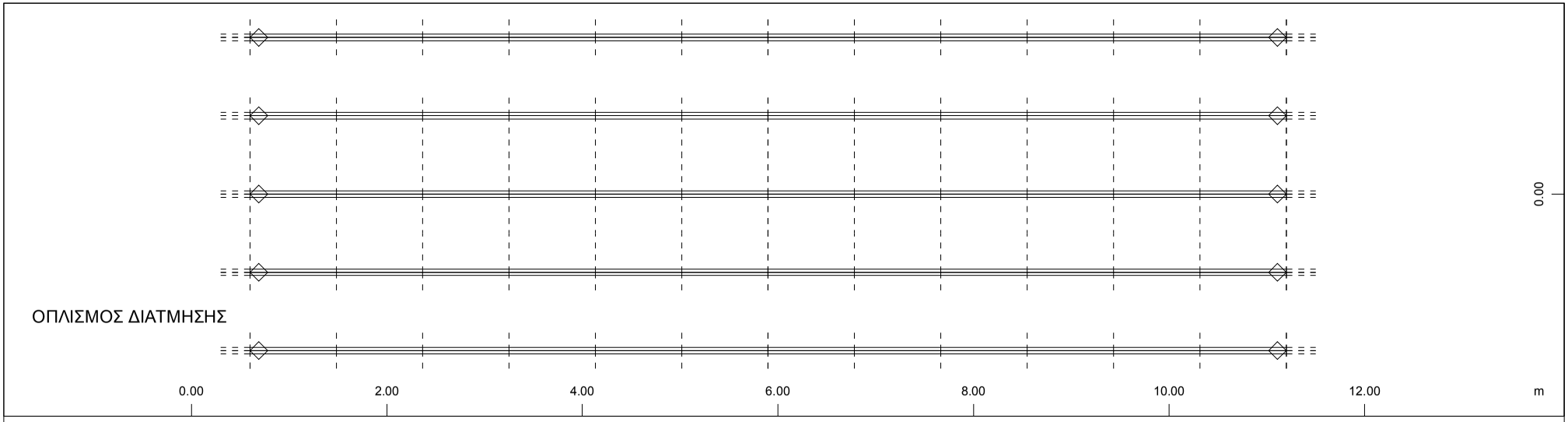


Z—X
Y

Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case 503 , 1 cm 3D = 0 cm2 (Max=0)

M 1 : 60

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΦΑΣΗ-1_SLS

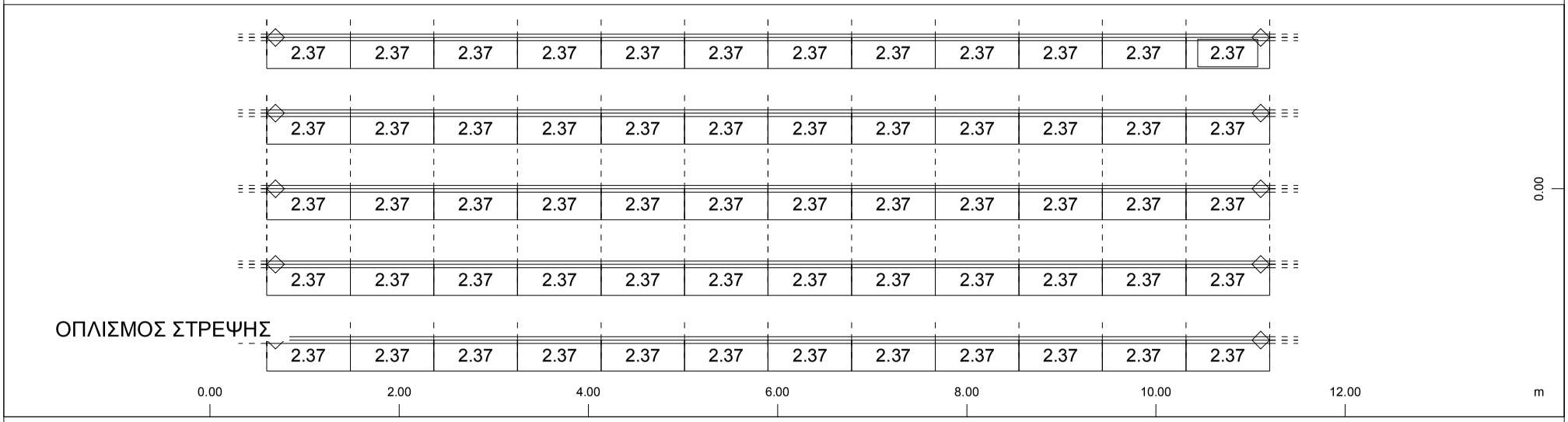


Z X Y

Sector of system Beam Elements Group 1

Beam Elements , Shear reinforcements (maximum), Design Case 503 , 1 cm 3D = 0 cm2/m (Max=0)

M 1 : 60



Z X Y

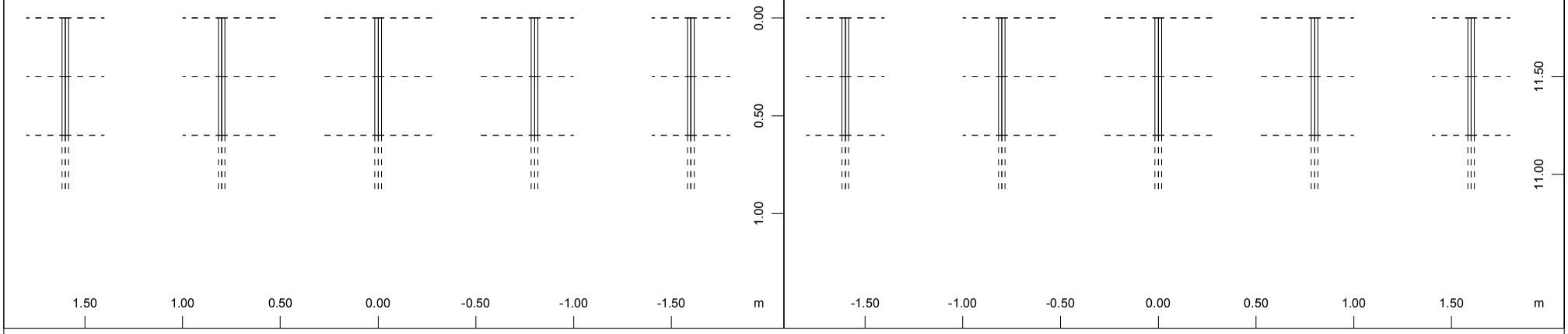
Sector of system Beam Elements Group 1

Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case 503 , 1 cm 3D = 4.48 cm2 (Max=2.37)

M 1 : 62

ΔΙΑΜΗΚΗ ΔΟΚΟΙ-ΣΤΗΡΙΞΗΣ

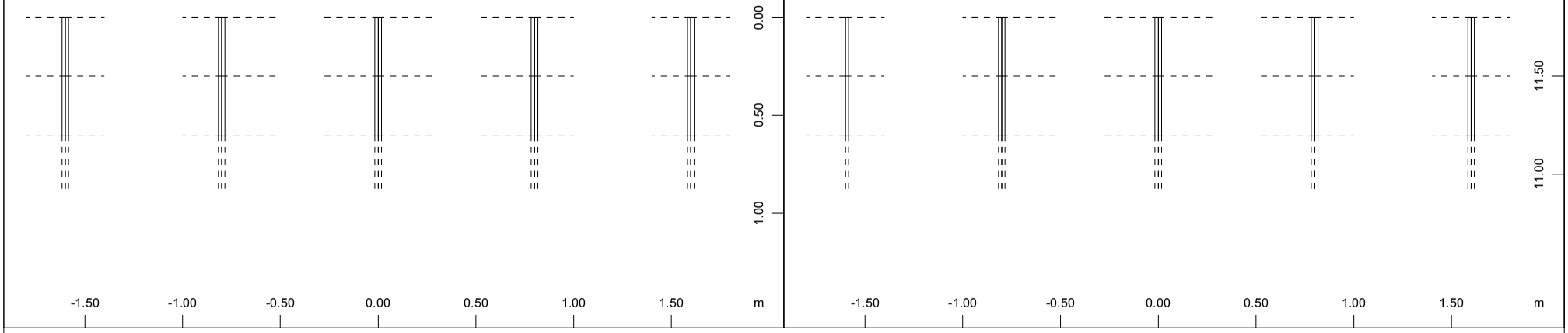
ΚΑΤΩ ΟΠΛΙΣΜΟΣ



Y Z X
Sector of system Beam Elements Group 2 M 1 : 30
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
503 , 1 cm 3D = 0 cm2 (Max=0)

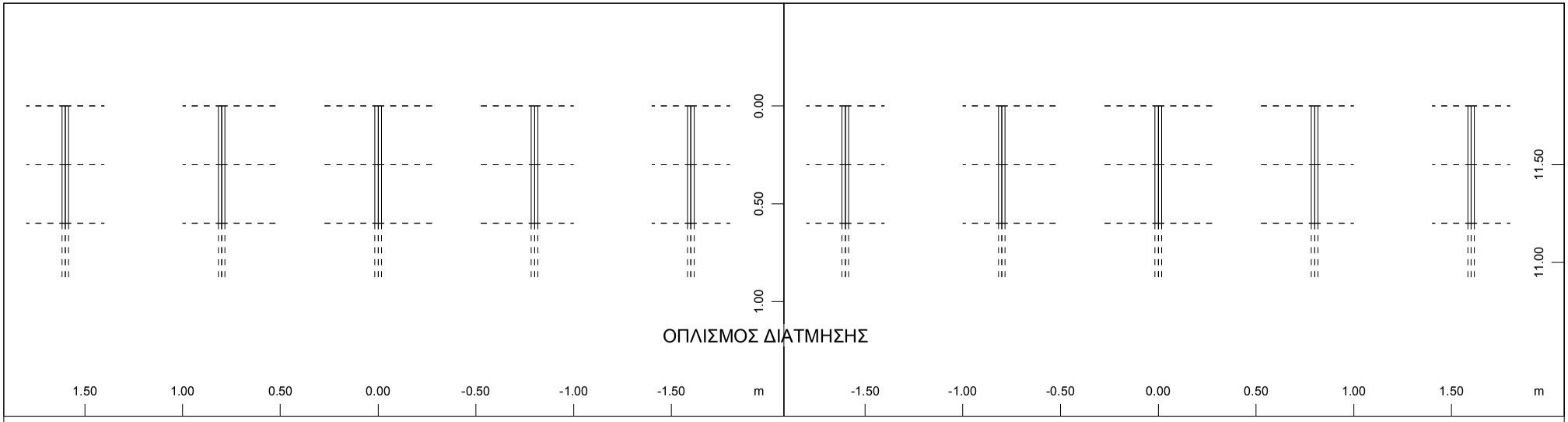
X Z Y
Sector of system Beam Elements Group 2 M 1 : 30
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
503 , 1 cm 3D = 0 cm2 (Max=0)

ΠΑΝΩ ΟΠΛΙΣΜΟΣ



Z Y X
Sector of system Beam Elements Group 2 M 1 : 30
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
503 , 1 cm 3D = 0 cm2 (Max=0)

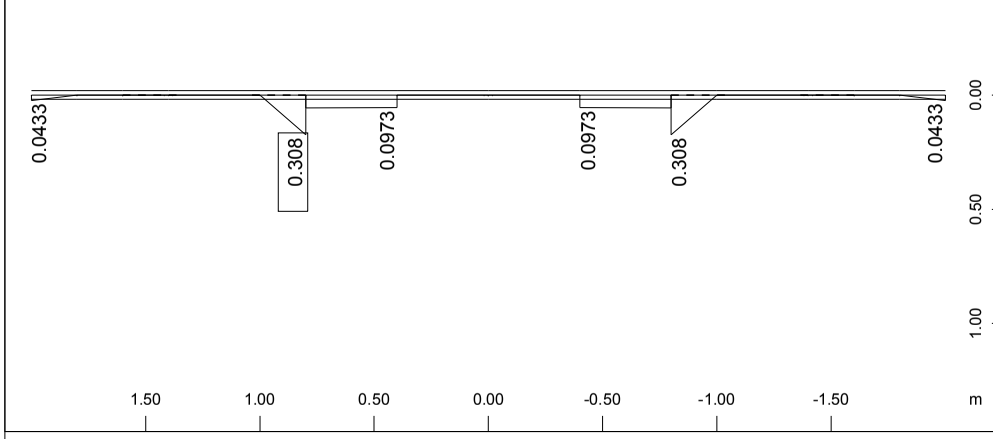
X Z Y
Sector of system Beam Elements Group 2 M 1 : 30
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
503 , 1 cm 3D = 0 cm2 (Max=0)



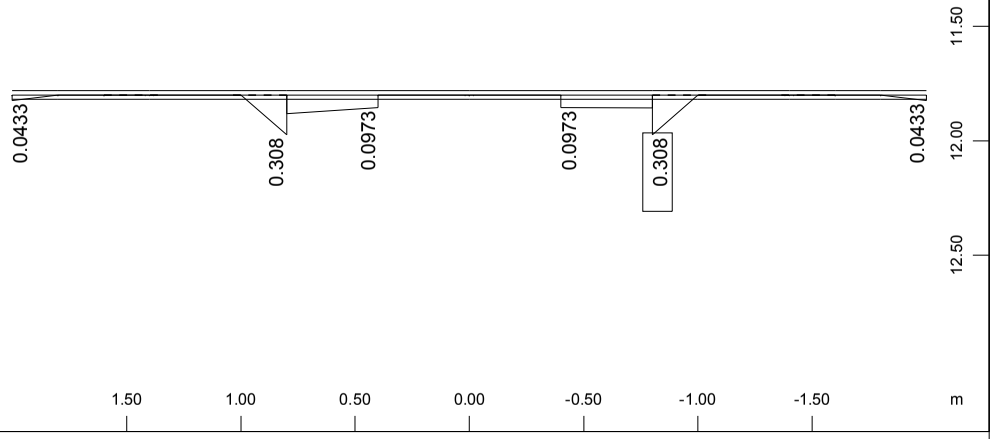
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΦΑΣΗ-1_SLS

ΠΑΣΣΑΛΟΔΕΣΜΟΣ

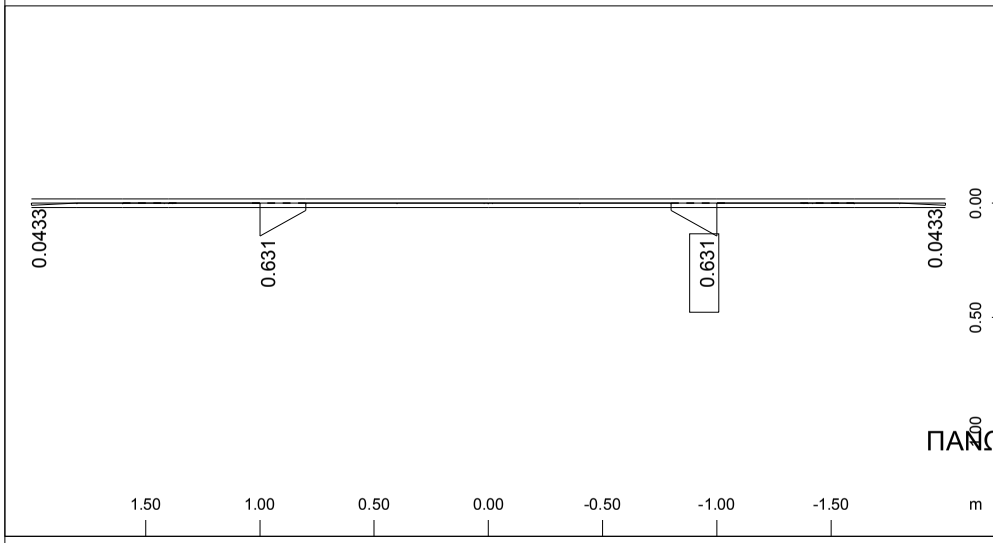
ΚΑΤΩ ΟΠΛΙΣΜΟΣ



Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
503 , 1 cm 3D = 0.581 cm2 (Max=0.308)

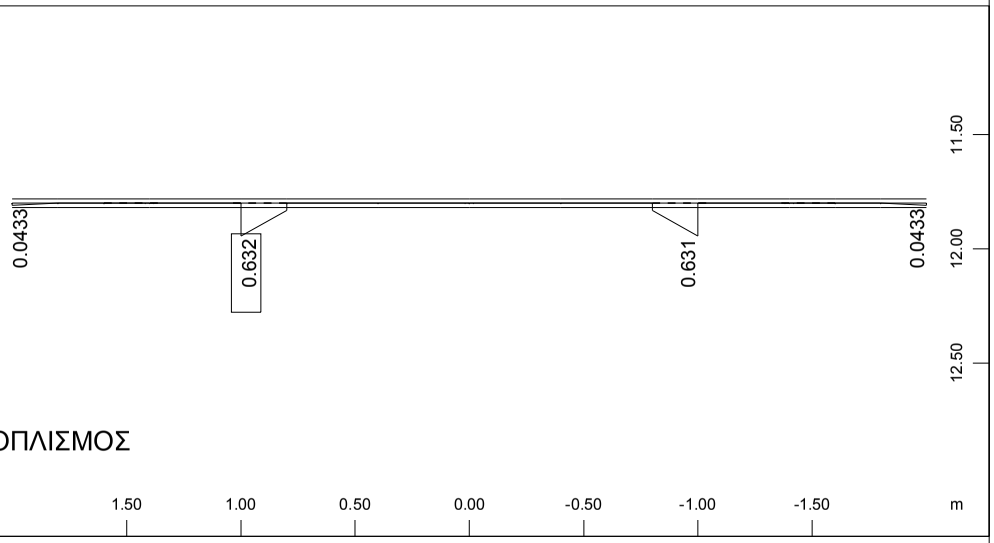


Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
503 , 1 cm 3D = 0.581 cm2 (Max=0.308)

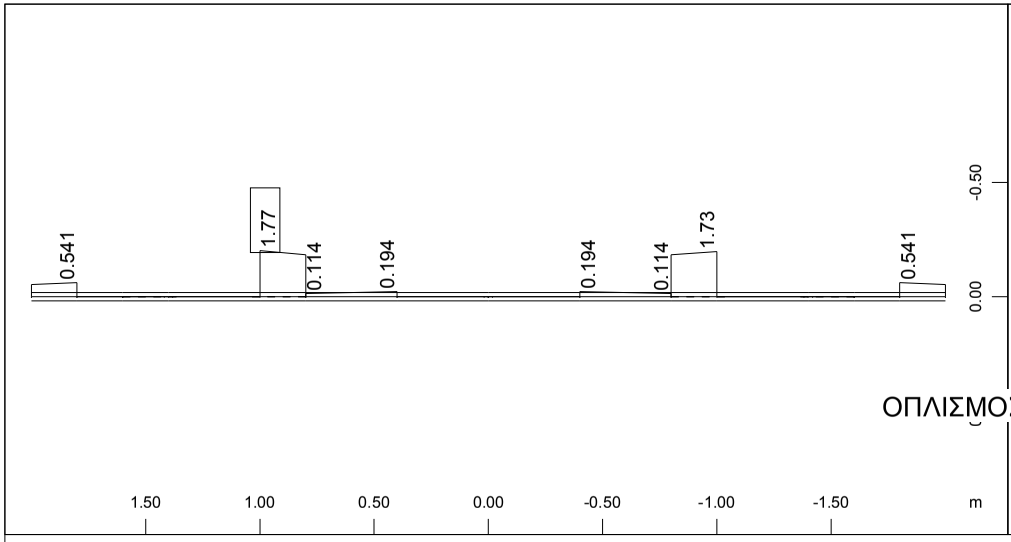


Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
503 , 1 cm 3D = 1.45 cm2 (Max=0.631)

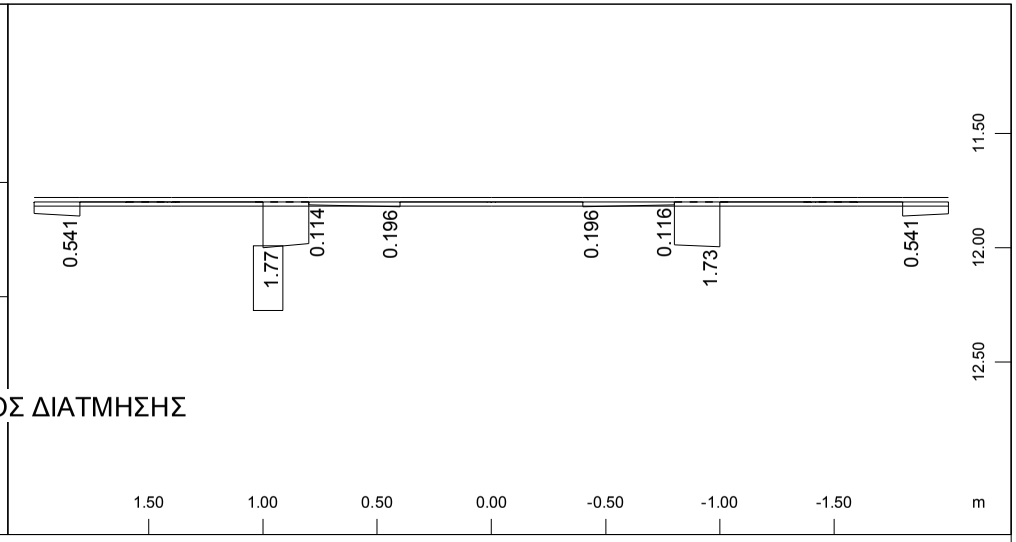
ΠΑΝΩ ΟΠΛΙΣΜΟΣ



Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
503 , 1 cm 3D = 1.45 cm2 (Max=0.632)

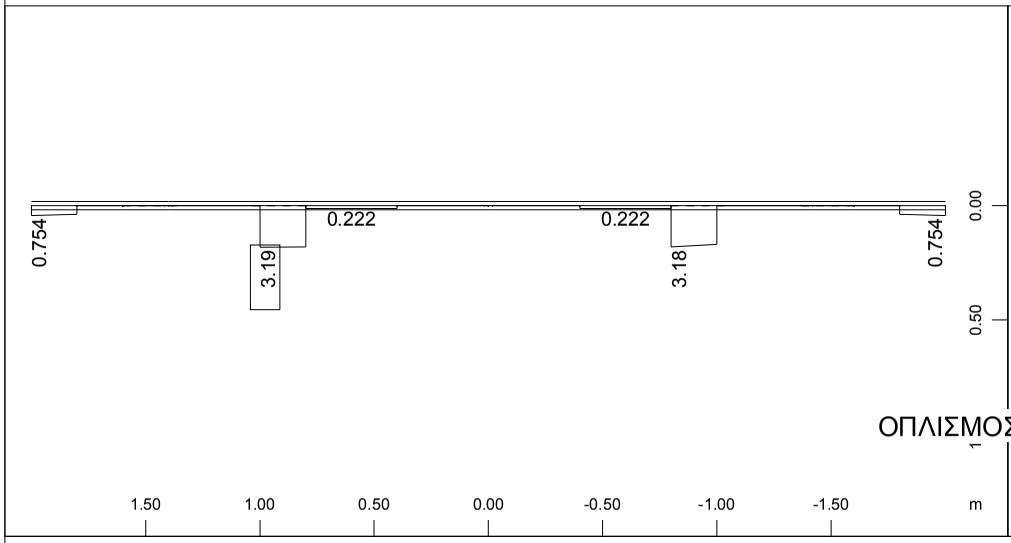


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Shear reinforcements (maximum), Design Case 503
, 1 cm 3D = 2.91 cm2/m (Max=1.77)

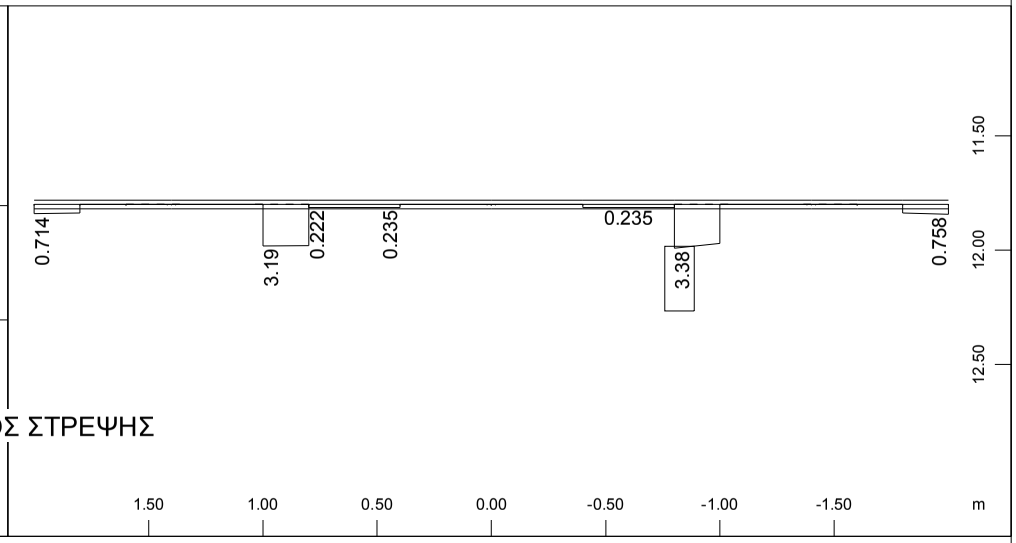


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Shear reinforcements (maximum), Design Case 503
, 1 cm 3D = 2.91 cm2/m (Max=1.77)

ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ

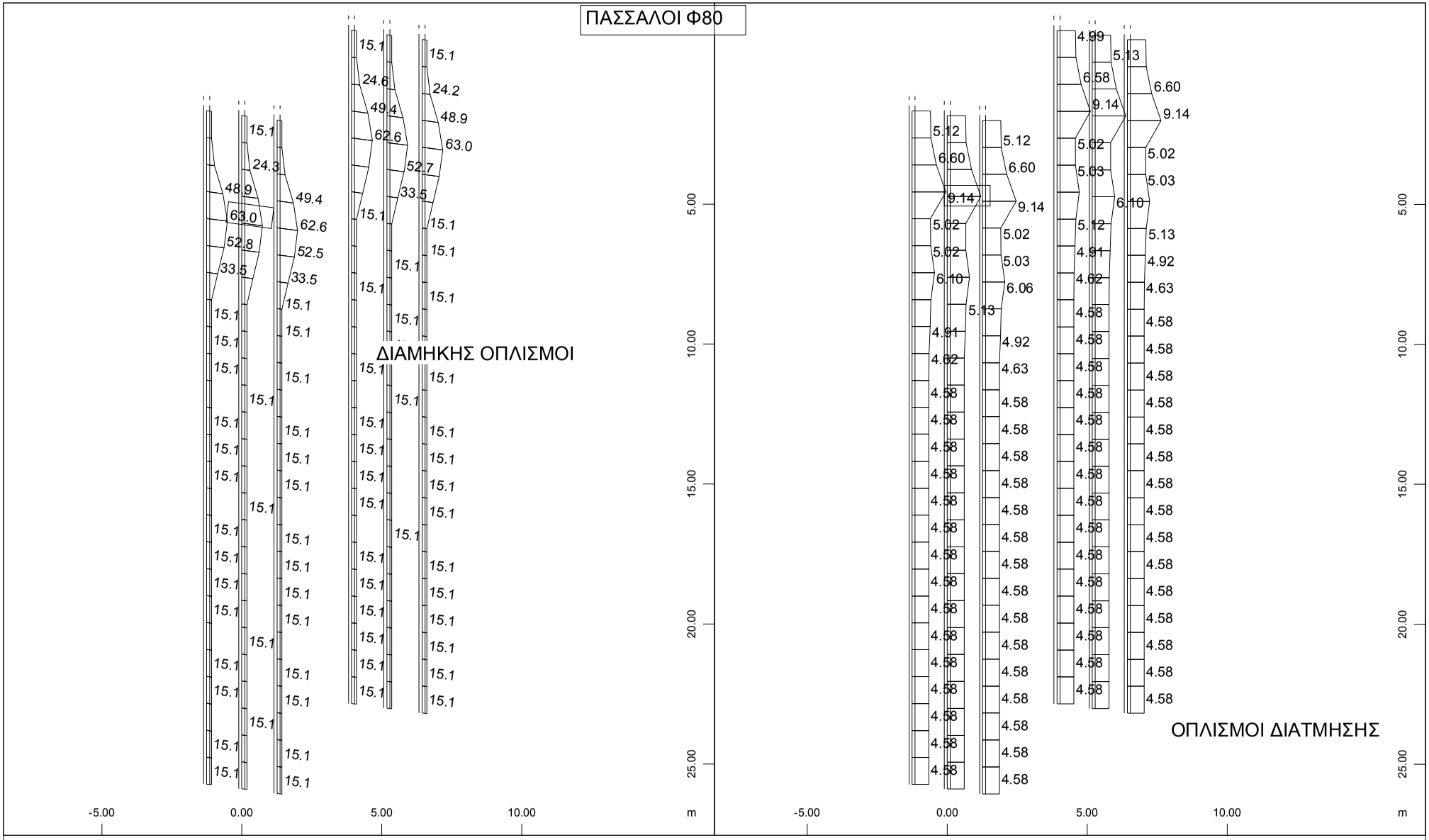


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
503 , 1 cm 3D = 5.81 cm2 (Max=3.19)



Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
503 , 1 cm 3D = 5.81 cm2 (Max=3.38)

ΟΠΛΙΣΜΟΣ ΣΤΡΕΨΗΣ



Sector of system Beam Elements Group 12
Beam Elements , Longitudinal Reinforcements Lay. 1, Design
Case 503 , 1 cm 3D = 145.3 cm2 (Max=63.0)

M 1 : 191
X * 0.502
Y * 0.906
Z * 0.962

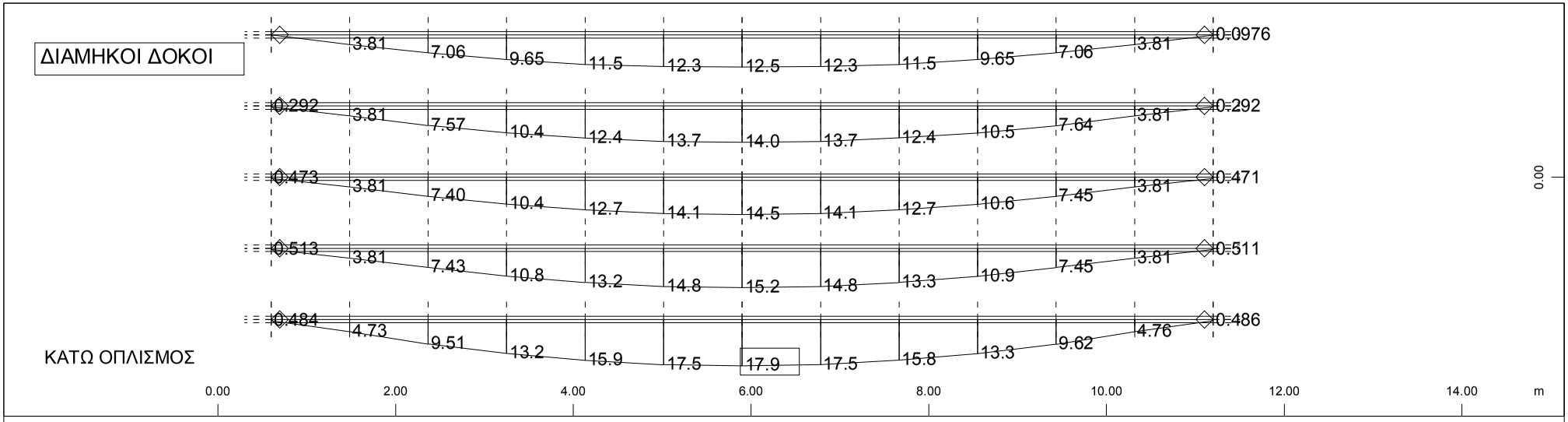
Sector of system Beam Elements Group 12
Beam Elements , Shear reinforcements (maximum), Design Case
503 , 1 cm 3D = 14.5 cm2/m (Max=9.14)

M 1 : 191
X * 0.502
Y * 0.906
Z * 0.962

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΦΑΣΗ-1_SLS

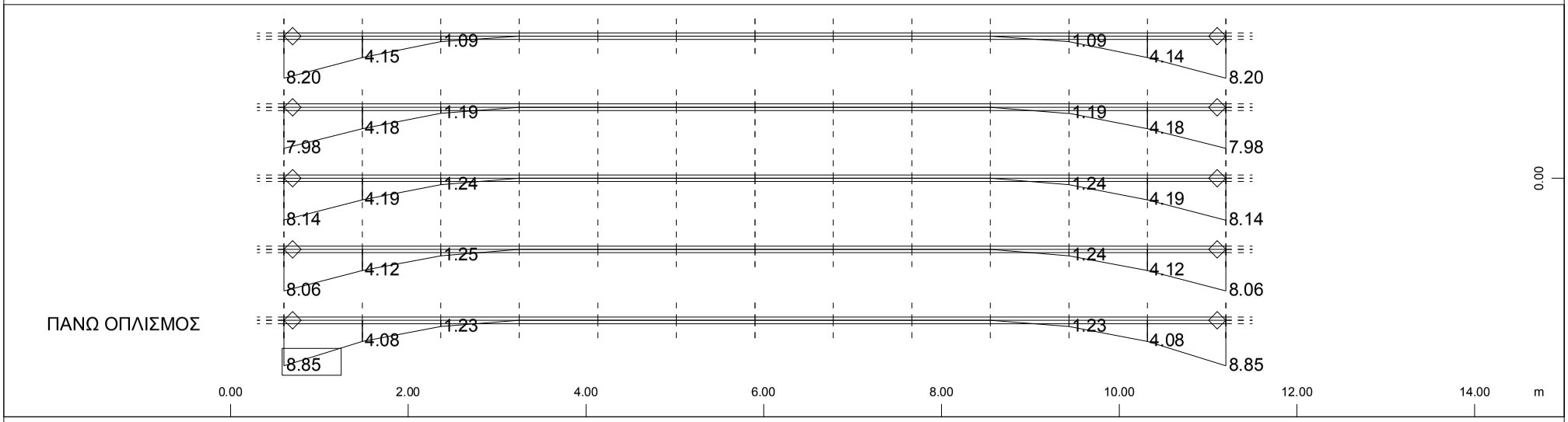
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00

7) ΦΑΣΗ-2_ ΕΛΕΓΧΟΣ ΦΟΡΕΑ ΣΕ ULS-STATIKA



Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case 507 , 1 cm 3D = 22.4 cm2 (Max=17.9)

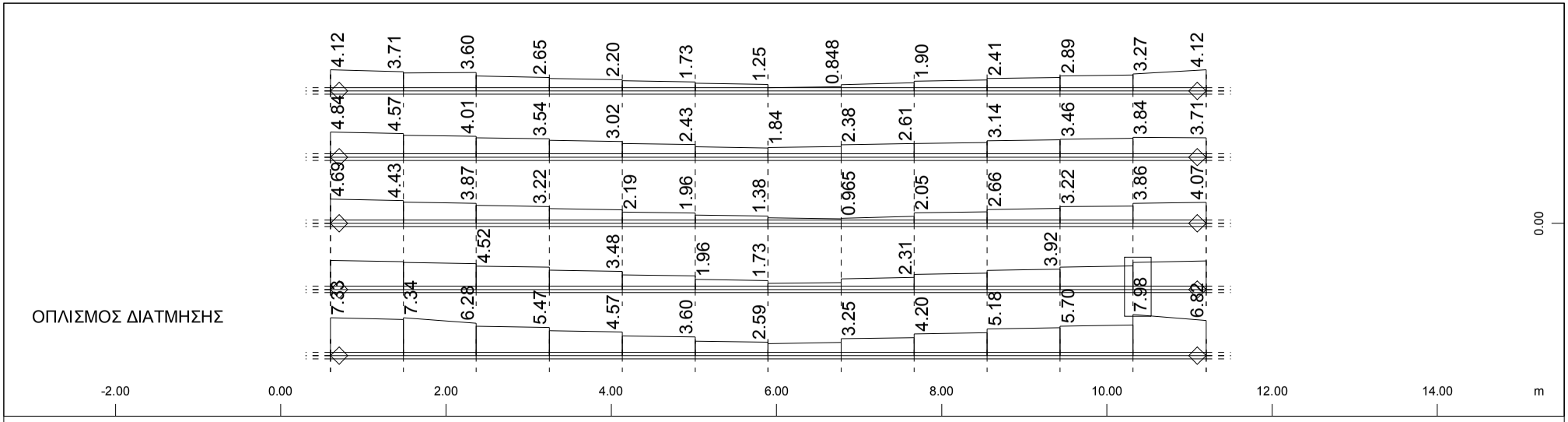
M 1 : 66



Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case 507 , 1 cm 3D = 11.2 cm2 (Max=8.85)

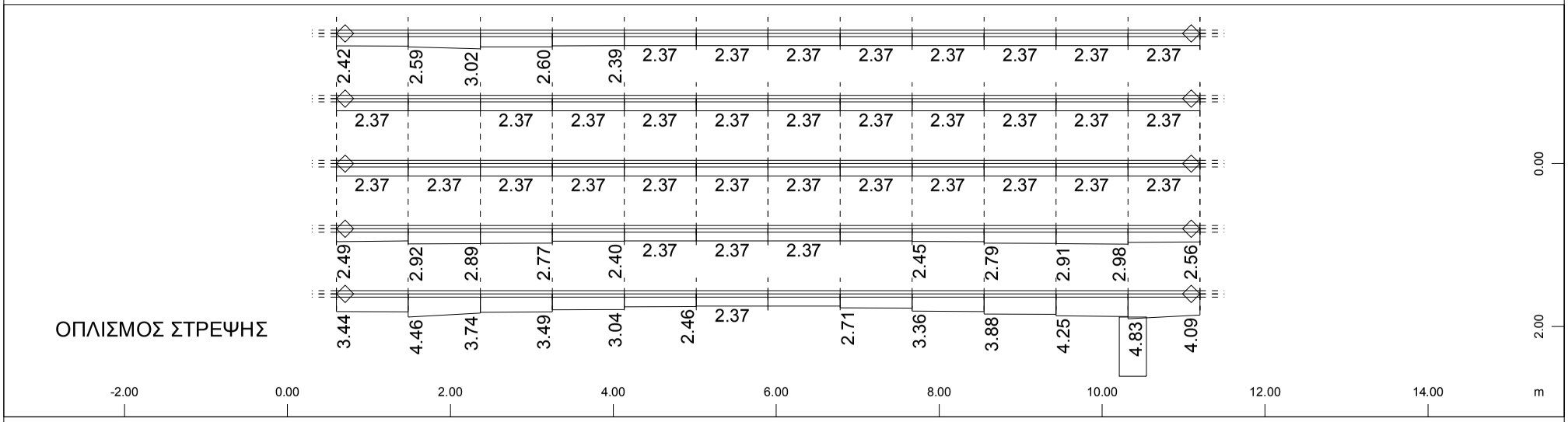
M 1 : 66

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΦΑΣΗ-2_ULS



Sector of system Beam Elements Group 1
Beam Elements , Shear reinforcements (maximum), Design Case 507 , 1 cm 3D = 11.2 cm2/m (Max=7.98)

M 1 : 71

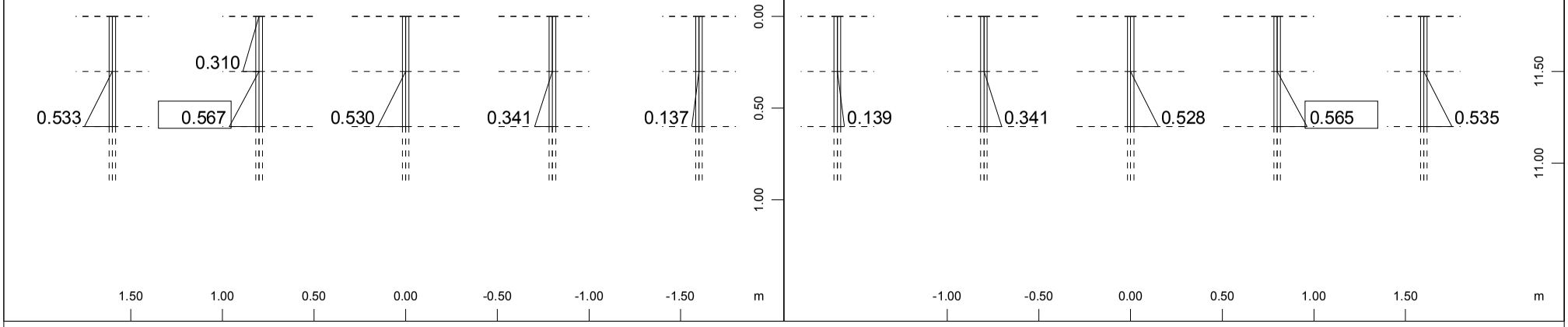


Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case 507 , 1 cm 3D = 11.2 cm2 (Max=4.83)

M 1 : 72

ΔΙΑΜΗΚΗ ΔΟΚΟΙ-ΣΤΗΡΙΞΗΣ

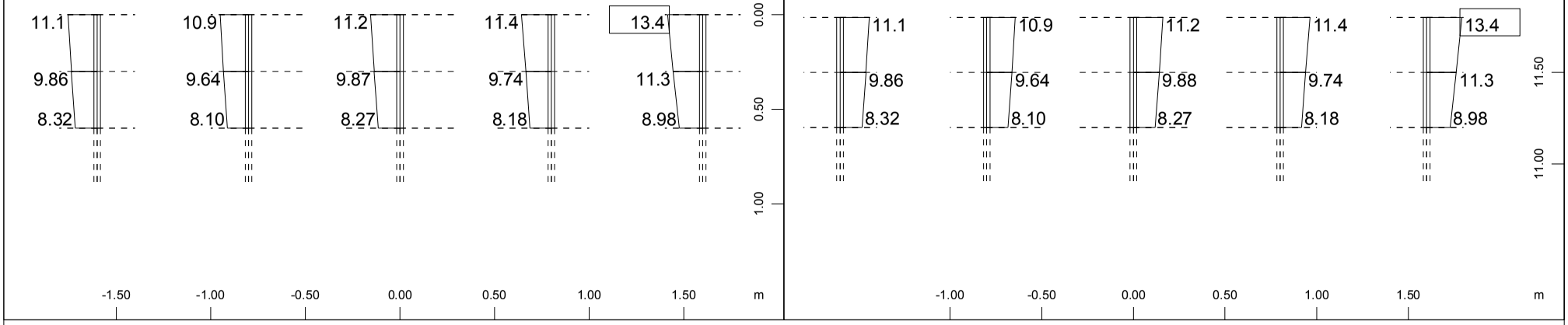
ΚΑΤΩ ΟΠΛΙΣΜΟΣ



Y Z X
Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
507 , 1 cm 3D = 1.12 cm2 (Max=0.567)

X Z Y
Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
507 , 1 cm 3D = 1.12 cm2 (Max=0.565)

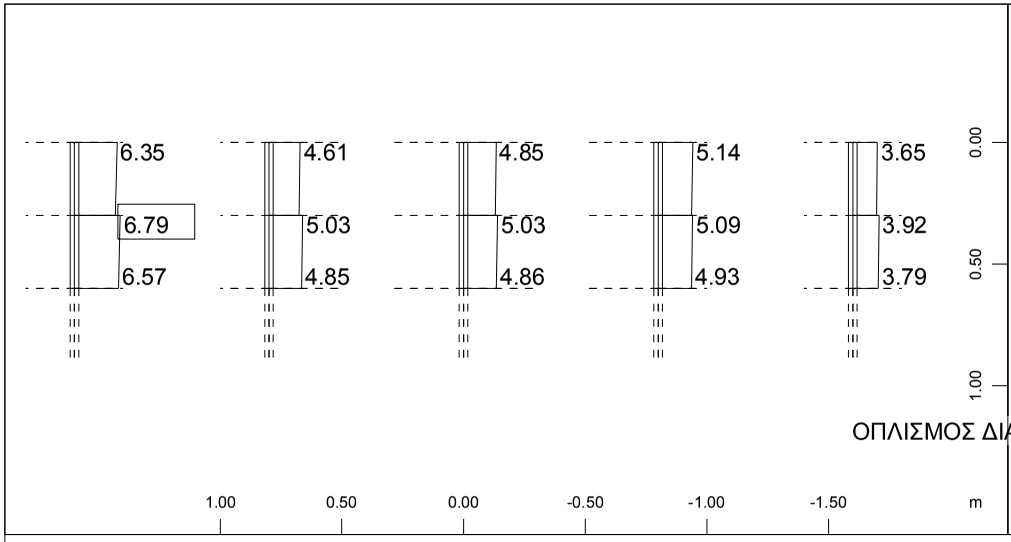
ΠΑΝΩ ΟΠΛΙΣΜΟΣ



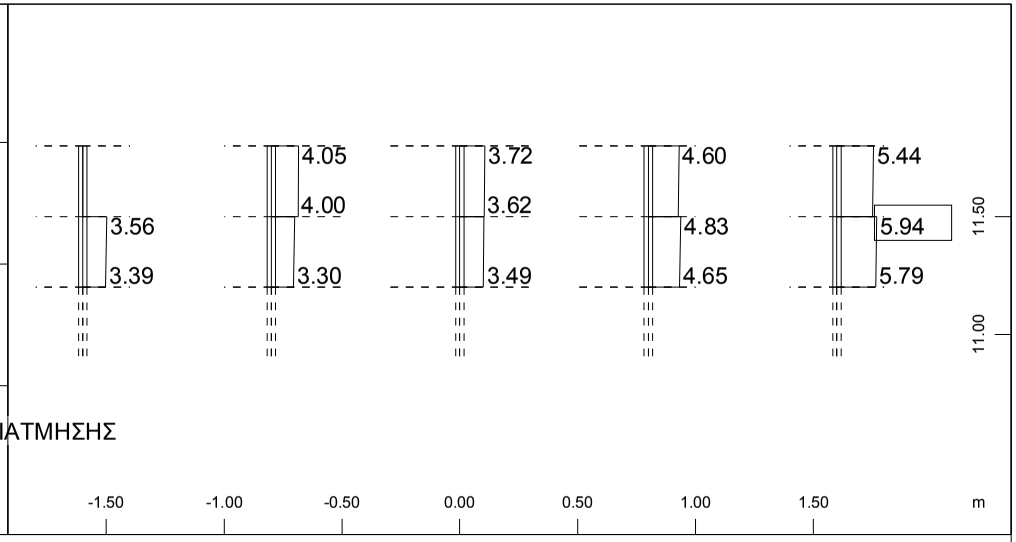
Z Y X
Sector of system Beam Elements Group 2 M 1 : 31
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
507 , 1 cm 3D = 22.4 cm2 (Max=13.4)

X Z Y
Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
507 , 1 cm 3D = 22.4 cm2 (Max=13.4)

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΦΑΣΗ-2_ULS

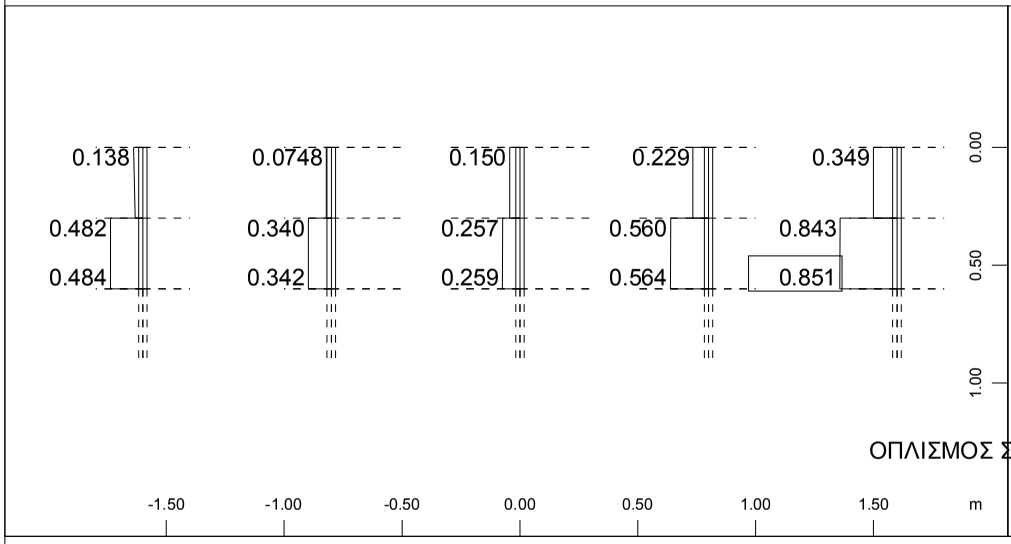


Sector of system Beam Elements Group 2 M 1 : 31
Beam Elements , Shear reinforcements (maximum), Design Case 507
, 1 cm 3D = 11.2 cm2/m (Max=6.79)

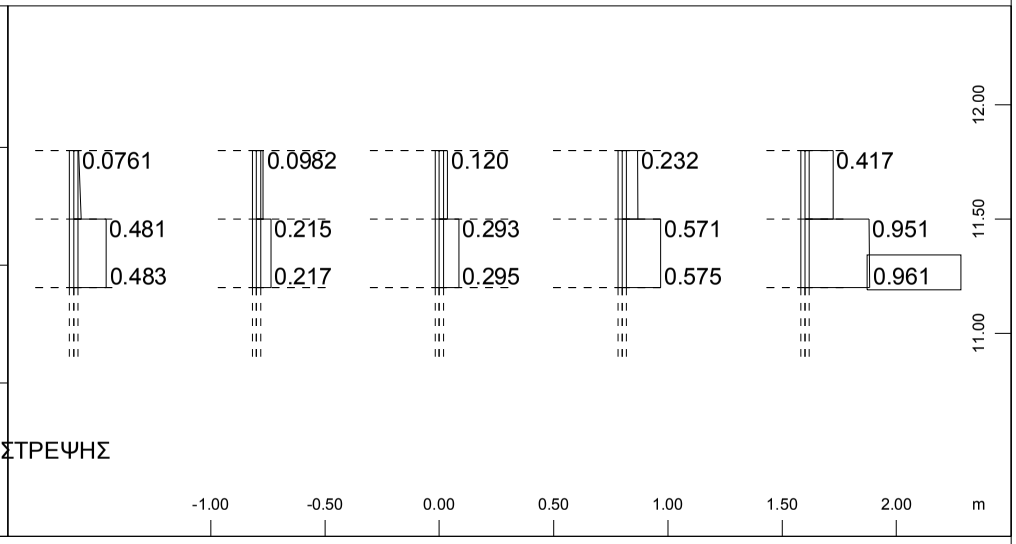


Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Shear reinforcements (maximum), Design Case 507
, 1 cm 3D = 11.2 cm2/m (Max=5.94)

ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ



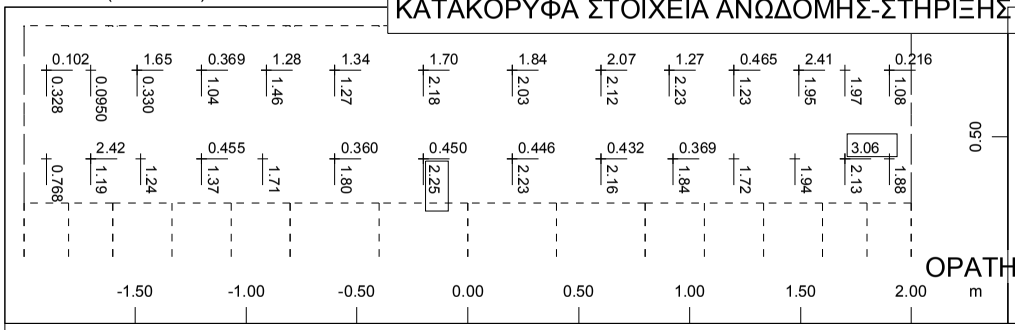
Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
507 , 1 cm 3D = 1.12 cm2 (Max=0.851)



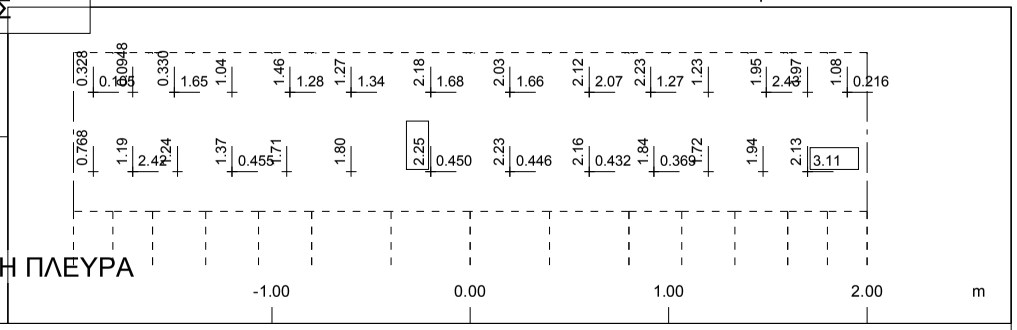
Sector of system Beam Elements Group 2 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
507 , 1 cm 3D = 1.12 cm2 (Max=0.961)

ΟΠΛΙΣΜΟΣ ΣΤΡΕΨΗΣ

ΚΑΤΑΚΟΡΥΦΑ ΣΤΟΙΧΕΙΑ ΑΝΩΔΟΜΗΣ-ΣΤΗΡΙΞΗΣ

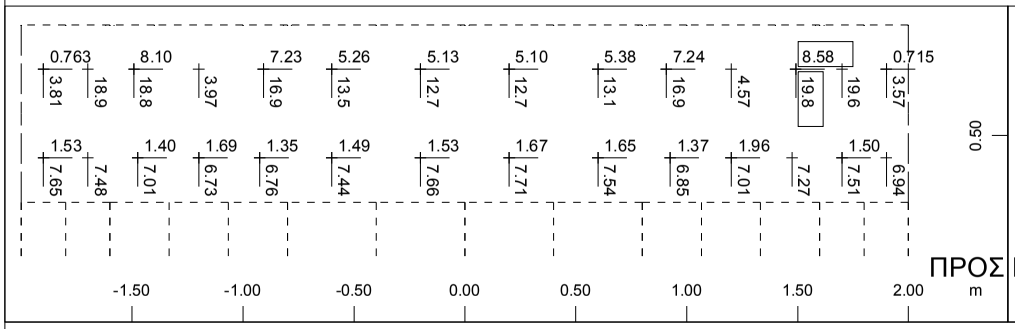


Sector of system Quadrilateral Elements Group 8 M 1 : 34
upper Reinforcements in Elements in cm2/m, Design Case 521 ULS
design (Max=3.06)

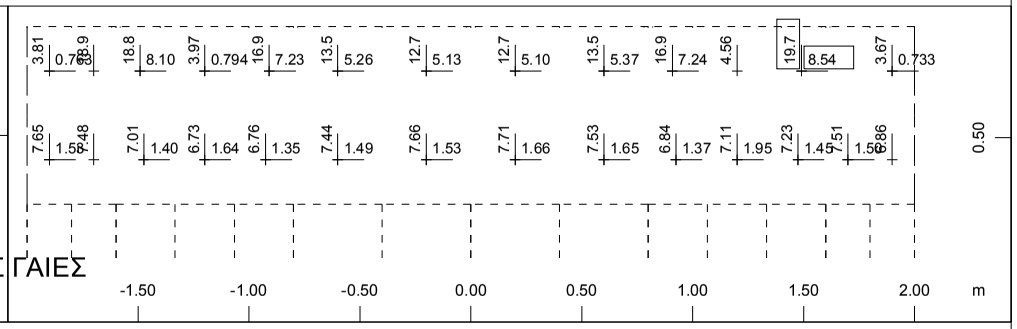


Sector of system Quadrilateral Elements Group 8 M 1 : 38
upper Reinforcements in Elements in cm2/m, Design Case 521 ULS
design (Max=3.11)

ΟΡΑΤΗ ΠΛΕΥΡΑ

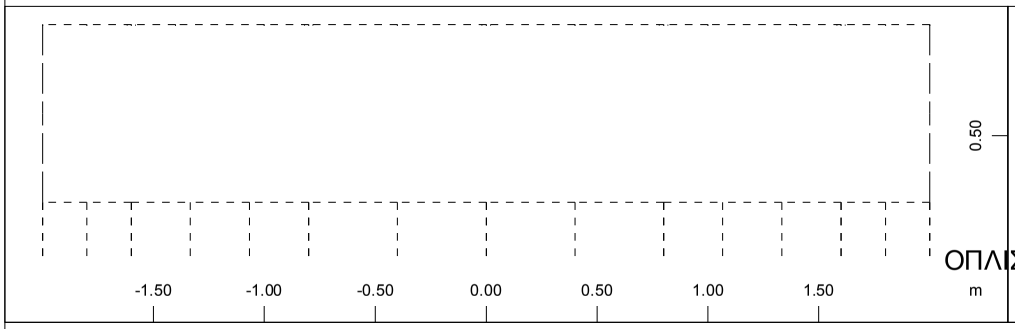


Sector of system Quadrilateral Elements Group 8 M 1 : 34
lower Reinforcements in Elements in cm2/m, Design Case 521 ULS
design (Max=19.8)

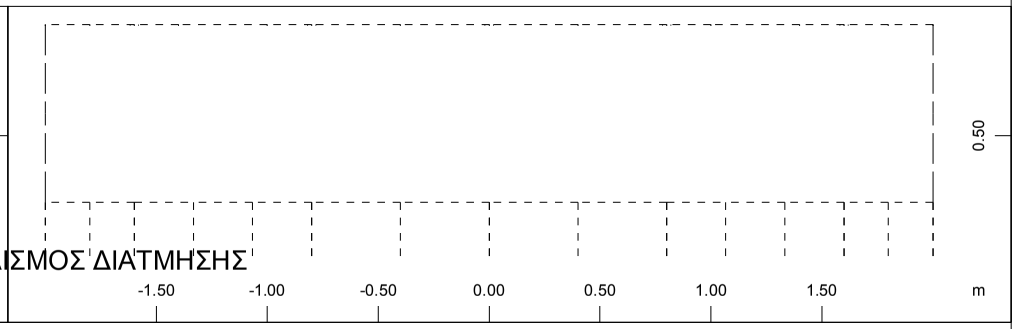


Sector of system Quadrilateral Elements Group 8 M 1 : 34
lower Reinforcements in Elements in cm2/m, Design Case 521 ULS
design (Max=19.7)

ΠΡΟΣ ΓΑΙΕΣ



Sector of system Quadrilateral Elements Group 8 M 1 : 34
Shear reinforcement from middle of element in cm2/m2, Design
Case 521 ULS design (Max=0)

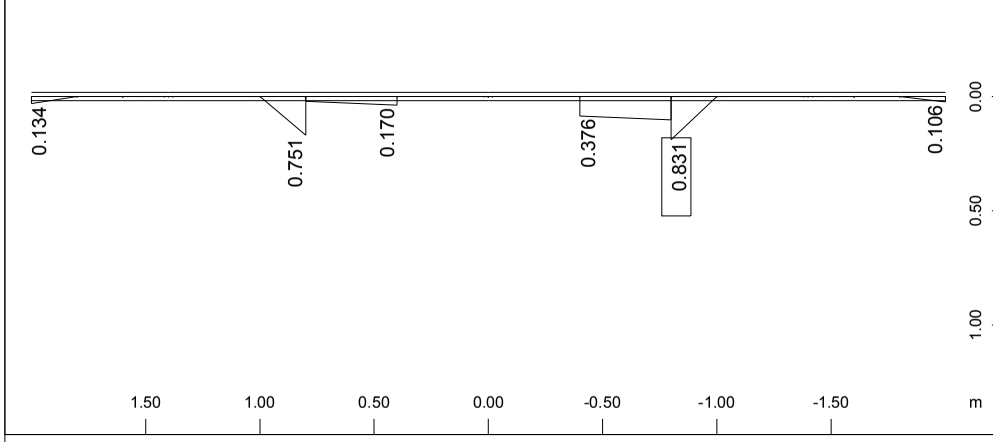


Sector of system Quadrilateral Elements Group 8 M 1 : 34
Shear reinforcement from middle of element in cm2/m2, Design
Case 521 ULS design (Max=0)

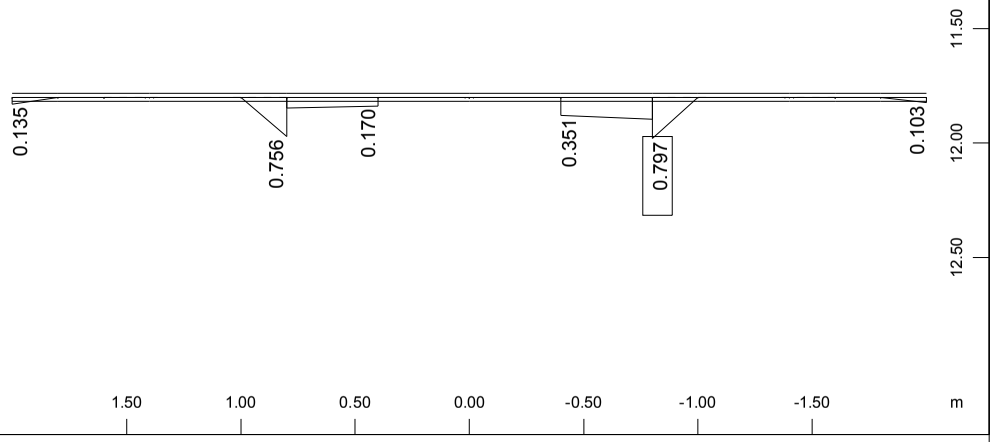
ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ

ΠΑΣΣΑΛΟΔΕΣΜΟΣ

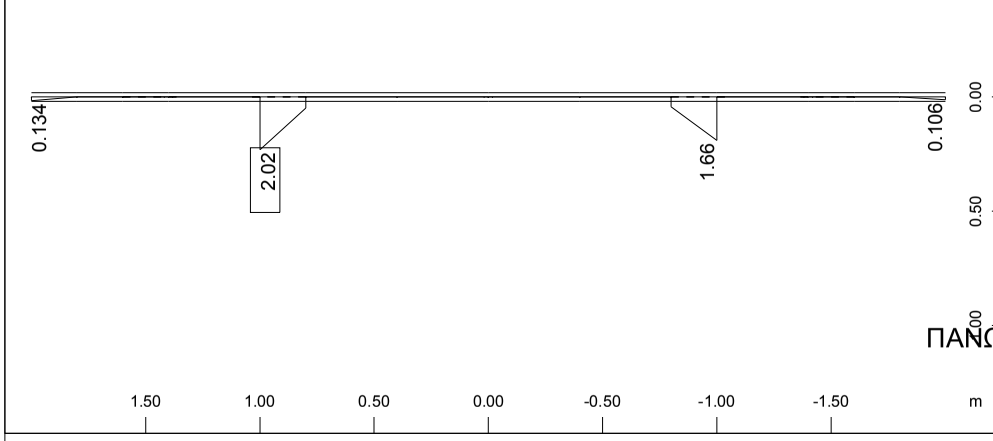
ΚΑΤΩ ΟΠΛΙΣΜΟΣ



Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
506 , 1 cm 3D = 1.45 cm2 (Max=0.831)

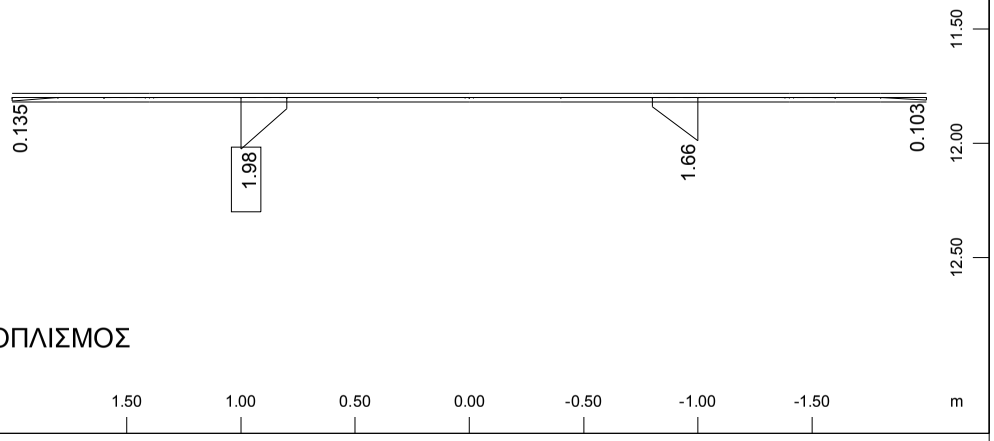


Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
506 , 1 cm 3D = 1.45 cm2 (Max=0.797)

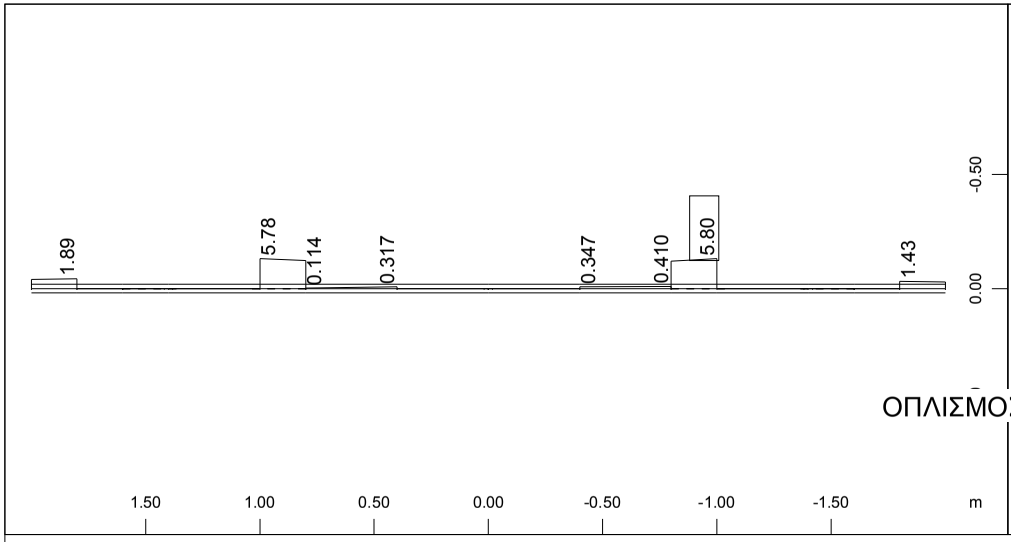


Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
506 , 1 cm 3D = 2.91 cm2 (Max=2.02)

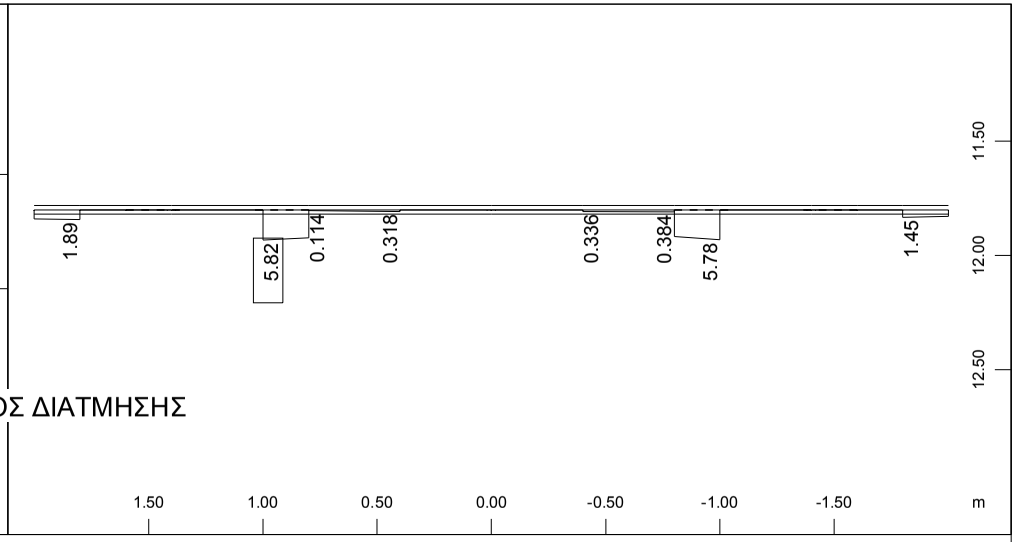
ΠΑΝΩ ΟΠΛΙΣΜΟΣ



Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
506 , 1 cm 3D = 2.91 cm2 (Max=1.98)

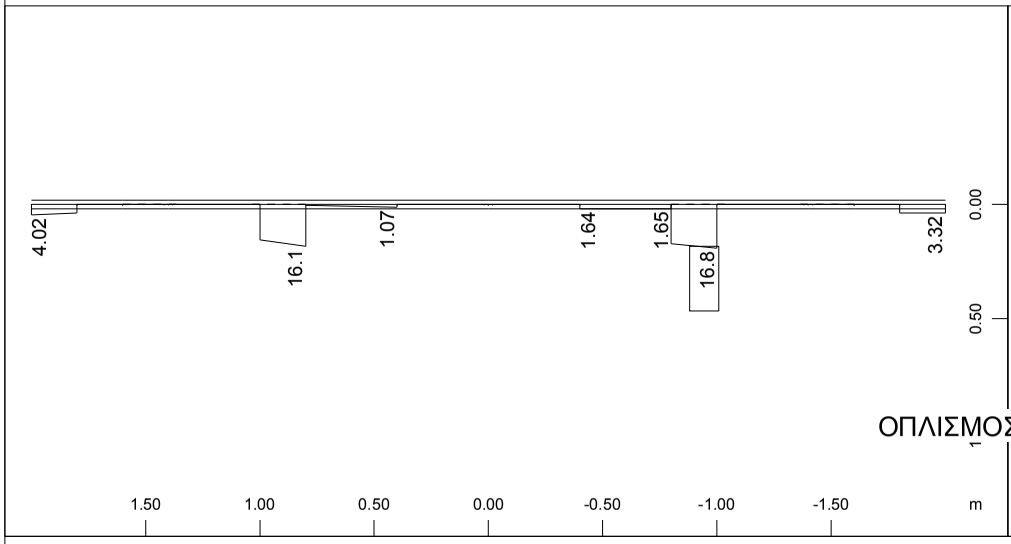


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Shear reinforcements (maximum), Design Case 506
, 1 cm 3D = 14.5 cm2/m (Max=5.80)

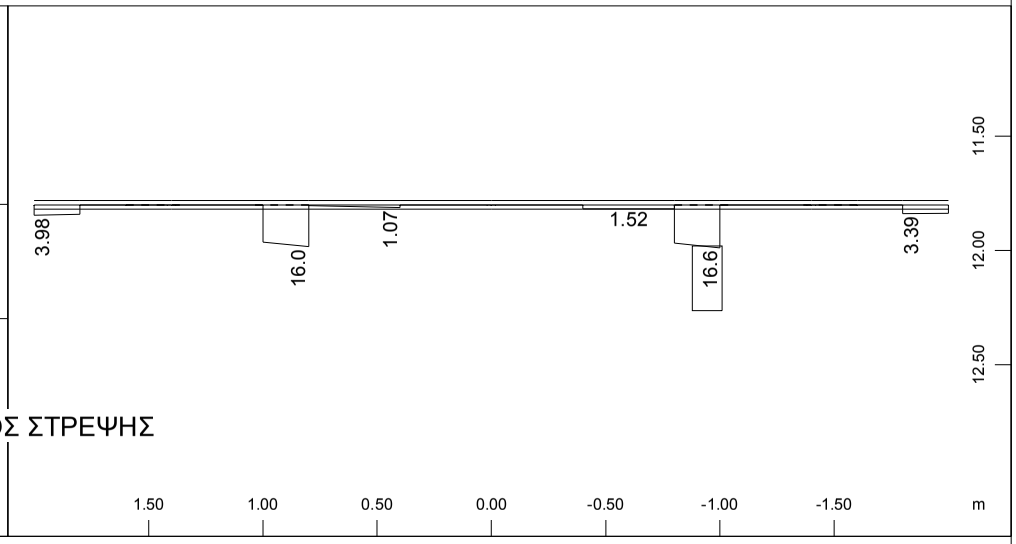


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Shear reinforcements (maximum), Design Case 506
, 1 cm 3D = 14.5 cm2/m (Max=5.82)

ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ

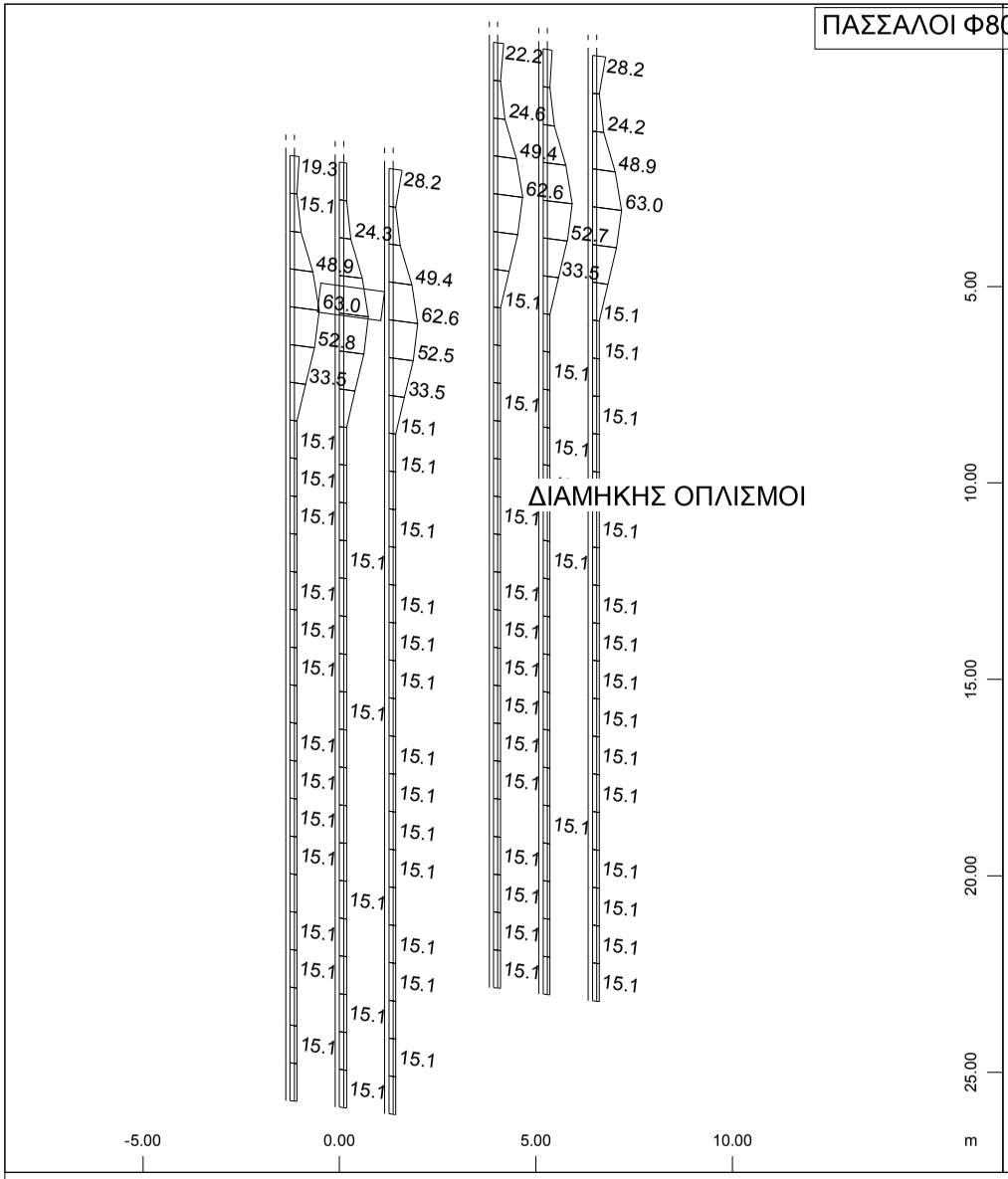


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
506 , 1 cm 3D = 29.1 cm2 (Max=16.8)



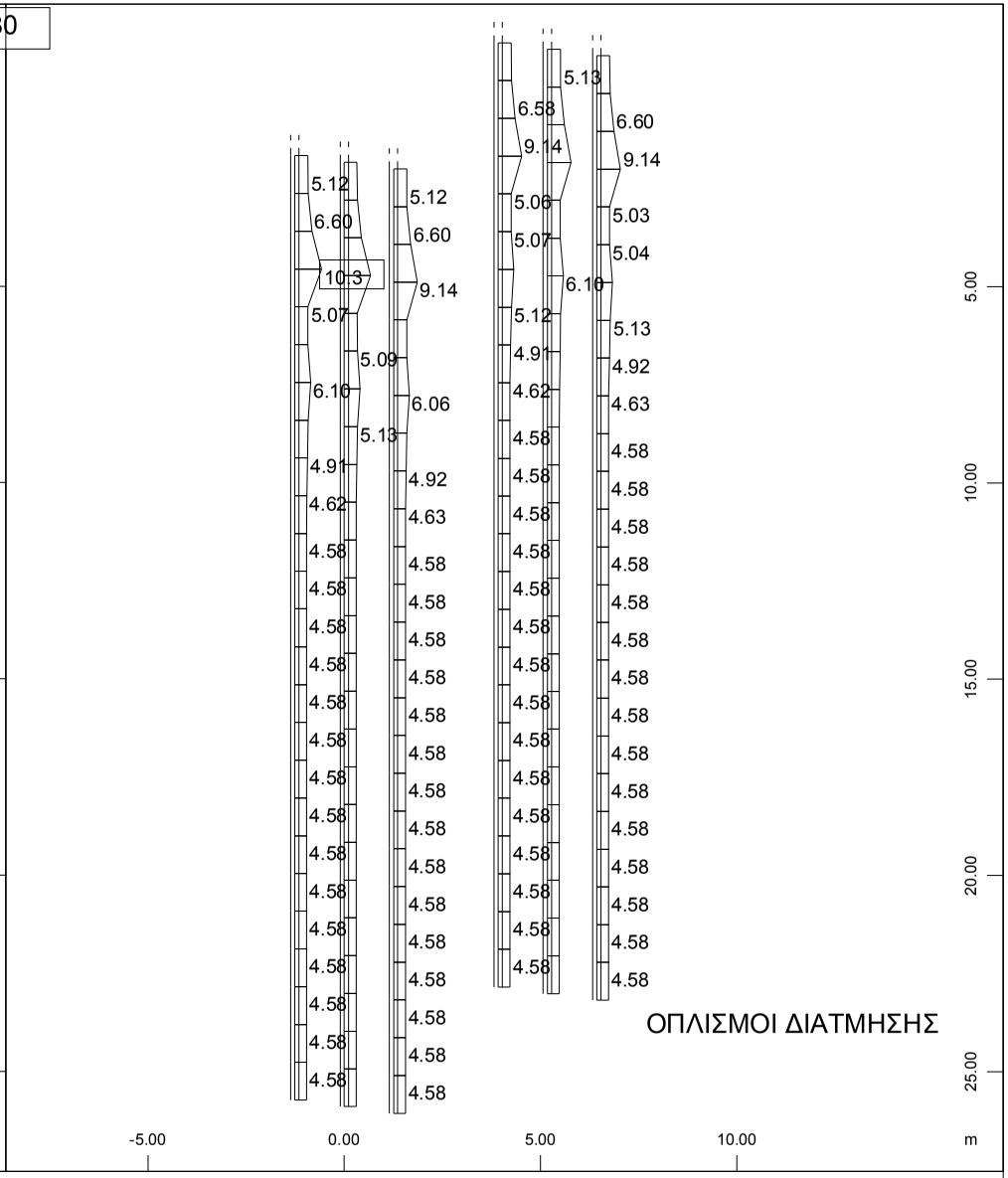
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
506 , 1 cm 3D = 29.1 cm2 (Max=16.6)

ΟΠΛΙΣΜΟΣ ΣΤΡΕΨΗΣ



Sector of system Beam Elements Group 12
Beam Elements , Longitudinal Reinforcements Lay. 1, Design
Case 506 , 1 cm 3D = 145.3 cm2 (Max=63.0)

M 1 : 191
X * 0.502
Y * 0.906
Z * 0.962

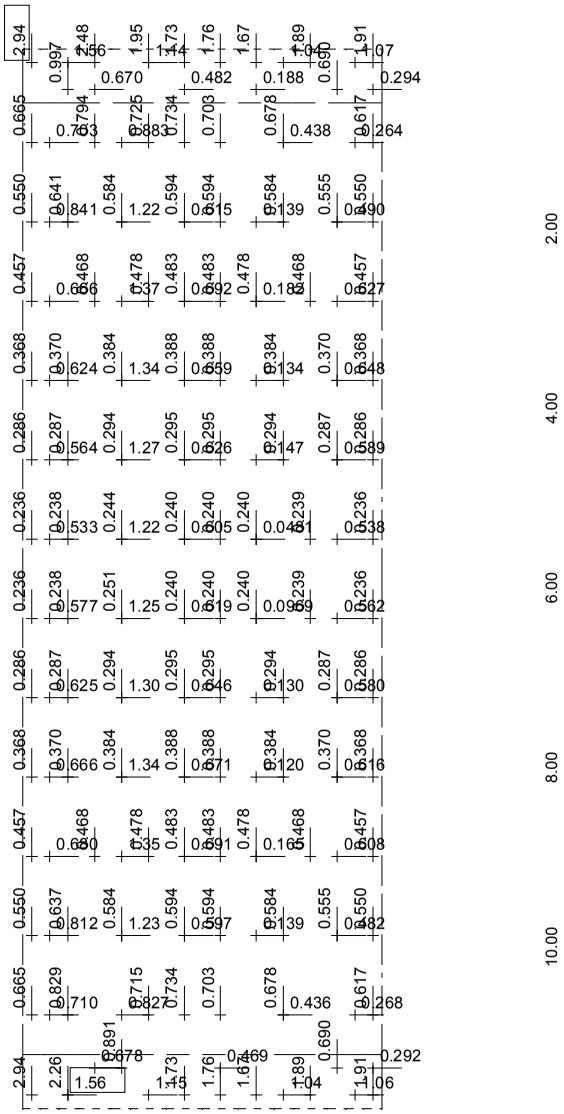


Sector of system Beam Elements Group 12
Beam Elements , Shear reinforcements (maximum), Design Case
506 , 1 cm 3D = 29.1 cm2/m (Max=10.3)

M 1 : 191
X * 0.502
Y * 0.906
Z * 0.962

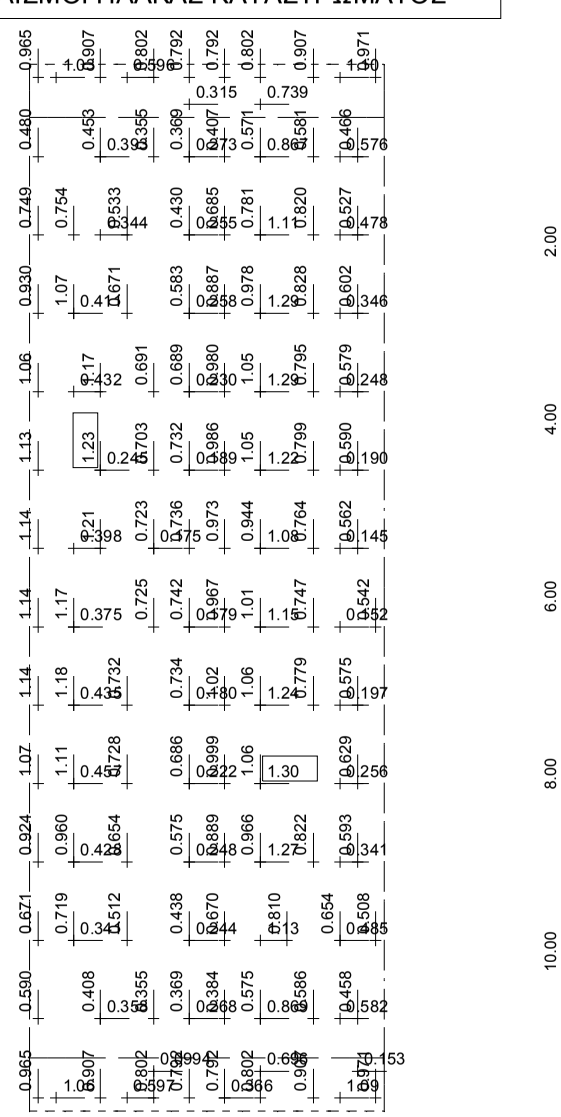
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΦΑΣΗ-2_ULS

ΟΠΛΙΣΜΟΙ ΠΛΑΚΑΣ ΚΑΤΑΣΤΡΩΜΑΤΟΣ



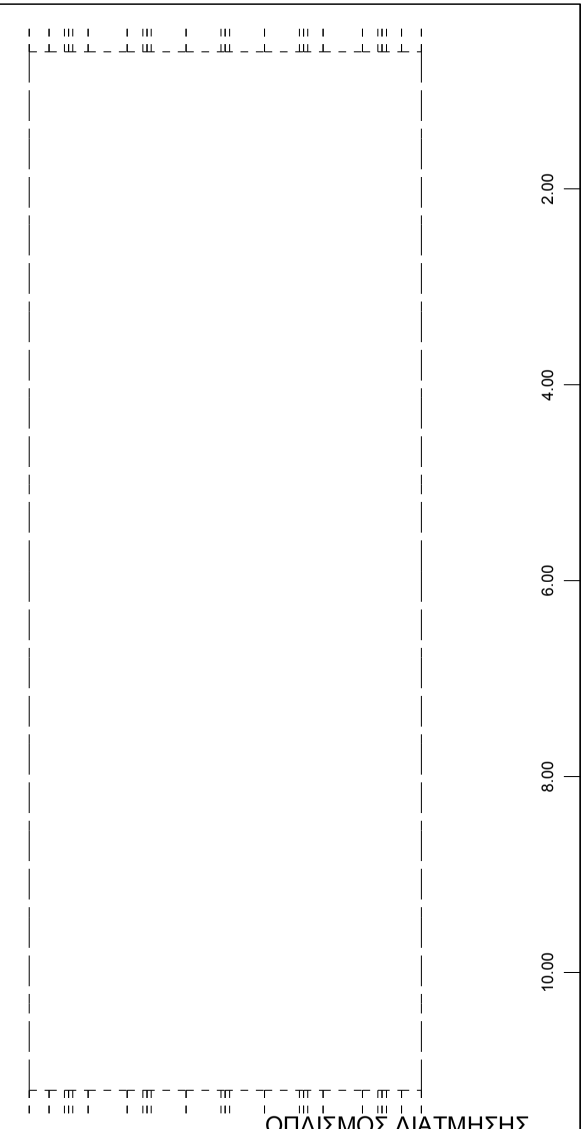
Sector of system Quadrilateral Elements Group 3 4
upper Reinforcements in Elements in cm2/m, Design Case
521 ULS design (Max=2.94)

M 1 : 84



Sector of system Quadrilateral Elements Group 3 4
lower Reinforcements in Elements in cm2/m, Design Case
521 ULS design (Max=1.30)

M 1 : 85



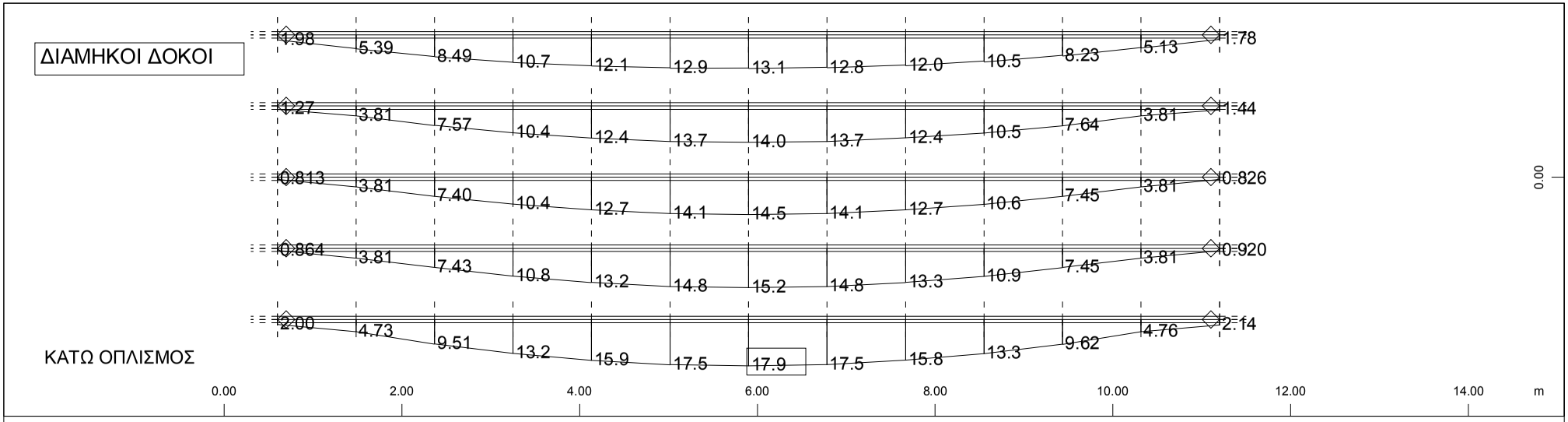
Sector of system Quadrilateral Elements Group 3
Shear reinforcement from middle of element in cm2/m2,
Design Case 521 ULS design (Max=0)

M 1 : 77

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΦΑΣΗ-2_ULS

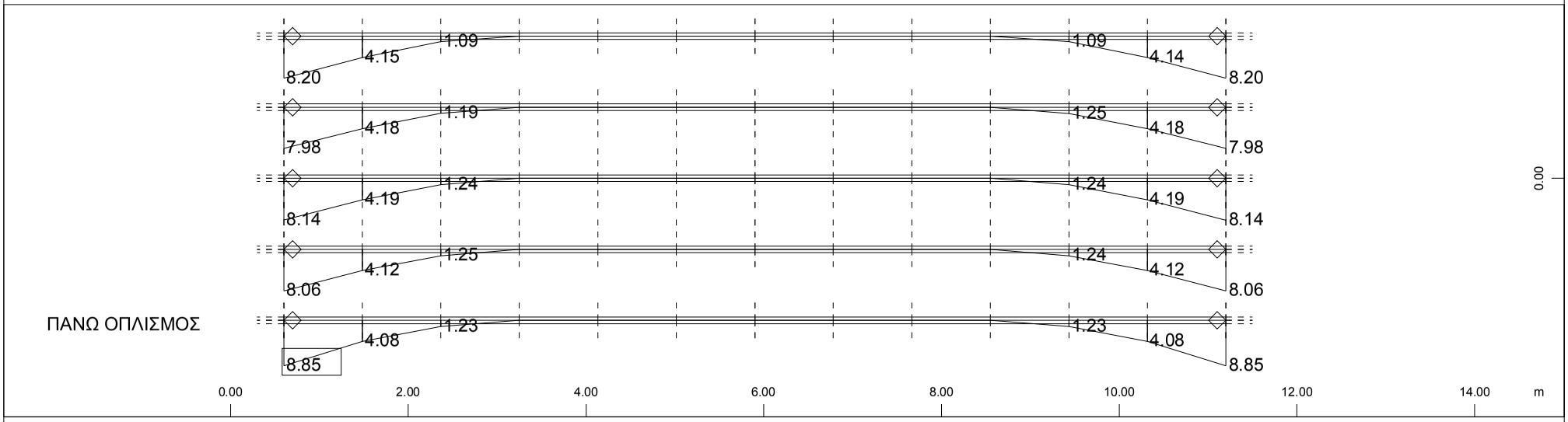
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00

8) ΦΑΣΗ-2_ ΕΛΕΓΧΟΣ ΦΟΡΕΑ ΣΕ ULS-ΠΡΟΣΚΡΟΥΣΗΣ



Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case 516 , 1 cm 3D = 22.4 cm2 (Max=17.9)

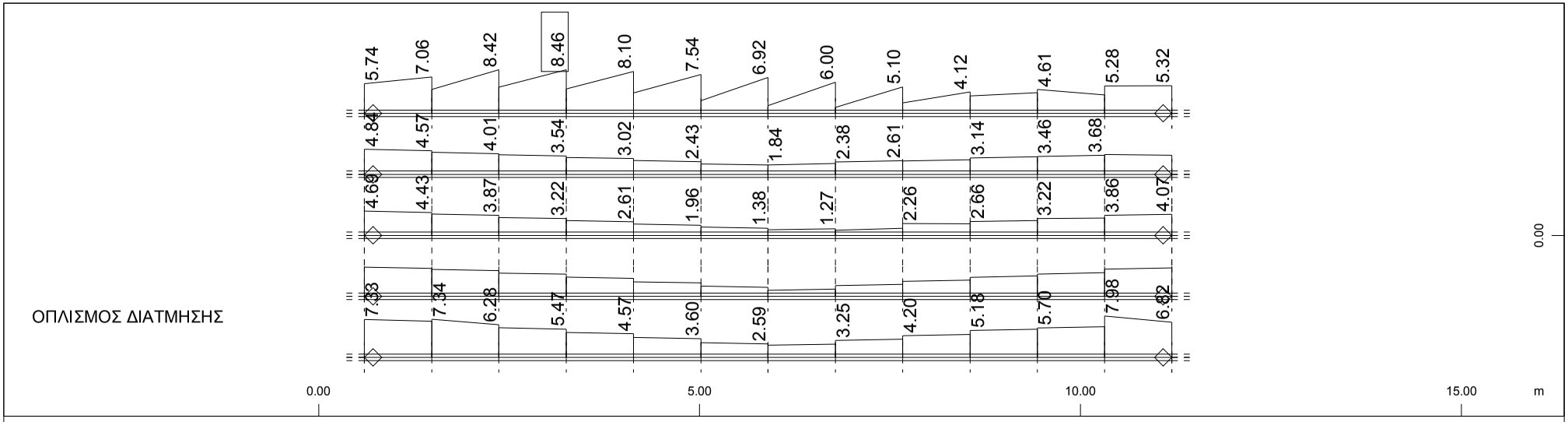
M 1 : 66



Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case 516 , 1 cm 3D = 11.2 cm2 (Max=8.85)

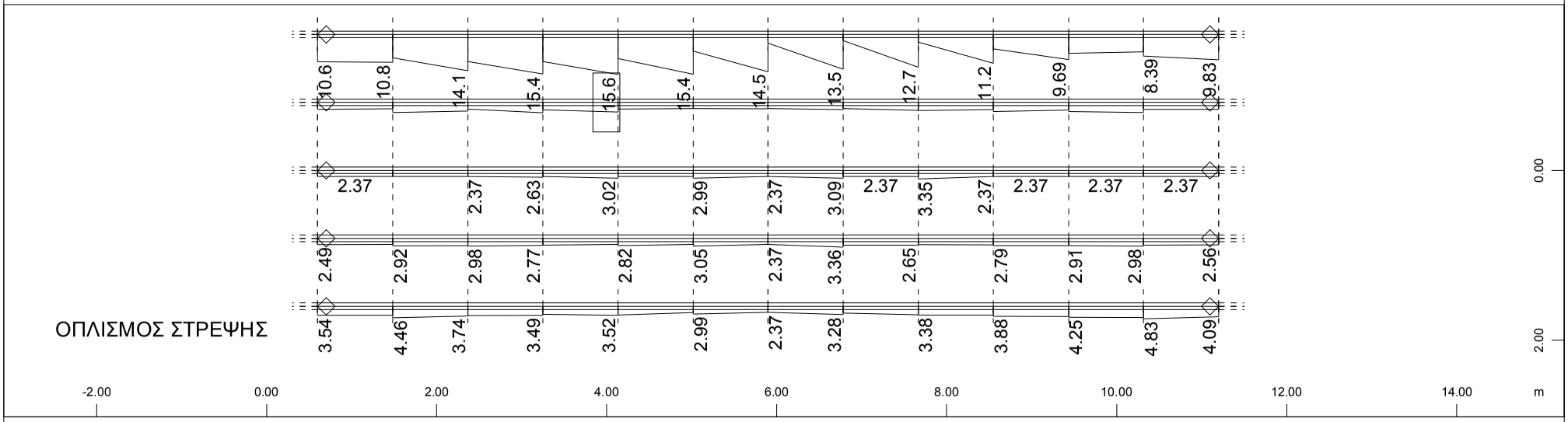
M 1 : 66

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΦΑΣΗ-2_CRASH



Sector of system Beam Elements Group 1
Beam Elements , Shear reinforcements (maximum), Design Case 516 , 1 cm 3D = 11.2 cm2/m (Max=8.46)

M 1 : 77

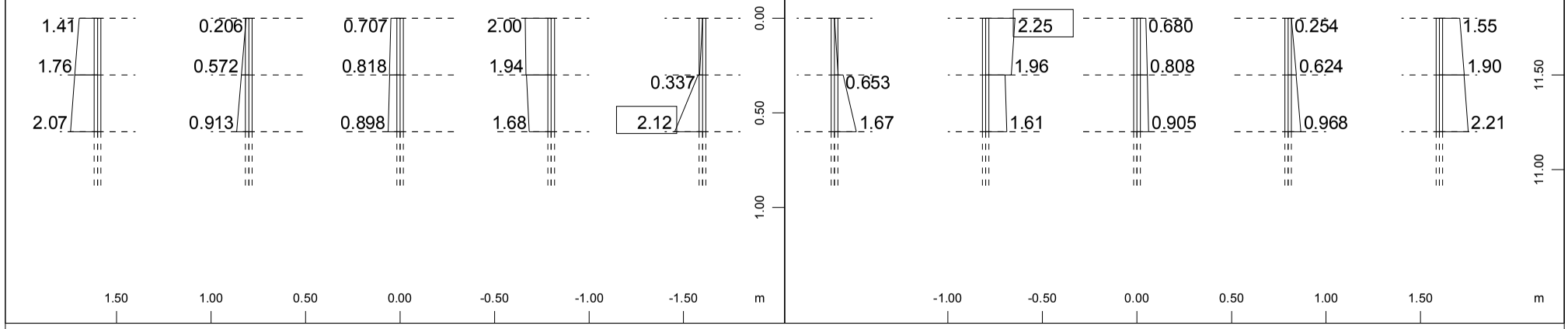


Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case 516 , 1 cm 3D = 22.4 cm2 (Max=15.6)

M 1 : 69

ΔΙΑΜΗΚΗ ΔΟΚΟΙ-ΣΤΗΡΙΞΗΣ

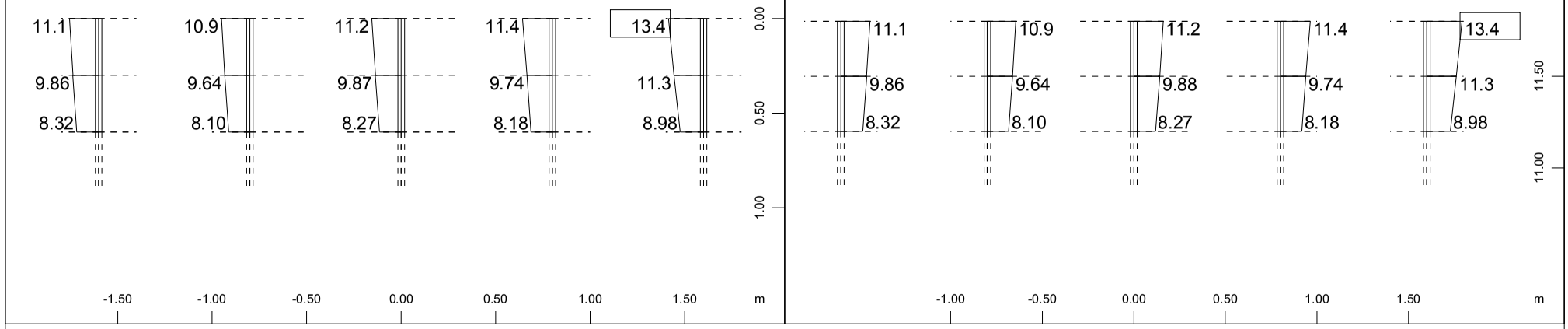
ΚΑΤΩ ΟΠΛΙΣΜΟΣ



Y—Z X
Sector of system Beam Elements Group 2 M 1 : 31
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
516 , 1 cm 3D = 4.48 cm2 (Max=2.12)

X Z—Y
Sector of system Beam Elements Group 2 M 1 : 31
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
516 , 1 cm 3D = 4.48 cm2 (Max=2.25)

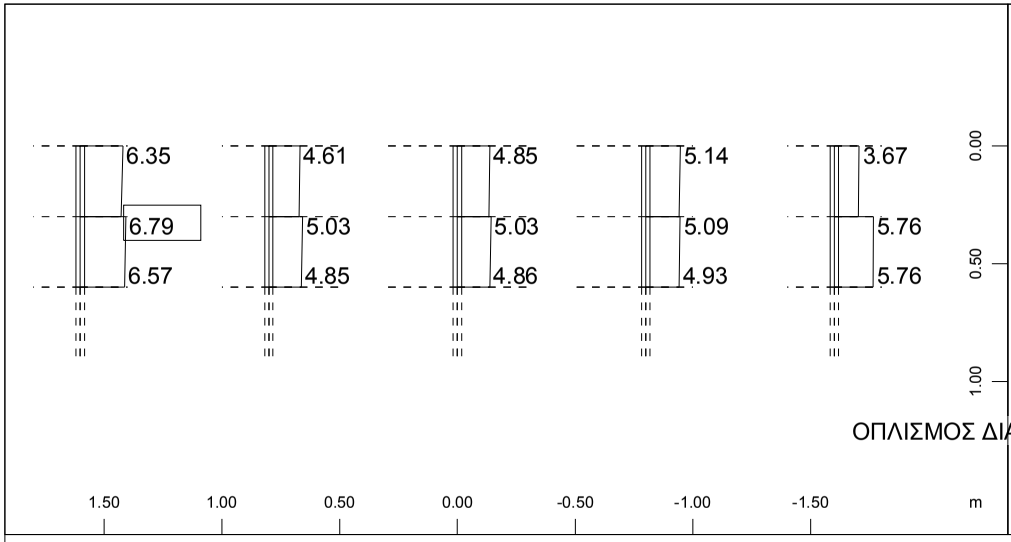
ΠΑΝΩ ΟΠΛΙΣΜΟΣ



Z—Y X
Sector of system Beam Elements Group 2 M 1 : 31
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
516 , 1 cm 3D = 22.4 cm2 (Max=13.4)

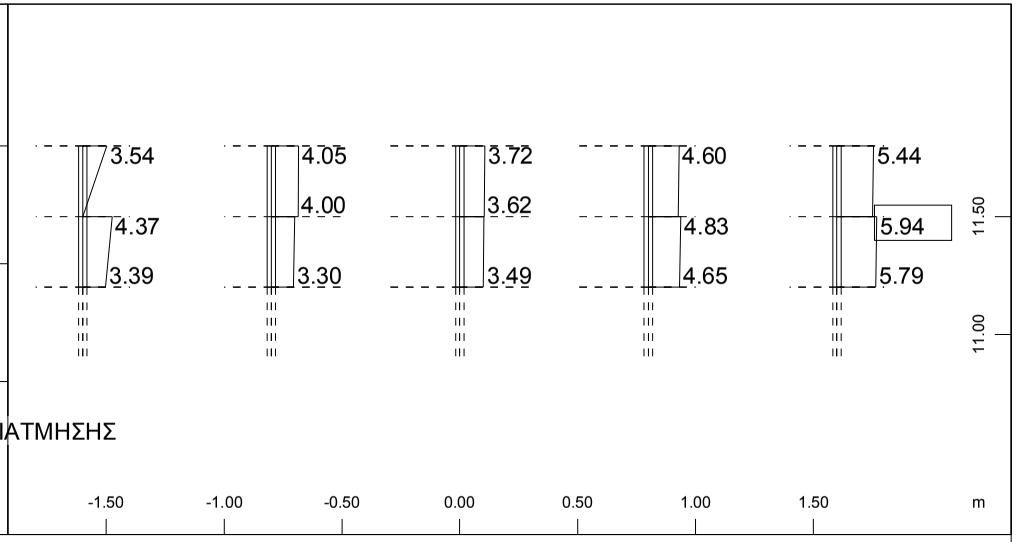
X Z—Y
Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
516 , 1 cm 3D = 22.4 cm2 (Max=13.4)

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΦΑΣΗ-2_CRASH

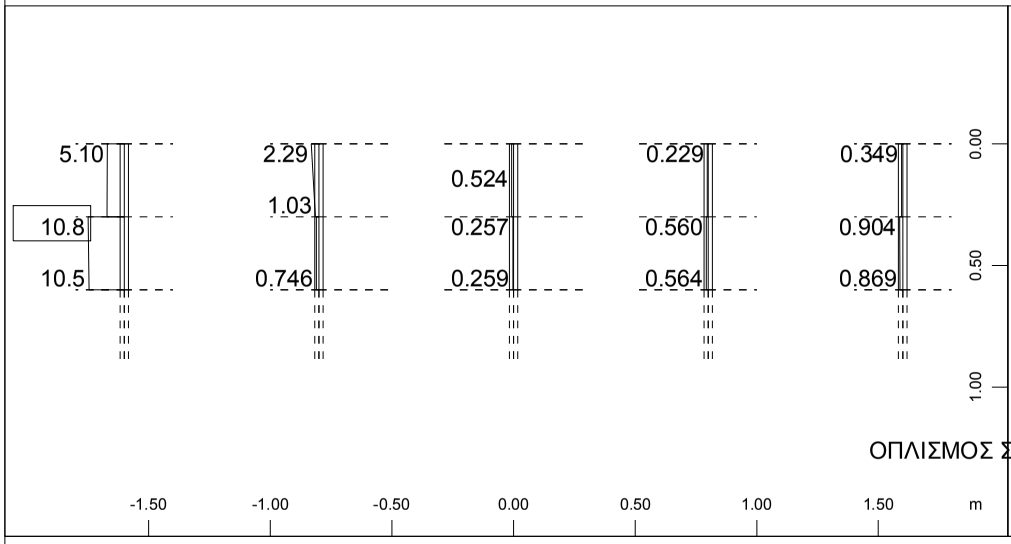


ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ

Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Shear reinforcements (maximum), Design Case 516
, 1 cm 3D = 11.2 cm2/m (Max=6.79)

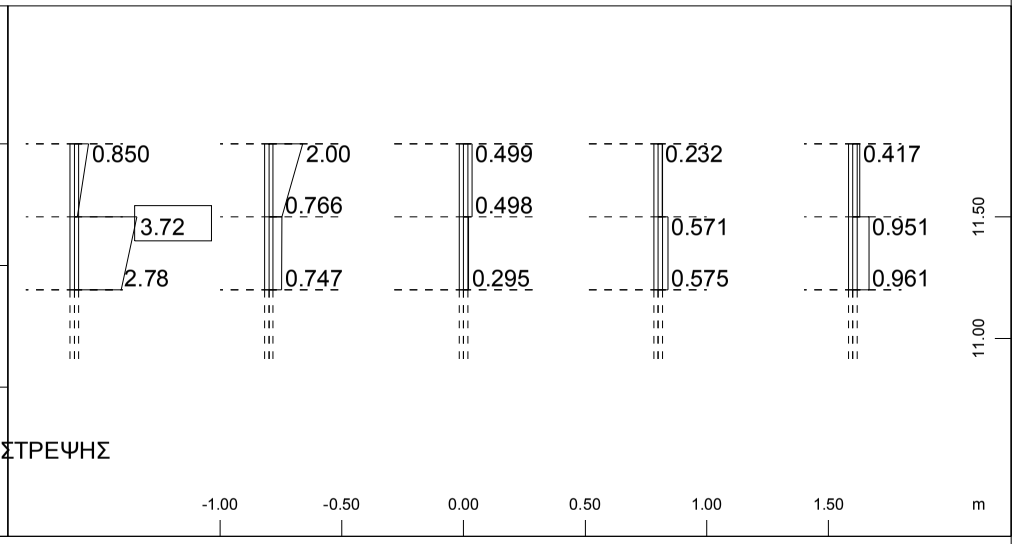


Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Shear reinforcements (maximum), Design Case 516
, 1 cm 3D = 11.2 cm2/m (Max=5.94)



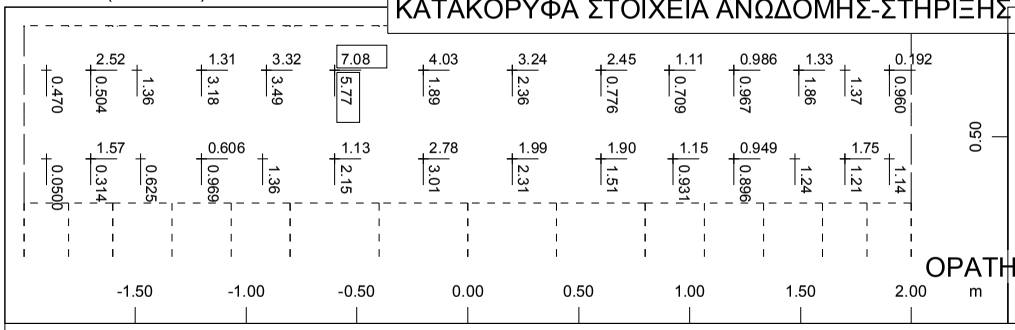
ΟΠΛΙΣΜΟΣ ΣΤΡΕΨΗΣ

Sector of system Beam Elements Group 2 M 1 : 31
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
516 , 1 cm 3D = 22.4 cm2 (Max=10.8)



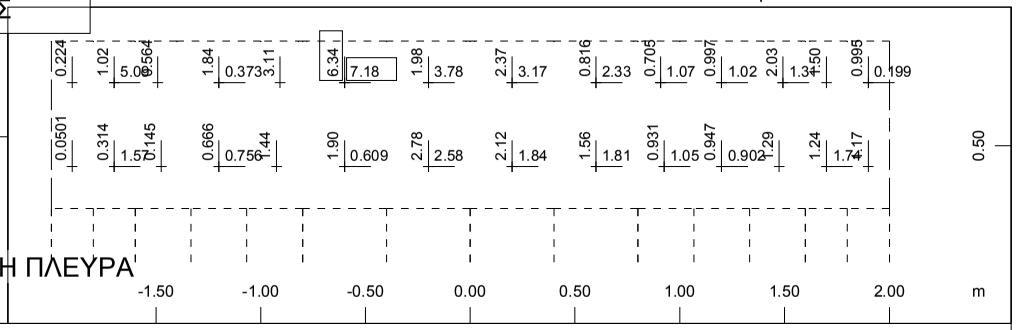
Sector of system Beam Elements Group 2 M 1 : 31
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
516 , 1 cm 3D = 4.48 cm2 (Max=3.72)

ΚΑΤΑΚΟΡΥΦΑ ΣΤΟΙΧΕΙΑ ΑΝΩΔΟΜΗΣ-ΣΤΗΡΙΞΗΣ

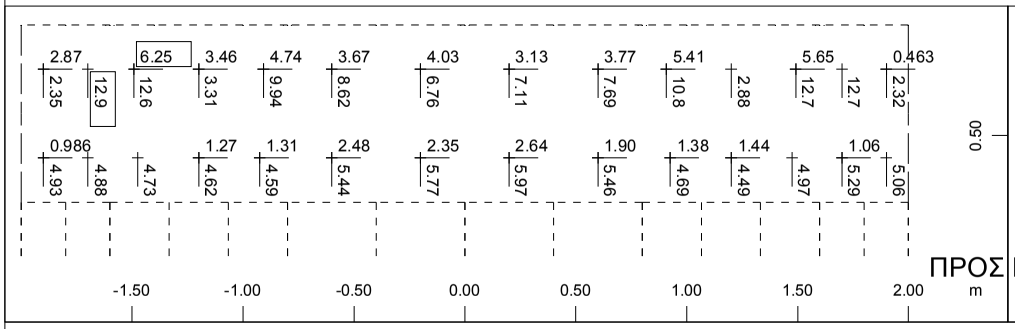


Sector of system Quadrilateral Elements Group 8 M 1 : 34
upper Reinforcements in Elements in cm2/m, Design Case 524 ULS
design (Max=7.08)

ΟΡΑΤΗ ΠΛΕΥΡΑ

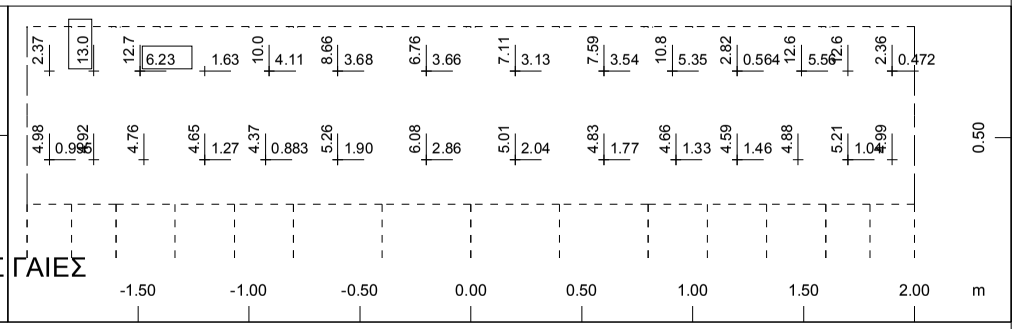


Sector of system Quadrilateral Elements Group 8 M 1 : 36
upper Reinforcements in Elements in cm2/m, Design Case 524 ULS
design (Max=7.18)

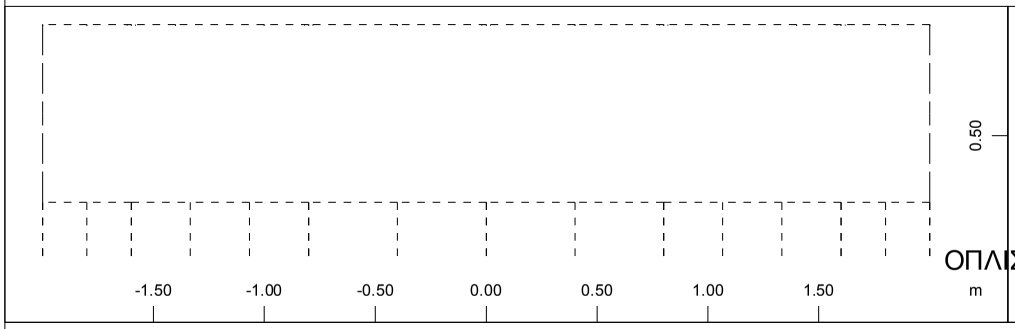


Sector of system Quadrilateral Elements Group 8 M 1 : 34
lower Reinforcements in Elements in cm2/m, Design Case 524 ULS
design (Max=12.9)

ΠΡΟΣ ΓΑΙΕΣ

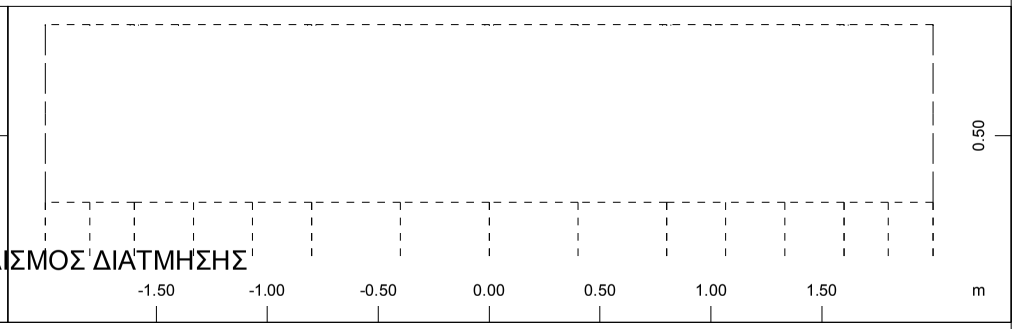


Sector of system Quadrilateral Elements Group 8 M 1 : 34
lower Reinforcements in Elements in cm2/m, Design Case 524 ULS
design (Max=13.0)



Sector of system Quadrilateral Elements Group 8 M 1 : 34
Shear reinforcement from middle of element in cm2/m2, Design
Case 524 ULS design (Max=0)

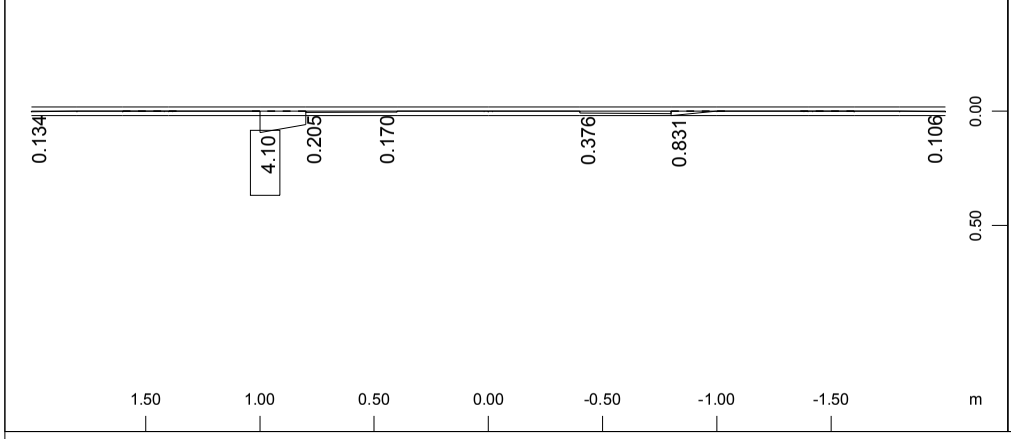
ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ



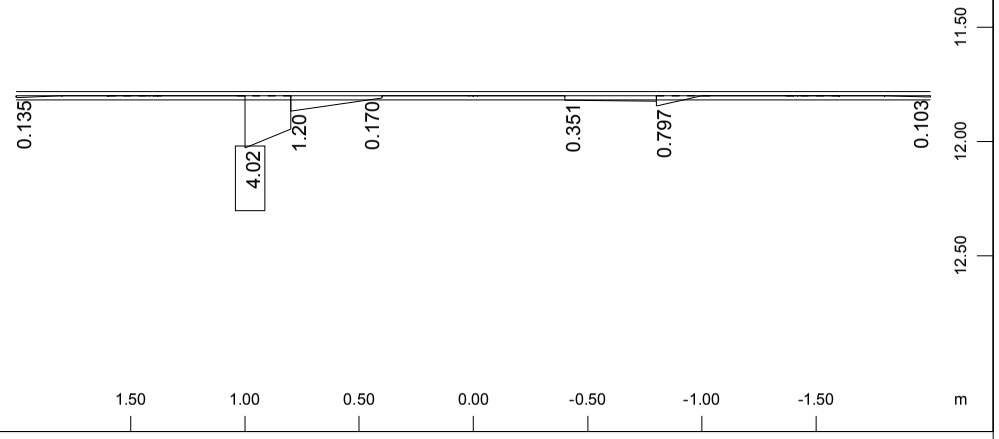
Sector of system Quadrilateral Elements Group 8 M 1 : 34
Shear reinforcement from middle of element in cm2/m2, Design
Case 524 ULS design (Max=0)

ΠΑΣΣΑΛΟΔΕΣΜΟΣ

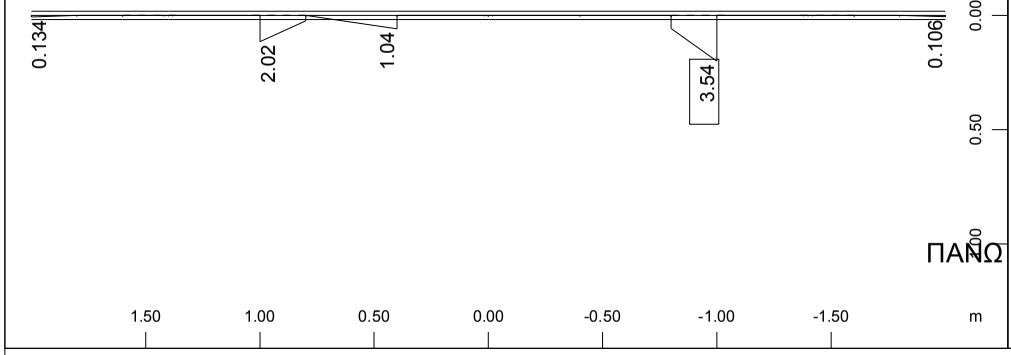
ΚΑΤΩ ΟΠΛΙΣΜΟΣ



Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
515 , 1 cm 3D = 14.5 cm2 (Max=4.10)

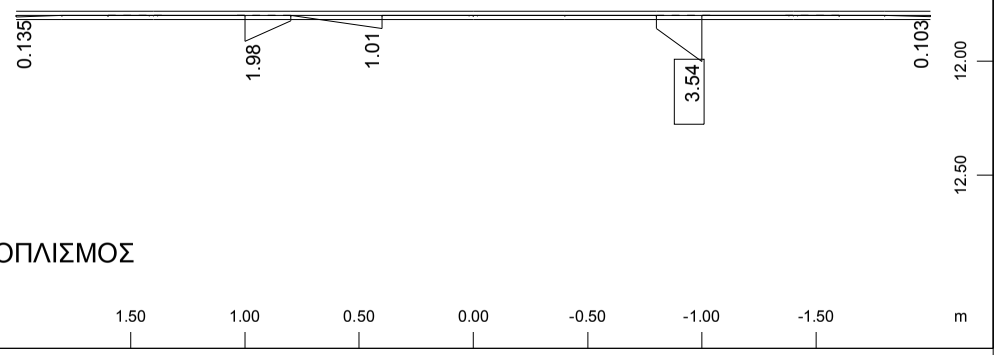


Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
515 , 1 cm 3D = 5.81 cm2 (Max=4.02)

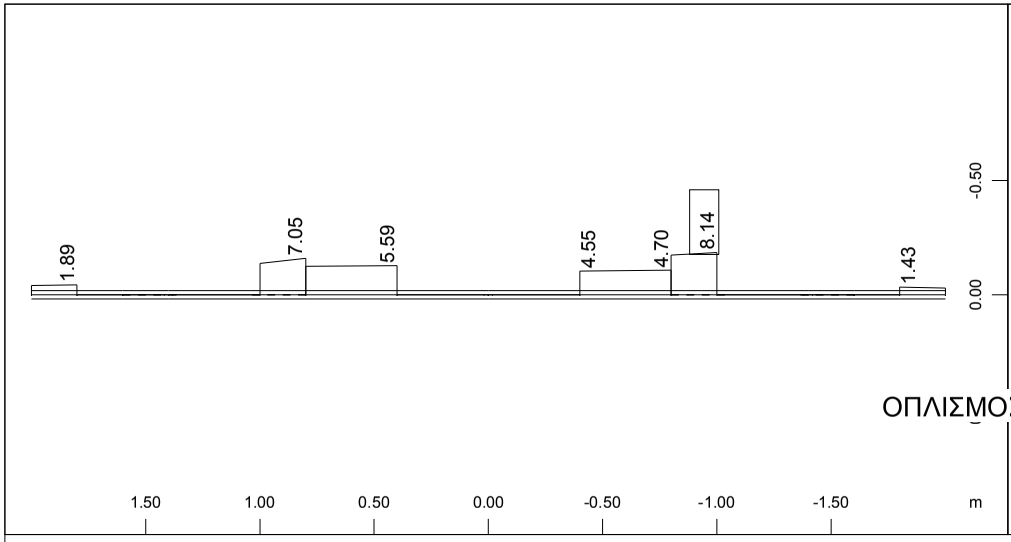


Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
515 , 1 cm 3D = 5.81 cm2 (Max=3.54)

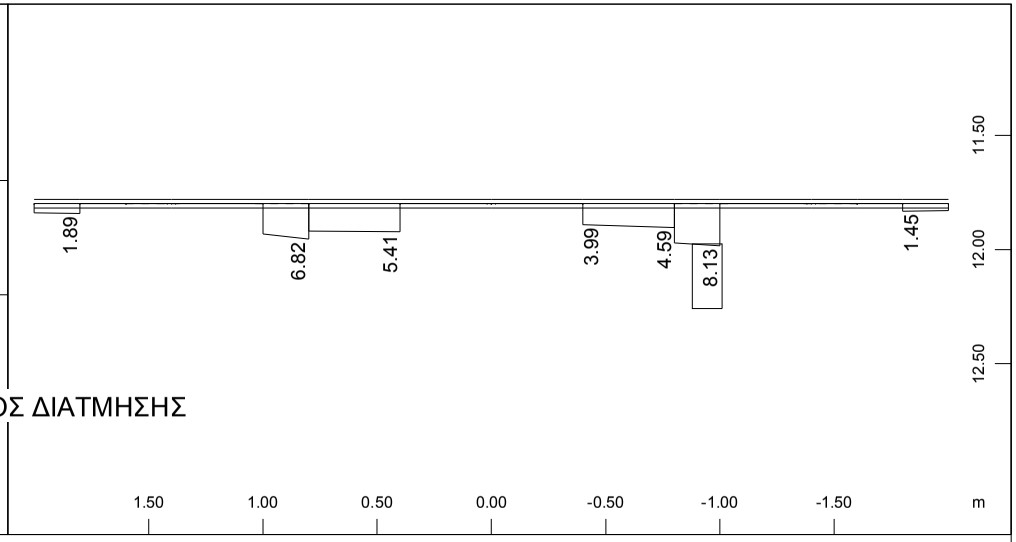
ΠΑΝΩ ΟΠΛΙΣΜΟΣ



Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
515 , 1 cm 3D = 5.81 cm2 (Max=3.54)

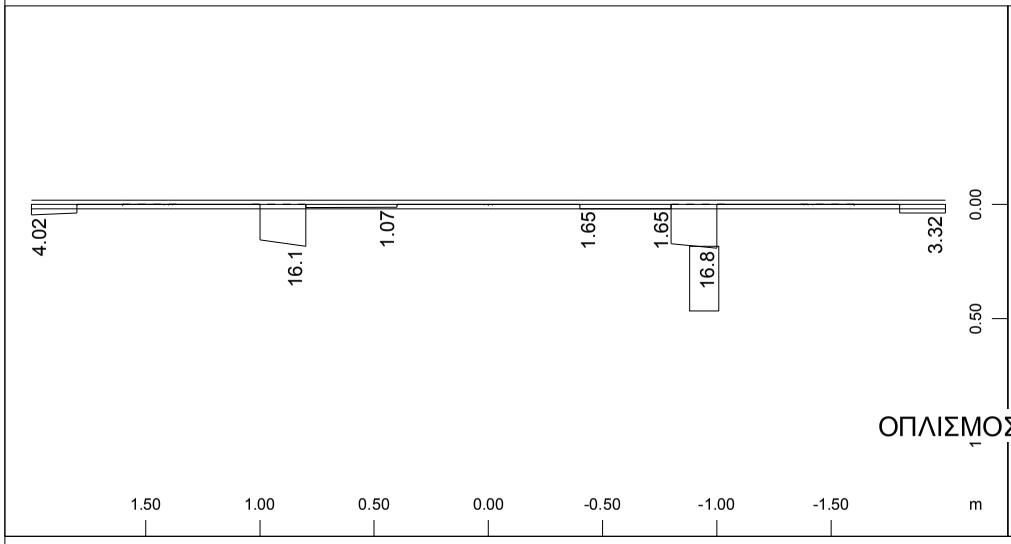


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Shear reinforcements (maximum), Design Case 515
, 1 cm 3D = 14.5 cm2/m (Max=8.14)

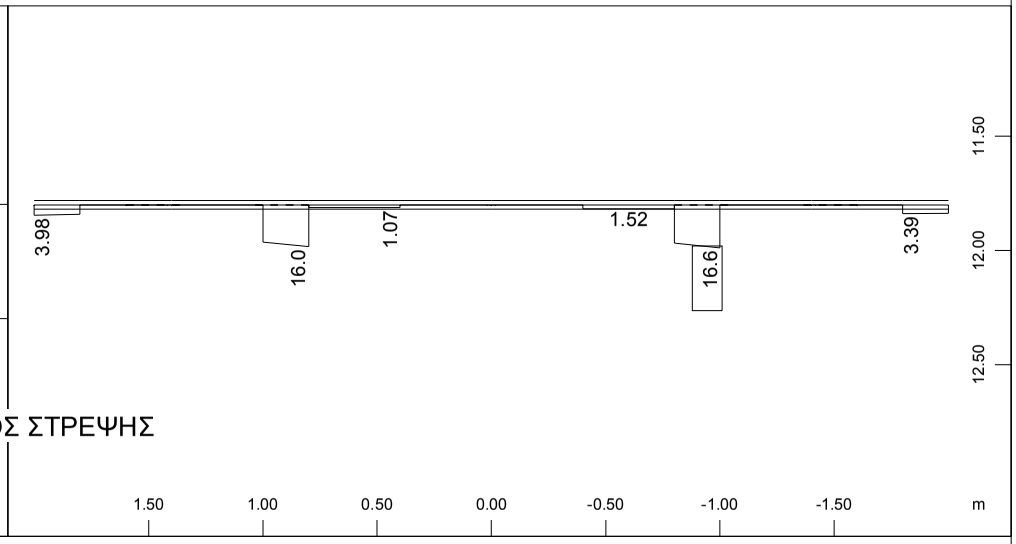


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Shear reinforcements (maximum), Design Case 515
, 1 cm 3D = 14.5 cm2/m (Max=8.13)

ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ

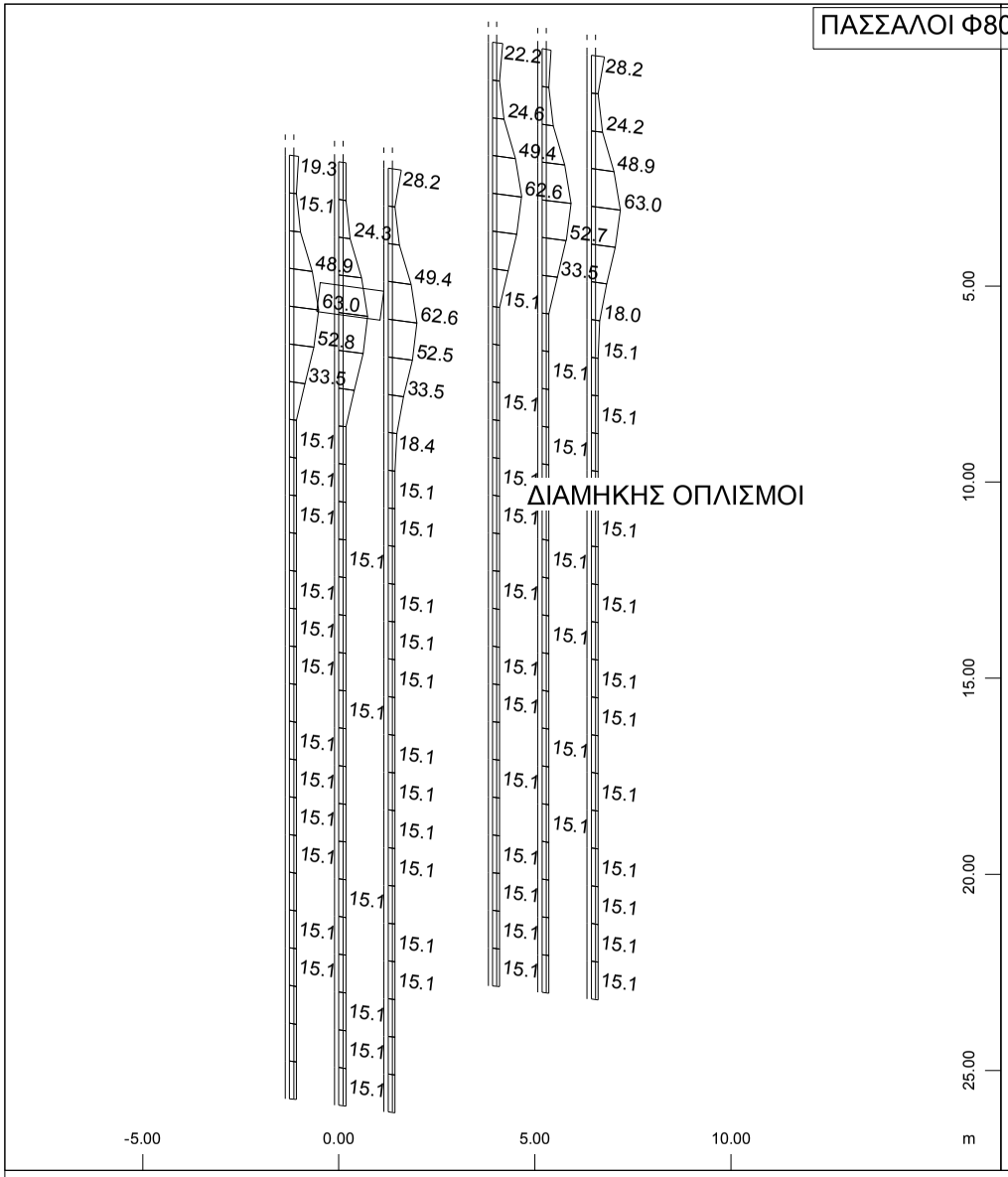


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
515 , 1 cm 3D = 29.1 cm2 (Max=16.8)



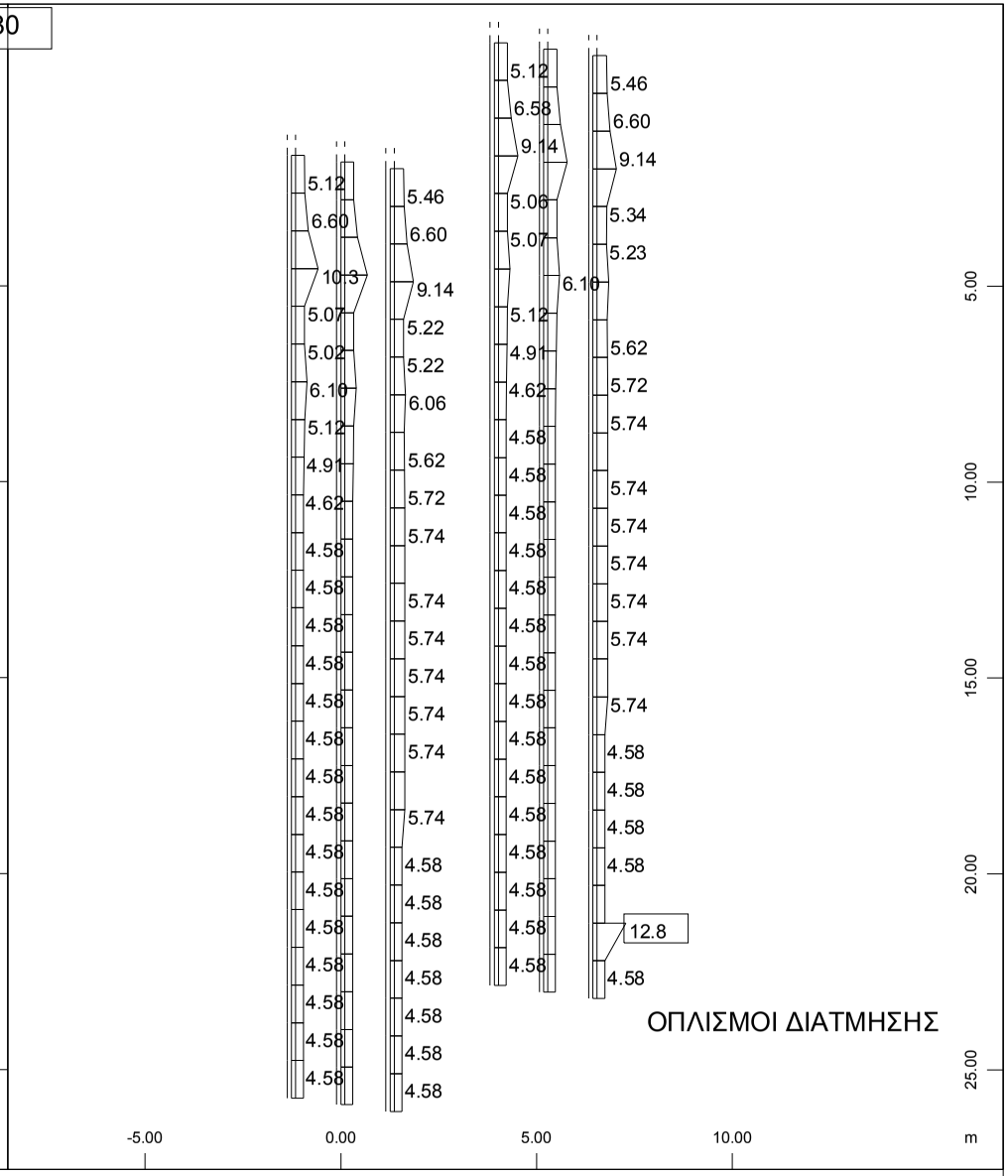
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
515 , 1 cm 3D = 29.1 cm2 (Max=16.6)

ΟΠΛΙΣΜΟΣ ΣΤΡΕΨΗΣ



Sector of system Beam Elements Group 12
Beam Elements , Longitudinal Reinforcements Lay. 1, Design
Case 515 , 1 cm 3D = 145.3 cm2 (Max=63.0)

M 1 : 191
X * 0.502
Y * 0.906
Z * 0.962

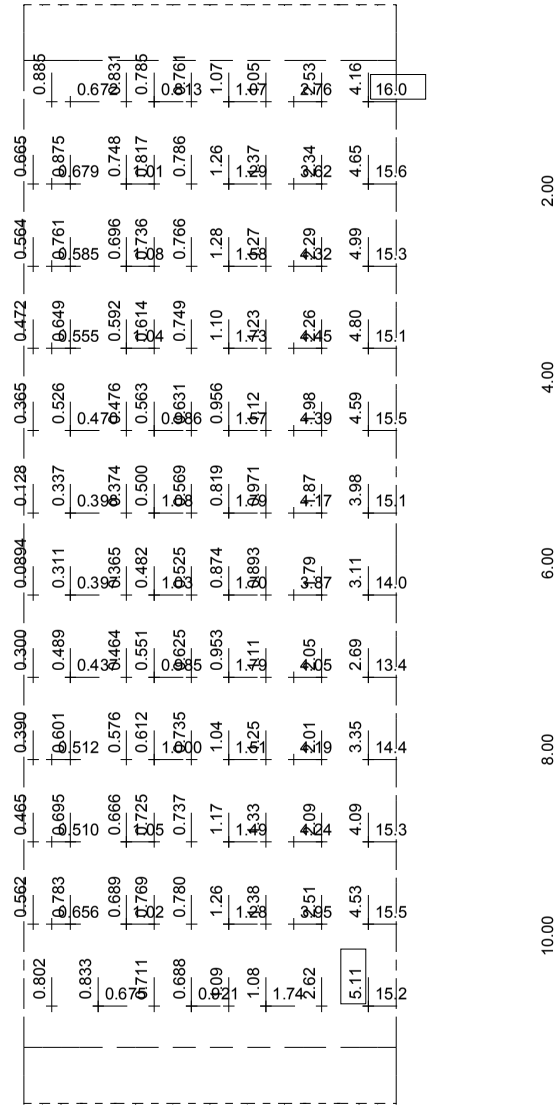


Sector of system Beam Elements Group 12
Beam Elements , Shear reinforcements (maximum), Design Case
515 , 1 cm 3D = 29.1 cm2/m (Max=12.8)

M 1 : 191
X * 0.502
Y * 0.906
Z * 0.962

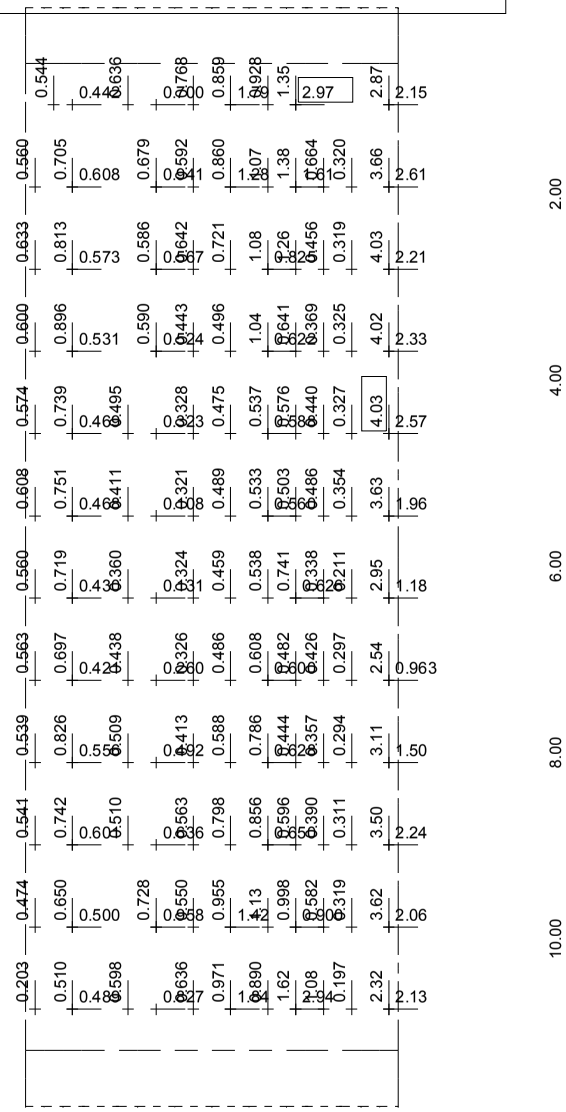
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΦΑΣΗ-2_CRASH

ΟΠΛΙΣΜΟΙ ΠΛΑΚΑΣ ΚΑΤΑΣΤΡΩΜΑΤΟΣ



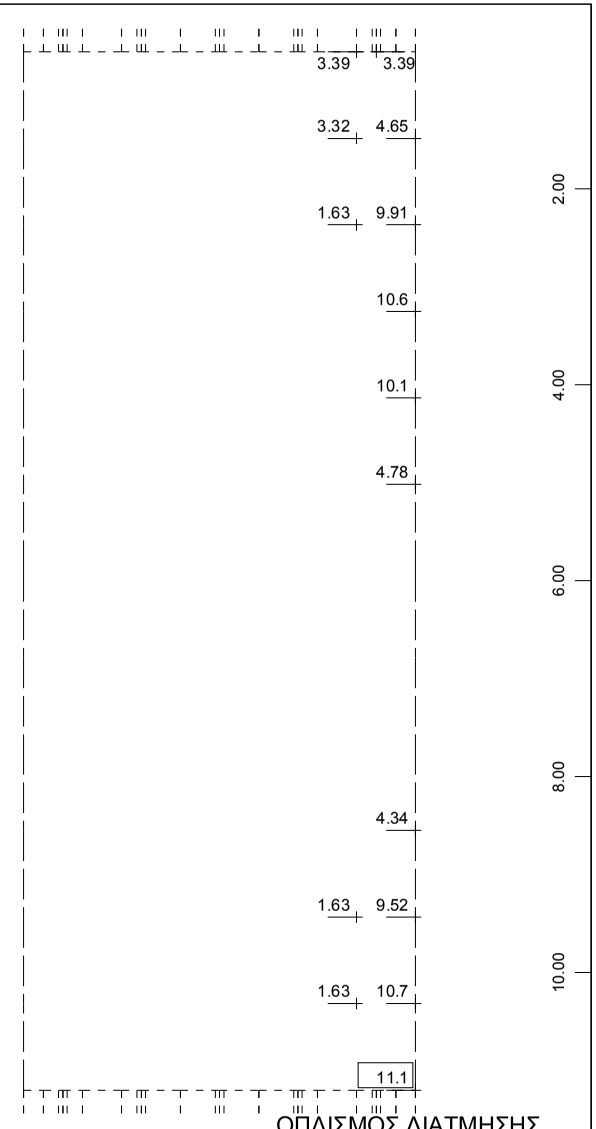
Sector of system Quadrilateral Elements Group 3 4
upper Reinforcements in Elements in cm2/m, Design Case
524 ULS design (Max=16.0)

M 1 : 81



Sector of system Quadrilateral Elements Group 3 4
lower Reinforcements in Elements in cm2/m, Design Case
524 ULS design (Max=4.03)

M 1 : 81

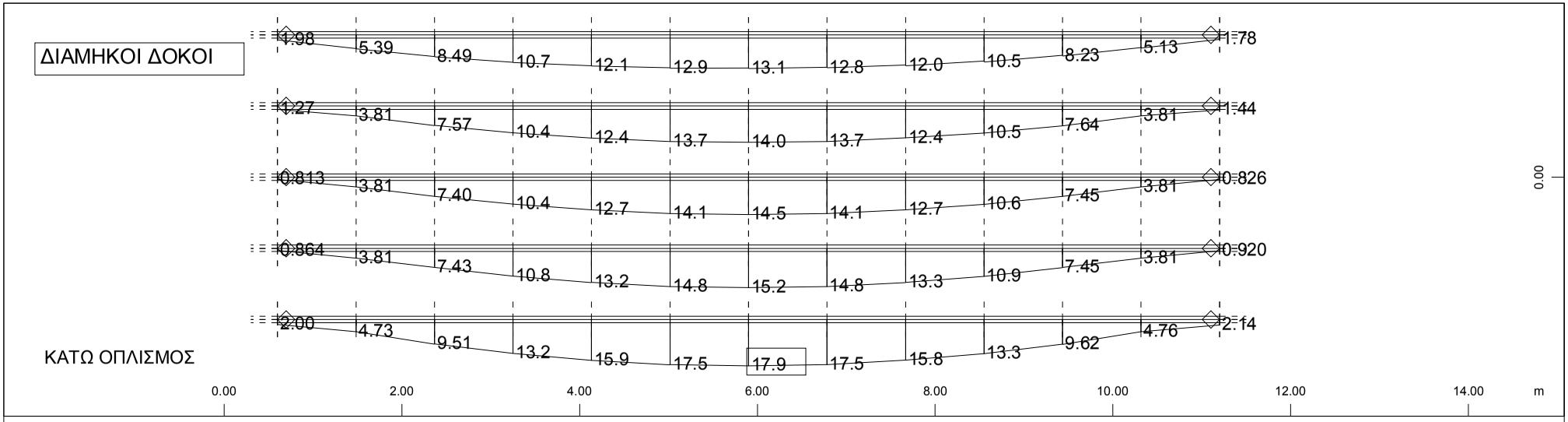


Sector of system Quadrilateral Elements Group 3
Shear reinforcement from middle of element in cm2/m2,
Design Case 524 ULS design (Max=11.1)

M 1 : 77

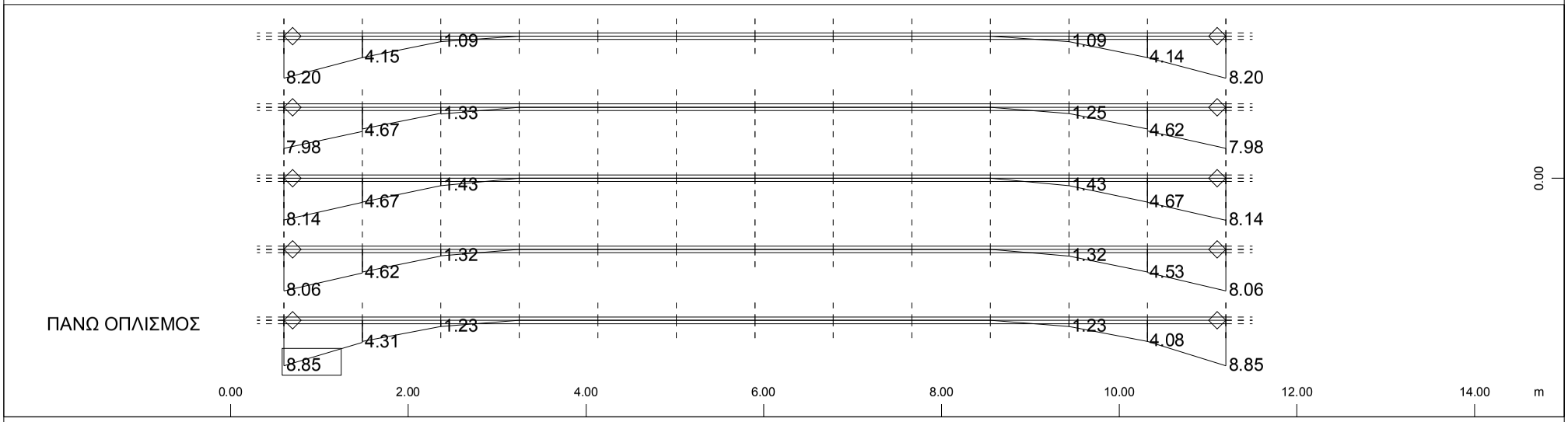
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00

9) ΦΑΣΗ-2_ ΕΛΕΓΧΟΣ ΦΟΡΕΑ ΣΕ SLS - ΡΗΓΜΑΤΩΣΗ



Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case 508 , 1 cm 3D = 22.4 cm2 (Max=17.9)

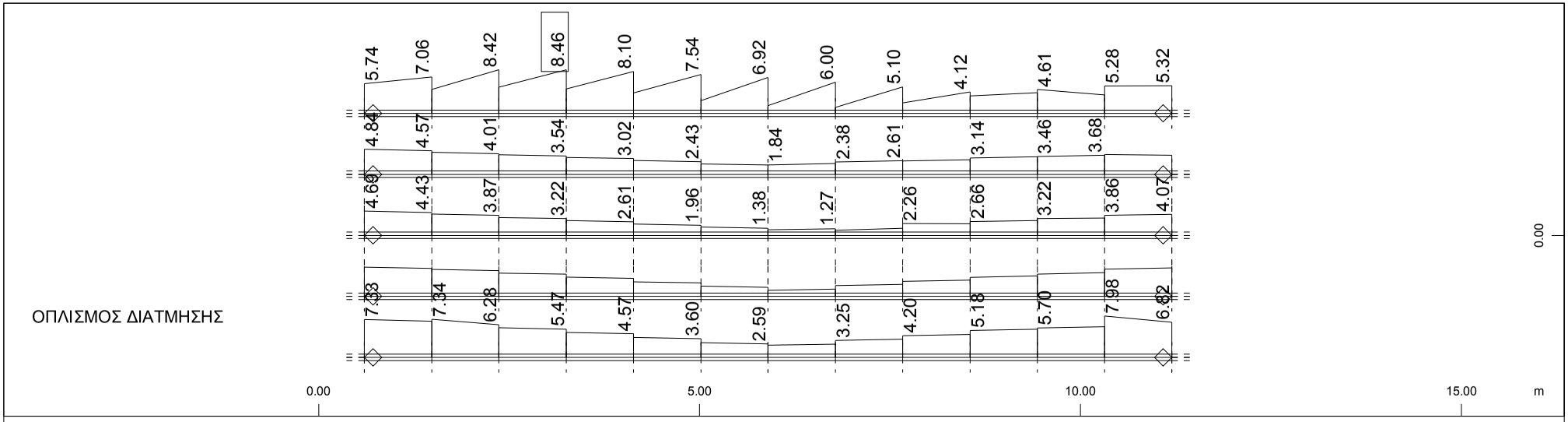
M 1 : 66



Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case 508 , 1 cm 3D = 11.2 cm2 (Max=8.85)

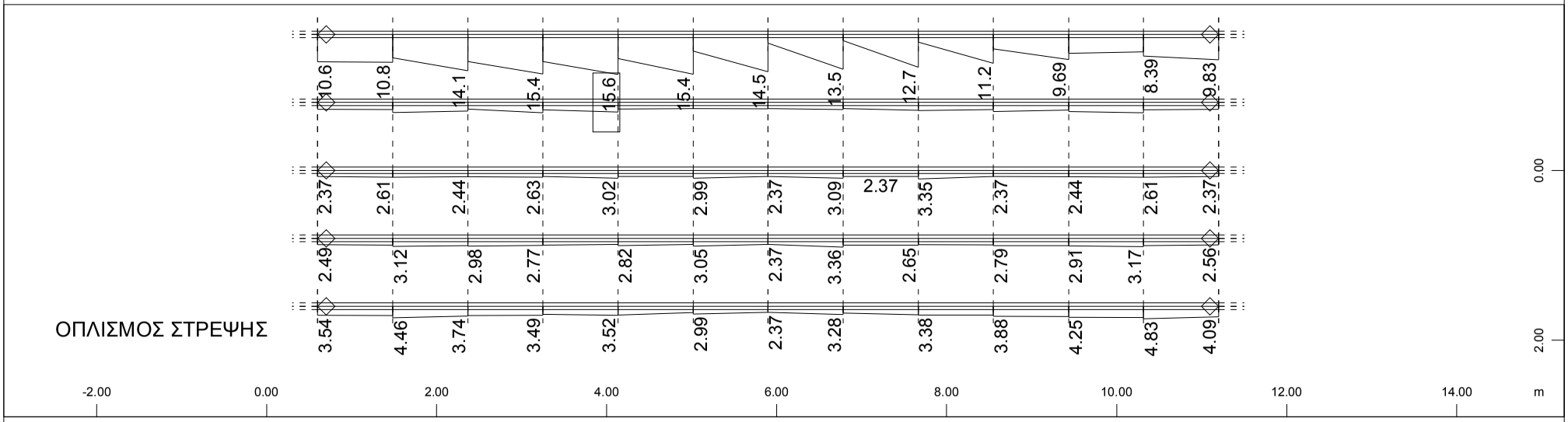
M 1 : 66

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΦΑΣΗ-2_SLS



Sector of system Beam Elements Group 1
Beam Elements , Shear reinforcements (maximum), Design Case 508 , 1 cm 3D = 11.2 cm2/m (Max=8.46)

M 1 : 77



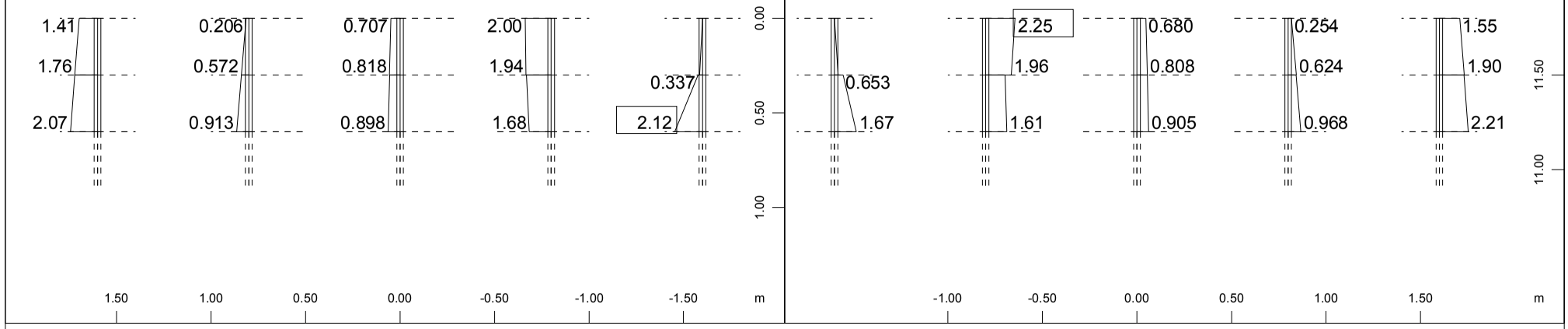
Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case 508 , 1 cm 3D = 22.4 cm2 (Max=15.6)

M 1 : 69

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΦΑΣΗ-2_SLS

ΔΙΑΜΗΚΗ ΔΟΚΟΙ-ΣΤΗΡΙΞΗΣ

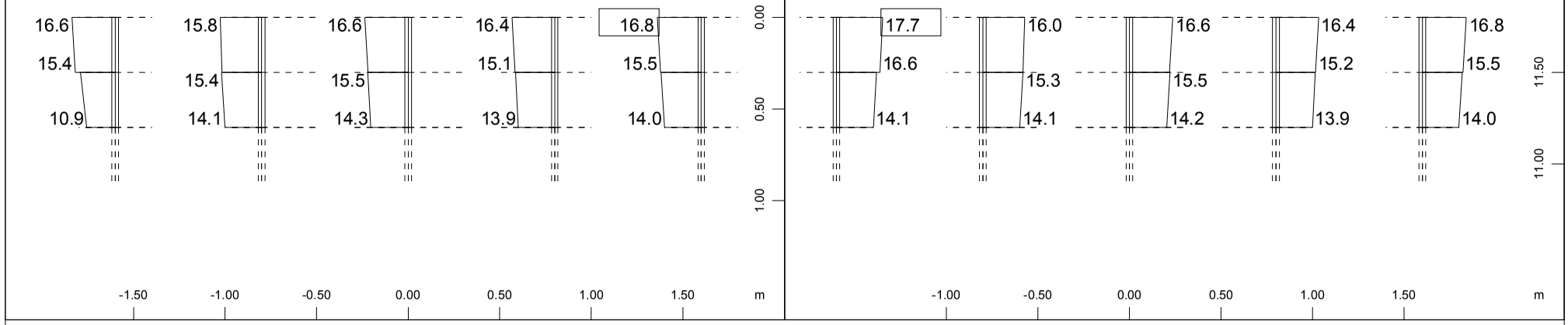
ΚΑΤΩ ΟΠΛΙΣΜΟΣ



Y—Z X
Sector of system Beam Elements Group 2
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
508 , 1 cm 3D = 4.48 cm2 (Max=2.12)
M 1 : 31

X Z—Y
Sector of system Beam Elements Group 2
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
508 , 1 cm 3D = 4.48 cm2 (Max=2.25)
M 1 : 31

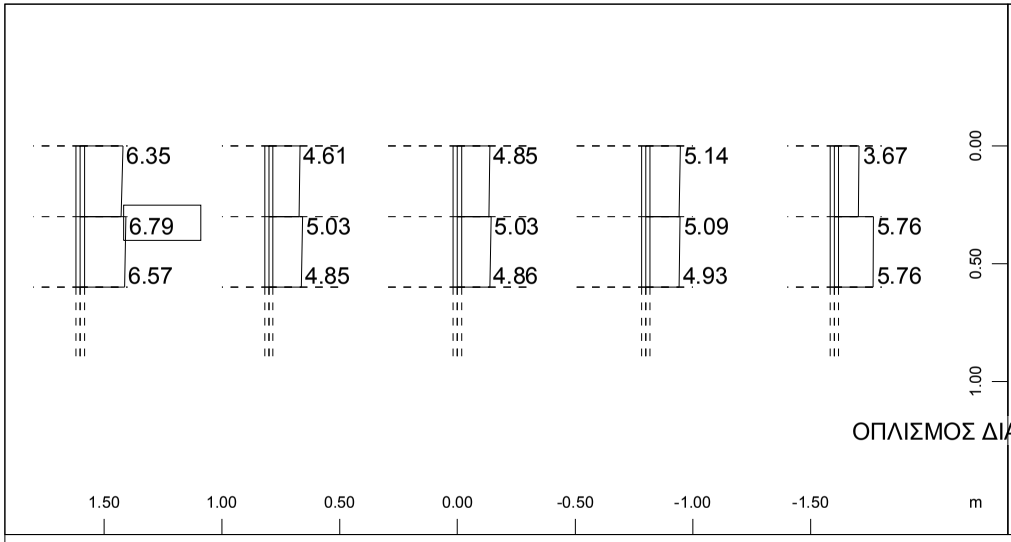
ΠΑΝΩ ΟΠΛΙΣΜΟΣ



Z—Y X
Sector of system Beam Elements Group 2
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
508 , 1 cm 3D = 22.4 cm2 (Max=16.8)
M 1 : 32

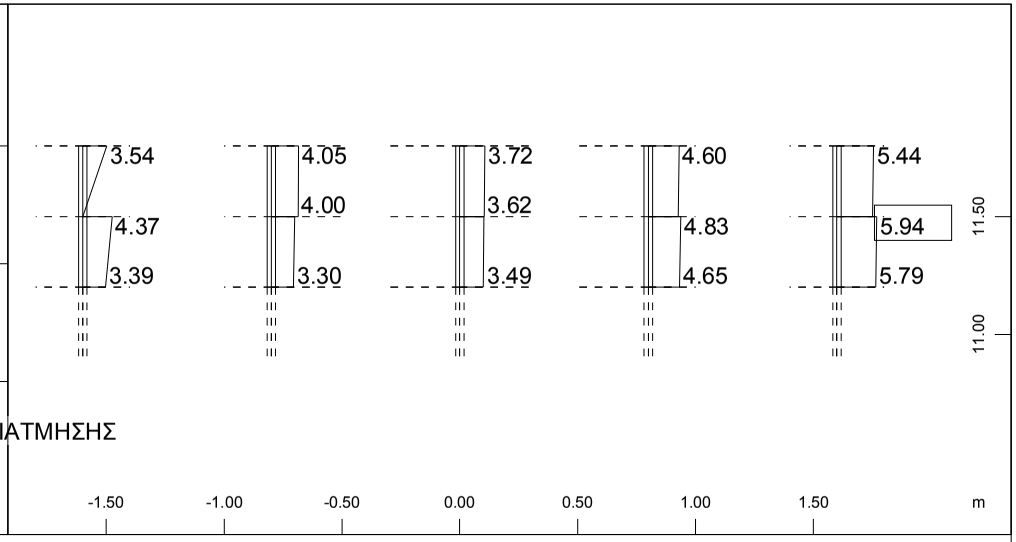
X Z—Y
Sector of system Beam Elements Group 2
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
508 , 1 cm 3D = 22.4 cm2 (Max=17.7)
M 1 : 32

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΦΑΣΗ-2_SLS

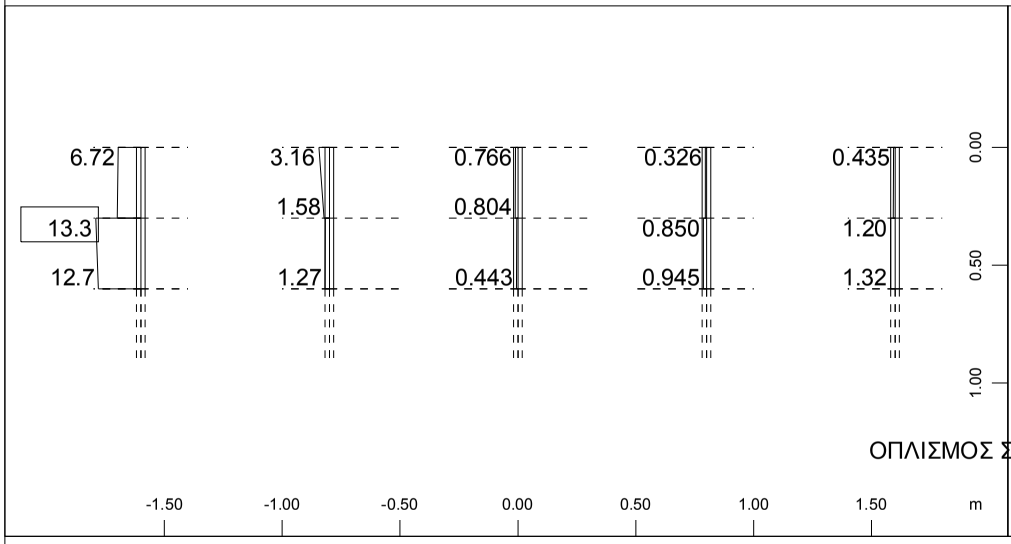


ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ

Y Z X Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Shear reinforcements (maximum), Design Case 508
, 1 cm 3D = 11.2 cm2/m (Max=6.79)

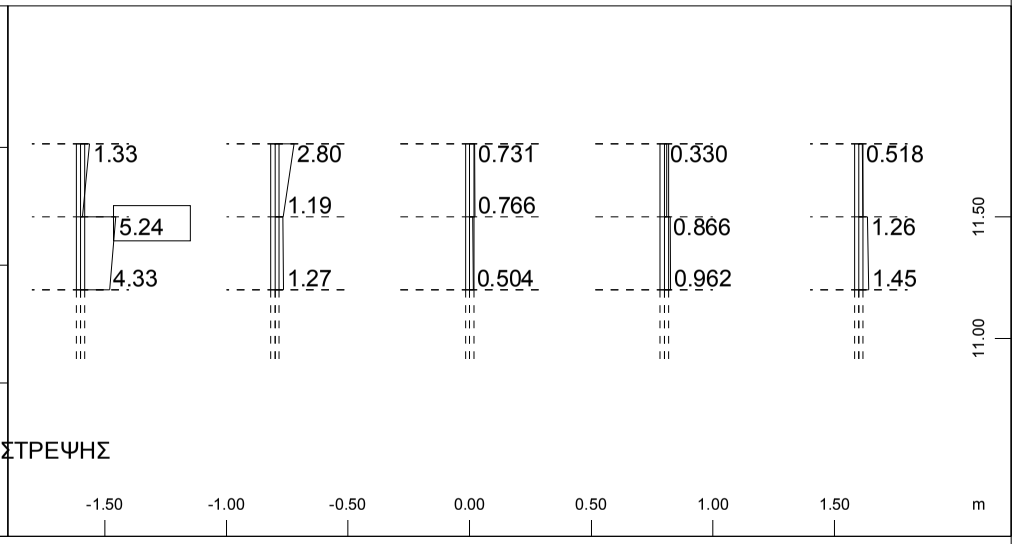


X Z Y Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Shear reinforcements (maximum), Design Case 508
, 1 cm 3D = 11.2 cm2/m (Max=5.94)



ΟΠΛΙΣΜΟΣ ΣΤΡΕΨΗΣ

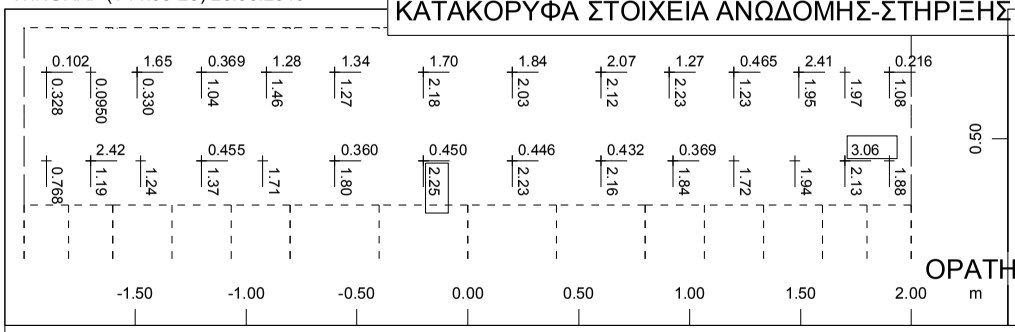
Z Y X Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
508 , 1 cm 3D = 22.4 cm2 (Max=13.3)



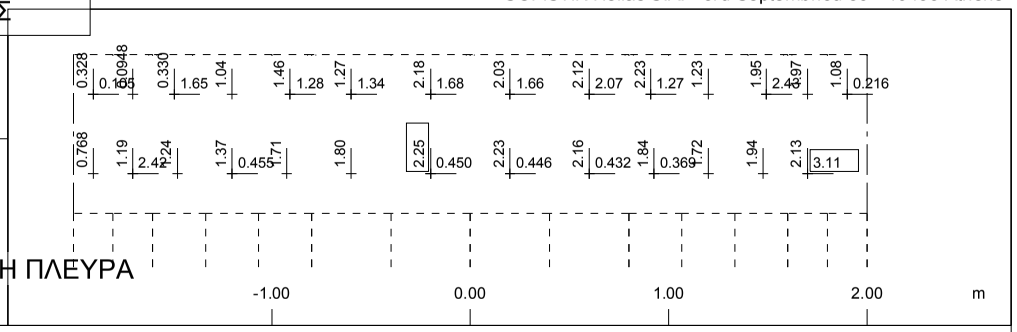
X Z Y Sector of system Beam Elements Group 2 M 1 : 31
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
508 , 1 cm 3D = 11.2 cm2 (Max=5.24)

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΦΑΣΗ-2_SLS

ΚΑΤΑΚΟΡΥΦΑ ΣΤΟΙΧΕΙΑ ΑΝΩΔΟΜΗΣ-ΣΤΗΡΙΞΗΣ

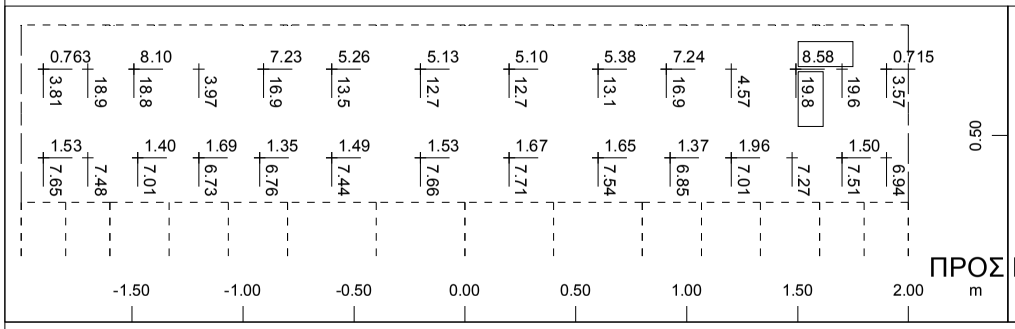


Sector of system Quadrilateral Elements Group 8 M 1 : 34
upper Reinforcements in Elements in cm2/m, Design Case 521 ULS
design (Max=3.06)

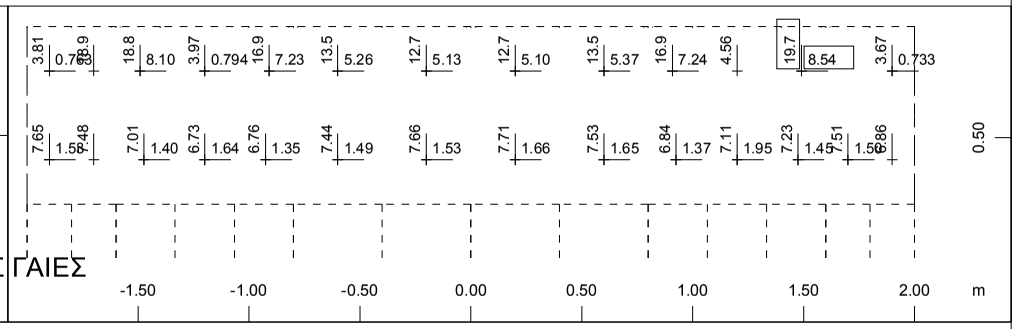


Sector of system Quadrilateral Elements Group 8 M 1 : 38
upper Reinforcements in Elements in cm2/m, Design Case 521 ULS
design (Max=3.11)

ΟΡΑΤΗ ΠΛΕΥΡΑ

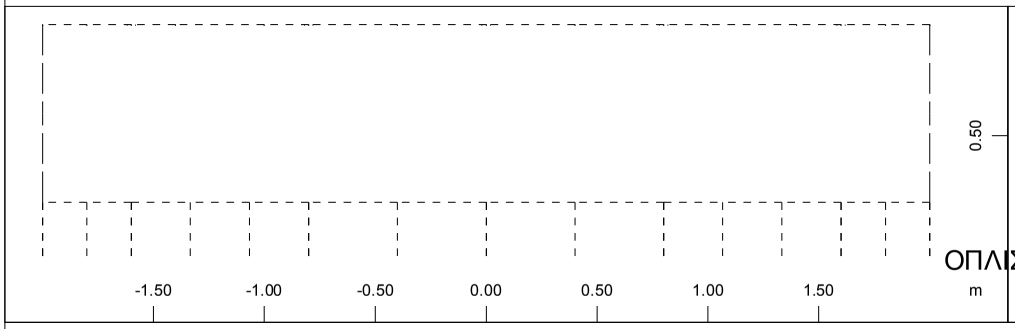


Sector of system Quadrilateral Elements Group 8 M 1 : 34
lower Reinforcements in Elements in cm2/m, Design Case 521 ULS
design (Max=19.8)

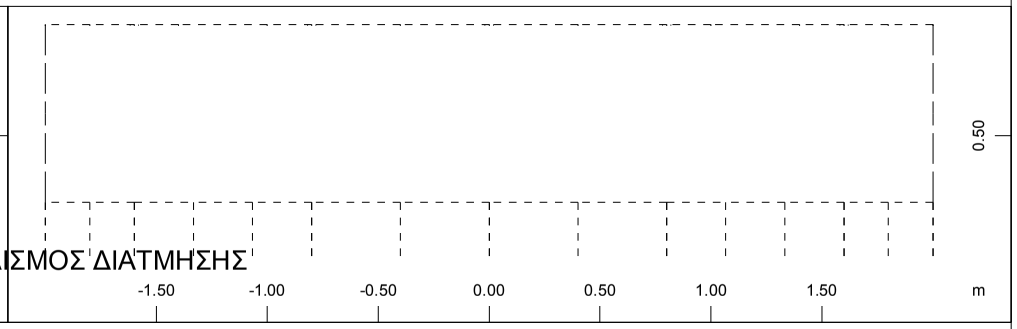


Sector of system Quadrilateral Elements Group 8 M 1 : 34
lower Reinforcements in Elements in cm2/m, Design Case 521 ULS
design (Max=19.7)

ΠΡΟΣ ΓΑΙΕΣ



Sector of system Quadrilateral Elements Group 8 M 1 : 34
Shear reinforcement from middle of element in cm2/m2, Design
Case 521 ULS design (Max=0)

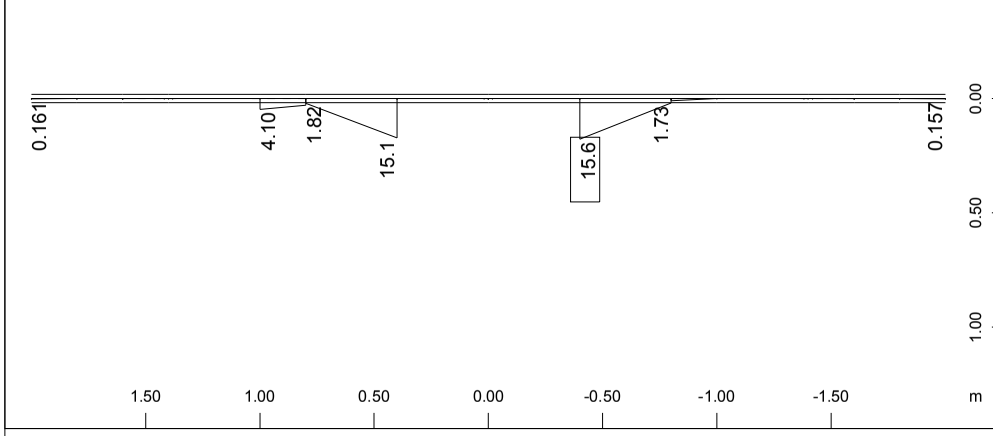


Sector of system Quadrilateral Elements Group 8 M 1 : 34
Shear reinforcement from middle of element in cm2/m2, Design
Case 521 ULS design (Max=0)

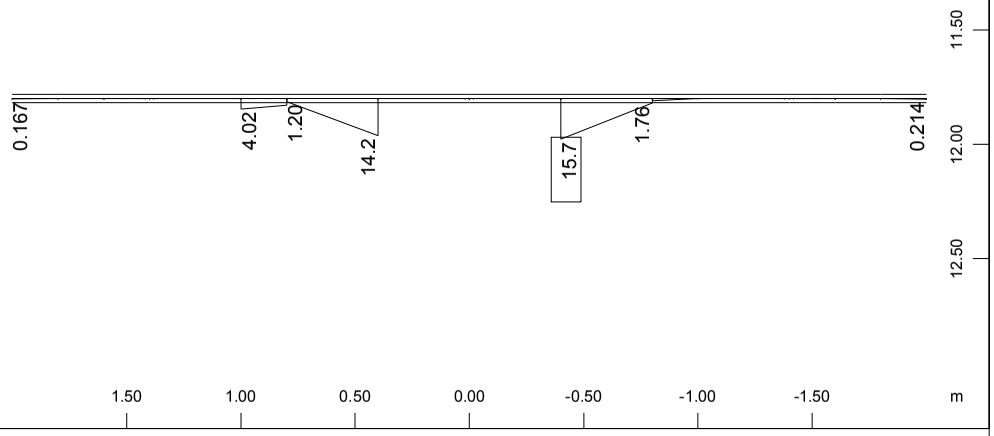
ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ

ΠΑΣΣΑΛΟΔΕΣΜΟΣ

ΚΑΤΩ ΟΠΛΙΣΜΟΣ

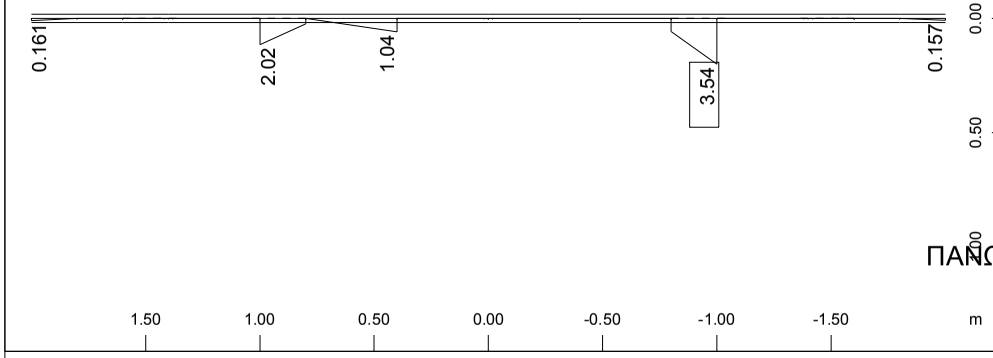


Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
508 , 1 cm 3D = 29.1 cm2 (Max=15.6)

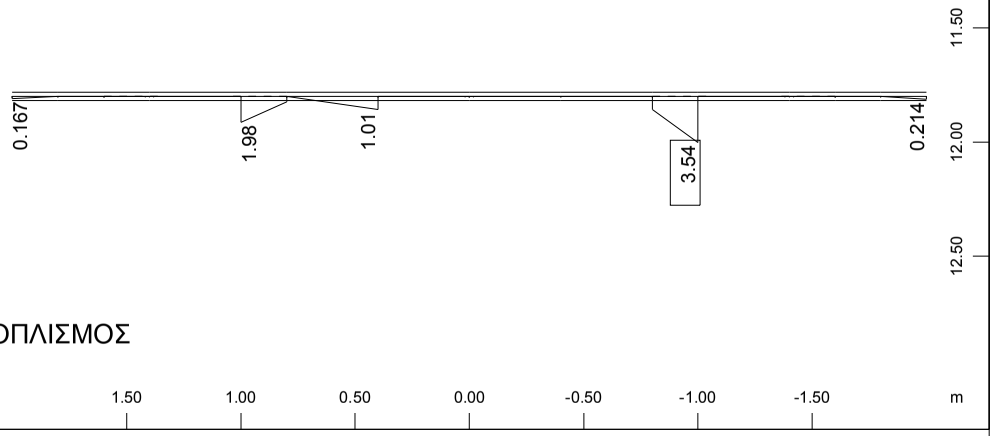


Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
508 , 1 cm 3D = 29.1 cm2 (Max=15.7)

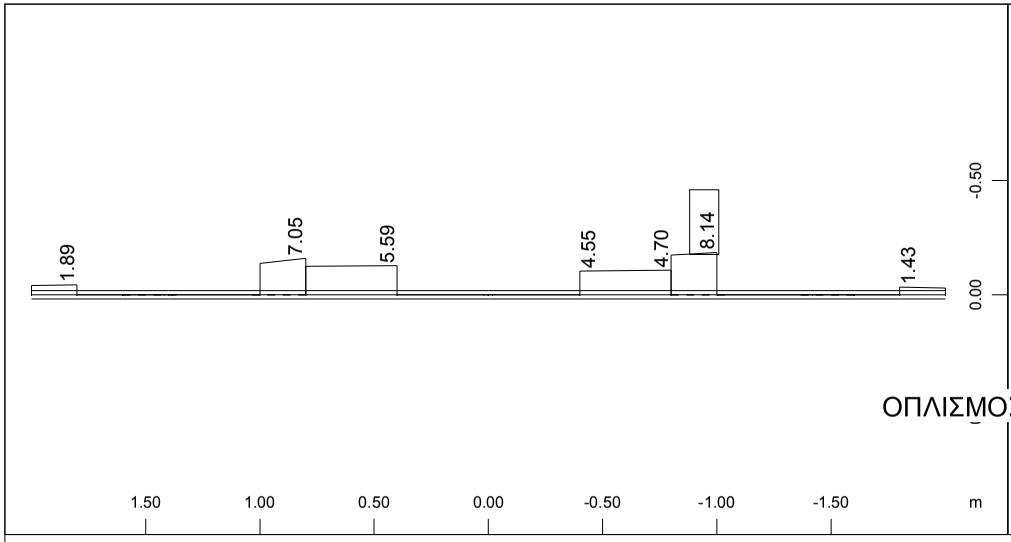
ΠΑΝΩ ΟΠΛΙΣΜΟΣ



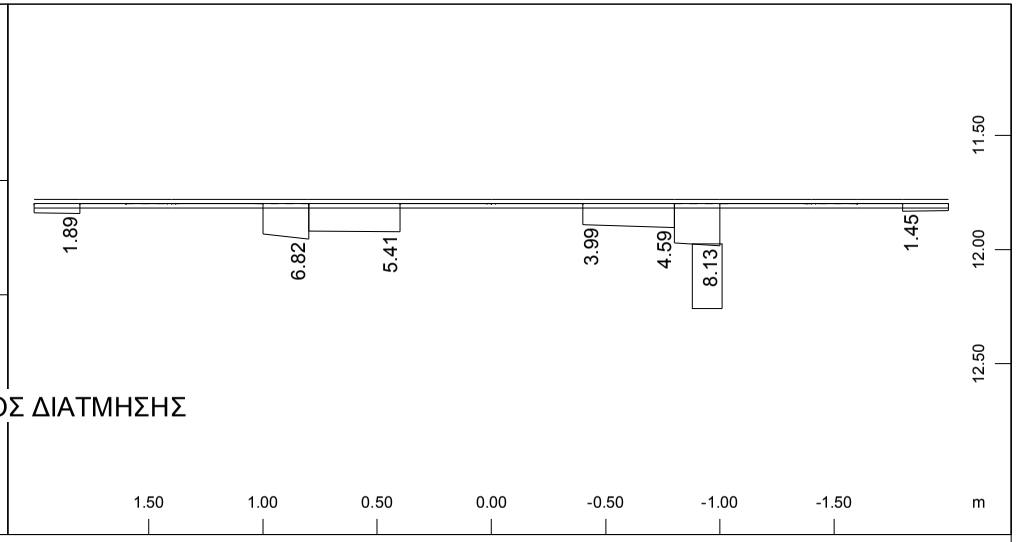
Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
508 , 1 cm 3D = 5.81 cm2 (Max=3.54)



Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
508 , 1 cm 3D = 5.81 cm2 (Max=3.54)

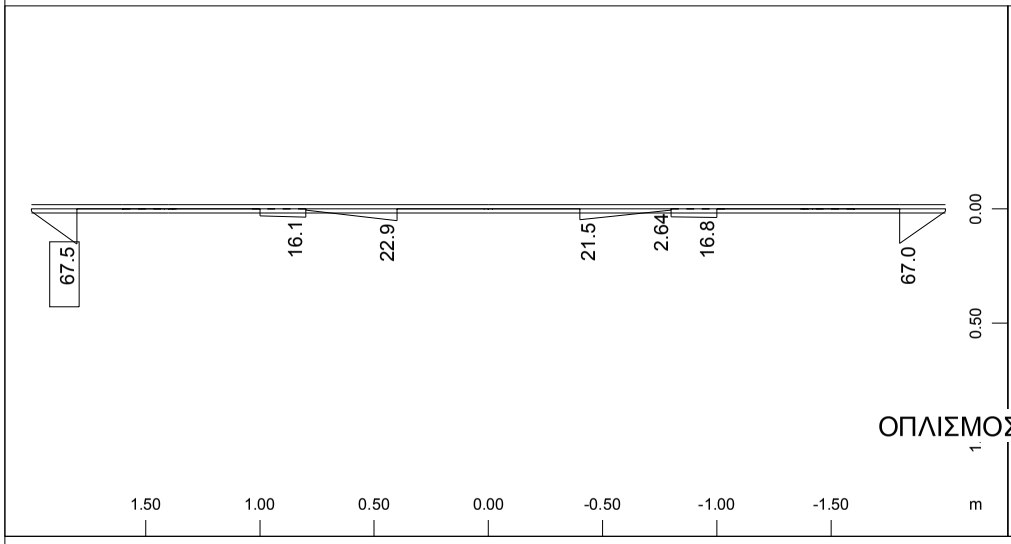


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Shear reinforcements (maximum), Design Case 508
, 1 cm 3D = 14.5 cm2/m (Max=8.14)

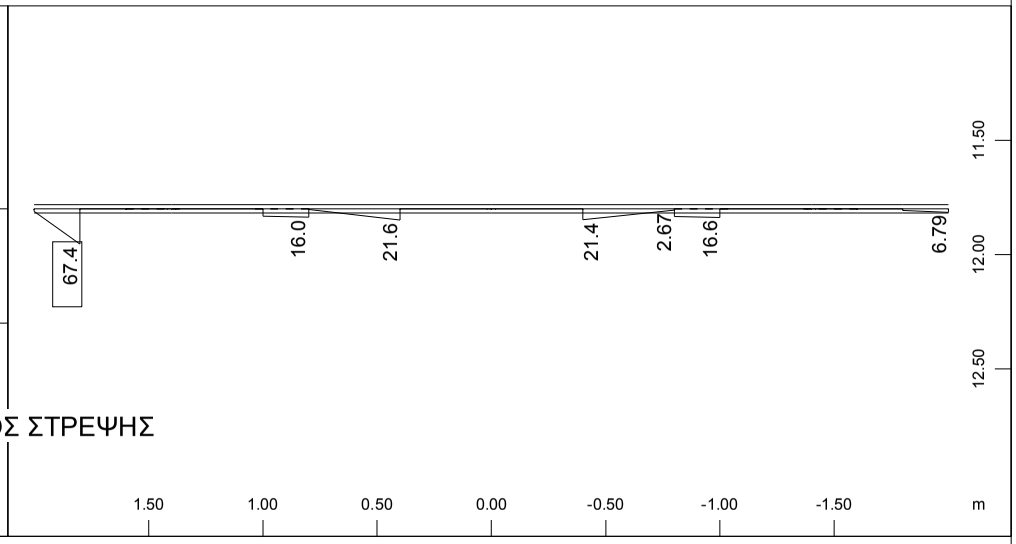


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Shear reinforcements (maximum), Design Case 508
, 1 cm 3D = 14.5 cm2/m (Max=8.13)

ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ

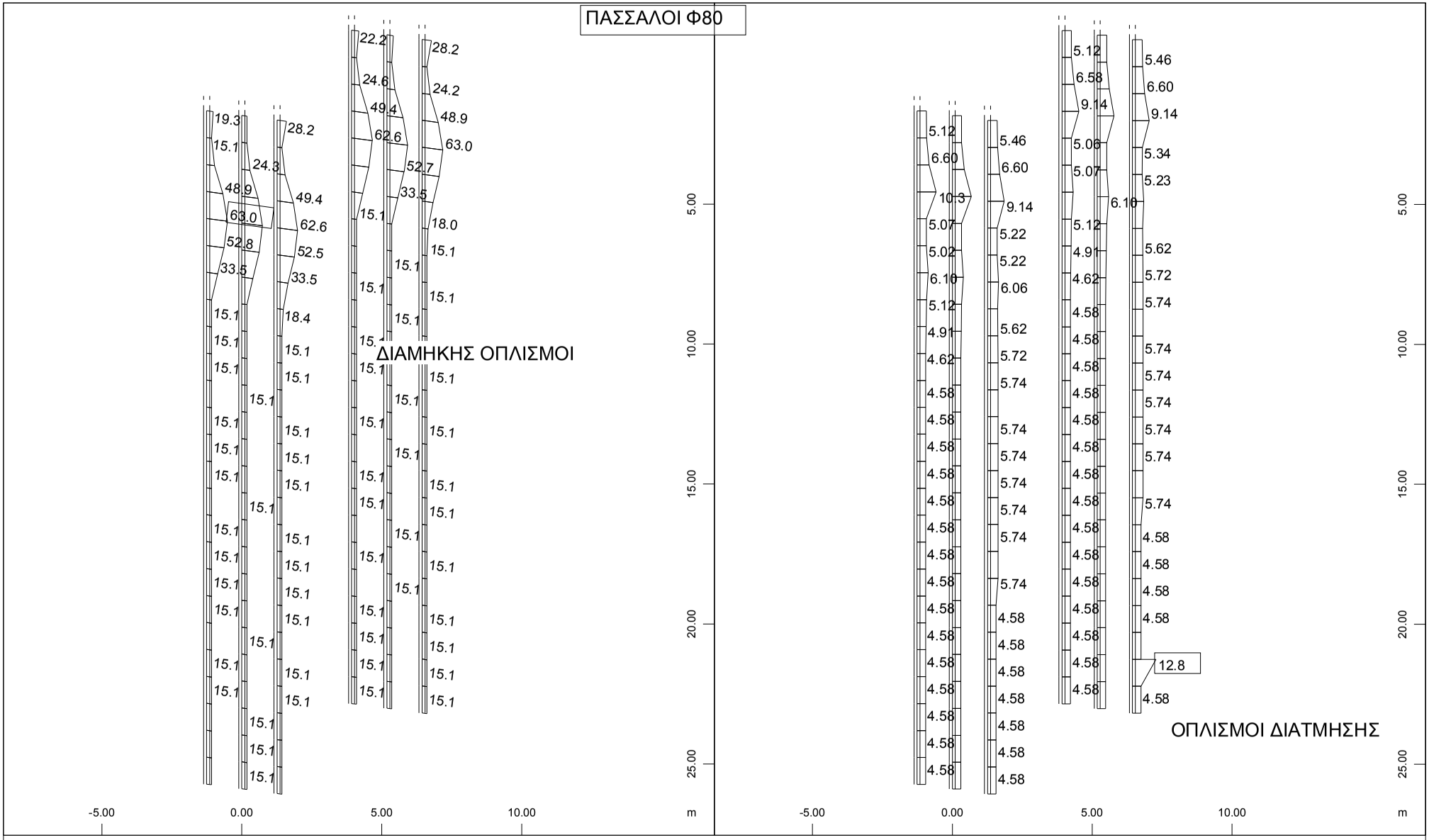


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
508 , 1 cm 3D = 145.3 cm2 (Max=67.5)



Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
508 , 1 cm 3D = 145.3 cm2 (Max=67.4)

ΟΠΛΙΣΜΟΣ ΣΤΡΕΨΗΣ



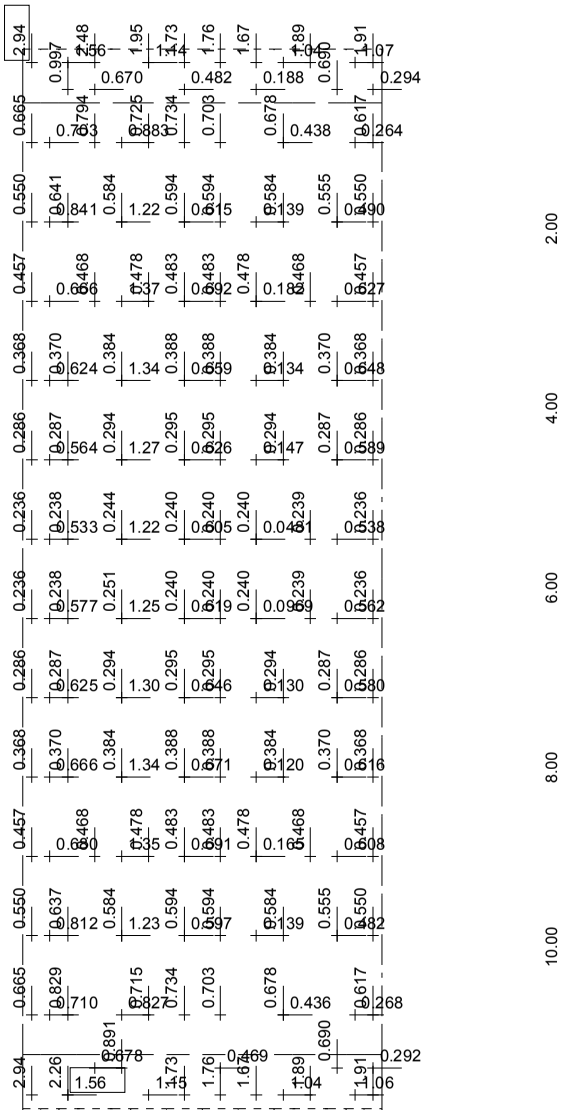
Sector of system Beam Elements Group 12
Beam Elements , Longitudinal Reinforcements Lay. 1, Design
Case 508 , 1 cm 3D = 145.3 cm2 (Max=63.0)

M 1 : 191
X * 0.502
Y * 0.906
Z * 0.962

Sector of system Beam Elements Group 12
Beam Elements , Shear reinforcements (maximum), Design Case
508 , 1 cm 3D = 29.1 cm2/m (Max=12.8)

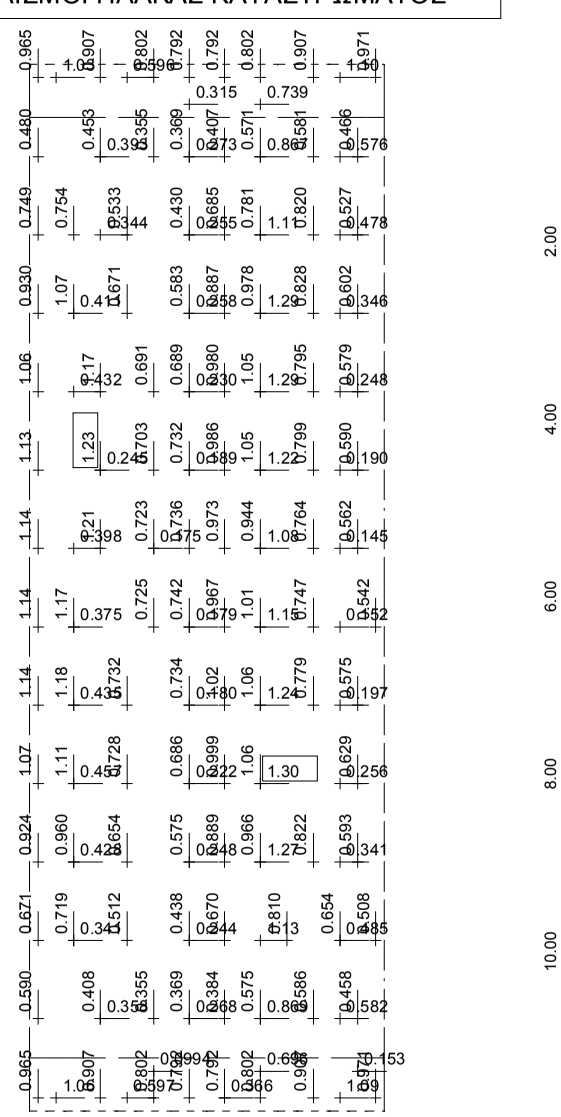
M 1 : 191
X * 0.502
Y * 0.906
Z * 0.962

ΟΠΛΙΣΜΟΙ ΠΛΑΚΑΣ ΚΑΤΑΣΤΡΩΜΑΤΟΣ



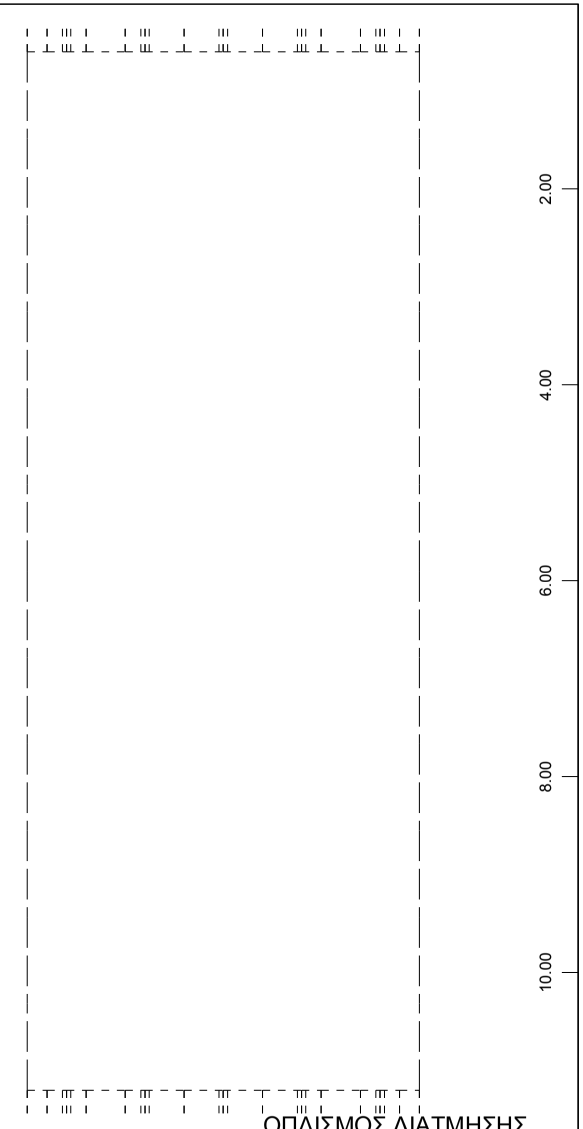
Sector of system Quadrilateral Elements Group 3 4
upper Reinforcements in Elements in cm2/m, Design Case
521 ULS design (Max=2.94)

M 1 : 84



Sector of system Quadrilateral Elements Group 3 4
lower Reinforcements in Elements in cm2/m, Design Case
521 ULS design (Max=1.30)

M 1 : 85



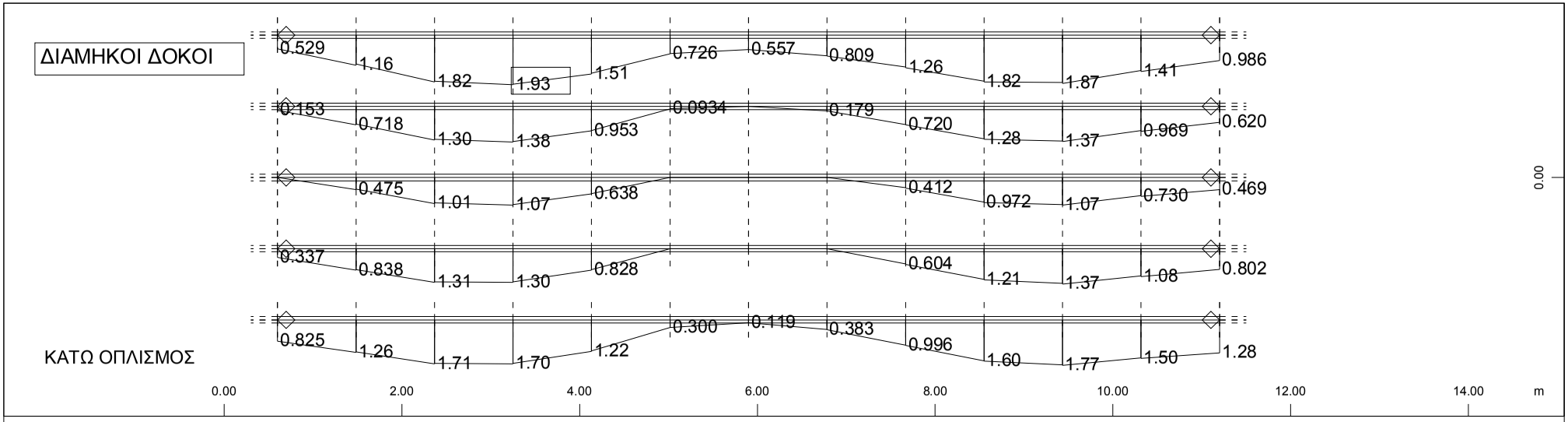
Sector of system Quadrilateral Elements Group 3
Shear reinforcement from middle of element in cm2/m2,
Design Case 521 ULS design (Max=0)

M 1 : 77

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΦΑΣΗ-2_SLS

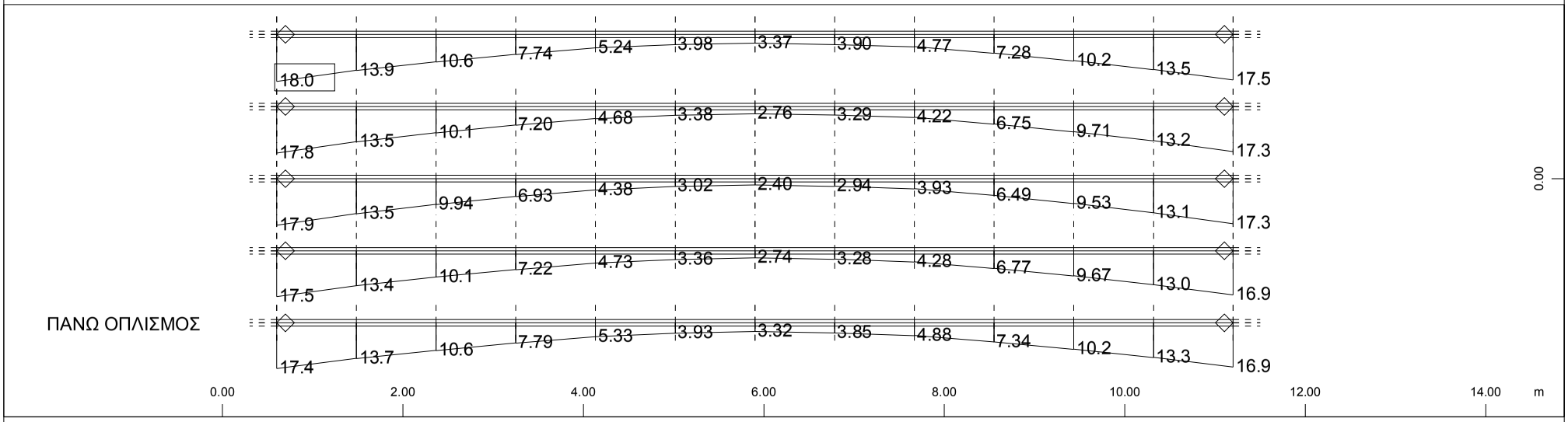
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00

10) ΦΑΣΗ-2_ ΕΛΕΓΧΟΣ ΦΟΡΕΑ ULS-ΣΕΙΣΜΙΚΑ (Q=1.50)



Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case 510 , 1 cm 3D = 2.24 cm2 (Max=1.93)

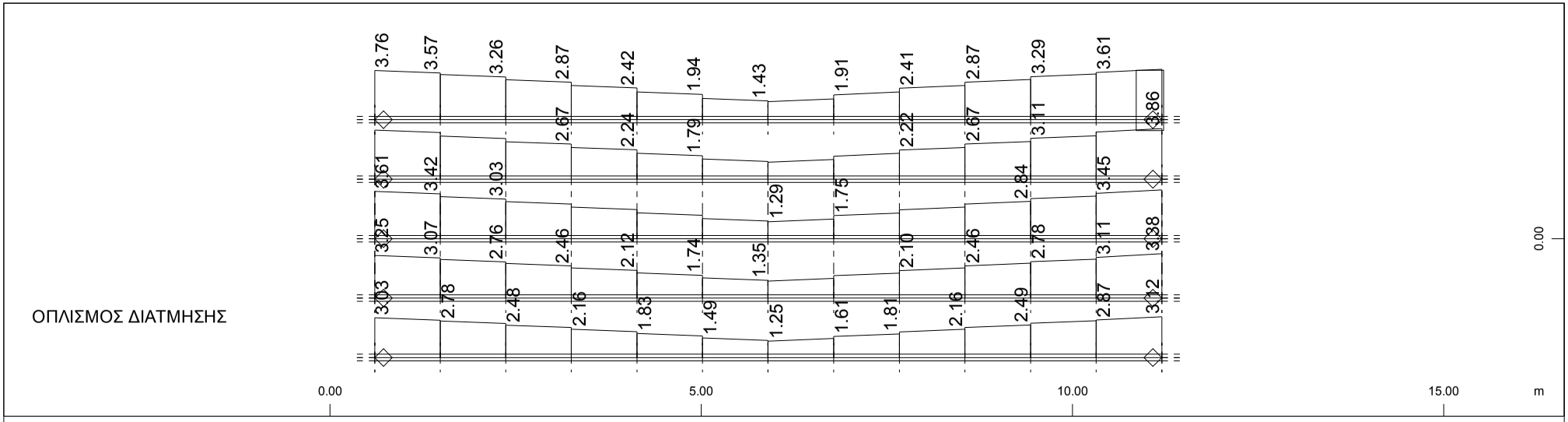
M 1 : 66



Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case 510 , 1 cm 3D = 22.4 cm2 (Max=18.0)

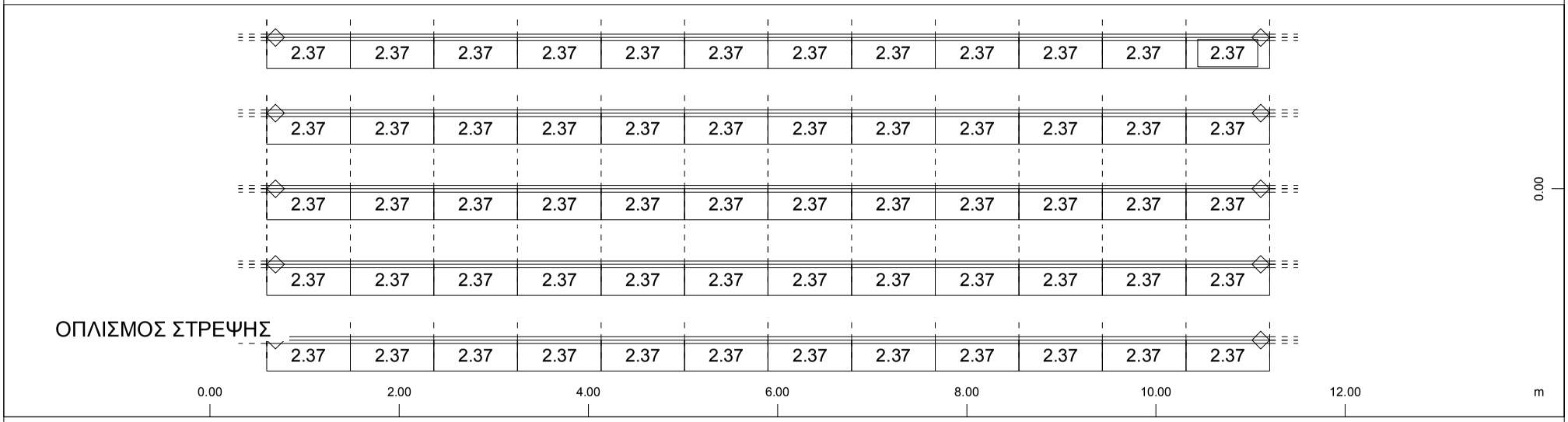
M 1 : 65

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΣΕΙΣΜΙΚΑ_Q=1.50



Sector of system Beam Elements Group 1
Beam Elements , Shear reinforcements (maximum), Design Case 510 , 1 cm 3D = 4.48 cm2/m (Max=3.86)

M 1 : 79



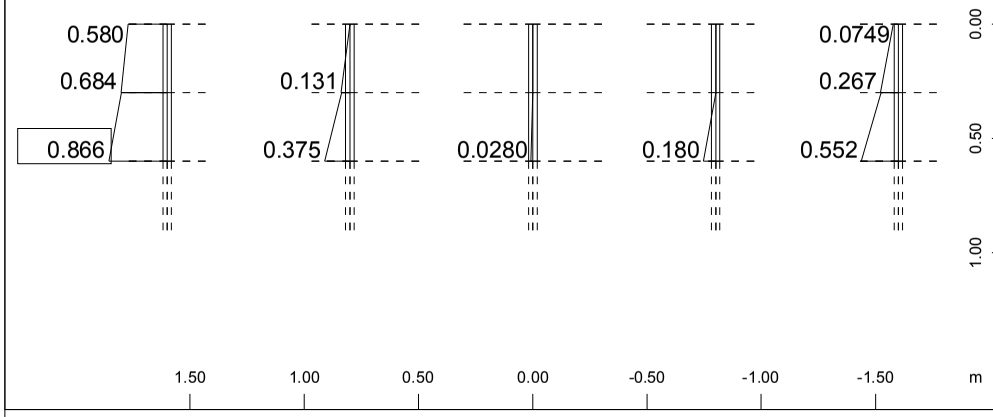
Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case 510 , 1 cm 3D = 4.48 cm2 (Max=2.37)

M 1 : 62

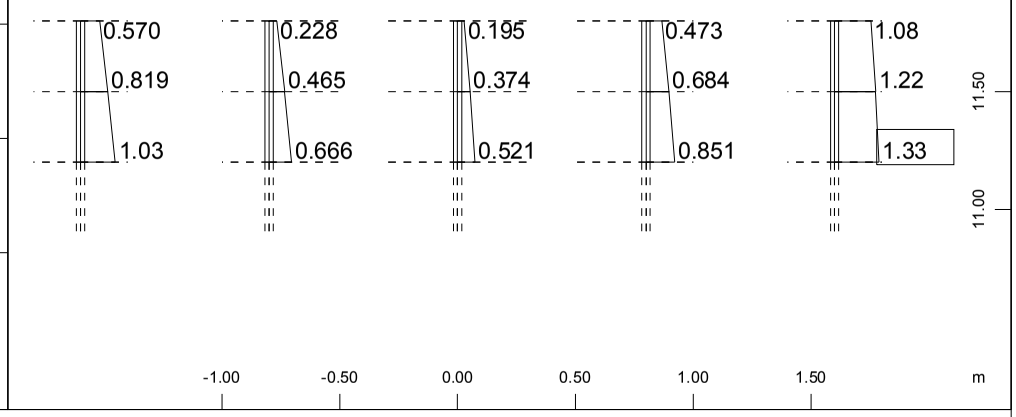
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΣΕΙΣΜΙΚΑ_Q=1.50

ΔΙΑΜΗΚΗ ΔΟΚΟΙ-ΣΤΗΡΙΞΗΣ

ΚΑΤΩ ΟΠΛΙΣΜΟΣ

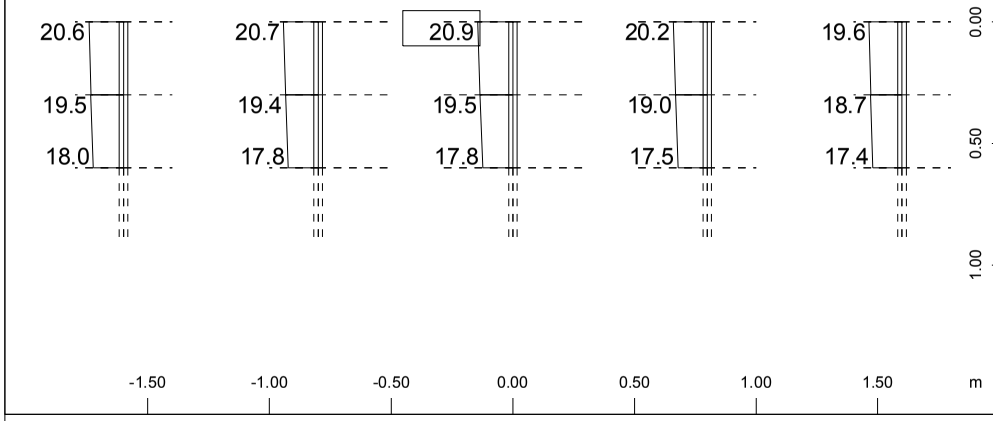


Y Z X
Sector of system Beam Elements Group 2 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
510 , 1 cm 3D = 1.12 cm2 (Max=0.866)

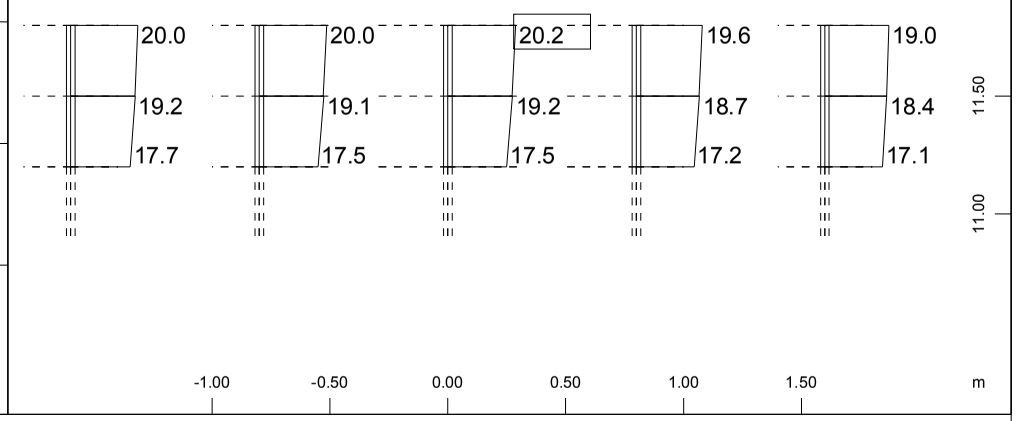


X Z Y
Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
510 , 1 cm 3D = 2.24 cm2 (Max=1.33)

ΠΑΝΩ ΟΠΛΙΣΜΟΣ

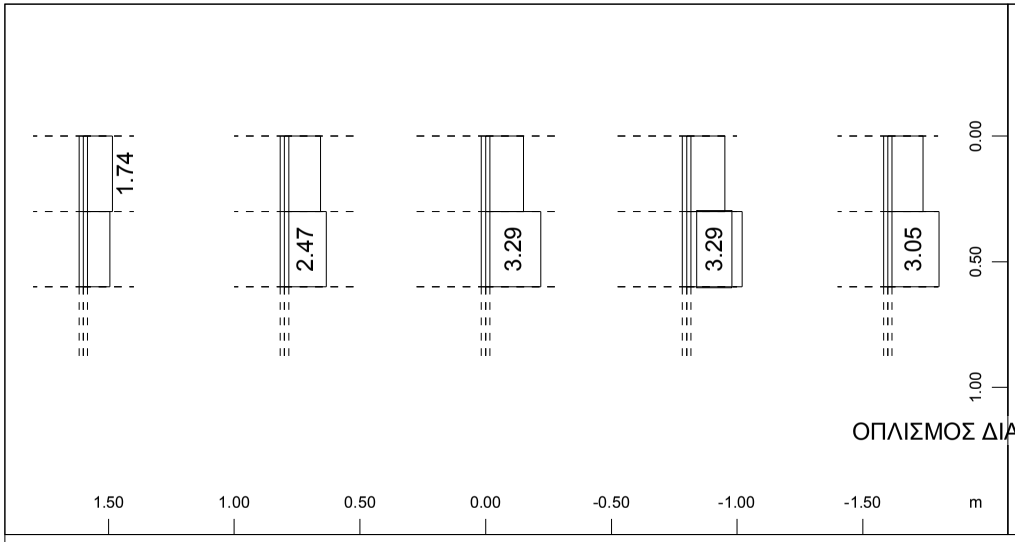


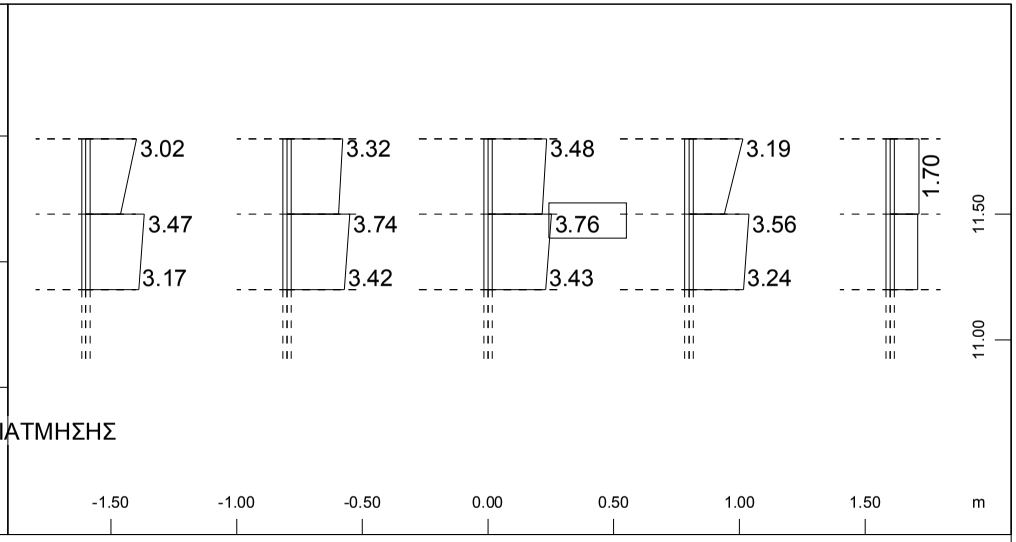
Z Y X
Sector of system Beam Elements Group 2 M 1 : 31
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
510 , 1 cm 3D = 44.8 cm2 (Max=20.9)



X Z Y
Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
510 , 1 cm 3D = 22.4 cm2 (Max=20.2)

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΣΕΙΣΜΙΚΑ_Q=1.50

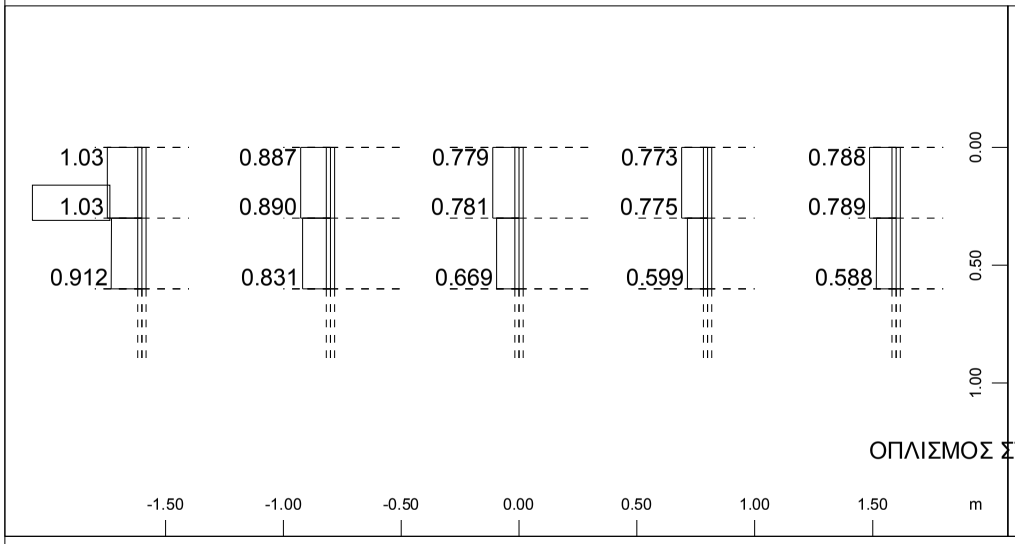


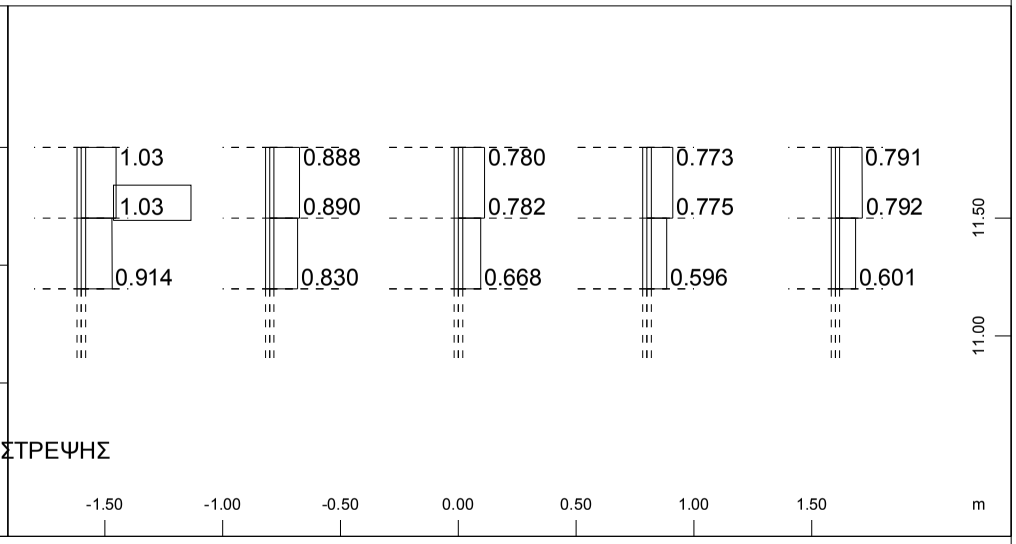


ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ

Y Z X Sector of system Beam Elements Group 2 M 1 : 30
Beam Elements , Shear reinforcements (maximum), Design Case 510
, 1 cm 3D = 4.48 cm2/m (Max=3.29)

X Z Y Sector of system Beam Elements Group 2 M 1 : 30
Beam Elements , Shear reinforcements (maximum), Design Case 510
, 1 cm 3D = 4.48 cm2/m (Max=3.76)





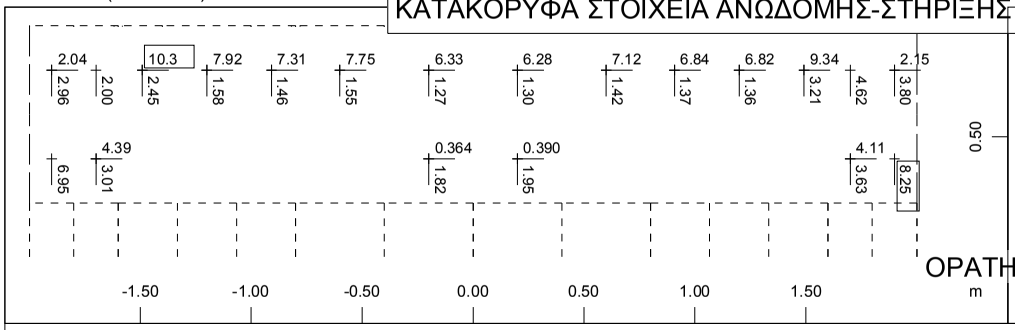
ΟΠΛΙΣΜΟΣ ΣΤΡΕΨΗΣ

Z Y X Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
510 , 1 cm 3D = 2.24 cm2 (Max=1.03)

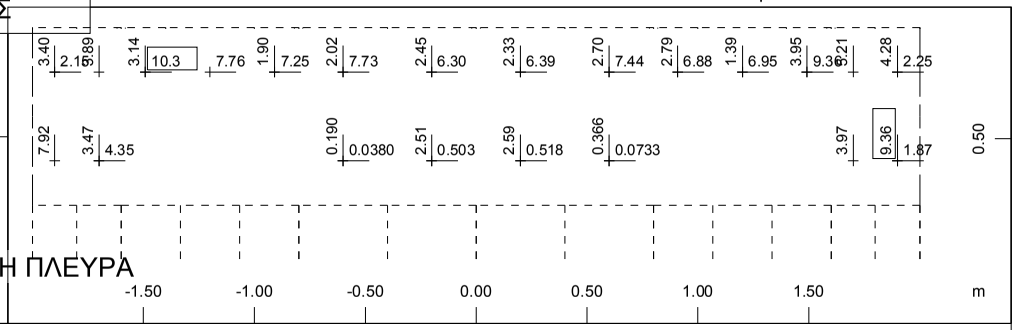
X Z Y Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
510 , 1 cm 3D = 2.24 cm2 (Max=1.03)

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΣΕΙΣΜΙΚΑ_Q=1.50

ΚΑΤΑΚΟΡΥΦΑ ΣΤΟΙΧΕΙΑ ΑΝΩΔΟΜΗΣ-ΣΤΗΡΙΞΗΣ

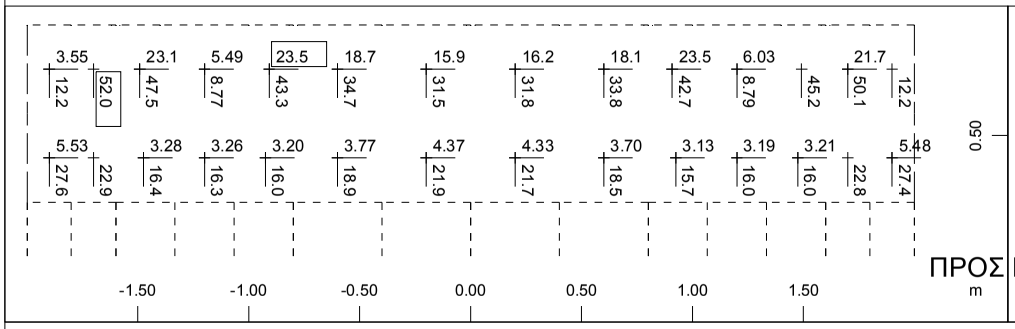


Sector of system Quadrilateral Elements Group 8 M 1 : 34
upper Reinforcements in Elements in cm2/m, Design Case 522 ULS
design (Max=10.3)

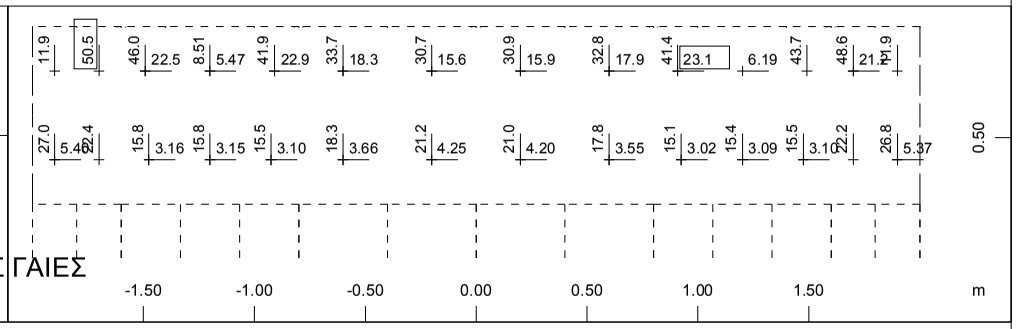


Sector of system Quadrilateral Elements Group 8 M 1 : 34
upper Reinforcements in Elements in cm2/m, Design Case 522 ULS
design (Max=10.3)

ΟΡΑΤΗ ΠΛΕΥΡΑ

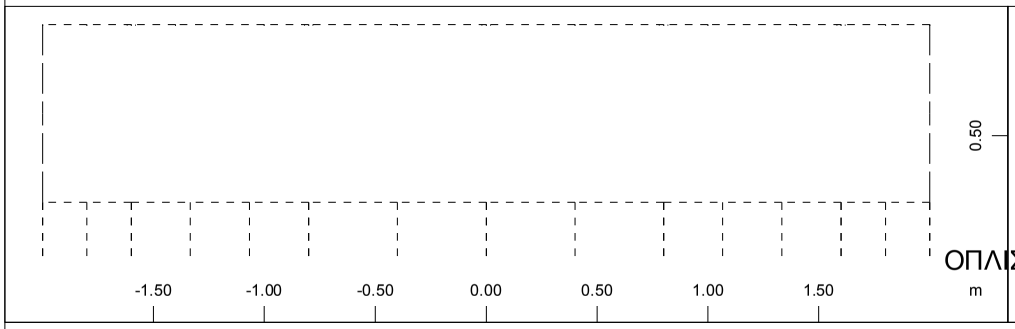


Sector of system Quadrilateral Elements Group 8 M 1 : 34
lower Reinforcements in Elements in cm2/m, Design Case 522 ULS
design (Max=52.0)

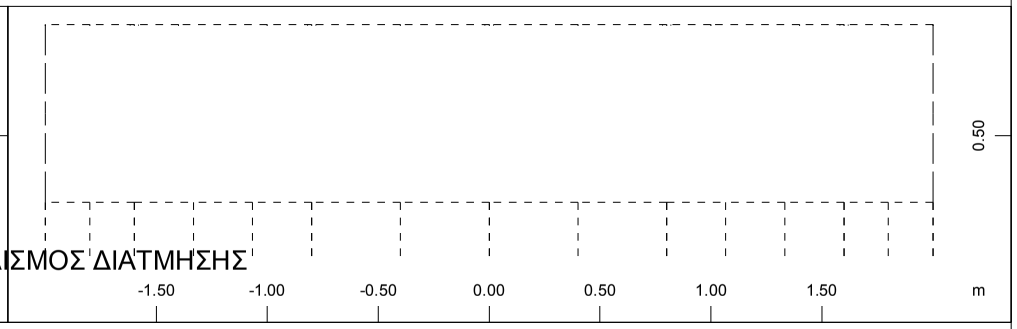


Sector of system Quadrilateral Elements Group 8 M 1 : 34
lower Reinforcements in Elements in cm2/m, Design Case 522 ULS
design (Max=50.5)

ΠΡΟΣ ΓΑΙΕΣ



Sector of system Quadrilateral Elements Group 8 M 1 : 34
Shear reinforcement from middle of element in cm2/m2, Design
Case 522 ULS design (Max=0)

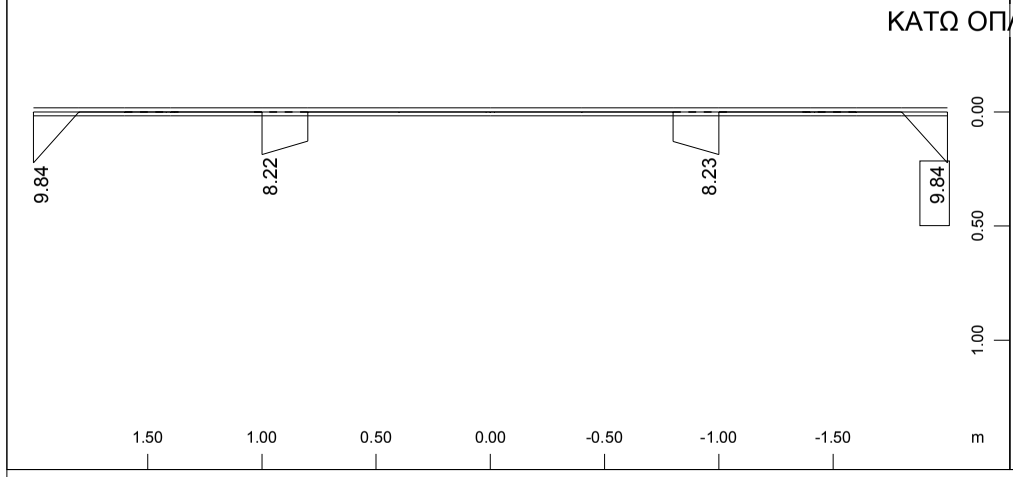


Sector of system Quadrilateral Elements Group 8 M 1 : 34
Shear reinforcement from middle of element in cm2/m2, Design
Case 522 ULS design (Max=0)

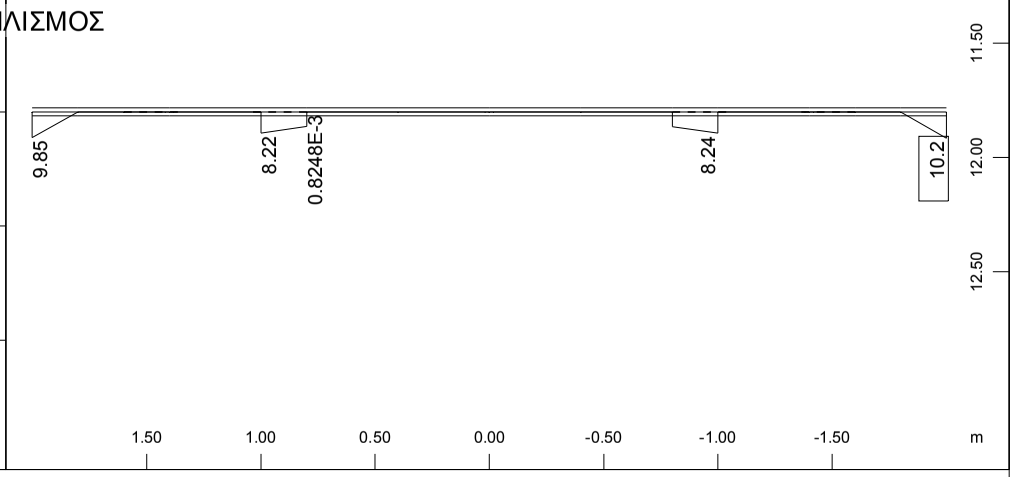
ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ

ΠΑΣΣΑΛΟΔΕΣΜΟΣ

ΚΑΤΩ ΟΠΛΙΣΜΟΣ

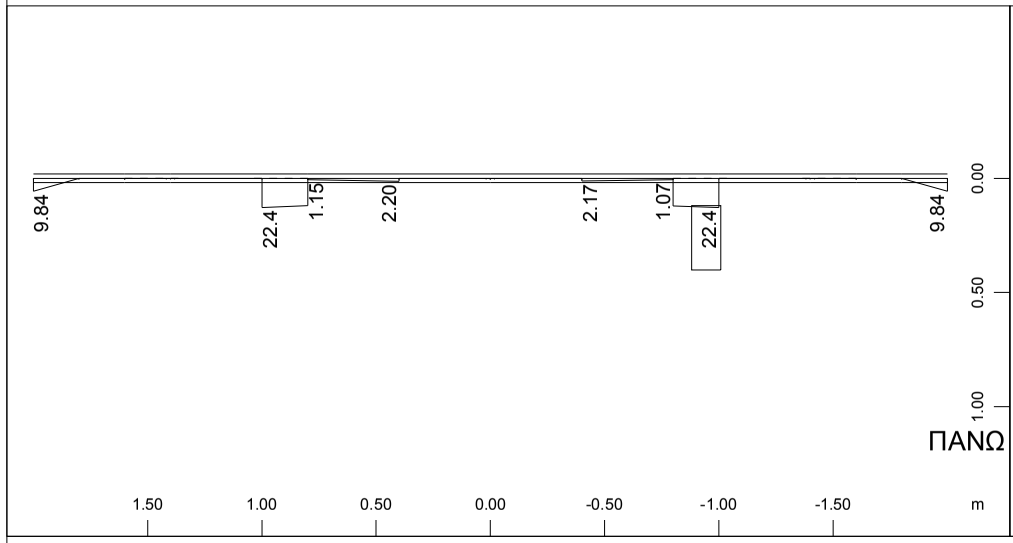


Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
509 , 1 cm 3D = 14.5 cm2 (Max=9.84)

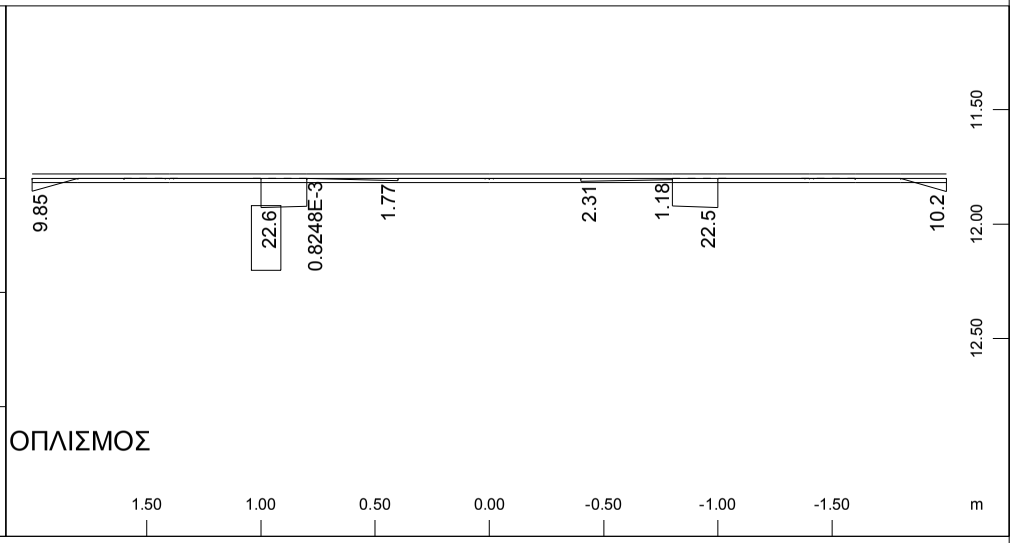


Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
509 , 1 cm 3D = 29.1 cm2 (Max=10.2)

ΠΑΝΩ ΟΠΛΙΣΜΟΣ

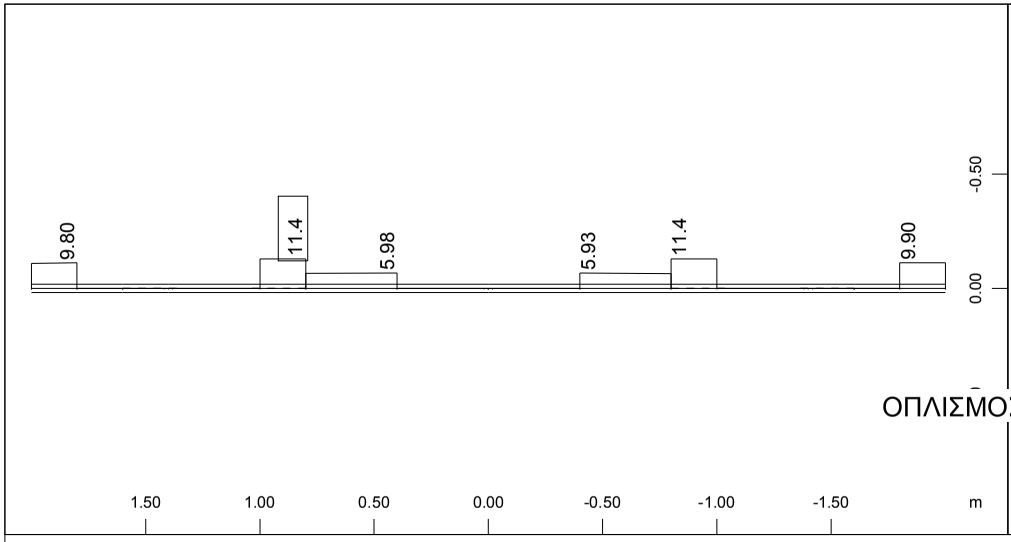


Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
509 , 1 cm 3D = 58.1 cm2 (Max=22.4)

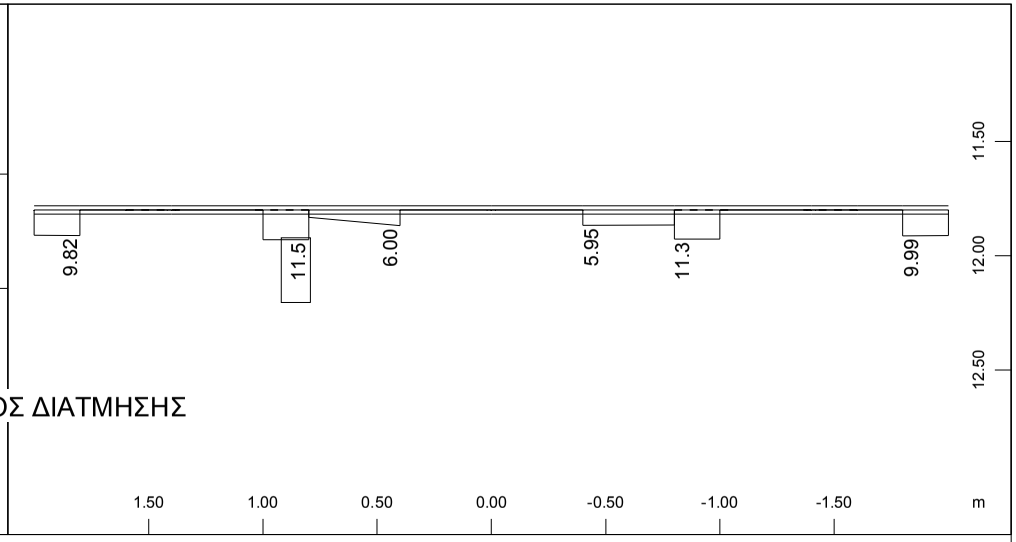


Y Z X
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
509 , 1 cm 3D = 58.1 cm2 (Max=22.6)

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΣΕΙΣΜΙΚΑ_Q=1.50

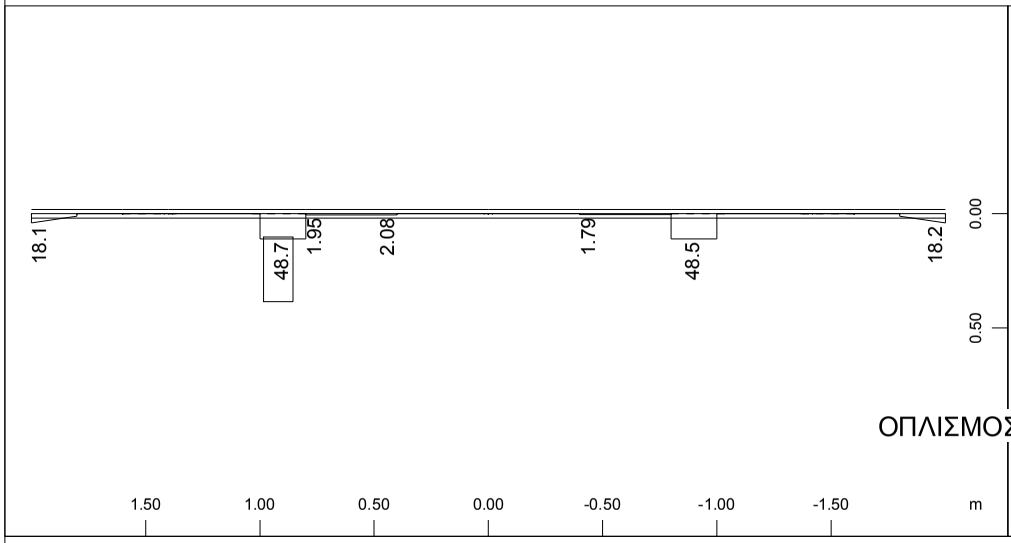


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Shear reinforcements (maximum), Design Case 509
, 1 cm 3D = 29.1 cm2/m (Max=11.4)

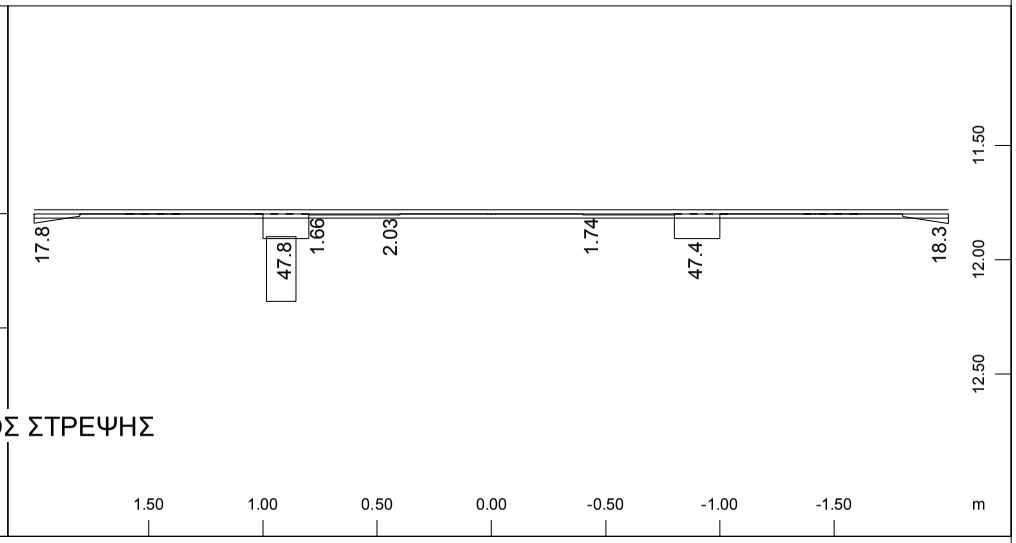


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Shear reinforcements (maximum), Design Case 509
, 1 cm 3D = 29.1 cm2/m (Max=11.5)

ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ



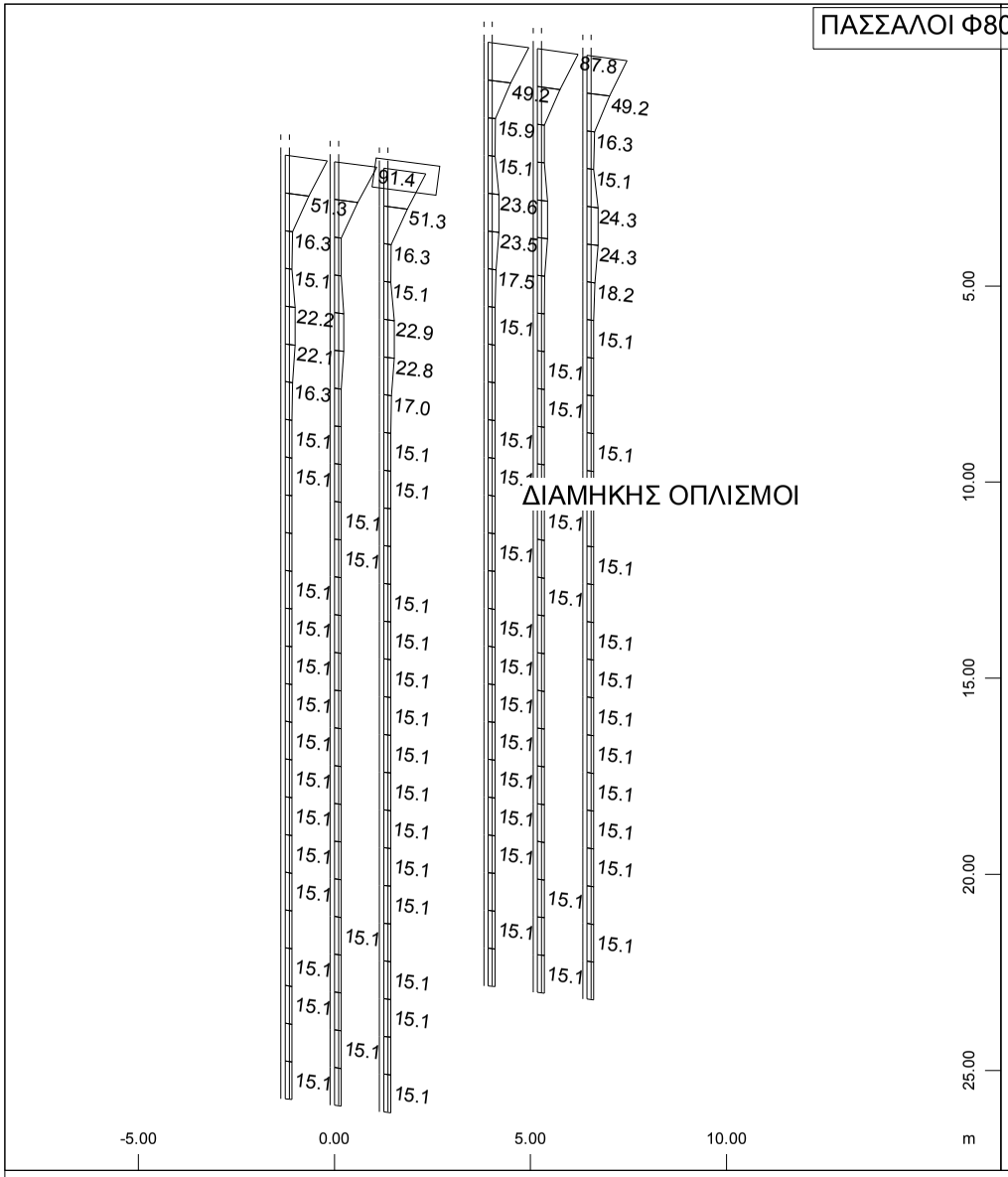
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
509 , 1 cm 3D = 145.3 cm2 (Max=48.7)



Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
509 , 1 cm 3D = 145.3 cm2 (Max=47.8)

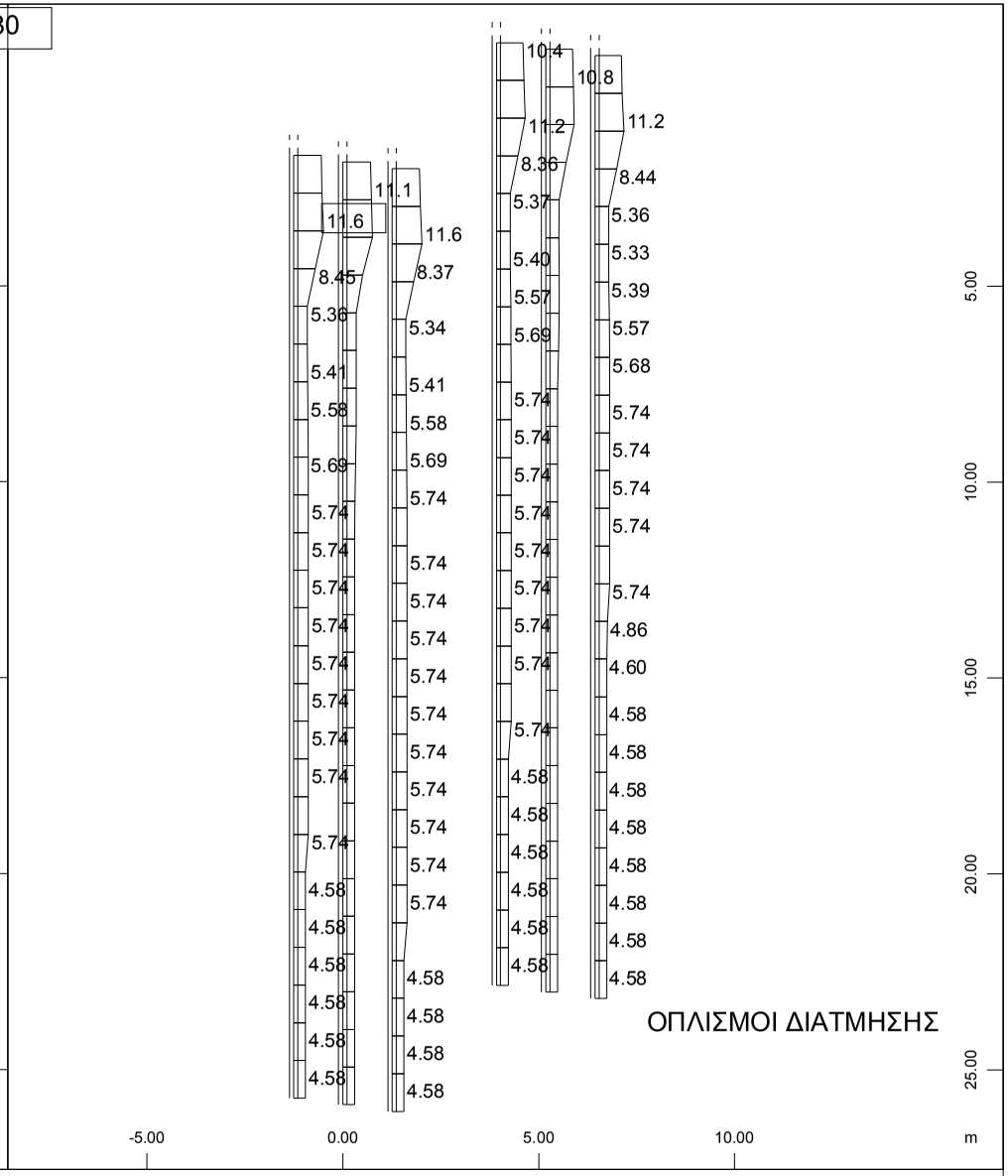
ΟΠΛΙΣΜΟΣ ΣΤΡΕΨΗΣ

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΣΕΙΣΜΙΚΑ_Q=1.50



Sector of system Beam Elements Group 12
Beam Elements , Longitudinal Reinforcements Lay. 1, Design
Case 509 , 1 cm 3D = 145.3 cm2 (Max=91.4)

M 1 : 191
X * 0.502
Y * 0.906
Z * 0.962

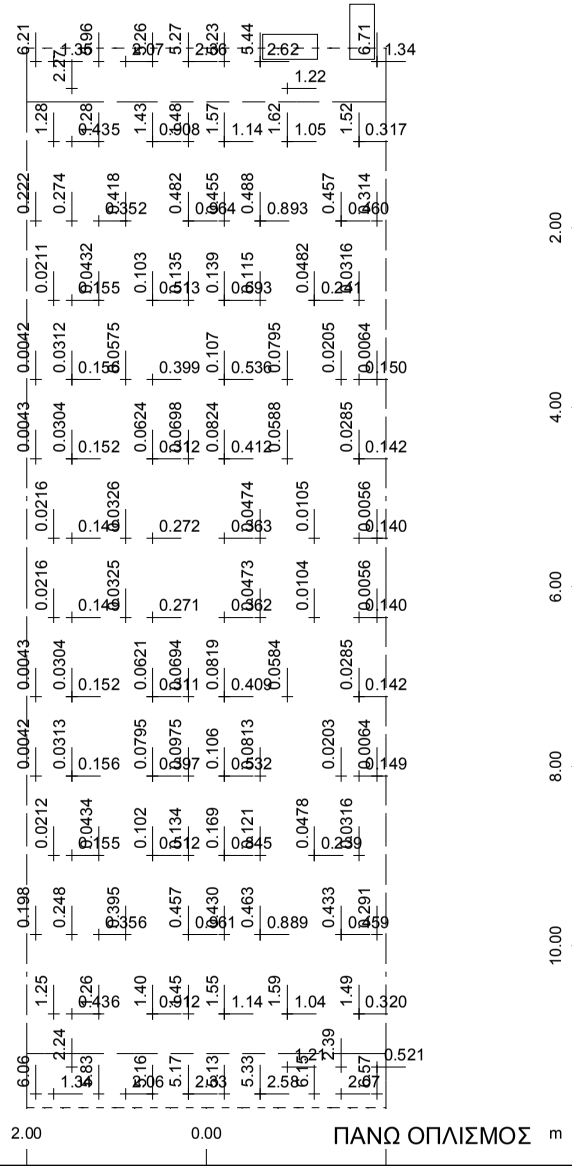


Sector of system Beam Elements Group 12
Beam Elements , Shear reinforcements (maximum), Design Case
509 , 1 cm 3D = 29.1 cm2/m (Max=11.6)

M 1 : 191
X * 0.502
Y * 0.906
Z * 0.962

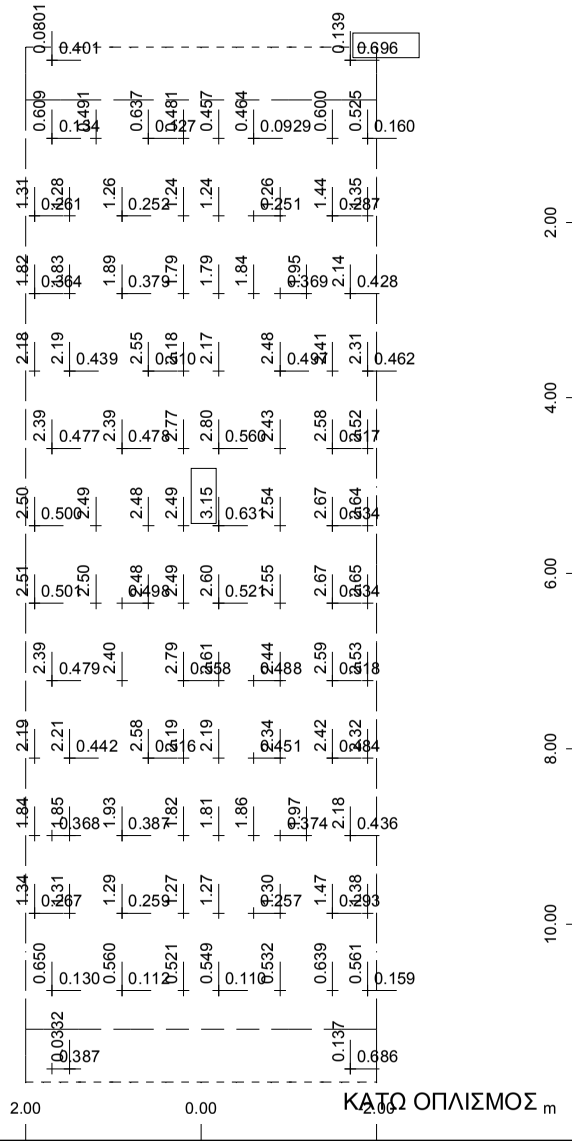
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΣΕΙΣΜΙΚΑ_Q=1.50

ΟΠΛΙΣΜΟΙ ΠΛΑΚΑΣ ΚΑΤΑΣΤΡΩΜΑΤΟΣ



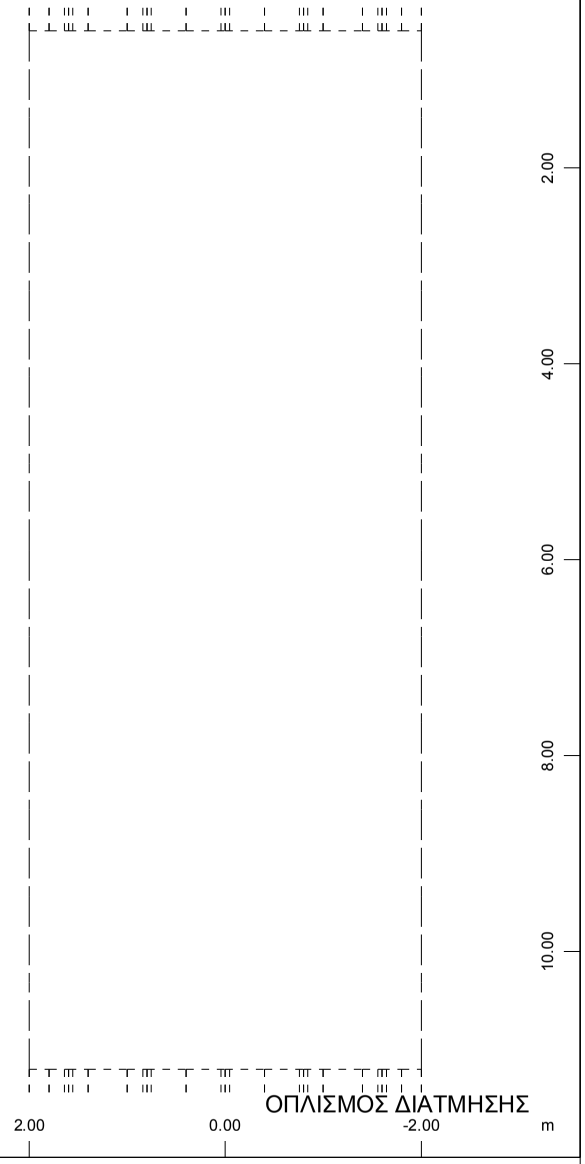
Sector of system Quadrilateral Elements Group 3 4
upper Reinforcements in Elements in cm2/m, Design Case
522 ULS design (Max=6.71)

M 1 : 84



Sector of system Quadrilateral Elements Group 3 4
lower Reinforcements in Elements in cm2/m, Design Case
522 ULS design (Max=3.15)

M 1 : 86



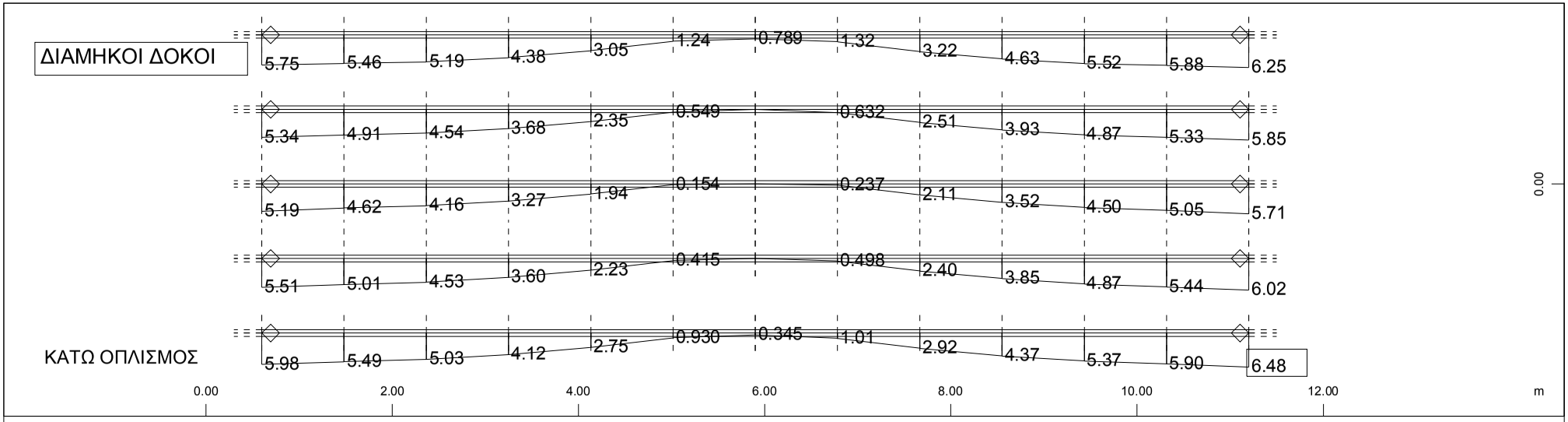
Sector of system Quadrilateral Elements Group 3
Shear reinforcement from middle of element in cm2/m2,
Design Case 522 ULS design (Max=0)

M 1 : 77

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΣΕΙΣΜΙΚΑ_Q=1.50

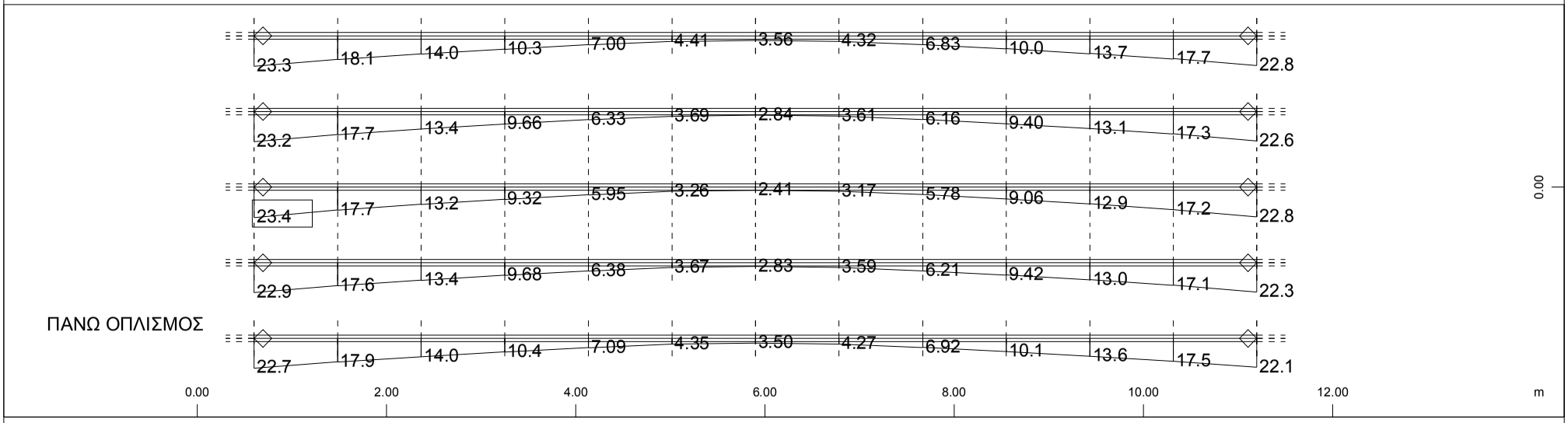
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00

11) ΦΑΣΗ-2_ ΕΛΕΓΧΟΣ ΦΟΡΕΑ ULS-ΣΕΙΣΜΙΚΑ (Q=1.00, γBd1=1.25)



Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case 512 , 1 cm 3D = 11.2 cm2 (Max=6.48)

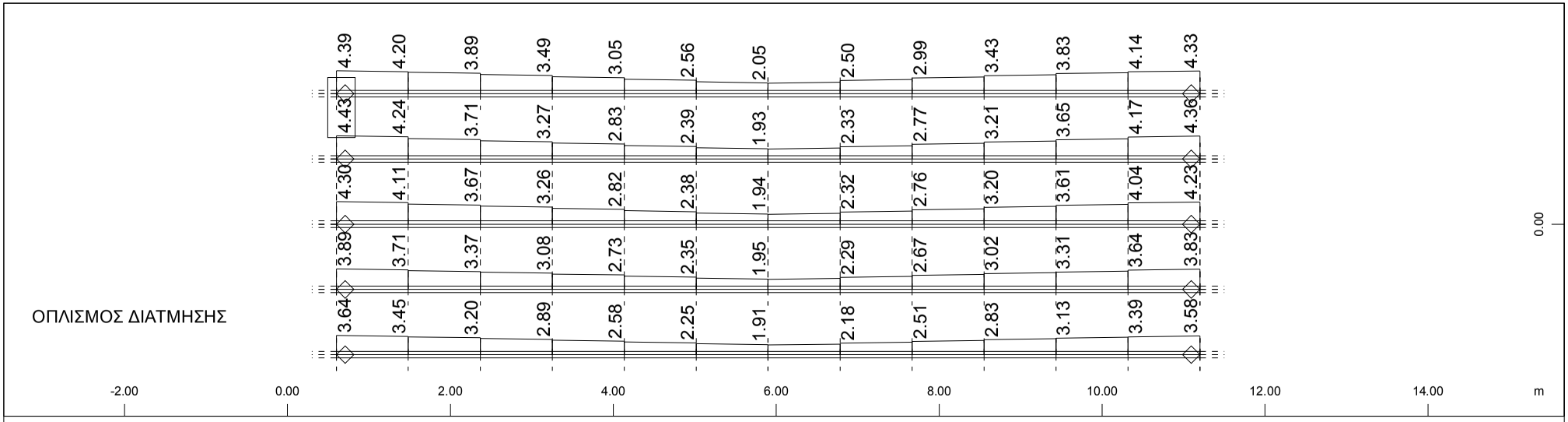
M 1 : 63



Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case 512 , 1 cm 3D = 44.8 cm2 (Max=23.4)

M 1 : 62

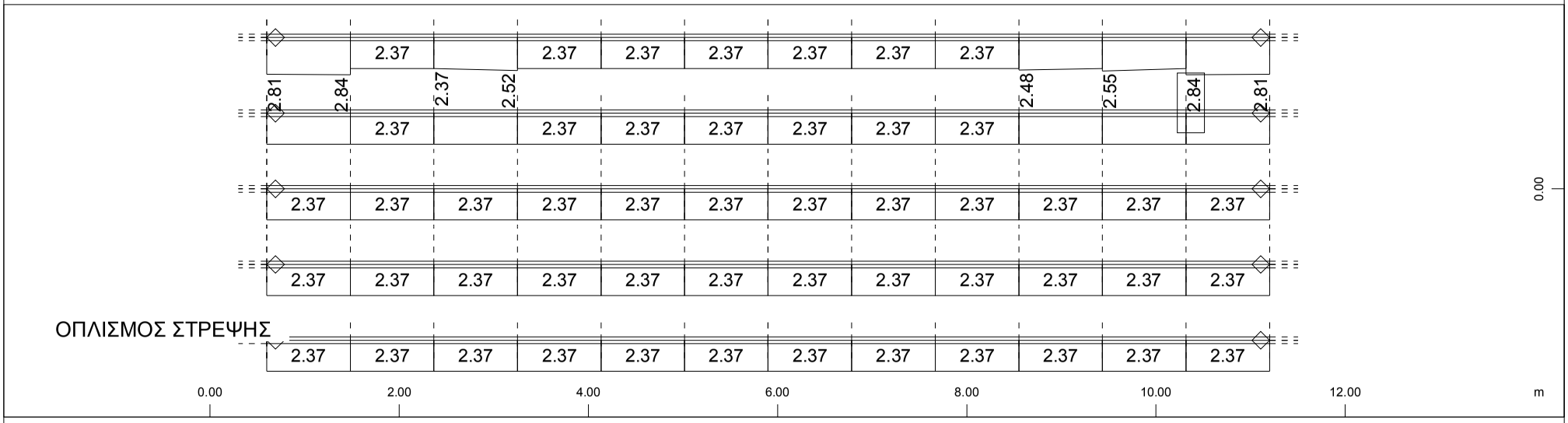
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΣΕΙΣΜΙΚΑ_Q=1.00



Z—X
Y

Sector of system Beam Elements Group 1
Beam Elements , Shear reinforcements (maximum), Design Case 512 , 1 cm 3D = 11.2 cm2/m (Max=4.43)

M 1 : 72



Z—X
Y

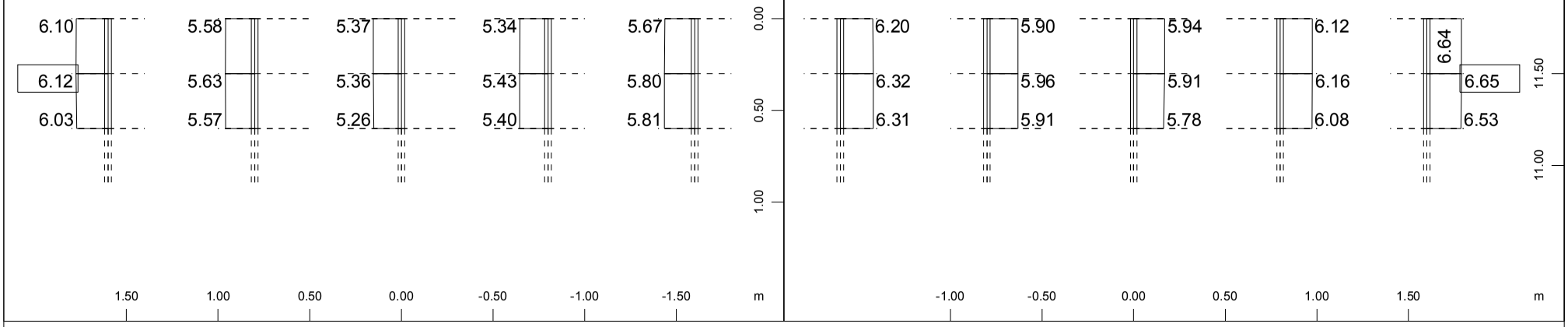
Sector of system Beam Elements Group 1
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case 512 , 1 cm 3D = 4.48 cm2 (Max=2.84)

M 1 : 62

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΣΕΙΣΜΙΚΑ_Q=1.00

ΔΙΑΜΗΚΗ ΔΟΚΟΙ-ΣΤΗΡΙΞΗΣ

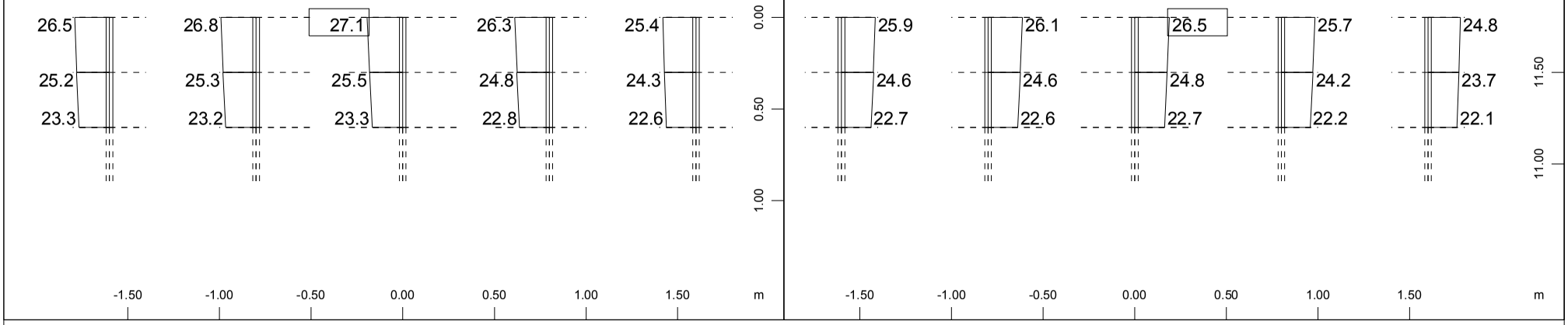
ΚΑΤΩ ΟΠΛΙΣΜΟΣ



Y Z X
Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
512 , 1 cm 3D = 11.2 cm2 (Max=6.12)

X Z Y
Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
512 , 1 cm 3D = 11.2 cm2 (Max=6.65)

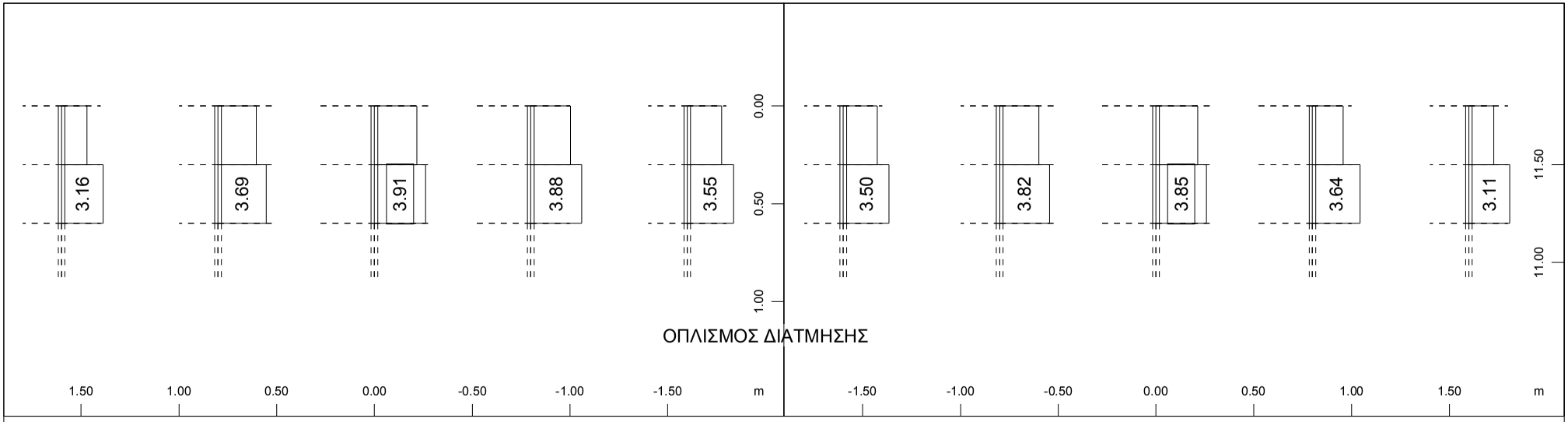
ΠΑΝΩ ΟΠΛΙΣΜΟΣ



Z Y X
Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
512 , 1 cm 3D = 44.8 cm2 (Max=27.1)

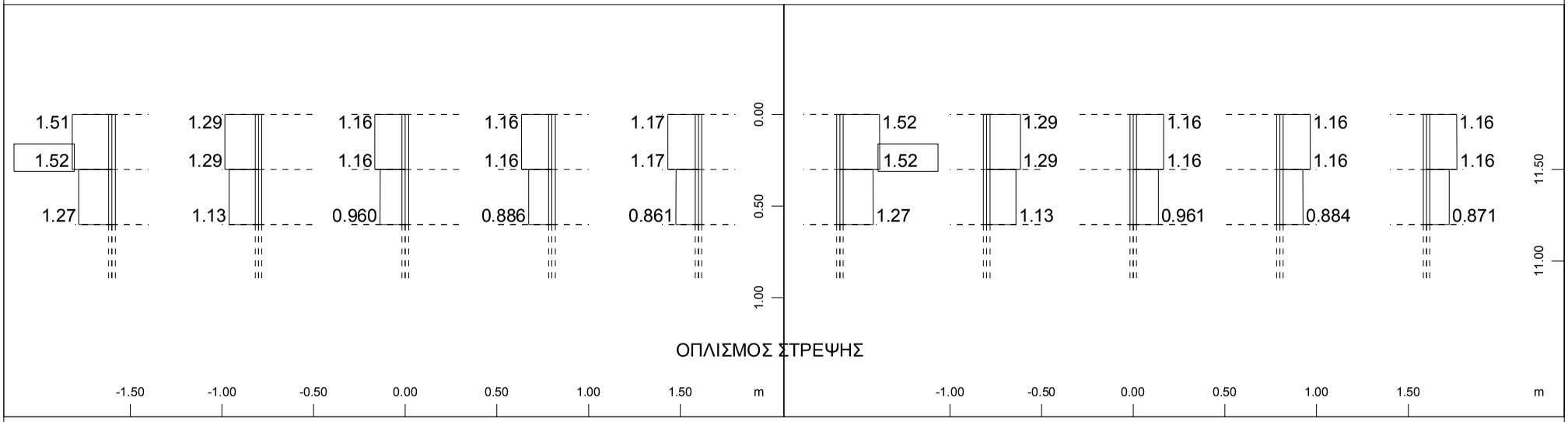
X Z Y
Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
512 , 1 cm 3D = 44.8 cm2 (Max=26.5)

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΣΕΙΣΜΙΚΑ_Q=1.00



Y Z X
Sector of system Beam Elements Group 2 M 1 : 30
Beam Elements , Shear reinforcements (maximum), Design Case 512
, 1 cm 3D = 4.48 cm2/m (Max=3.91)

X Z Y
Sector of system Beam Elements Group 2 M 1 : 30
Beam Elements , Shear reinforcements (maximum), Design Case 512
, 1 cm 3D = 4.48 cm2/m (Max=3.85)

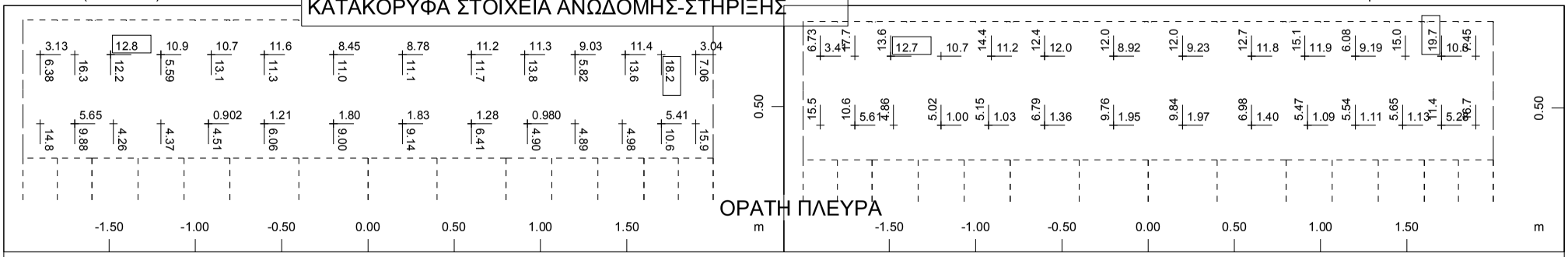


Z Y X
Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
512 , 1 cm 3D = 2.24 cm2 (Max=1.52)

X Z Y
Sector of system Beam Elements Group 2 M 1 : 32
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
512 , 1 cm 3D = 2.24 cm2 (Max=1.52)

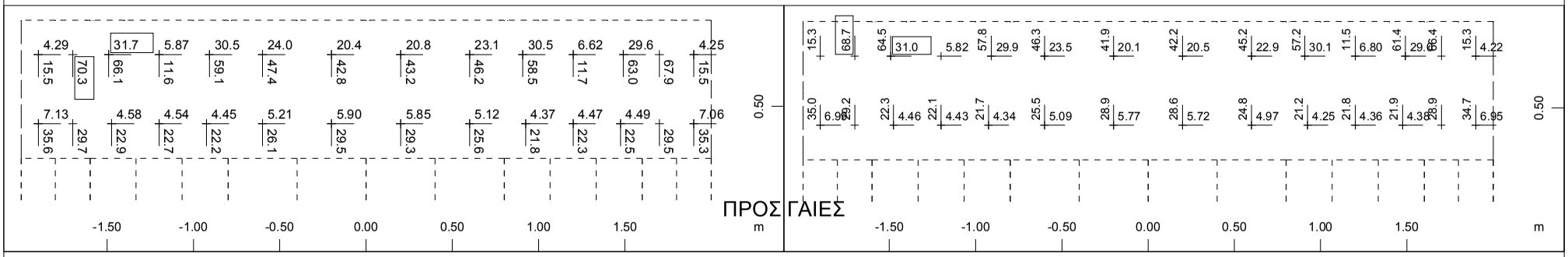
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΣΕΙΣΜΙΚΑ_Q=1.00

ΚΑΤΑΚΟΡΥΦΑ ΣΤΟΙΧΕΙΑ ΑΝΩΔΟΜΗΣ-ΣΤΗΡΙΞΗΣ



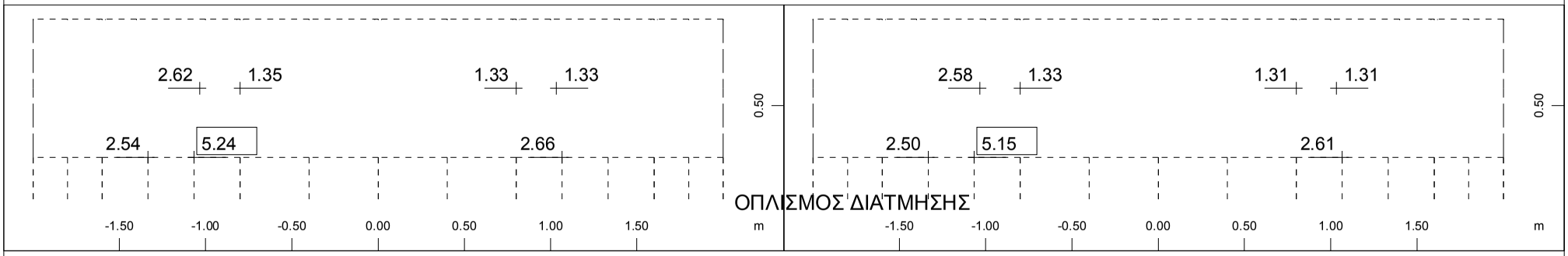
Sector of system Quadrilateral Elements Group 8 M 1 : 34
upper Reinforcements in Elements in cm2/m, Design Case 523 ULS
design (Max=18.2)

Sector of system Quadrilateral Elements Group 8 M 1 : 34
upper Reinforcements in Elements in cm2/m, Design Case 523 ULS
design (Max=19.7)



Sector of system Quadrilateral Elements Group 8 M 1 : 34
lower Reinforcements in Elements in cm2/m, Design Case 523 ULS
design (Max=70.3)

Sector of system Quadrilateral Elements Group 8 M 1 : 34
lower Reinforcements in Elements in cm2/m, Design Case 523 ULS
design (Max=68.7)

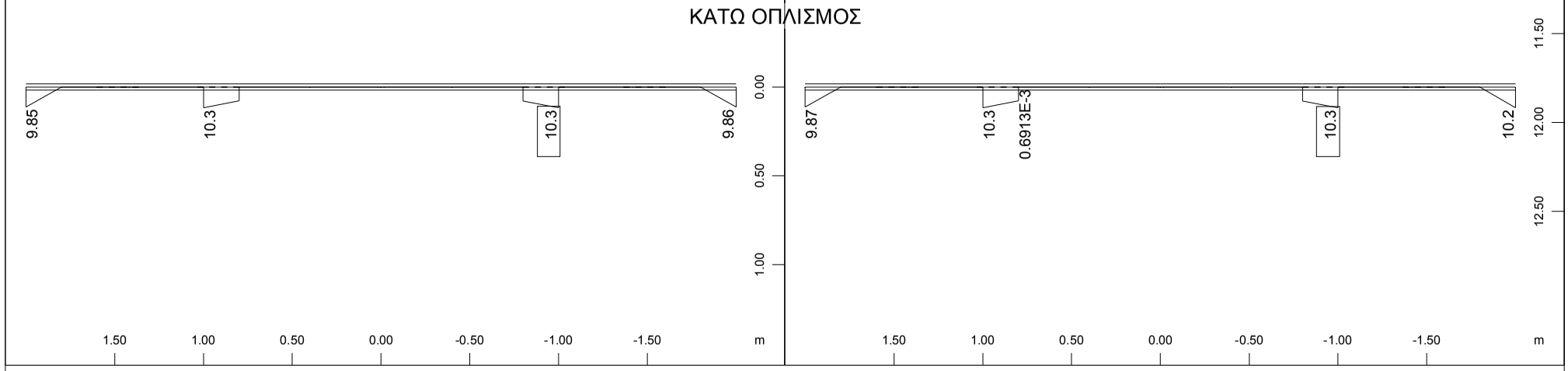


Sector of system Quadrilateral Elements Group 8 M 1 : 34
Shear reinforcement from middle of element in cm2/m2, Design
Case 523 ULS design (Max=5.24)

Sector of system Quadrilateral Elements Group 8 M 1 : 34
Shear reinforcement from middle of element in cm2/m2, Design
Case 523 ULS design (Max=5.15)

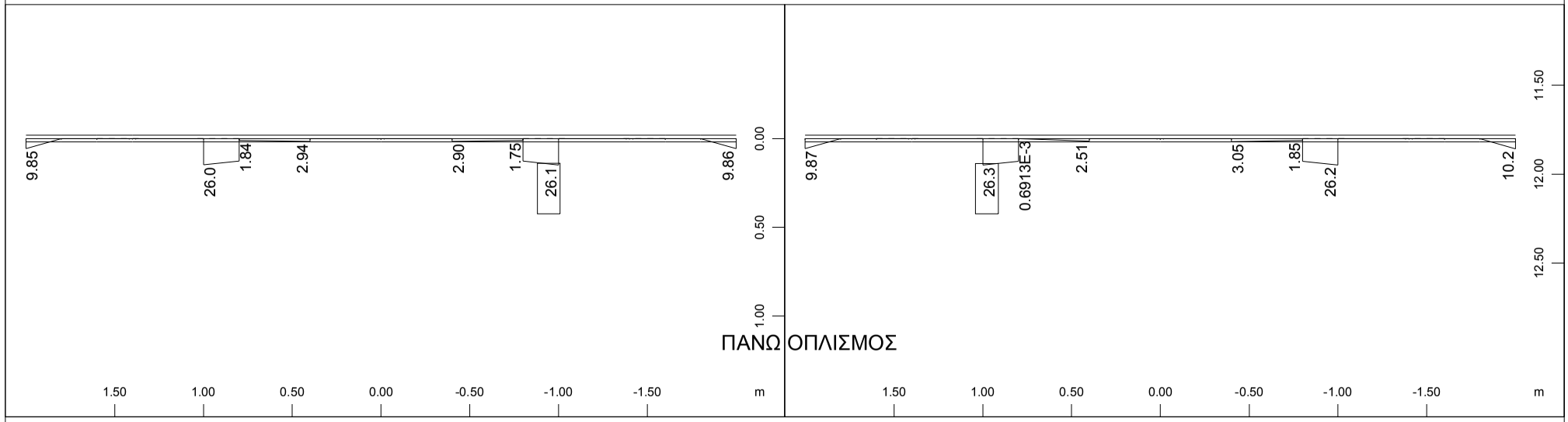
ΠΑΣΣΑΛΟΔΕΣΜΟΣ

ΚΑΤΩ ΟΠΛΙΣΜΟΣ



Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
511 , 1 cm 3D = 29.1 cm2 (Max=10.3)

Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 1, Design Case
511 , 1 cm 3D = 29.1 cm2 (Max=10.3)

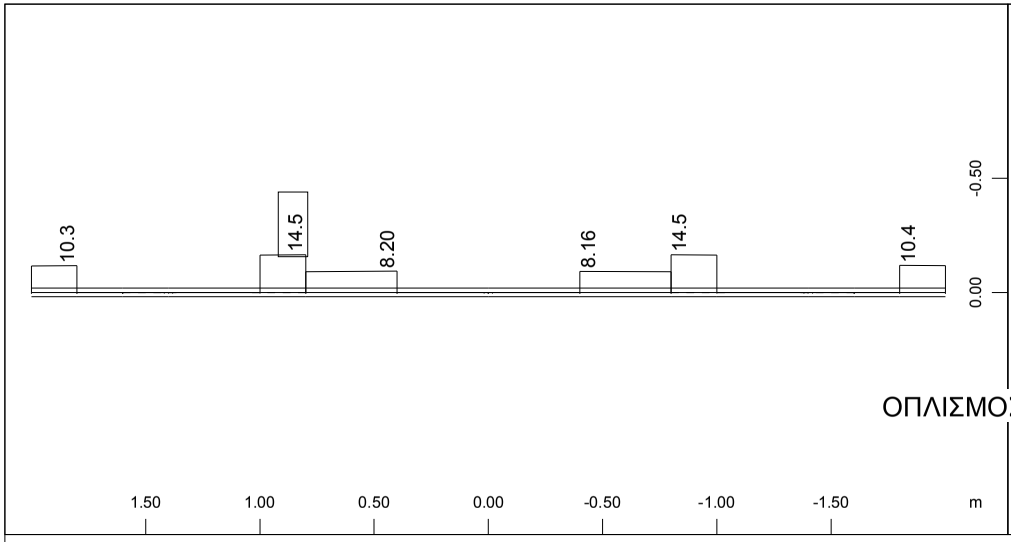


ΠΑΝΩ ΟΠΛΙΣΜΟΣ

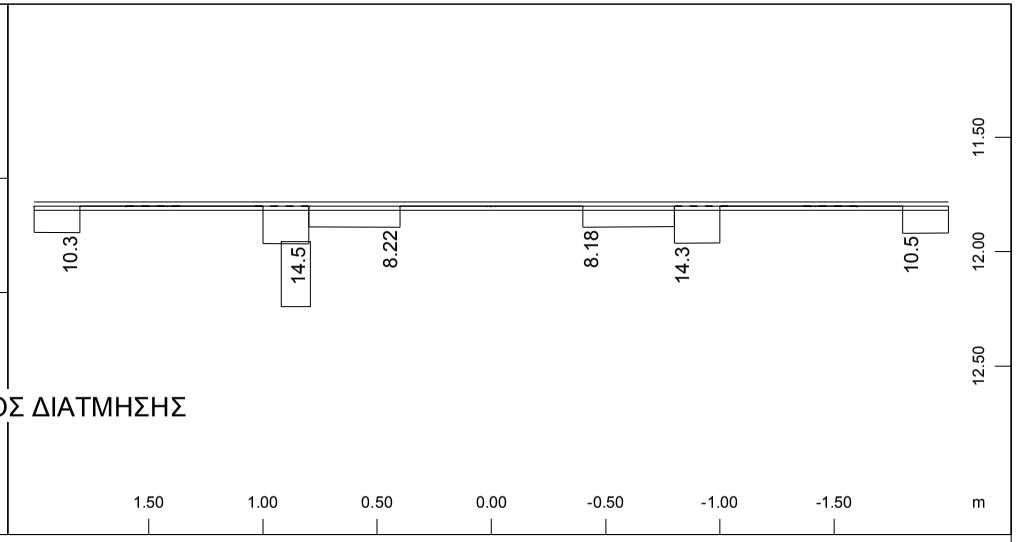
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
511 , 1 cm 3D = 58.1 cm2 (Max=26.1)

Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case
511 , 1 cm 3D = 58.1 cm2 (Max=26.3)

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΣΕΙΣΜΙΚΑ_Q=1.00

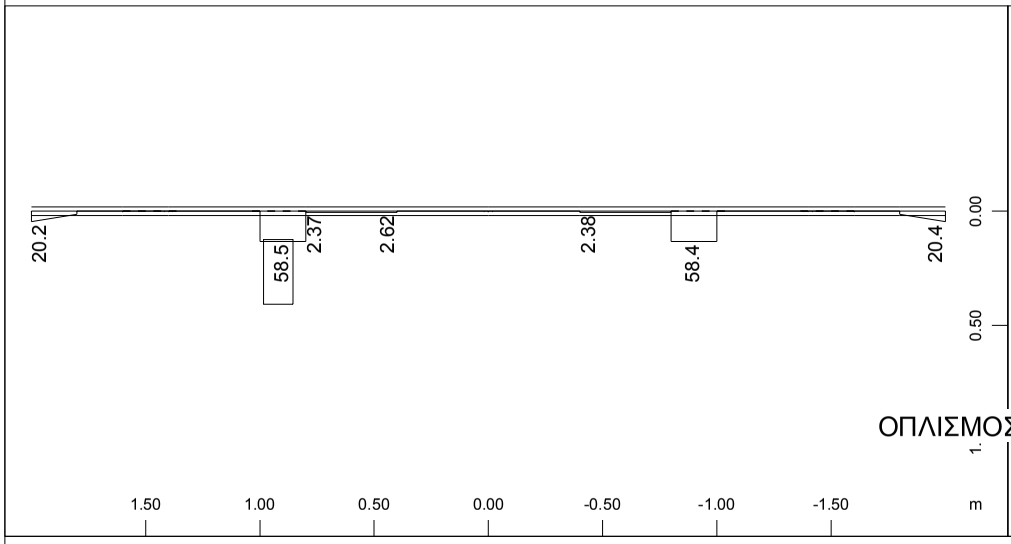


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Shear reinforcements (maximum), Design Case 511
, 1 cm 3D = 29.1 cm2/m (Max=14.5)

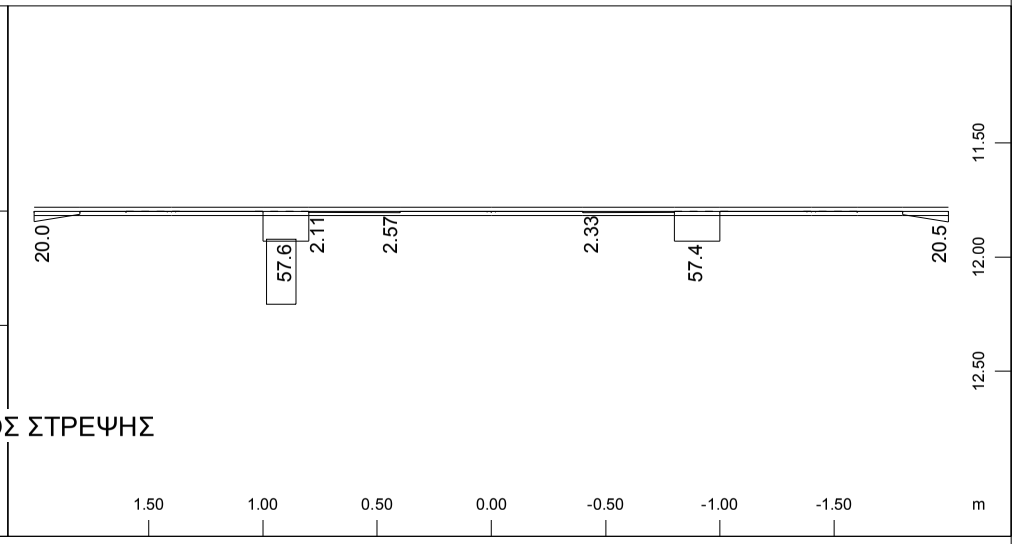


Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Shear reinforcements (maximum), Design Case 511
, 1 cm 3D = 29.1 cm2/m (Max=14.5)

ΟΠΛΙΣΜΟΣ ΔΙΑΤΜΗΣΗΣ



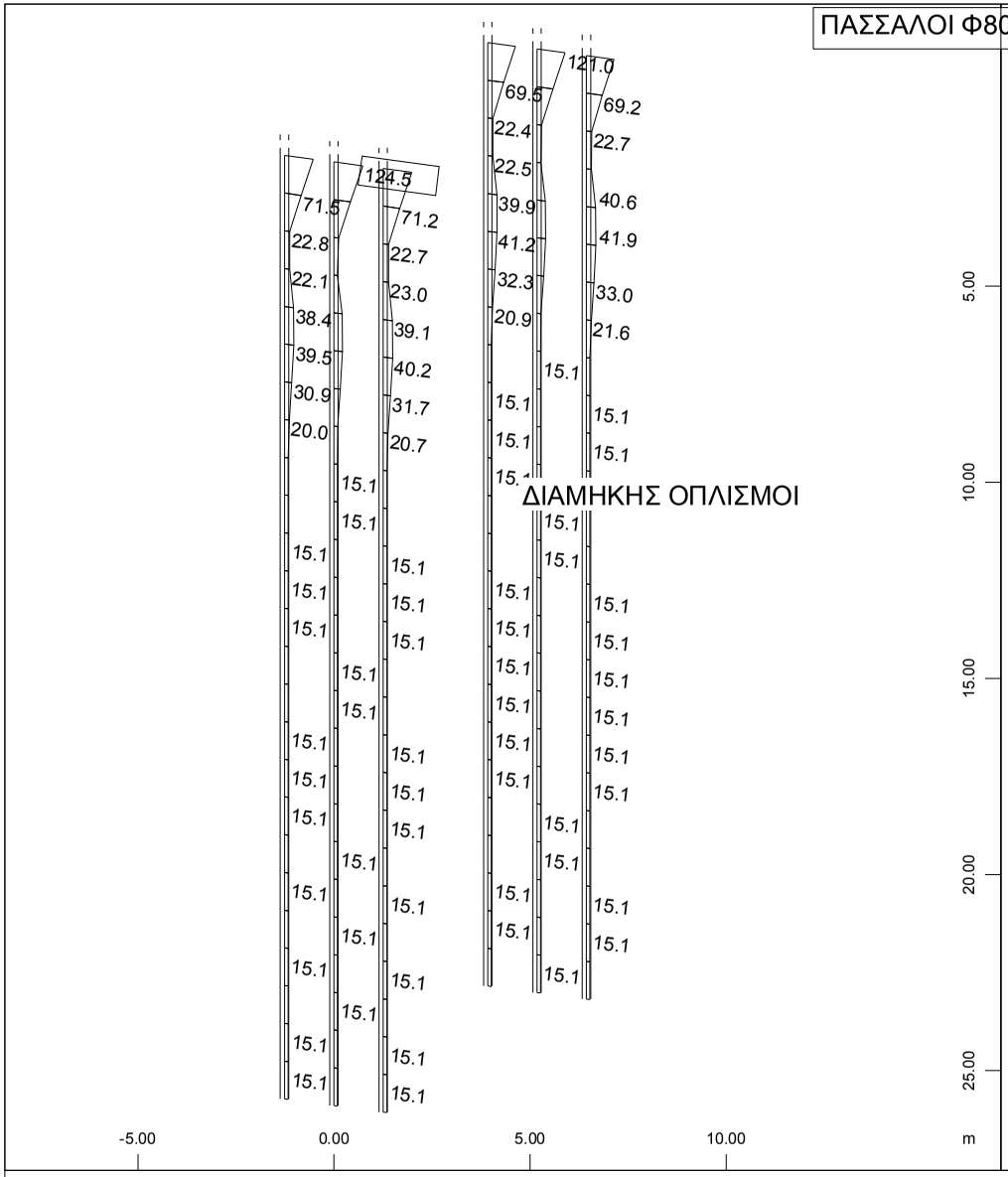
Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
511 , 1 cm 3D = 145.3 cm2 (Max=58.5)



Sector of system Beam Elements Group 10 M 1 : 33
Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case
511 , 1 cm 3D = 145.3 cm2 (Max=57.6)

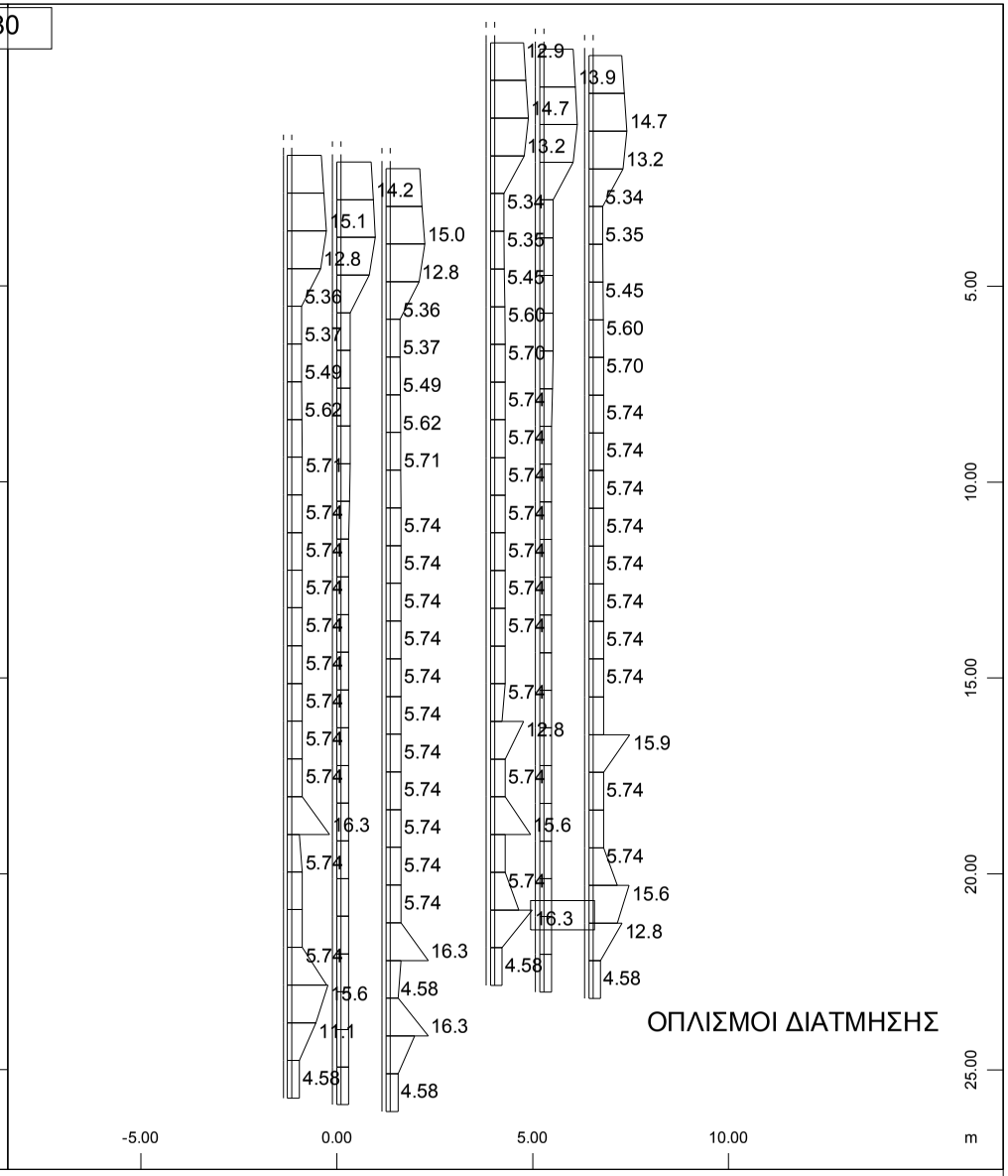
ΟΠΛΙΣΜΟΣ ΣΤΡΕΨΗΣ

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΣΕΙΣΜΙΚΑ_Q=1.00



Sector of system Beam Elements Group 12
Beam Elements , Longitudinal Reinforcements Lay. 1, Design
Case 511 , 1 cm 3D = 290.6 cm2 (Max=124.5)

M 1 : 191
X * 0.502
Y * 0.906
Z * 0.962

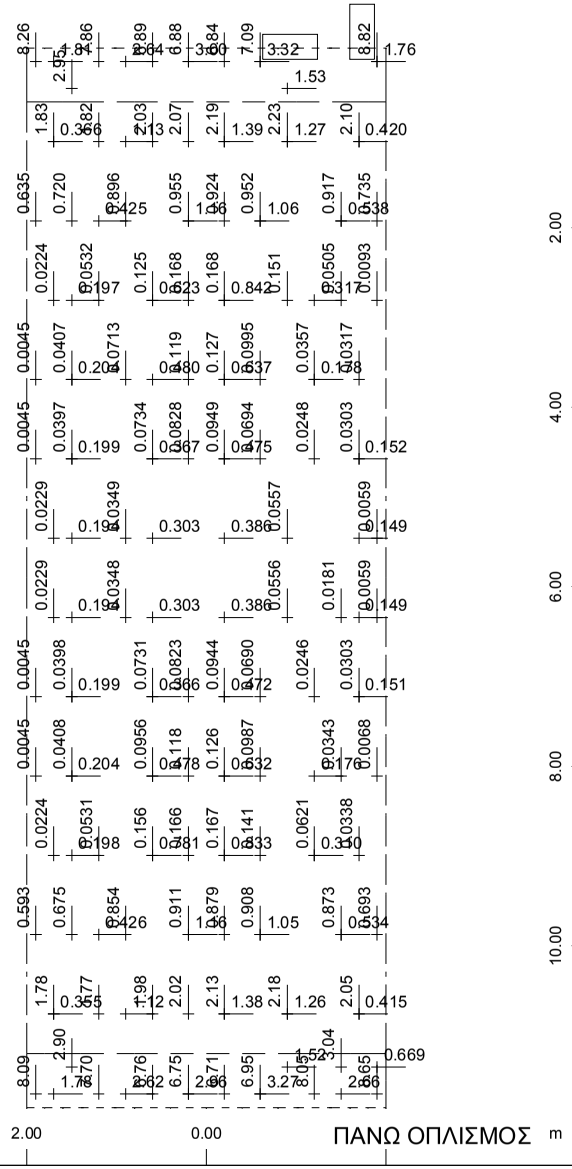


Sector of system Beam Elements Group 12
Beam Elements , Shear reinforcements (maximum), Design Case
511 , 1 cm 3D = 29.1 cm2/m (Max=16.3)

M 1 : 191
X * 0.502
Y * 0.906
Z * 0.962

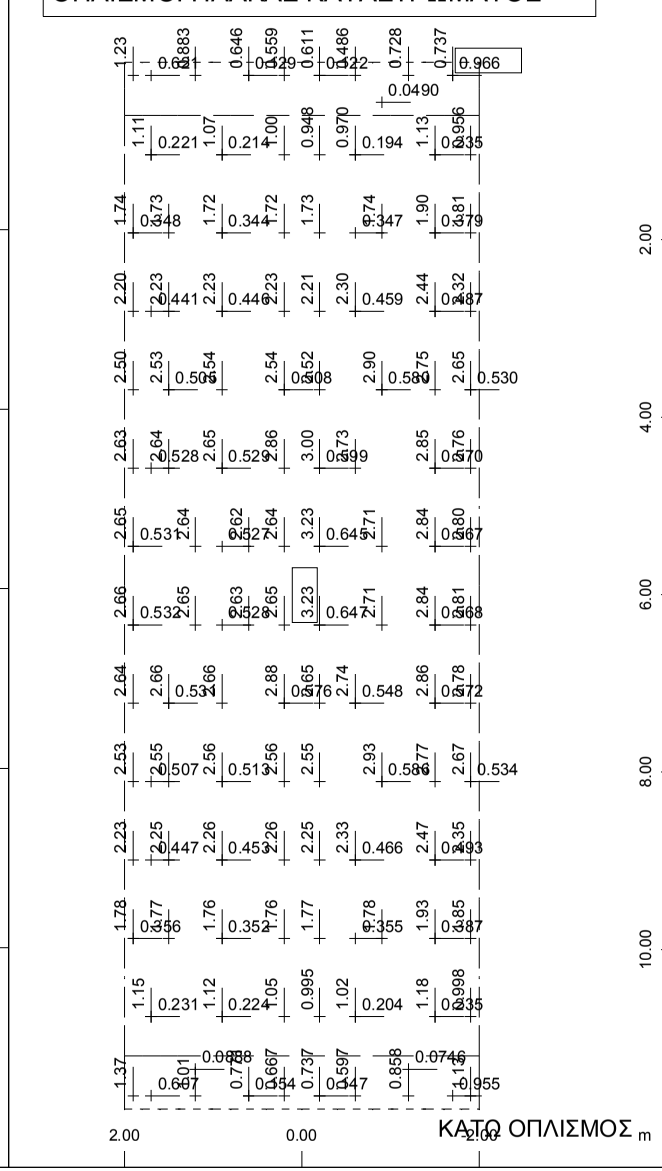
ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΣΕΙΣΜΙΚΑ_Q=1.00

ΟΠΛΙΣΜΟΙ ΠΛΑΚΑΣ ΚΑΤΑΣΤΡΩΜΑΤΟΣ



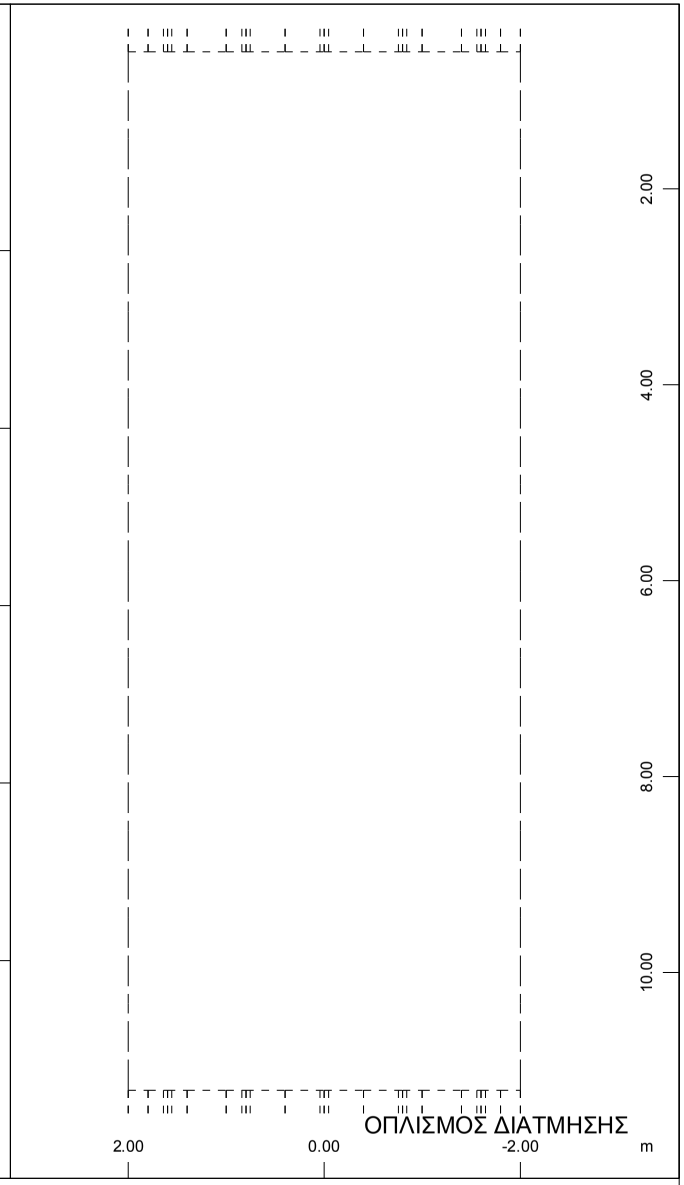
Sector of system Quadrilateral Elements Group 3 4
upper Reinforcements in Elements in cm2/m, Design Case
523 ULS design (Max=8.82)

M 1 : 84



Sector of system Quadrilateral Elements Group 3 4
lower Reinforcements in Elements in cm2/m, Design Case
523 ULS design (Max=3.23)

M 1 : 85



Sector of system Quadrilateral Elements Group 3
Shear reinforcement from middle of element in cm2/m2,
Design Case 523 ULS design (Max=0)

M 1 : 77

ΟΡΙΣΤΙΚΗ ΜΕΛΕΤΗ/ΤΕΧΝΙΚΟ ΤΑ/L=13.00
PRINT:ΟΠΛΙΣΜΟΙ_ΣΕΙΣΜΙΚΑ_Q=1.00